



Illinois Power Generating Company
1500 Eastport Plaza Dr.
Collinsville, IL 62234

July 28, 2022

Illinois Environmental Protection Agency
DWPC – Permits MC #15
Attn: Part 845 Coal Combustion Residual Rule Submittal
1021 North Grand Avenue East
P.O. Box 19276
Springfield, IL 62794-9276

Re: Coffeen Power Plant GMF Gypsum Stack Pond; IEPA ID # W1350150004-03

Dear Mr. LeCrone:

In accordance with 35 I.A.C. § 845.200, Illinois Power Generating Company (IPGC) is submitting a construction permit application for the Coffeen Power Plant GMF Gypsum Stack Pond (IEPA ID # W1350150004-03). One hardcopy is provided with this submittal.

The permit application was prepared in accordance with 35 I.A.C. § 845.220 (a) and (d). This submittal includes the completed permit forms as required by § 845.210.

Sincerely,

A handwritten signature in blue ink that reads "Cynthia E. Vodopivec".

Cynthia Vodopivec
SVP-Environmental Health and Safety

Enclosures



**Illinois Environmental Protection Agency
CCR Surface Impoundment Permit Application
Form CCR 1 – General Provisions**

Bureau of Water ID Number:

For IEPA Use Only

CCR Permit Number:

Facility Name:

SECTION 1: FACILITY, OPERATOR, AND OWNER INFORMATION (35 Ill. Adm. Code 845.210(b))

Facility, Operator, and Owner Information	1.1	Facility Name		
	1.2	Illinois EPA CCR Permit Number (if applicable)		
	1.3	Facility Contact Information		
		Name (first and last)	Title	Phone Number
		Email address		
	1.4	Facility Mailing Address		
		Street or P.O. box		
		City or town	State	Zip Code
	1.5	Facility Location		
		Street, route number, or other specific identifier		
		County name	County code (if known)	
	City or town	State	Zip Code	
1.6	Name of Owner/Operator			

Facility, Operator, and Owner Info	1.7	Owner/Operator Contact Information		
		Name (first and last)	Title	Phone Number
		Email address		
	1.8	Owner/Operator Mailing Address		
		Street or P.O. box		
	City or town	State	Zip Code	
SECTION 2: LEGAL DESCRIPTION (35 Ill. Adm. Code 845.210(c))				
Legal Description	2.1	Legal Description of the facility boundary		
SECTION 3: PUBLICLY ACCESSIBLE INTERNET SITE REQUIREMENTS (35 Ill. Adm. Code 845.810)				
Internet Site	3.1	Web Address(es) to publicly accessible internet site(s) (CCR website)		
	3.2	Is/are the website(s) titled "Illinois CCR Rule Compliance Data and Information"		
		Yes	No	
SECTION 4: IMPOUNDMENT IDENTIFICATION				
Impoundment Identification	4.1	List all the impoundment identification numbers for your facility and check the corresponding box to indicate that you have attached a written description for each impoundment.		
			Attached written description	
			Attached written description	
			Attached written description	
			Attached written description	
			Attached written description	
			Attached written description	

	<input type="checkbox"/>	Attached written description
	<input type="checkbox"/>	Attached written description
	<input type="checkbox"/>	Attached written description
	<input type="checkbox"/>	Attached written description

SECTION 5: CHECKLIST AND CERTIFICATION STATEMENT

Checklist and Certification Statement	5.1	In Column 1 below, mark the sections of Form 1 that you have completed and are submitting with your application. For each section, specify in Column 2 any attachments that you are enclosing.			
		Column 1		Column 2	
		Section 1: Facility, Operator, and Owner Information	<input checked="" type="checkbox"/>	w/attachments	<input type="checkbox"/>
		Section 2: Legal Description	<input checked="" type="checkbox"/>	w/attachments	<input checked="" type="checkbox"/>
		Section 3: Publicly Accessible Internet Site Requirement	<input checked="" type="checkbox"/>	w/attachments	<input type="checkbox"/>
		Section 4: Impoundment Identification	<input checked="" type="checkbox"/>	w/attachments	<input checked="" type="checkbox"/>
	5.2	Certification Statement			
		I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gather and evaluate the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations.			
		Name (print or type first and last name) of Owner/Operator Cynthia Vodopivec			Official Title SVP - Environmental
		Signature <i>Cynthia ? Vodopivec</i>			Date Signed 7-18-2022

Form
2CC



Illinois Environmental Protection Agency
CCR Surface Impoundment Permit Application
Form CCR 2CC – Closure Construction

Bureau of Water ID Number:

W1350150004

CCR Permit Number:

N/A

Facility Name:

Coffeen Power Plant

For IEPA Use Only

SECTION 1: DESIGN AND CONSTRUCTION PLANS (35 Ill. Adm. Code 845.220)

Design and Construction Plans (Construction History)	1.1	CCR surface impoundment name.
		Gypsum Stack Pond
	1.2	Identification number of the CCR surface impoundment (if one has been assigned by the Agency).
		N/A
	1.3	Describe the boundaries of the CCR surface impoundment (35 Ill. Adm. Code 845.210 (c)).
		Attachment J
	1.4	State the purpose for which the CCR surface impoundment is being used.
		Attachment A
	1.5	How long has the CCR surface impoundment been in operation?
		Attachment A
	1.6	List the types of CCR that have been placed in the CCR surface impoundment.
		Attachment B

Design and Construction Plans (Continued)	1.7	List the name of the watershed within which the CCR surface impoundment is located.		
		Attachment A		
	1.8	What is the size in acres of the watershed within which the CCR surface impoundment is located?		
		Attachment A		
	1.9	Check the corresponding boxes to indicate that you have attached the following:		
	<input checked="" type="checkbox"/>	A description of the physical and engineering properties of the foundation and abutment materials on which the CCR surface impoundment is constructed.		
	<input checked="" type="checkbox"/>	A statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR surface impoundment.		
	<input checked="" type="checkbox"/>	A statement of the method of site preparation and construction of each zone of the CCR surface impoundment.		
	<input checked="" type="checkbox"/>	A statement of the approximate dates of construction of each successive stage of construction of the CCR surface impoundment.		
	<input checked="" type="checkbox"/>	Drawings satisfying the requirements of 35 Ill. Adm. Code 845.220(a)(1)(F).		
	<input checked="" type="checkbox"/>	A description of the type, purpose, and location of existing instrumentation.		
	<input checked="" type="checkbox"/>	Area capacity curves for the CCR impoundment.		
	<input checked="" type="checkbox"/>	A description of each spillway and diversion design features and capacities and provide the calculations used in their determination.		
	<input checked="" type="checkbox"/>	The construction specifications and provisions for surveillance, maintenance, and repair of the CCR surface impoundment.		
1.10.1	Is there any record or knowledge of structural instability of the CCR surface impoundment?			
	<input type="checkbox"/>	Yes	<input checked="" type="checkbox"/>	No
1.10.2	If you answered yes to Item 1.10.1, provide detailed explanation of the structural instability.			

SECTION 2: NARRATIVE DESCRIPTION OF THE FACILITY (35 Ill. Adm. Code 845.220)

Narrative Description	2.1	List the types of CCR expected in the CCR surface impoundments.		
		Attachment B		
	2.2	Have you attached a chemical analysis of each type of expected CCR?		
		<input checked="" type="checkbox"/>	Yes	
	2.3	Estimate of the maximum capacity of the surface impoundment in gallons or cubic yards.		
		Attachment B		
2.4	The rate at which CCR and non-CCR waste streams currently enter the CCR impoundment in gallons per day and dry tons.			
	Attachment B	GPD	Attachment B	dTn
2.5	Estimate length of time the CCR surface impoundment will receive CCR and non-CCR waste streams.			
	Attachment B			
2.6	Have you attached an on-site transportation plan that includes all existing and planned roads in the facility that will be used during the operation of the CCR surface impoundment?			
	<input checked="" type="checkbox"/>	Yes		

SECTION 3: MAPS (35 Ill. Adm. Code 845.220)

Maps	3.1	Check the corresponding boxes to indicate that you have attached the following maps:		
		<input checked="" type="checkbox"/>	A site location map on the most recent United States Geological Survey (USGS) quadrangle of the area from the 7 ½ minute series (topographic) or on another map whose scale clearly shows the information required in 35 Ill. Adm. Code 845.220(a)(3).	
		<input checked="" type="checkbox"/>	Site plans maps satisfying the requirements of 35 Ill. Adm. Code 845.220(a)(4).	

SECTION 4: ATTACHMENTS

Attachments	4.1	Check the corresponding boxes to indicate that you have attached the following:		
		<input checked="" type="checkbox"/>	A narrative description of the proposed construction of, or modification to, a CCR surface impoundment and any projected changes in the volume or nature of the CCR or non-CCR waste streams.	
		<input checked="" type="checkbox"/>	Plans and specifications fully describing the design, nature, function, and interrelationship of each individual component of the facility.	
		<input checked="" type="checkbox"/>	The signature and seal of a qualified professional engineer.	
		<input checked="" type="checkbox"/>	Certification that the owner or operator of the CCR surface impoundment completed the public notification and public meetings required under 35 Ill. Adm. Code 845.240.	

Attachments (Continued)	<input checked="" type="checkbox"/>	A summary of the issues raised by the public during the public notification and public meetings.
	<input checked="" type="checkbox"/>	A summary of any revisions, determinations, or other considerations made in response to those issues raised by the public during the public notification and public meetings.
	<input checked="" type="checkbox"/>	A list of interested persons in attendance who would like to be added to the Agency's listserv for the facility.
	<input checked="" type="checkbox"/>	Certification that all contractors, subcontractors, and installers utilized to construct, install, modify, or close a CCR surface impoundment are participants in a training program that is approved by and registered with the U.S. Department of Labor's Employment and Training Administration and that includes instruction in erosion control and environmental remediation.
	<input checked="" type="checkbox"/>	Certification that all contractors, subcontractors, and installers utilized to construct, install, modify, or close a CCR surface impoundment are participants in a training program that is approved by and registered with the U.S. Department of Labor's Employment and Training Administration and that includes instruction in the operation of heavy equipment and excavation.

SECTION 5: GROUNDWATER MONITORING PROGRAM

Groundwater Monitoring	5.1	Indicate that you have attached the following components of a new groundwater monitoring program or any modifications to an existing groundwater monitoring program by checking the corresponding boxes:
	<input checked="" type="checkbox"/>	A hydrogeologic site investigation meeting the requirements of 35 Ill. Adm. Code 845.620, if applicable.
	<input checked="" type="checkbox"/>	Design and construction plans of a groundwater monitoring system meeting the requirements of 35 Ill. Adm. Code 845.630.
	<input checked="" type="checkbox"/>	A proposed groundwater sampling and analysis program that includes selection of the statistical procedures to be used for evaluating groundwater monitoring data as required by 35 Ill. Adm. Code 845.640 and 845.650.

SECTION 6: CLOSURE (35 Ill. Adm. Code 845.220(d))

Closure	6.1	What is the closure prioritization category under 35 Ill. Adm. Code 845.700(g), if applicable?
		Attachment E
	6.2	Indicate that you have attached the following by checking the corresponding boxes:
	<input checked="" type="checkbox"/>	The final closure plan, as specified in 35 Ill. Adm. Code 845.720(b), which includes the closure alternatives analysis required by 35 Ill. Adm. Code 845.710.
	<input checked="" type="checkbox"/>	Proposed schedule to complete closure.
<input checked="" type="checkbox"/>	Post-closure care plan as specified in 35 Ill. Adm. Code 845.780(d).	

SECTION 7: GROUNDWATER MODELING (35 Ill. Adm. Code 845.220(d)(3))

Groundwater	7.1	Indicate that you have attached the following by checking the corresponding boxes:
	<input checked="" type="checkbox"/>	The results of groundwater contaminant transport modeling and calculations showing how the closure will achieve compliance with the applicable groundwater standards.
	<input checked="" type="checkbox"/>	All modeling inputs and assumptions.
	<input checked="" type="checkbox"/>	Description of the fate and transport of contaminants with the selected corrective action over time.

	<input checked="" type="checkbox"/>	Capture zone modeling, if applicable.
	<input checked="" type="checkbox"/>	Any necessary licenses and software needed to review and access both the model and the data contained within the model.



PERMIT APPLICATION

**Part 845 Construction Permit Application for the Gypsum
Management Facility Gypsum Stack Pond**
Coffeen Power Plant

Submitted to:

Illinois Environmental Protection Agency

1021 North Grand Avenue East
P.O. Box 19276
Springfield, Illinois 62794-9276

Submitted by:

Illinois Power Resources Generating, LLC

1500 Eastport Plaza Drive
Collinsville, Illinois 62234

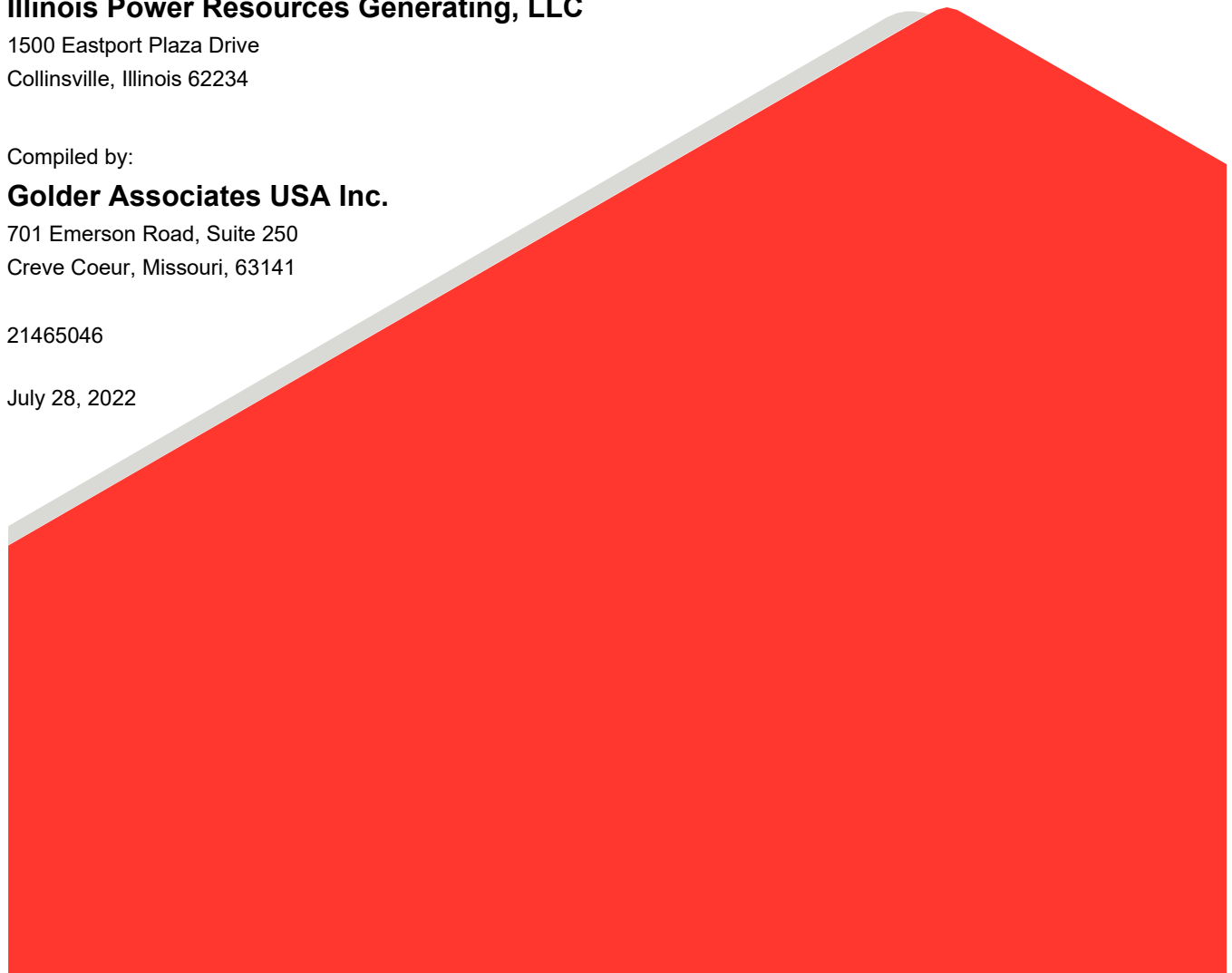
Compiled by:

Golder Associates USA Inc.

701 Emerson Road, Suite 250
Creve Coeur, Missouri, 63141

21465046

July 28, 2022



Executive Summary

Introduction

Illinois Power Resources Generating, LLC (IPRG) owns and operates the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) at the Coffeen Power Plant in Montgomery County, Illinois. The GMF GSP is a coal combustion residuals (CCR) surface impoundment that was used to manage gypsum produced at the Coffeen Power Plant from the time construction of the GMF GSP was completed in 2010 until the power plant was retired in December 2019. Since the retirement of the Coffeen Power Plant, The GMF GSP has no longer received CCR or any other waste stream. IPRG is submitting this Part 845 Construction Permit Application for the GMF GSP to provide the Illinois Environmental Protection Agency (IEPA) with the information required under 35 Illinois Administrative Code (I.A.C.) 845, Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments (Part 845) for closure of the GMF GSP.

Pursuant to 35 I.A.C. § 845.700(c), the GMF GSP has been designated as Category 5. Corrective action is expected at the Site. An evaluation of potential corrective measures and corrective actions has not yet been completed but will be conducted consistent with the requirements in 35 I.A.C. § 845.670.

Closure Method Selection

As required under 35 I.A.C. § 845.710, a closure alternatives analysis has been completed to identify the most appropriate closure method for the GMF GSP. The following closure alternatives were evaluated:

- closure by removal of CCR with disposal in an off-site landfill
- hybrid closure with a final cover system constructed over CCR consolidated in the northern portion of the impoundment

Based on the findings of the closure alternatives analysis, the GMF GSP will be closed with a final cover system with CCR consolidated in the northern portion of the impoundment (i.e., hybrid closure). All gypsum (CCR) in the southern portion of the impoundment will be removed and relocated to the northern portion of the impoundment, which will be closed in accordance with 35 I.A.C. § 845.750. Under this hybrid approach, approximately 63% of the current CCR footprint within the impoundment will be removed. The closed GMF GSP will contain approximately 295,500 cubic yards of CCR.

Closure Method Description

The closed GMF GSP facility will have final cover slopes of 25H:1V (4%) to accommodate moderate settlement and promote drainage. A berm will be constructed at the south end of the consolidated footprint for stability. The location of the berm has been selected to accommodate the estimated 295,500 CY of CCR and 38,000 CY of clayey soil from the GMF GSP to be contained within the consolidated footprint based on the grading plan presented. The general sequencing plan for the closure-in-place option is as follows:

- Pump out ponded water [approximately 106 million gallons (MG)] from the GMF GSP to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- Resume pumping of the perimeter drains surrounding the GSP to lower the groundwater level beneath the GSP and outside the existing liner system to facilitate closure construction. Discharge water to the existing

drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.

- A temporary water management system will be constructed within the GMF GSP, including ditches and sumps. The system will maintain the GMF GSP in an unwatered state by collecting contact stormwater during closure construction. Stormwater flow will be conveyed through Outfall 023 to the existing drainage to the east where it will be managed in accordance with the NPDES permit for the site.
- Once the ponded water has been removed from the GMF GSP, the CCR in the consolidated footprint will be dewatered. Approximately 131,000 CY of CCR will be dewatered as needed to enable relocation. It is anticipated that approximately 8.8 MG of water removal will be required to dewater the CCR. The CCR will dewater to some degree by gravity, but dewatering by pumping from trenches and sumps is expected to be necessary. Liquid waste and water flowing to sumps will be managed in accordance with the NPDES permit for the site and discharged through Outfall 023.
- CCR will be removed from the berm footprint and relocated into the consolidated footprint. The berm will be constructed in an east-west orientation at the east end of the consolidated footprint. The upstream face of the berm will be lined with a composite liner system consisting of 60-mil HDPE geomembrane overlying a compacted clay layer, which will tie into the existing composite liner system.
- All remaining CCR south of the berm in the GMF GSP will be collected and deposited north of the berm.
- Geosynthetic components of the existing liner system and PWRS south of the berm in the GMF GSP will be removed and disposed in the closure footprint or hauled away for disposal. It is anticipated that up to 1 foot of clay soil beneath the geomembrane may also be removed. The soils will be visually observed for signs of CCR. If soils with signs of CCR are observed, they will be removed and deposited north of the berm (for the purposes of conceptual design, assume 1 foot, or approximately 38,000 CY, will need to be removed).
- Compacted fill, composed of locally available soils, would be placed only as needed to achieve final cover subgrade. The compacted fill is anticipated to be compacted to a minimum of 95% of the standard Proctor maximum dry density to reduce settlement.
- Construction of an alternate final cover system, consisting of (from top to bottom):
 - 24-inch final protective soil layer. The final protective soil layer would include 18 inches of protective soil cover overlain by a 6-inch-thick topsoil layer, and be revegetated with grass species selected to reduce maintenance based on soil type. The 18 inches of protective soil cover will be constructed of locally available soils removed from the embankment containment berm and compacted to between 80% and 95% of the standard Proctor maximum dry density for establishment of vegetation and protection of the underlying geomembrane. Protective soil layer material is likely to be primarily low-plasticity silt or clay based on review of site geotechnical information.
 - Drainage geocomposite
 - 40-mil linear low-density polyethylene (LLDPE) geomembrane layer
- All areas of the cover system will be sloped at a minimum of 4% to positively drain to the exterior of the GMF GSP.

- To prevent impoundment of water in the south end of the current GMF GSP footprint after CCR removal, existing earthen embankments not required for the consolidated footprint will be removed on the eastern side of the GMF GSP and a channel will be excavated to allow stormwater to flow through existing NPDES Outfall 023 into the existing drainage to the east.
- The final ground surface of the southern part of the GMF GSP will be sloped to drain a minimum of 0.5% towards the channel excavated in the southeast corner, in order to allow post-closure, non-contact stormwater to gravity flow into the existing drainage. Soil fill, sourced from existing berms no longer required to contain waste in the consolidated footprint or from the on-Site soil borrow area southeast of AP1, will be used to achieve the necessary slopes. Additional fill will come from off-Site borrow sources assumed to be located within 2 miles of the GMF GSP.
- Vegetation will be established on the final surface of the GMF GSP. Stormwater best management practices (BMPs) such as erosion control blankets will be used, as needed, to reduce erosion during vegetation establishment.
- After vegetation is established, BMPs will be removed, and closure construction will be considered complete.

Document Organization

IEPA Application Form CCR 1 and IEPA Application Form CCR 2CC precede this Executive Summary. A checklist identifying the required elements of the Part 845 Construction Permit Application and the location in this document where each element can be found follows this Executive Summary. Supplemental information required under Part 845 is organized in a set of appendices that follow the checklist:

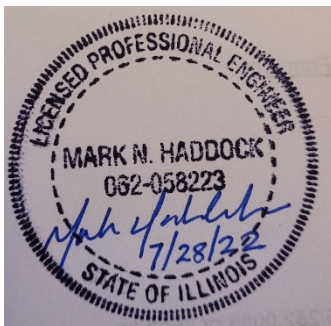
- Appendix A (History of Construction) provides general information about the GMF GSP and describes its design and construction.
- Appendix B (Narrative Description) describes the types and generation rates of CCR managed in the GMF GSP.
- Appendix C (Map Package) includes a Site Location Map and a Site Plan Map depicting important site features and information.
- Appendix D (Hydrogeologic Site Characterization) describes hydrogeologic conditions in the vicinity of the GMF GSP.
- Appendix E (Closure Priority Categorization) identifies the closure priority category assigned to the GMF GSP.
- Appendix F (Groundwater Modeling) describes the results of groundwater modeling that has been conducted to assess the expected fate and transport of chemical constituents following closure of the GMF GSP (a link to access the groundwater model files will be transmitted to the IEPA separately). Includes an Evaluation of Potential Exceedances of Groundwater Protection Standards, which summarizes the potential exceedances of groundwater protection standards that have been detected at the GMF GSP and identifies the alternative sources that have caused them.
- Appendix G (Final Closure Plan) provides design information for closure of the GMF GSP, as well as the results of a closure alternatives analysis that has been conducted to determine the most effective approach for closure of the GMF GSP.

- Appendix H (Groundwater Monitoring Plan) describes the monitoring locations and procedures that will be used to assess groundwater quality after closure of the GMF GSP.
- Appendix I (Post-closure Care Plan) describes the procedures that will be followed to maintain the closed GMF GSP during the post-closure care period.
- Appendix J (Legal Description) provides the land description of the GMF GSP facility boundary.
- Appendix K (Public Meetings Information) provides the information pertaining to the public notification and public meetings required under Part 845.
- Appendix L (Contractor Training Certification) certifies that personnel used to construct, install, modify, or close the GMF GSP will participate in required training programs.

Signature Page

Golder Associates USA Inc.

I, Mark Haddock, being a registered professional engineer in good standing in the State of Illinois, certify to the best of my knowledge that the information contained in this construction permit application has been prepared in accordance with recognized and generally accepted engineering practices.



Mark Haddock, PE
Principal

[https://golderassociates.sharepoint.com/sites/145229/project files/6 deliverables/reports/01-gmf_permit_app/gsp/20220728_final/21465046-coffeen_gsp_permit_application_20220728.docx](https://golderassociates.sharepoint.com/sites/145229/project%20files/6%20deliverables/reports/01-gmf_permit_app/gsp/20220728_final/21465046-coffeen_gsp_permit_application_20220728.docx)

Regulation	Requirement	Location in Permit Application
Section 845.220 - Construction Permits		
a)	All construction permit applications must contain the following information and documents.	
1)	Design and Construction Plans (Construction History)	Appendix A
A)	Identifying information	Appendix A
i)	The name and address of the person or persons owning or operating the CCR surface impoundment	Appendix A
ii)	The name associated with the CCR surface impoundment	Appendix A
iii)	The identification number of the CCR surface impoundment if one has been assigned by the Agency	Appendix A
B)	A statement of the purpose for which the CCR surface impoundment is being used, how long the CCR surface impoundment has been in operation, and the types of CCR that have been placed in the CCR surface impoundment	Appendix A
C)	The name and size in acres of the watershed within which the CCR surface impoundment is located	Appendix A
D)	A description of the physical and engineering properties of the foundation and abutment materials on which the CCR surface impoundment is constructed.	Appendix A
E)	A statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR surface impoundment; the method of site preparation and construction of each zone of the CCR surface impoundment; and the approximate dates of construction of each successive stage of construction of the CCR surface impoundment	Appendix A
F)	At a scale that details engineering structures and appurtenances relevant to the design, construction, operation, and maintenance of the CCR surface impoundment, detailed dimensional drawings of the CCR surface impoundment, including a plan view and cross-sections of the length and width of the CCR surface impoundment, showing all zones, foundation improvements, drainage provisions, spillways, diversion ditches, outlets, instrument locations, and slope protection, in addition to the normal operating pool surface elevation and the maximum pool surface elevation following peak discharge from the inflow design flood, the expected maximum depth of CCR within the CCR surface impoundment, and any identifiable natural or manmade features that could adversely affect operation of the CCR surface impoundment due to malfunction or mis-operation.	Appendix A
G)	A description of the type, purpose, and location of existing instrumentation	Appendix A
H)	Area-capacity curves for the CCR surface impoundment	Appendix A
I)	A description of each spillway and diversion design features and capacities and calculations used in their determination.	Appendix A
J)	The construction specifications and provisions for surveillance, maintenance, and repair of the CCR surface impoundment.	Appendix A
K)	Any record or knowledge of structural instability of the CCR surface impoundment	Appendix A
2)	Narrative Description of the Facility. The permit application must contain a written description of the facility with supporting documentation describing the procedures and plans that will be used at the facility to comply with the requirements of this Part. The descriptions must include, but are not limited to, the following information:	Appendix B, Appendix G
A)	The types of CCR expected in the CCR surface impoundment, including a chemical analysis of each type of expected CCR;	Appendix B
B)	An estimate of the maximum capacity of each surface impoundment in gallons or cubic yards	Appendix B
C)	The rate at which CCR and non-CCR waste streams currently enter the CCR surface impoundment in gallons per day and dry tons;	Appendix B
D)	The estimated length of time the CCR surface impoundment will receive CCR and non-CCR waste streams; and	Appendix B
E)	An on-site transportation plan that includes all existing and planned roads in the facility that will be used during the operation of the CCR surface impoundment	Appendix B
3)	Site Location Map. All permit applications must contain a site location map on the most recent United States Geological Survey (USGS) quadrangle of the area from the 7½ minute series (topographic), or on such other map whose scale clearly shows the following information:	Appendix C
A)	The facility boundaries and all adjacent property, extending at least 1000 meters (3280 feet) beyond the boundary of the facility;	Appendix C
B)	All surface waters;	Appendix C
C)	The prevailing wind direction	Appendix C
D)	The limits of all 100-year floodplains	Appendix C
E)	All natural areas designated as a Dedicated Illinois Nature Preserve under the Illinois Natural Areas Preservation Act [525 ILCS 30];	Appendix C
F)	All historic and archaeological sites designated by the National Historic Preservation Act (16 USC 470 et seq.) and the Illinois Historic Sites Advisory Council Act [20 ILCS 3410]; and	Appendix C
G)	All areas identified as critical habitat under the Endangered Species Act of 1973 (16 USC 1531 et seq.) and the Illinois Endangered Species Protection Act [520 ILCS 10].	Appendix C
4)	Site Plan Map. The application must contain maps, including cross-sectional maps of the site boundaries, showing the location of the facility. The following information must be shown	Appendix C
A)	The entire facility, including any proposed and all existing CCR surface impoundment location;	Appendix C
B)	The boundaries, both above and below ground level, of the facility and all CCR surface impoundments or landfills containing CCR included in the facility	Appendix C
C)	All existing and proposed groundwater monitoring wells; and	Appendix C
D)	All main service corridors, transportation routes, and access roads to the facility	Appendix C
5)	A narrative description of the proposed construction of, or modification to, a CCR surface impoundment and any projected changes in the volume or nature of the CCR or non-CCR waste streams	Appendix G
6)	Plans and specifications fully describing the design, nature, function and interrelationship of each individual component of the facility.	Appendix A, Appendix G
7)	A new groundwater monitoring program or any modification to an existing groundwater monitoring program that includes but is not limited to the following information:	
A)	A hydrogeologic site investigation meeting the requirements of Section 845.620, if applicable	Appendix D
B)	Design and construction plans of a groundwater monitoring system meeting the requirements of Section 845.630; and	Appendix H

Regulation	Requirement	Location in Permit Application
Section 845.220 - Construction Permits (Continued)		
C)	A proposed groundwater sampling and analysis program that includes selection of the statistical procedures to be used for evaluating groundwater monitoring data, as required by Sections 845.640 and 845.650.	Appendix H
8)	The signature and seal of a qualified professional engineer	Executive Summary
9)	Certification that the owner or operator of the CCR surface impoundment completed the public notification and public meetings required under Section 845.240, a summary of the issues raised by the public, a summary of any revisions determinations, or other considerations made in response to those issues, and a list of interested persons in attendance who would like to be added to the Agency's listserv for the facility.	Appendix K
b)	New Construction. In addition to the requirements in subsection (a), all construction permit applications to build a new CCR surface impoundment, lateral expansion of a CCR surface impoundment, or retrofit of an existing CCR surface impoundment must also contain the following information and documents	Not applicable - not new construction, lateral expansion, or retrofit
1)	Plans and specifications that demonstrate the proposed CCR surface impoundment will meet the location standards in the following sections:	
A)	Section 845.300 (Placement Above the Uppermost Aquifer)	
B)	Section 845.310 (Wetlands);	
C)	Section 845.320 (Fault Areas);	
D)	Section 845.330 (Seismic Impact Zones); and	
E)	Section 845.340 (Unstable Areas and Floodplains)	
2)	Plans and specifications demonstrate the proposed CCR surface impoundment will meet the following design criteria:	
A)	The CCR surface impoundment will have a liner meeting the liner requirements of Section 845.400(b) or (c);	
B)	The CCR surface impoundment will have a leachate collection system meeting the requirements of Section 845.420 and	
C)	The CCR surface impoundment, if not incised, will be constructed with slope protection, as required by Section 845.430.	
3)	CCR fugitive dust control plan (see Section 845.500(b))	
4)	Preliminary written closure plan (see Section 845.720(a))	
5)	Initial written post-closure care plan, if applicable (see Section 845.780(d))	
c)	Corrective Action Construction. In addition to the requirements in subsection (a), all construction permit applications that include any corrective action performed under Subpart F must also contain the following information and documents:	
1)	Corrective action plan (see Section 845.670);	
2)	Groundwater modeling, including:	
A)	The results of groundwater contaminant transport modeling and calculations showing how the closure will achieve compliance with the applicable groundwater standards	
B)	All modeling inputs and assumptions;	
C)	Description of the fate and transport of contaminants, with the selected closure over time; and	
D)	Capture zone modeling, if applicable.	
3)	Any necessary licenses and software needed to review and access both the models and the data contained within the models required by subsection (c)(2);	
4)	Corrective action groundwater monitoring program, including identification of revisions to the groundwater monitoring system for corrective action; and	
5)	Any interim measures necessary to reduce the contaminants leaching from the CCR surface impoundment, and/or potential exposures to human or ecological receptors, including an analysis of the factors specified in Section 845.680(a)(3).	
d)	Closure Construction. In addition to the requirements in subsection (a), all construction permit applications for closure of the CCR surface impoundment under Subpart G must contain the following information and documents:	Appendix E Appendix G Appendix F Appendix F Appendix F Appendix F Not applicable Provided separately if applicable Appendix G Appendix I
1)	Closure prioritization category under Section 845.700(g), if applicable	
2)	Final closure plan, as specified in Section 845.720(b), which includes the closure alternatives analysis required by Section 845.710;	
3)	Groundwater modeling, including	
A)	The results of groundwater contaminant transport modeling and calculations showing how the closure will achieve compliance with the applicable groundwater standards	
B)	All modeling inputs and assumptions	
C)	Description of the fate and transport of contaminants, with the selected closure over time;	
D)	Capture zone modeling, if applicable; and	
E)	Any necessary licenses and software needed to review and access both the model and the data contained within the model.	
4)	Proposed schedule to complete closure; and	
5)	Post-closure care plan as specified in Section 845.780(d), if applicable	
e)	Owners or operators of CCR surface impoundments who submitted a closure plan to the agency before May 1, 2019, and who complete closure before July 30, 2021, shall not be required to obtain a construction permit for closure under subsection (d). [415 ILCS 5/22.58(e)]	Not applicable - closure not completed before July 31, 2021
f)	A single construction permit application may be submitted for new construction, corrective action, and closure if the construction is related to the same multiphased project. The permit application for a project with multiple phases must contain all information required by subsections (a), (b), (c) and (d), as applicable.	Not applicable - not a multiphased project
g)	Duration of Construction Permits	Not applicable - permit application is for closure Acknowledged
1)	For any construction permit that is not for the closure or retrofit of the CCR surface impoundment, the construction permit must be issued for fixed terms not to exceed 3 years.	
2)	For any construction permit for the closure or retrofit of a CCR surface impoundment, the construction permit must be issued for an initial fixed term expiring within the timeframe approved by the Agency in the construction permit or five years, whichever is less. The Agency may renew a construction permit for closure or retrofit in two-year increments under Section 845.760(b).	

Regulation	Requirement	Location in Permit Application
Section 845.620 - Hydrogeologic Site Characterization		
a)	The owner or operator of the CCR surface impoundment must design and implement a hydrogeologic site characterization.	Appendix D
b)	The hydrogeologic site characterization must include, but is not limited to, the following:	
1)	Geologic well logs/boring logs;	Appendix D
2)	Climatic aspects of the site, including seasonal and temporal fluctuations in groundwater flow	Appendix D
3)	Identification of nearby surface water bodies and drinking water intakes	Appendix D
4)	Identification of nearby pumping wells and associated uses of the groundwater	Appendix D
5)	Identification of nearby dedicated nature preserves	Appendix D
6)	Geologic setting;	Appendix D
7)	Structural characteristics;	Appendix D
8)	Geologic cross-sections;	Appendix D
9)	Soil characteristics;	Appendix D
10)	Identification of confining layers	Appendix D
11)	Identification of potential migration pathways	Appendix D
12)	Groundwater quality data;	Appendix D
13)	Vertical and horizontal extent of the geologic layers to a minimum depth of 100 feet below land surface, including lithology and stratigraphy	Appendix D
14)	A map displaying any known underground mines beneath a CCR surface impoundment	Appendix D
15)	Chemical and physical properties of the geologic layers to a minimum depth of 100 feet below land surface;	Appendix D
16)	Hydraulic characteristics of the geologic layers identified as migration pathways and geologic layers that limit migration, including:	Appendix D
A)	Water table depth;	Appendix D
B)	Hydraulic conductivities;	Appendix D
C)	Effective and total porosities;	Appendix D
D)	Direction and velocity of groundwater flow; and	Appendix D
E)	Map of the potentiometric surface;	Appendix D
17)	Groundwater classification under 35 Ill. Adm. Code 620; and	Appendix D
18)	Any other information requested by the Agency that is relevant to the hydrogeologic site characterization	Appendix D
Section 845.630 - Groundwater Monitoring Systems (excerpts providing requirements for the construction permit application)		
g)	The owner or operator must obtain a certification from a qualified professional engineer stating that the groundwater monitoring system has been designed and constructed to meet the requirements of this Section. If the groundwater monitoring system includes the minimum number of monitoring wells specified in subsection (c)(1), the certification must document the basis supporting this determination. The certification must be submitted to the Agency with the appropriate permit application.	Appendix H
Section 845.640 - Groundwater Sampling and Analysis Requirements (excerpts providing requirements for the construction permit application)		
a)	The groundwater monitoring program must include consistent sampling and analysis procedures that are designed to ensure monitoring results that provide an accurate representation of groundwater quality at the background and downgradient wells required by Section 845.630. The owner or operator of the CCR surface impoundment must develop a sampling and analysis program that includes procedures and techniques for:	Appendix H
b)	The groundwater monitoring program must include sampling and analysis methods that are appropriate for groundwater sampling and that accurately measure constituents and other monitoring parameters in groundwater samples.	Appendix H
1)	Sample collection;	Appendix H
2)	Sample preservation and shipment;	Appendix H
3)	Analytical procedures;	Appendix H
4)	Chain of custody control; and	Appendix H
5)	Quality assurance and quality control	Appendix H
f)	Statistical Methods	Appendix H
1)	The owner or operator of the CCR surface impoundment must select one of the statistical methods specified in subsection (f)(1) to be used in evaluating groundwater monitoring data for each specified constituent. The statistical test chosen must be conducted separately for each constituent in each monitoring well.	Appendix H
2)	The owner or operator of the CCR surface impoundment must obtain a certification from a qualified professional engineer stating that the selected statistical method is appropriate for evaluating the groundwater monitoring data for the CCR surface impoundment. The certification must include a narrative description of the statistical method selected to evaluate the groundwater monitoring data. The certification must be submitted to the Agency with the appropriate permit application.	Appendix H
Section 845.710 - Closure Alternatives (excerpts providing requirements for the construction permit application)		
b)	Before selecting a closure method, the owner or operator of each CCR surface impoundment must complete a closure alternatives analysis. The closure alternatives analysis must examine the following for each closure alternative:	Appendix G
1)	The long- and short-term effectiveness and protectiveness of the closure method, including identification and analyses of the following factors:	Appendix G
A)	The magnitude of reduction of existing risks;	Appendix G
B)	The magnitude of residual risks in terms of likelihood of future releases of CCR	Appendix G
C)	The type and degree of long-term management required, including monitoring, operation, and maintenance;	Appendix G
D)	The short-term risks that might be posed to the community or the environment during implementation of such a closure, including potential threats to human health and the environment associated with excavation, transportation, and re-disposal of contaminants;	Appendix G
E)	The time until closure and post-closure care or the completion of groundwater monitoring under Section 845.740(b) is completed;	Appendix G
F)	The potential for exposure of humans and environmental receptors to remaining wastes, considering the potential threat to human health and the environment associated with excavation, transportation, re-disposal, containment or changes in groundwater flow;	Appendix G
G)	The long-term reliability of the engineering and institutional controls, including an analysis of any off-site, nearby destabilizing activities; and	Appendix G
H)	Potential need for future corrective action of the closure alternative	Appendix G

Regulation	Requirement	Location in Permit Application
Section 845.710 - Closure Alternatives (excerpts providing requirements for the construction permit application) (Continued)		
2)	The effectiveness of the closure method in controlling future releases based on analyses of the following factors:	Appendix G
A)	The extent to which containment practices will reduce further releases; and	Appendix G
B)	The extent to which treatment technologies may be used	Appendix G
3)	The ease or difficulty of implementing a potential closure method based on analyses of the following types of factors:	Appendix G
A)	Degree of difficulty associated with constructing the technology	Appendix G
B)	Expected operational reliability of the technologies	Appendix G
C)	Need to coordinate with and obtain necessary approvals and permits from other agencies;	Appendix G
D)	Availability of necessary equipment and specialists; and	Appendix G
E)	Available capacity and location of needed treatment, storage, and disposal services	Appendix G
4)	The degree to which the concerns of the residents living within communities where the CCR will be handled, transported and disposed are addressed by the closure method	Appendix G
c)	In the closure alternative analysis, the owner or operator of the CCR surface impoundment must:	Appendix G
1)	Analyze complete removal of the CCR as one closure alternative, along with the modes for transporting the removed CCR, including by rail, barge, low-polluting trucks, or a combination of these transportation modes;	Appendix G
2)	Identify whether the facility has an onsite landfill with remaining capacity that can legally accept CCR, and, if not, whether constructing an onsite landfill is possible; and	Appendix G
3)	Include any other closure method in the alternatives analysis if requested by the Agency.	Appendix G
d)	The analysis for each alternative completed under this Section must:	Appendix G
1)	Meet or exceed a class 4 estimate under the AACE Classification Standard, incorporated by reference in Section 845.150, or a comparable classification practice as provided in the AACE Classification Standard	Appendix G
2)	Contain the results of groundwater contaminant transport modeling and calculations showing how the closure alternative will achieve compliance with the applicable groundwater protection standards;	Appendix G
3)	Include a description of the fate and transport of contaminants with the closure alternative over time including consideration of seasonal variations; and	Appendix G
4)	Assess impacts to waters in the state.	Appendix G
e)	At least 30 days before submission of a construction permit application for closure, the owner or operator of the CCR surface impoundment must discuss the results of the closure alternatives analysis in a public meeting with interested and affected parties as required by Section 845.240	Appendix K
f)	After completion of the public meeting under subsection (e), the owner or operator of a CCR surface impoundment must select a closure method and submit a final closure plan to the Agency under Section 845.720(b). All materials demonstrating completion of the closure alternatives analysis specified in this Section must be submitted with the final closure plan.	Appendix G
Section 845.720 - Closure Plan (excerpts providing requirements for the construction permit application)		
b)	Final Closure Plan	Appendix G
1)	The owner or operator of a CCR surface impoundment must submit to the Agency, as a part of a construction permit application for closure, a final closure plan. The plan must be submitted before the installation of a final cover system or removal of CCR from the surface impoundment for the purpose of closure.	Appendix G
3)	The final closure plan must identify the proposed selected closure method, and must include the information required in subsection (a)(1) and the closure alternatives analysis as specified in Section 845.710.	Appendix G
5)	The owner or operator of the CCR surface impoundment must obtain and submit with its construction permit application for closure a written certification from a qualified professional engineer that the final written closure plan meets the requirements of this Part.	Appendix G
Section 845.750 - Closure with a Final Cover System (excerpts providing requirements for the construction permit application)		
c)	Final Cover System. If a CCR surface impoundment is closed by leaving CCR in place, the owner or operator must install a final cover system that is designed to minimize infiltration and erosion, and, at a minimum, meets the requirements of this subsection (c). The final cover system must consist of a low permeability layer and a final protective layer. The design of the final cover system must be included in the preliminary and final written closure plans required by Section 845.720 and the construction permit application for closure submitted to the agency.	Appendix G
Section 845.780 - Post-Closure Care Requirements (excerpts providing requirements for the construction permit application)		
d)	Written Post-Closure Care Plan	Appendix I
1)	Content of the Plan. The owner or operator of a CCR surface impoundment must prepare a written post-closure care plan that includes, at a minimum, the information specified in this subsection (d)(1):	Appendix I
A)	A description of the monitoring and maintenance activities required in subsection (b) for the CCR surface impoundment and the frequency at which these activities will be performed	Appendix I
B)	The name, address, telephone number, and email address of the person or office to contact about the facility during the post-closure care period; and	Appendix I
C)	A description of the planned uses of the property during the post-closure care period. Post-closure use of the property must not disturb the integrity of the final cover, liners, or any other component of the containment system, or the function of the monitoring systems unless necessary to comply with the requirements in this Part. Any other disturbance is allowed if the owner or operator of the CCR surface impoundment demonstrates that disturbance of the final cover, liner, or other component of the containment system, including any removal of CCR, will not increase the potential threat to human health or the environment. The demonstration must be certified by a qualified professional engineer and must be submitted to the Agency.	Appendix I
4)	The owner or operator of the CCR surface impoundment must obtain a written certification from a qualified professional engineer that the initial, and any amendment of the, written post-closure care plan meets the requirements of this Section.	Appendix I

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History of Construction



October 2016

Illinois Power Generating Company
134 CIPS Lane
Coffeen, IL 62017

**RE: History of Construction
USEPA Final CCR Rule, 40 CFR § 257.73(c)
Coffeen Power Station
Coffeen, Illinois**

On behalf of Illinois Power Generating Company, AECOM has prepared the following history of construction for Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond at the Coffeen Power Station in accordance with 40 CFR § 257.73(c).

BACKGROUND

40 CFR § 257.73(c)(1) requires the owner or operator of an existing coal combustion residual (CCR) surface impoundment that either (1) has a height of five feet or more and a storage volume of 20 acre-feet or more, or (2) has a height of 20 feet or more to compile a history of construction by October 17, 2016 that contains, to the extent feasible, the information specified in 40 CFR § 257.73(c)(1)(i)–(xii).

The history of construction presented herein was compiled based on existing documentation, to the extent that it is reasonably and readily available (see 80 Fed. Reg. 21302, 21380 [April 17, 2015]), and AECOM's site experience. AECOM's document review included record drawings, geotechnical investigations, construction specifications, etc. for Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond at the Coffeen Power Station.

HISTORY OF CONSTRUCTION

§ 257.73(c)(1)(i): The name and address of the person(s) owning or operating the CCR unit; the name associated with the CCR unit; and the identification number of the CCR unit if one has been assigned by the state.

Owner: Illinois Power Generating Company

Address: 1500 Eastport Plaza Drive
Collinsville, IL 62234

CCR Units: Ash Pond No. 1
Ash Pond No. 2
GMF Pond, IDNR Dam ID No. IL50579
GMF Recycle Pond, IDNR Dam ID No. IL50578

Ash Pond No. 1 and Ash Pond No. 2 do not have a state assigned identification number.

§ 257.73(c)(1)(ii): The location of the CCR unit identified on the most recent USGS 7¹/₂ or 15 minute topographic quadrangle map or a topographic map of equivalent scale if a USGS map is not available.

The locations of Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond have been identified on an USGS 7-1/2 minute topographic quadrangle map in **Appendix A**.

§ 257.73(c)(1)(iii): A statement of the purpose for which the CCR unit is being used.

The following captures the purposes of the CCR units:

- Ash Pond No. 1 is being used to store and dispose of bottom ash and other-non-CCR waste and to clarify recycled process water for plant operations. Ash Pond No. 2 (inactive) was used to store and dispose of bottom ash and fly ash.
- The GMF Pond is being used to store and dispose of gypsum and to clarify recycled process water for plant operations.
- The GMF Recycle Pond was used to store and dispose of gypsum from the plant's scrubber operations prior to the in-service date of the GMF Pond in 2010. The GMF Recycle Pond currently only receives and stores clear process water from the GMF Pond.

Notice of intent to close Ash Pond No. 2 was provided in November, 2015.¹

¹ This history of construction report was prepared on a facility-wide basis for CCR surface impoundments at the Coffeen Power Station. The inclusion of Ash Pond No. 2 in this history of construction report does not concede and should not be construed to concede that Ash Pond No. 2 is subject to the Design Criteria or all Operating Criteria in the CCR Rule.

§ 257.73(c)(1)(iv): The name and size in acres of the watershed where the CCR unit is located.

Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond are located in the Coffeen Lake Watershed with a 12-digit Hydrologic Unit Code (HUC) of 071402030304 and a drainage area of 11,695 acres (USGS, 2016).

§ 257.73(c)(1)(v): A description of the physical and engineering properties of the foundation and abutment materials on which the CCR unit is constructed.

The foundation and abutment materials of Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond consist of native fine-grained soils of wind-blown origin (loess), with some coarse-grained layers, underlain by glacial till. The physical properties of the fine-grained soils are described as low- to medium-plasticity silty clay, sandy lean clay, or lean clay with sand, often with trace amounts of gravel; or high plasticity fat clay, often with trace amounts of sand. The clay soils vary from soft to very stiff, moist to wet, and brown to gray. The physical properties of the coarse-grained soils are described as clayey sand, silty sand, or fine to coarse sand, with trace amounts of gravel. The sand is wet and varies from loose to dense and brown to gray. A thin layer of native silty or sandy lean clay is located immediately above the glacial till deposits. The clay is very soft to medium stiff, low to medium plasticity, wet, and orange brown to gray. The physical properties of the glacial till are described as lean clay, or silty to sandy lean clay, with trace amounts of fine gravel, hard, low plasticity, moist to wet, and brown to gray. An available summary of the engineering property typical ranges of the foundation and abutment materials is presented in **Table 1** below. The engineering properties are based on previous geotechnical explorations and laboratory testing.

Ash Pond No. 1 and Ash Pond No. 2 are enclosed impoundments with embankments and do not have abutments. The GMF Pond and GMF Recycle Pond were constructed as incised impoundments enclosed by embankments.

Table 1. Summary of Foundation and Abutment Material Engineering Properties

Material	Unit Weight (pcf)	Effective (drained) Shear Strength Parameters		Total (undrained) Shear Strength Parameters	Post-Earthquake Shear Strength
		Cohesion, c' (psf)	Friction Angle, ϕ' (deg)	S_u/p'	S_{ur}/p'
Foundation Clay (Under Embankment)	125	0	32	$S_u/p' = 0.39-0.45$, Min. $S_u = 700$ psf	Peak Undrained
Foundation Clay (Free Field)	125	0	30	$S_u/p' = 0.24-0.28$, Min. $S_u = 450$ psf	Peak Undrained
Soft Foundation Clay	125	0	30	$S_u/p' = 0.22-0.28$, Min. $S_u = 275$ psf	$S_u/p' = 0.13-0.16$, Min. $S_{ur} = 200$ psf
Glacial Till	135	0	40	$S_u/p' = 0.45-0.64$, Min. $S_u = 700$ psf	Peak Undrained

§ 257.73(c)(1)(vi): A statement of the type, size, range, and physical and engineering properties of the materials used in constructing each zone or stage of the CCR unit; the method of site preparation and construction of each zone of the CCR unit; and the approximate dates of construction of each successive stage of construction of the CCR unit.

Physical properties for the embankment construction materials for Ash Pond No. 1, Ash Pond No. 2, GMF Pond, and GMF Recycle Pond are described as silty clay, sandy lean clay, or lean clay with sand, with trace amounts of fine gravel. The fill is soft to very stiff in consistency, low to medium plasticity, moist to wet, and brown to gray. Trace amounts of organic material and ash are present. The embankment fill generally appears to be well-compacted. An available summary of the engineering properties of the embankment construction materials is presented in **Table 2** below. The engineering properties are based on previous geotechnical explorations and laboratory testing.

Table 2. Summary of Construction Material Engineering Properties for Embankments

Material	Unit Weight (pcf)	Peak Drained Shear Strength		Peak Undrained Shear Strength	Post-Earthquake Shear Strength
		Cohesion, c' (psf)	Friction Angle, f' (deg)	S_u/p'	S_{ur}/p'
Embankment Fill	135	0	31	$S_u/p' = 0.60$, Min. $S_u = 450$ psf	Peak Undrained

The GMF Pond and GMF Recycle Pond contain liner systems. The liner system within the GMF Pond consists of a 60-mil textured high density polyethylene (HDPE) geomembrane underlain by a 3-foot thick layer of compacted clay. A typical cross section profile of the GMF Pond liner system is shown on drawing C-10206 (sh. 9) presented in **Appendix B**. An available summary of the engineering properties of the GMF Pond liner construction materials from Hanson (2008) is presented in **Table 3** below. The liner system within the GMF Recycle Pond consists of a 60-mil textured HDPE geomembrane underlain by smooth-drum rolled native soil. A typical cross section profile of the GMF Recycle Pond liner system is shown on drawing C-10206 (sh. 20) presented in **Appendix B**.

Table 3. Summary of Construction Material Engineering Properties for Liner

Material	Unit Weight (pcf)	Effective (drained) Shear Strength Parameters		Total (undrained) Shear Strength Parameters	
		c' (psf)	Φ' (°)	c (psf)	Φ (°)
Clay Liner	121.2	0	28.3	1950	0

The method of site preparation and construction of Ash Pond No. 1 and Ash Pond No. 2 is not reasonably and readily available. Site preparation and construction of the GMF Pond and GMF Recycle Pond were completed in accordance with the applicable construction specification (see § 257.73(c)(1)(xi) below).

The approximate dates of construction of each successive stage of construction of Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond are provided in **Table 4** below.

Table 4. Approximate dates of construction of each successive stage of construction.

Date	Event
1964	Construction of Ash Pond No. 1
1971	Construction of Ash Pond No. 2
1978-1979	Installation of internal embankment and new recycle intake structure in Ash Pond No. 1 and abandonment of existing outfall structure
1984-1985	Closure of Ash Pond No. 2 by installing a clay cover
2000	Installation of a sheet pile wall to facilitate construction of drainage flume along the northeast corner of the Ash Pond No. 1
2009	Installation of well dewatering system in Ash Pond No. 2
2008-2010	Construction of the GMF Pond and the GMF Recycle Pond

§ 257.73(c)(1)(vii): At a scale that details engineering structures and appurtenances relevant to the design, construction, operation, and maintenance of the CCR unit, detailed dimensional drawings of the CCR unit, including a plan view and cross sections of the length and width of the CCR unit, showing all zones, foundation improvements, drainage provisions, spillways, diversion ditches, outlets, instrument locations, and slope protection, in addition to the normal operating pool surface elevation and the maximum pool surface elevation following peak discharge from the inflow design flood, the expected maximum depth of CCR within the CCR surface impoundment, and any identifiable natural or manmade features that could adversely affect operation of the CCR unit due to malfunction or mis-operation.

Drawings that contain items pertaining to the requested information for Ash Pond No. 1, Ash Pond No. 2, the GMF Pond, and the GMF Recycle Pond are listed in **Table 5** below. Items marked as "Not Available" are items not found during a review of the reasonably and readily available record documentation.

Table 5. List of drawings containing items pertaining to the information requested in § 257.73(c)(1)(vii).

	Ash Pond No. 1	Ash Pond No. 2	GMF Pond	GMF Recycle Pond
Dimensional plan view (all zones)	B-35, S-44, S-45	B-560, A1000 (sh. 1)	C-10206 (sh. 4, 9, 10)	C-10206 (sh. 4, 19)
Dimensional cross sections	B-35, S-47 to S-50	B-561	C-10206 (sh. 9)	C-10206 (sh. 20)
Foundation Improvements	Not Applicable	Not Applicable	C-10206 (sh. 10)	C-10206 (sh. 20)
Drainage Provisions	Not Applicable	A1000 (sh. 4)	C-10206 (sh. 15, 16, 20)	C-10206 (sh. 21)
Spillways and Outlets	S-8, S-49	W1008 (sh. 2)	C-10206 (sh. 20)	C-10206 (sh. 22)
Diversion Ditches	Not Applicable	A1000 (sh. 1)	Not Applicable	Not Applicable
Instrument Locations	Plate 2, Figure 2A	Figure 2B	Not Applicable	C-10206 (sh. 19)
Slope Protection	S-49	B-561	C-10206 (sh. 9)	C-10206 (sh. 20)
Normal Operating Pool Elevation	S-8, S-49	Not Applicable	C-10201-25	Not Available
Maximum Pool Elevation	S-8	Not Applicable	C-10201-25	Not Available
Approximate Maximum Depth of CCR in 2016	15 feet	28 feet	16 feet	12 feet

All drawings referenced in **Table 5** above can be found in **Appendix B** and **Appendix C**.

Based on the review of the drawings listed above, no natural or manmade features that could adversely affect operation of these CCR units due to malfunction or mis-operation were identified.

§ 257.73(c)(1)(viii): A description of the type, purpose, and location of existing instrumentation.

Existing instrumentation at Ash Pond No. 1 and Ash Pond No. 2 include vibrating-wire and open-standpipe piezometers. The purpose of the piezometers is to measure the phreatic surface within and around the impoundments. Two (2) open-standpipe piezometers (AP-P1 and AP-P2) were installed at Ash Pond No. 2 in 2009 and the locations are presented on Figure 2A in **Appendix C**. Two (2) open-standpipe piezometers (B-2 and B-4) were installed at Ash Pond No. 1 in 2010 and the locations are presented on Plate 2 in **Appendix C**. Twelve (12) open-standpipe and vibrating-wire piezometers were installed at Ash Pond No. 1 and Ash Pond No. 2 in 2015 and the locations are presented on Figure 2A in **Appendix C**.

The GMF Pond does not contain existing instrumentation. Existing instrumentation at the GMF Recycle Pond consists of one (1) ultrasonic level transmitter. The purpose of the ultrasonic level transmitter is to measure the water level within the GMF Recycle Pond. The location of the ultrasonic level transmitter is shown on drawing C-10206 (sh. 19) presented in **Appendix B**.

§ 257.73(c)(1)(ix): Area-capacity curves for the CCR unit.

Area-capacity curves for Ash Pond No. 2 and the GMF Recycle Pond are not reasonably and readily available. The area-capacity curves for Ash Pond No. 1 and the GMF Pond are presented in **Figures 1 and 2**, respectively, below. “Area-capacity curves”, as defined by 40 CFR § 257.53, “means graphic curves which readily show the reservoir water surface area, in acres, at different elevations from the bottom of the reservoir to the maximum water surface, and the capacity or volume, in acre-feet, of the water contained in the reservoir at various elevations.”

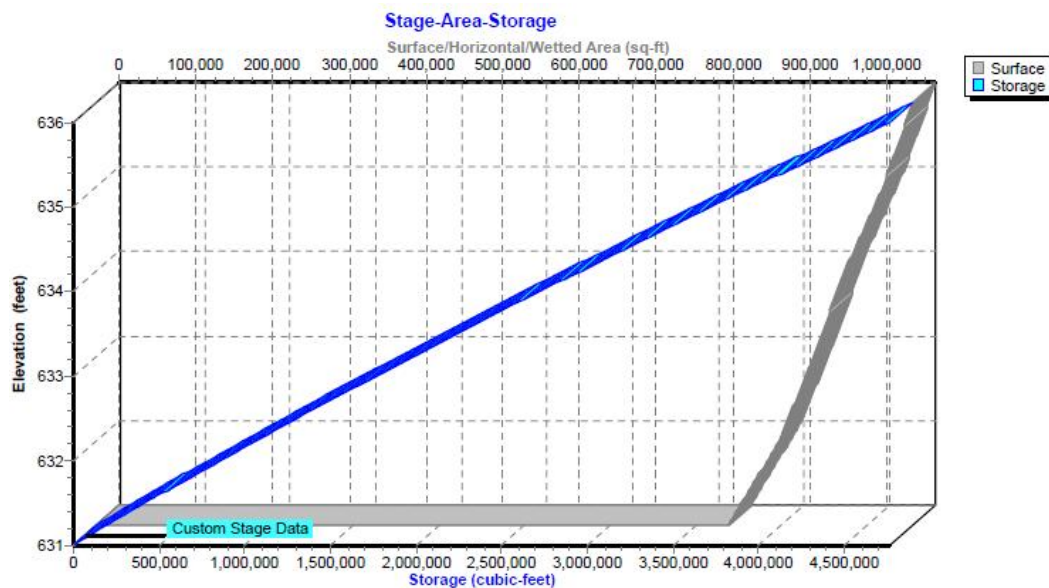


Figure 1. Area-capacity curve for Ash Pond No. 1

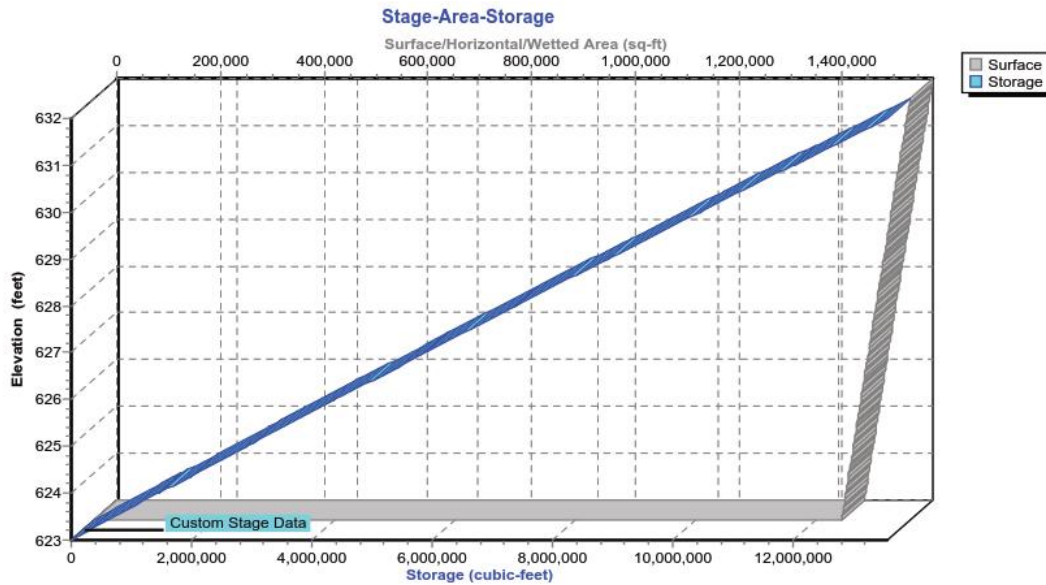


Figure 2. Area-capacity curve for GMF Pond

The area-capacity curves shown were taken from the pond modeling analysis. Actual pond capacity is limited to the approximate berm elevation listed in **Table 6** below. Any information above berm elevation should be disregarded.

§ 257.73(c)(1)(x): A description of each spillway and diversion design features and capacities and calculations used in their determination.

Ash Pond No. 1 contains a concrete intake structure that drains into a 48-inch diameter (dia.) steel pipe. The steel pipe leads to the recycle pump house. In 2016, the discharge capacity of Ash Pond No. 1 was evaluated using HydroCAD 10 software modeling a 1,000-year, 24-hour rainfall event. The results of the HydroCAD 10 analysis are presented below in **Table 6**.

Ash Pond No. 2 was closed in 1984-1985 by installing a clay cover. Non-contact stormwater is collected in ditches along the clay cover and drain off the pond cover via concrete-lined ditch outlets. CCR-contact stormwater collected within the pond is pumped into the GMF Pond via the well dewatering system at the discretion of plant personnel. The capacity of the diversion ditches and well pumps during a model rainfall event has not been evaluated.

The GMF Pond contains a 14-inch high-density polyethylene (HDPE) pipe culvert for normal flow and a weir-like spillway for high water flow. The GMF Pond also contains a 10-inch dia. HDPE siphon pipe used for dewatering. In 2016, the discharge capacity of the GMF Pond was evaluated using HydroCAD 10 software modeling a 1,000-year, 24-hour rainfall event. The results of the HydroCAD 10 analysis are presented below in **Table 6**.

The GMF Recycle Pond contains a decant structure that drains into two (2) 18-inch dia. HDPE pipes that lead to a pump house. The capacity of the decant structure during a model rainfall event has not been evaluated.

Table 6. Results of HydroCAD 10 analyses

	Ash Pond No. 1	GMF Pond
Approximate Minimum Berm Elevation ¹ (ft)	635.0	631.0
Approximate Emergency Spillway Elevation ¹ (ft)	Not Applicable	624.0
Starting Pool Elevation ¹ (ft)	631.0	621.2
Peak Elevation ¹ (ft)	632.0	623.8
Time to Peak (hr)	24.4	24.1
Surface Area (ac)	20.4	33.4
Storage ² (ac-ft)	19.5	88.3

Note: 1. Elevations are based on NAVD88 datum
 2. Storage given is from Starting Pool Elevation to Peak Elevation.

§ 257.73(c)(1)(xi): The construction specifications and provisions for surveillance, maintenance, and repair of the CCR unit.

The construction specifications for Ash Pond No. 1 and Ash Pond No. 2 are not reasonably and readily available. The construction specifications for the GMF Pond and the GMF Recycle Pond are located in *Project Specifications, Gypsum Stack and Recycle Pond Construction* presented in **Appendix D**.

The provisions for surveillance, maintenance, and repair of Ash Pond No. 1 are located in *Operation & Maintenance Manual for #1 Ash Pond* presented in **Appendix E**. The provisions for surveillance, maintenance, and repair of Ash Pond No. 2 are not reasonably and readily available. The provisions for surveillance, maintenance, and repair of the GMF Pond and the GMF Recycle Pond are located in *Operation and Maintenance Manual, Gypsum Management Facility* presented in **Appendix F**.

The operations and maintenance plans for the CCR units identified in this report are currently being revised by Illinois Power Generating Company.

§ 257.73(c)(1)(xii): Any record or knowledge of structural instability of the CCR unit.

In March, 2009, shallow sloughing was observed along the eastern embankment of Ash Pond No. 2. The sloughing was inspected by Hanson Professional Services Inc. A dewatering

system was installed in Ash Pond No. 2 to lower the phreatic surface within the pond. In December, 2015, additional sloughing was observed on the embankment of Ash Pond No. 2 and on the embankment of Ash Pond No. 1. The sloughing was believed to be caused by recent heavy rains and was repaired. Photos of the 2015 sloughing repair are presented in **Appendix G**.

There is no record or knowledge of structural instability at the GMF Pond and the GMF Recycle Pond at Coffeen Power Station.

LIMITATIONS

The signature of AECOM's authorized representative on this document represents that to the best of AECOM's knowledge, information and belief in the exercise of its professional judgment, it is AECOM's professional opinion that the aforementioned information is accurate as of the date of such signature. Any recommendation, opinion or decisions by AECOM are made on the basis of AECOM's experience, qualifications and professional judgment and are not to be construed as warranties or guaranties. In addition, opinions relating to environmental, geologic, and geotechnical conditions or other estimates are based on available data and that actual conditions may vary from those encountered at the times and locations where data are obtained, despite the use of due care.

Sincerely,



Claudia Prado
Program Manager



Victor Modeer, P.E., D.GE
Senior Project Manager

REFERENCES

Hanson Professional Services Inc. (2008), *Support Document for IDNR/OWR Permit Application, Coffeen Power Generating Station Gypsum Management Facility Montgomery County, Illinois*

United States Environmental Protection Agency (USEPA). (2015). *Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals From Electric Utilities; Final Rule*. 40 CFR Parts 257 and 261, 80 Fed. Reg. 21302, 21380 April 17, 2015.

United States Geological Survey (USGS). (2016). The National Map Viewer. <http://viewer.nationalmap.gov/viewer/>. USGS data first accessed in March of 2016.

APPENDICES

Appendix A: History of Construction Vicinity Map

Appendix B: Coffeen Power Station Drawings

Appendix C: Coffeen Power Station Boring and Piezometer Locations

Appendix D: Project Specifications, Gypsum Stack and Recycle Pond Construction (Hanson 2008)

Appendix E: Operation & Maintenance Manual for #1 Ash Pond

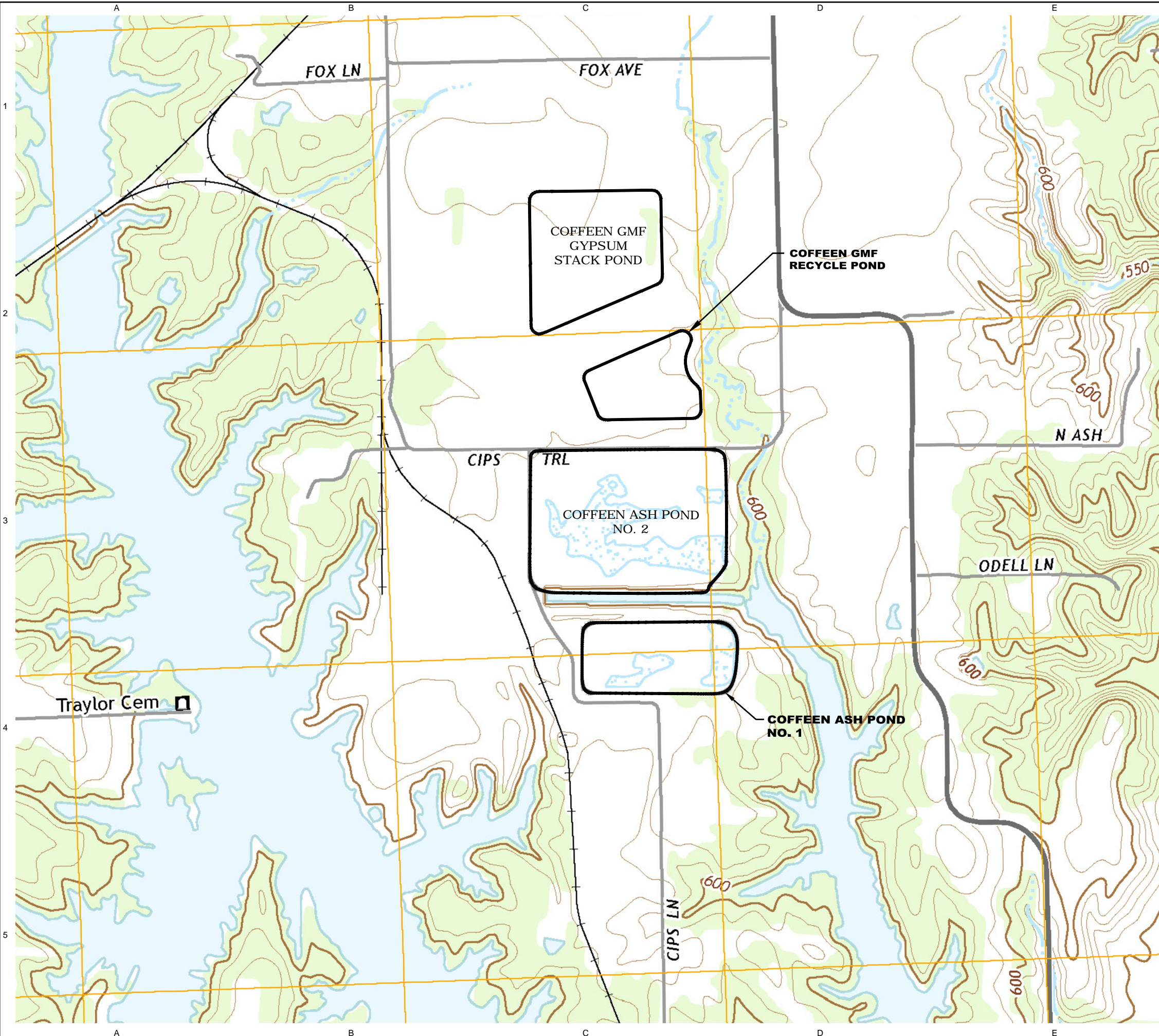
Appendix F: Operation and Maintenance Manual, Gypsum Management Facility Operation (2015)

Appendix G: Photos of 2015 Sloughing Repair



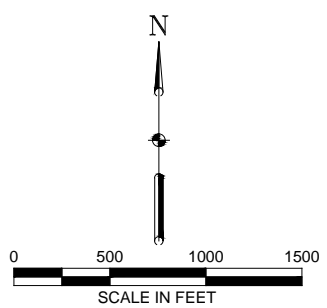
Appendix A: History of Construction Vicinity Map

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LEGEND
 CCR UNITS

SOURCE:
 MAP PROVIDED FROM ELECTRONIC USGS DIGITAL RASTER GRAPHIC 7.5 MINUTE TOPOGRAPHIC MAP OF COFFEEN, ILLINOIS, REVISED 2015.



AECOM
 1001 Highlands Plaza Drive, Suite 300
 St. Louis, Mo. 63110
 314 429-0100 (phone)
 314 429-0462 (fax)

ILLINOIS POWER GENERATING COMPANY
 134 CIPS Lane
 Coffeen, IL 62017

HISTORY OF CONSTRUCTION
 COFFEEN POWER STATION
 COFFEEN, ILLINOIS

ISSUED FOR BIDDING _____ DATE BY _____

ISSUED FOR CONSTRUCTION _____ DATE BY _____

REVISIONS		
NO.	DESCRIPTION	DATE
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△		
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△		

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 DRAWN BY: DJD
 DESIGNED BY: DJD
 CHECKED BY: MN
 DATE CREATED: 2016-04-13
 PLOT DATE:
 SCALE: 1" = 500'
 ACAD VER: 2014

SHEET TITLE
HISTORY OF CONSTRUCTION VICINITY MAP

01



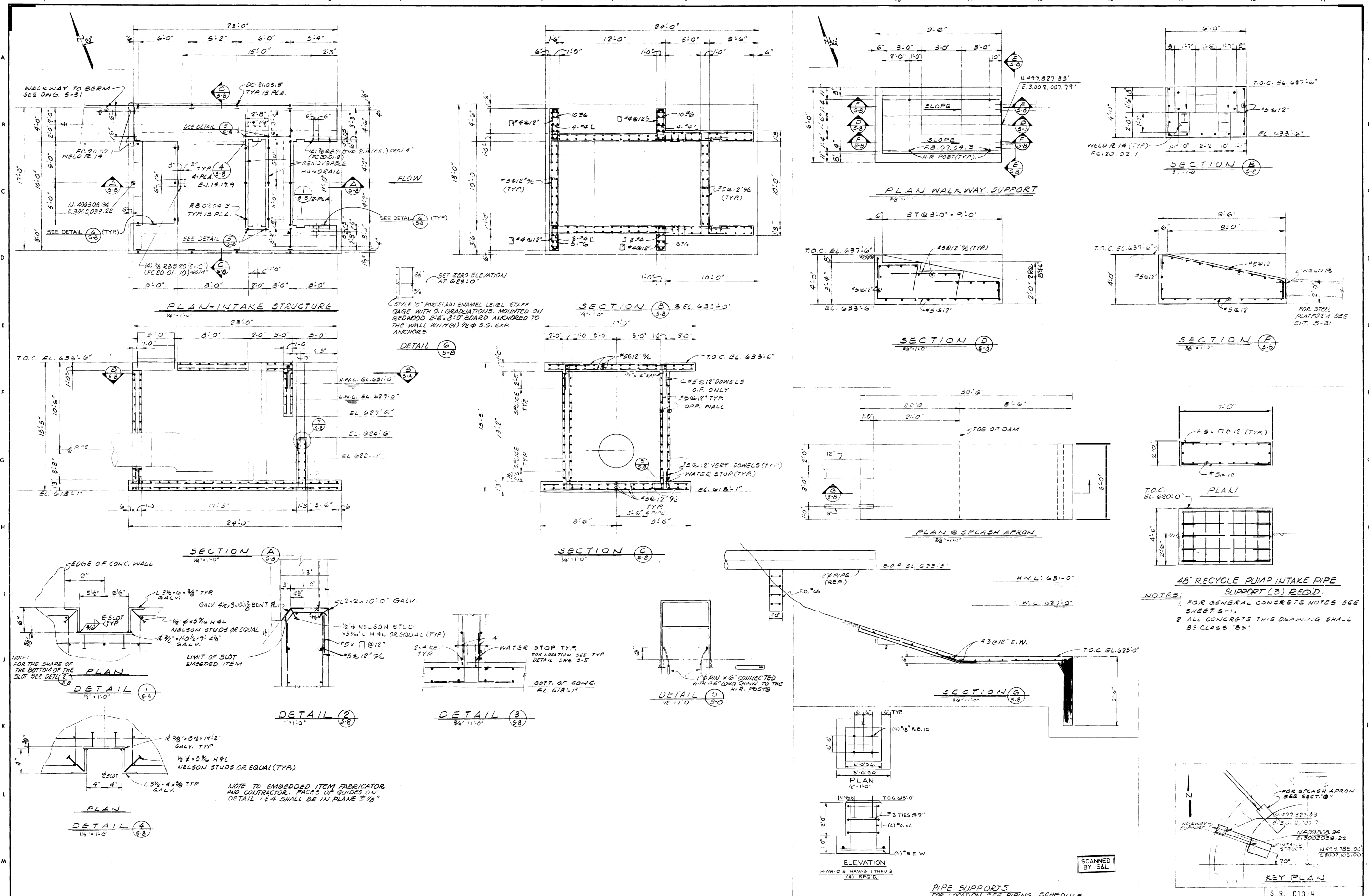
Appendix B: Coffeen Power Station Drawings

1. "Earthwork & Grading Plan", Drawing No. B-35, Revision S, 8 September, 1995, Sargent & Lundy Engineers.
2. "Concrete Recycle Pump House – Intake Structure", Drawing No. S-8, Revision 6, 23 February, 1996, Stearns-Roger Incorporated.
3. "Civil Layout & Grading Plan Sheet 4", Drawing No. S-44, Revision 6, 23 February, 1996, Stearns-Roger Incorporated.
4. "Civil Layout & Grading Plan Sheet 5", Drawing No. S-45, Revision 9, 23 February, 1996, Stearns-Roger Incorporated.
5. "Civil Miscellaneous Sections and Details, Sheet 2", Drawing No. S-47, Revision 2, 23 February, 1996, Stearns-Roger Incorporated.
6. "Civil Ash Pond No 1 – Sections and Details", Drawing No. S-48, Revision 2, 23 February, 1996, Stearns-Roger Incorporated.
7. "Civil Miscellaneous Sections and Details, Sheet 4", Drawing No. S-49, Revision 4, 23 February, 1996, Stearns-Roger Incorporated.
8. "Civil Miscellaneous Sections and Details", Drawing No. S-50, Revision 4, 23 February, 1996, Stearns-Roger Incorporated.
9. "Ash Storage Area, Plan", Drawing No. B-560, Revision A, 9 February, 1971, Sargent & Lundy Engineers.
10. "Ash Storage Area, Sections & Details", Drawing No. B-561, Revision A, 9 February, 1971, Sargent & Lundy Engineers.
11. "Overall Site Plan, Dewatering System, Ash Pond #2", Drawing No. A1000 (sh. 1), Revision A, 12 October, 2009, Ameren Energy Resources Generating.
12. "Site Details, Dewatering System, Ash Pond #2", Drawing No. A1000 (sh. 4), Revision A, 12 October, 2009, Ameren Energy Resources Generating.
13. "Proposed Site Plan, CCB Management Facility", Drawing No. C-10206 (sh. 4), Revision 0, 5 January, 2011, Ameren Energy Generating.
14. "Groundwater Monitoring & Boring Plan, CCB Management Facility", Drawing No. C-10206 (sh. 5), Revision 0, 5 January, 2011, Ameren Energy Generating.
15. "Anchor Trench and Liner System, CCB Management Facility", Drawing No. C-10206 (sh. 9), Revision 0, 5 January, 2011, Ameren Energy Generating.
16. "Cell G1-Foundation Grade & Control Data, CCB Management Facility", Drawing No. C-10206 (sh. 10), Revision 0, 5 January, 2011, Ameren Energy Generating.
17. "Cell G1-Process Water Recovery System, CCB Management Facility", Drawing No. C-10206 (sh. 15), Revision 0, 5 January, 2011, Ameren Energy Generating.
18. "Cell G1-PWRS Drain Details, CCB Management Facility", Drawing No. C-10206 (sh. 16), Revision 0, 5 January, 2011, Ameren Energy Generating.
19. "Recycle Pond Plan & Control Data, CCB Management Facility", Drawing No. C-10206 (sh. 19), Revision 0, 5 January, 2011, Ameren Energy Generating.
20. "Recycle Pond – Process Water Transfer Channel Details, CCB Management Facility", Drawing No. C-10206 (sh. 20), Revision 0, 5 January, 2011, Ameren Energy Generating.



Appendix B: Coffeen Power Station Drawings (continued)

21. "Recycle Pond–Process Water Decant Sections & Details, CCB Management Facility", Drawing No. C-10206 (sh. 21), Revision 0, 5 January, 2011, Ameren Energy Generating.
22. "Recycle Pond – Emergency Spillway Sections & Details, CCB Management Facility", Drawing No. C-10206 (sh. 22), Revision 0, 5 January, 2011, Ameren Energy Generating.
23. "Ash Pond #2, Drainage Modifications", Drawing No. W1008 (sh. 2), Ameren Energy Generating.



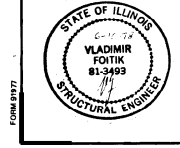
48" RECYCLE PUMP INTAKE PIPE SUPPORT (3) REQD.

NOTES:

- FOR GENERAL CONCRETE NOTES SEE SHEET S-1.
- ALL CONCRETE THIS DRAWING SHALL BE CLASS 'B5'.

PIPE SUPPORTS FOR LOCATION SEE PIPING SCHEDULE

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2	REVISED ELEV. ADDED NOTE. DIMS. SET. CALLOUT REINF.	5/1/18	TVT			M-2	YARD PIPING PLAN	02/01/18										
3	ADDED DETAIL 5. 2" W. REINFORCE. SUPPLEMENTARY BAR. REMOVE 2" S. PANEL. 1" S. S. REINFORCE. STRIKE SUPPORTS.	9/2/18	TVT			S-45	LAYOUT & GRADING PLAN											
4	REMOVED PIPE SUPPORTS & HANDRAILS.	4/1/18	TVT			M-43	YARD PIPING, SECTIONS & DETAILS											
5	FIELD REVISION	4/1/18	JFD															
6	SCANNED BY S&L	2/2/18																



SCANNED BY S&L

CONCRETE
 RECYCLE PUMP HOUSE - INTAKE STRUCTURE
 & MISCELLANEOUS FOUNDATIONS

WASTEWATER MANAGEMENT FACILITIES
 CENTRAL ILLINOIS PUBLIC SERVICE COMPANY
 COFFEEN POWER STATION

SCALE: AS NOTED

Stearns-Roger
 INCORPORATED

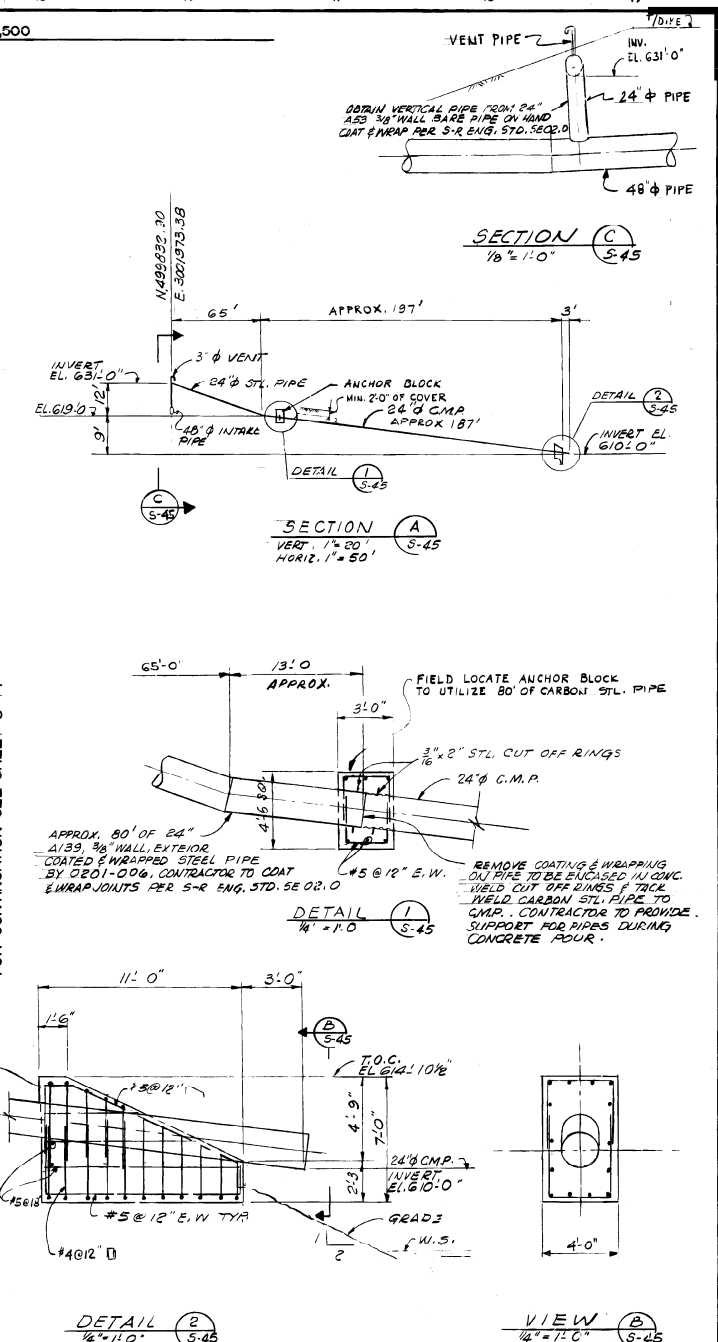
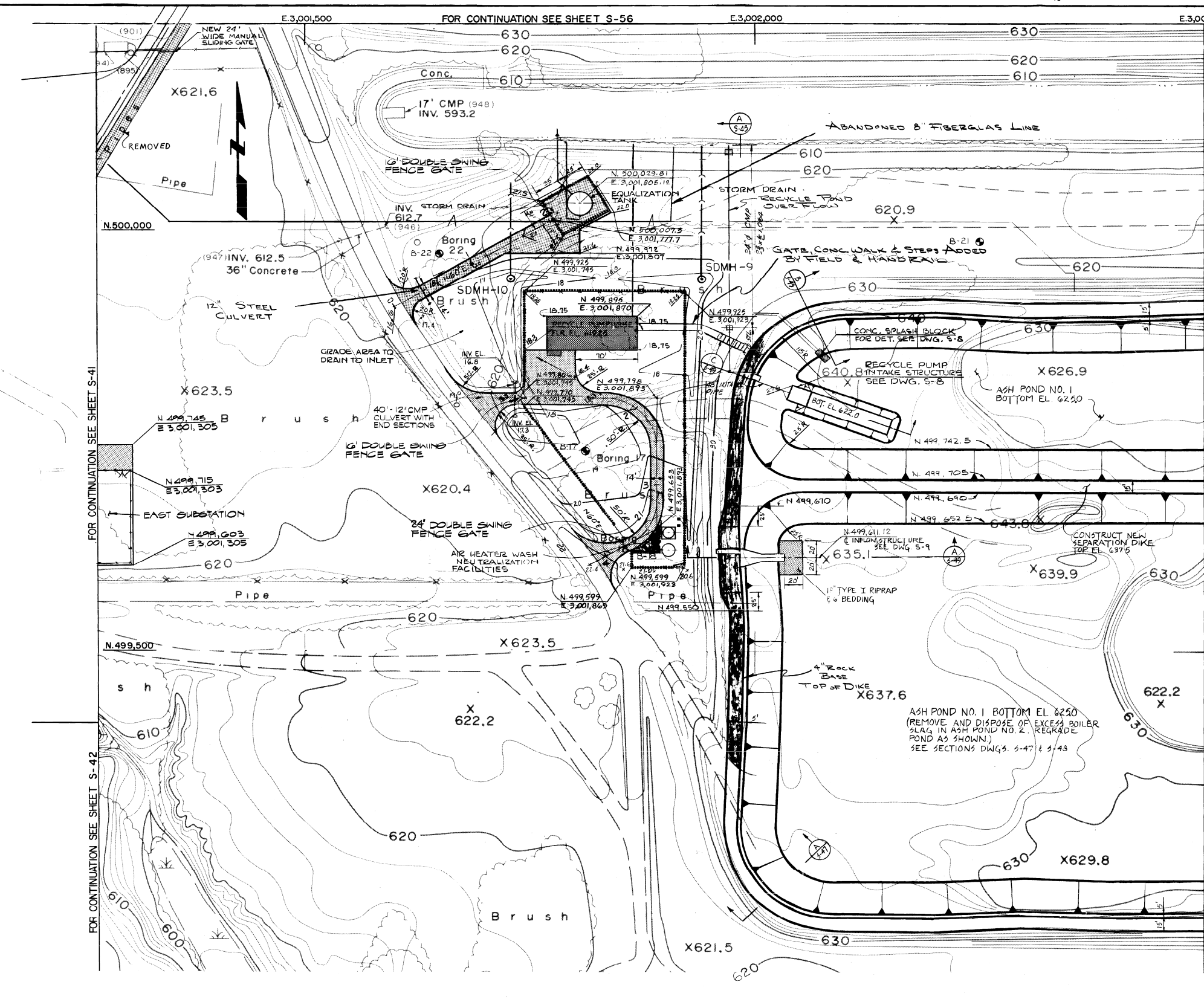
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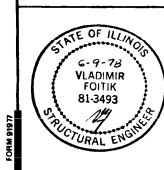
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SHEET NO. 3-8

REV. 1



- NOTES:**
1. FOR ROAD PROFILE AND SECTIONS SEE DWG S-53.
 2. FOR LEGEND SEE DWG S-40
 3. FOR ASH POND NO. 1 CROSS SECTIONS SEE DWGS S-47, S-48 & S-49.
 4. FOR STORM DRAIN PROFILE AND MANHOLES, SEE DWG S-52.
 5. FOR SITE PLAN SEE DWG S-40
 6. FOR FENCE DETAILS SEE DWG S-57



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SCANNED BY S&L

S.R. NO. Y2-5

CIVIL
LAYOUT & GRADING PLAN SHEET 5

WASTEWATER MANAGEMENT FACILITIES
CENTRAL ILLINOIS PUBLIC SERVICE COMPANY
COFFEE POWER STATION

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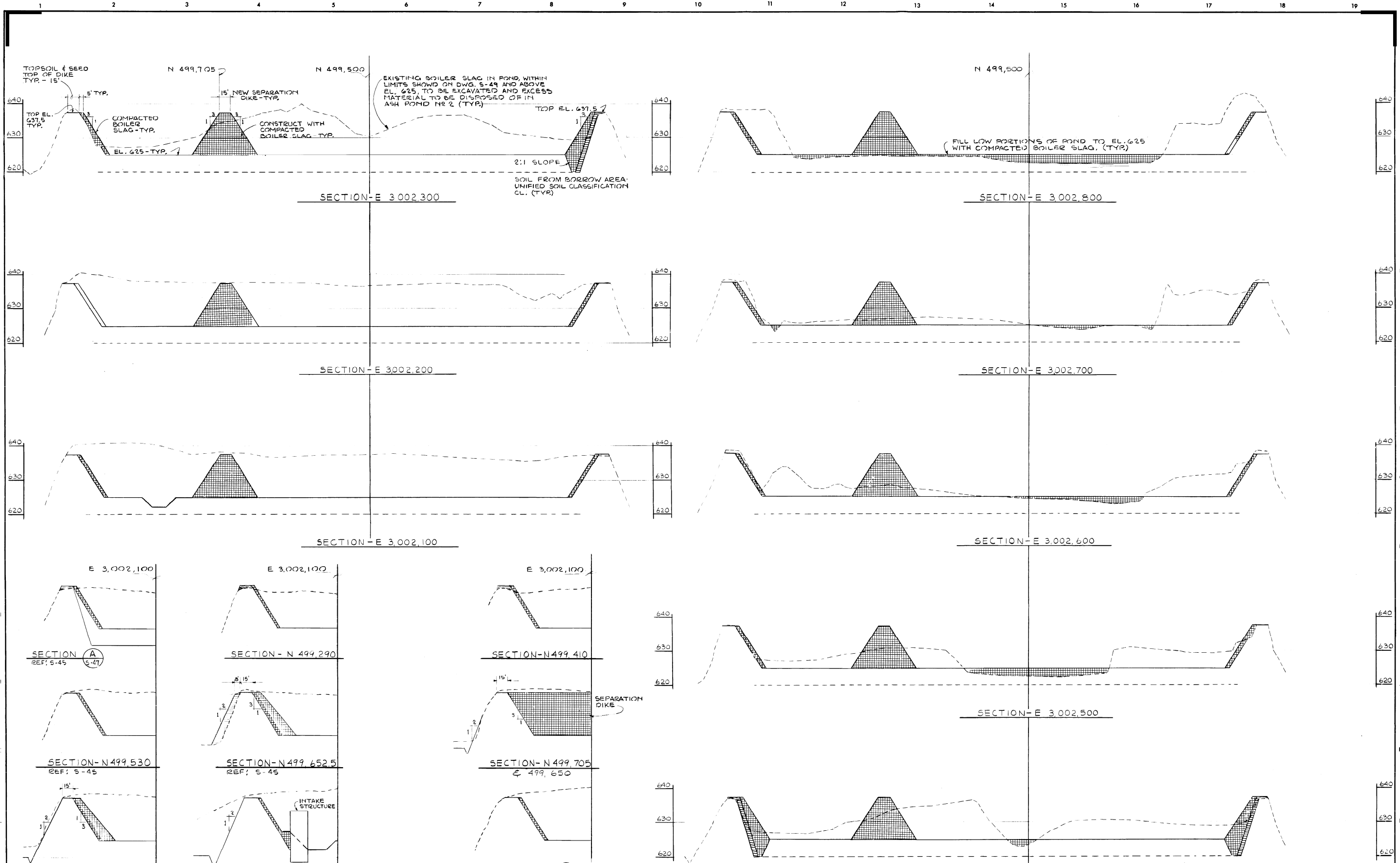
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INCORPORATED

ORDER NO. C-20000

DWG. NO. L-22886

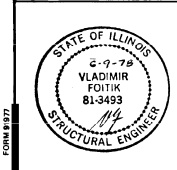
SHEET NO. S-45

REV. 1



NOTES:
 1. FOR LEGEND SEE DWG. S-40
 2. FOR ASH POND NO.1 PLANS SEE DWGS. S-44 AND S-45
 3. FOR SITE PLAN SEE DWG. S-40

SCANNED BY S&L



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SR NO Y2-7

DWG. NO. L-22886

CIVIL
MISCELLANEOUS SECTIONS AND DETAILS
SHEET 2

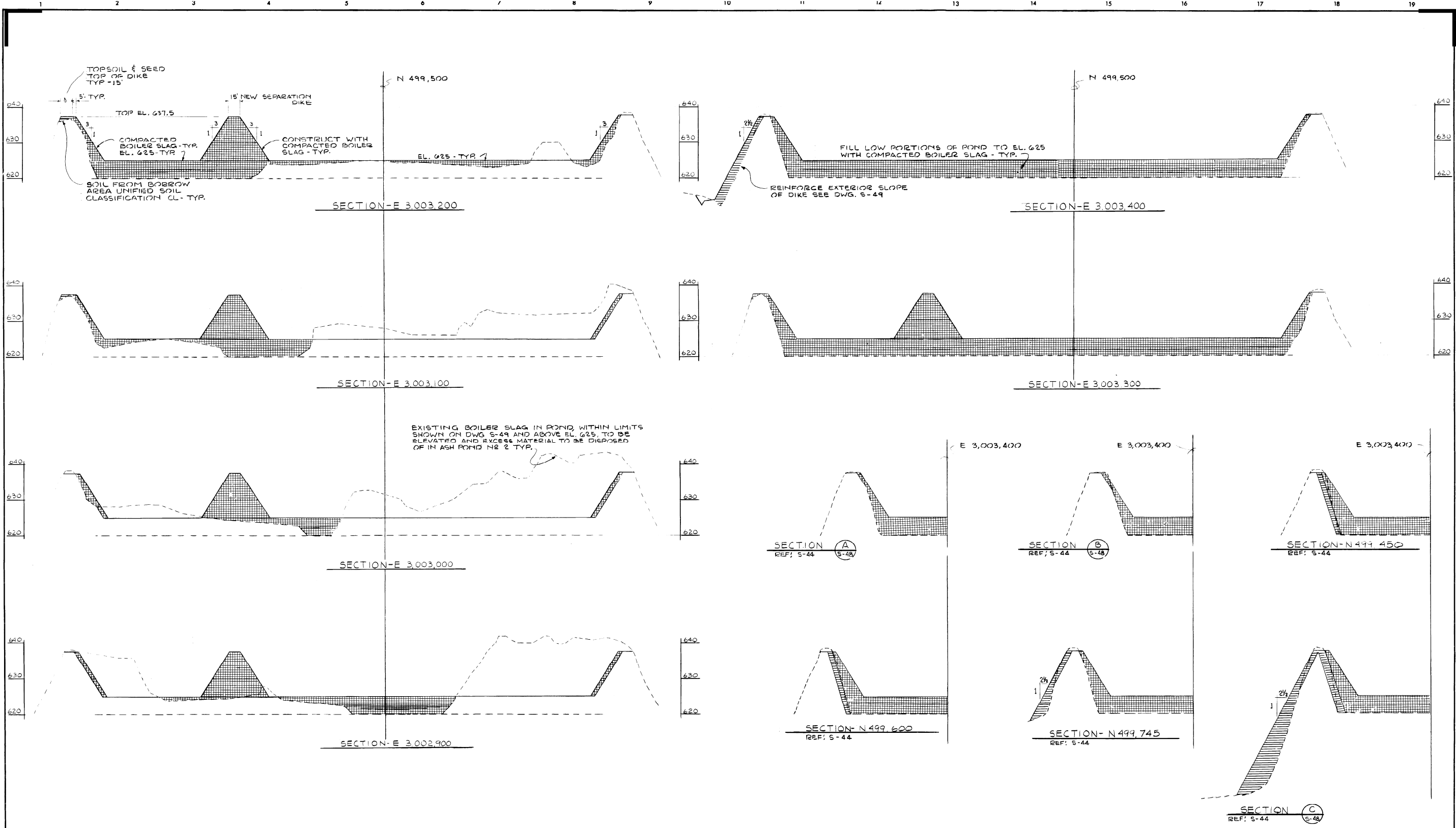
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CENTRAL ILLINOIS PUBLIC SERVICE COMPANY
COFFEE POWER STATION

0201
SHEET NO. S-47

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VERT 1"=10'

Stearns-Roger
INCORPORATED

ORDER NO. C-20000



- NOTES:
 1. FOR LEGEND SEE DWG. S-40
 2. FOR ASH POND N#1 PLANS SEE DWGS. S-44 AND S-45
 3. FOR SITE PLAN SEE DWG. S-40

SCANNED BY S&L



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2	SCANNED BY S&L	3/23/96																

SR NO Y2-8

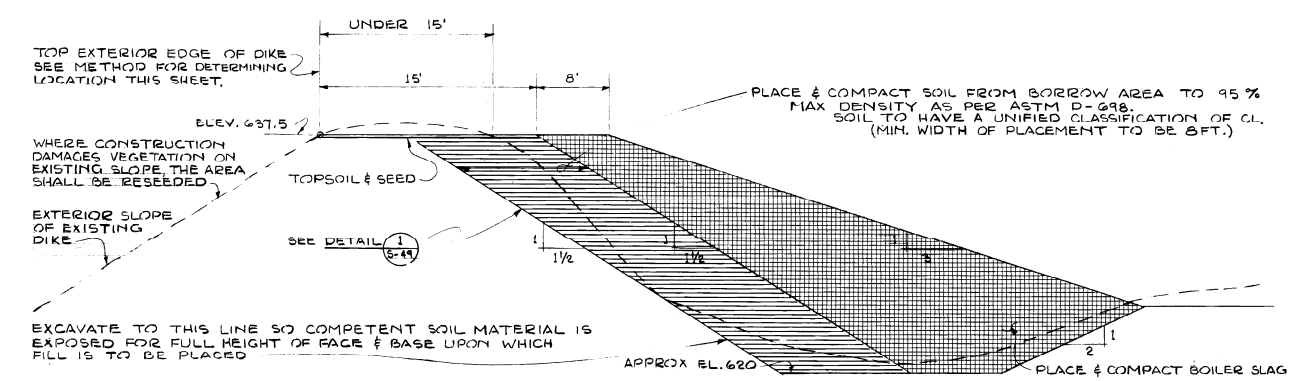
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ASH POND NO 1 - SECTIONS AND DETAILS

WASTEWATER MANAGEMENT FACILITIES
CENTRAL ILLINOIS PUBLIC SERVICE COMPANY
COFFEEN POWER STATION

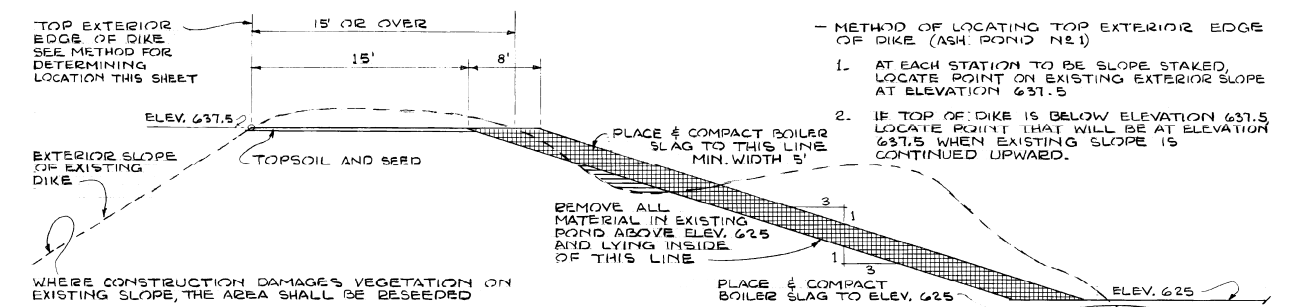
Stearns-Roger
INCORPORATED

DWS. NO. L-22888
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ORDER NO. C-20000

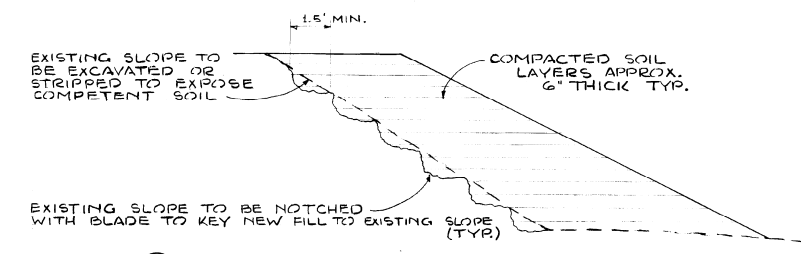
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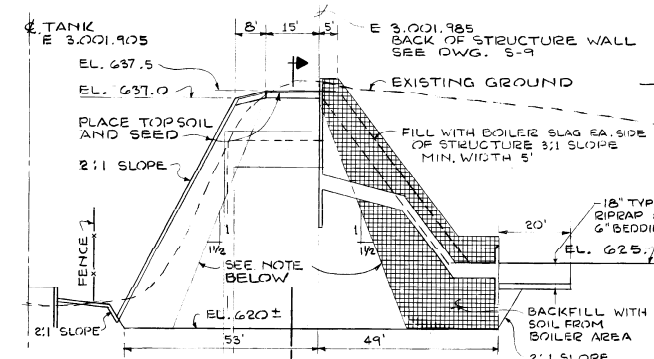
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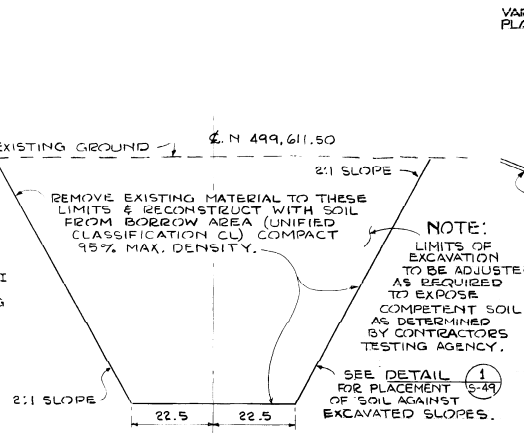
ASH POND No. 1 SCALE 1"=5'
TYPICAL DIKE SECTION WHERE WIDTH OF EXISTING DIKE AT ELEV. 637.5 IS 15' OR OVER



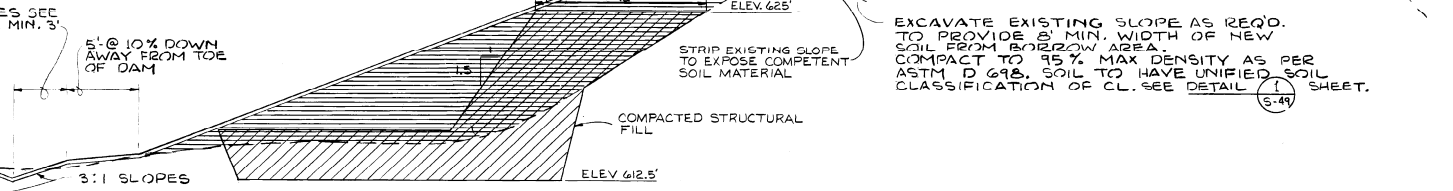
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PLACING NEW FILL AGAINST EXISTING SLOPE



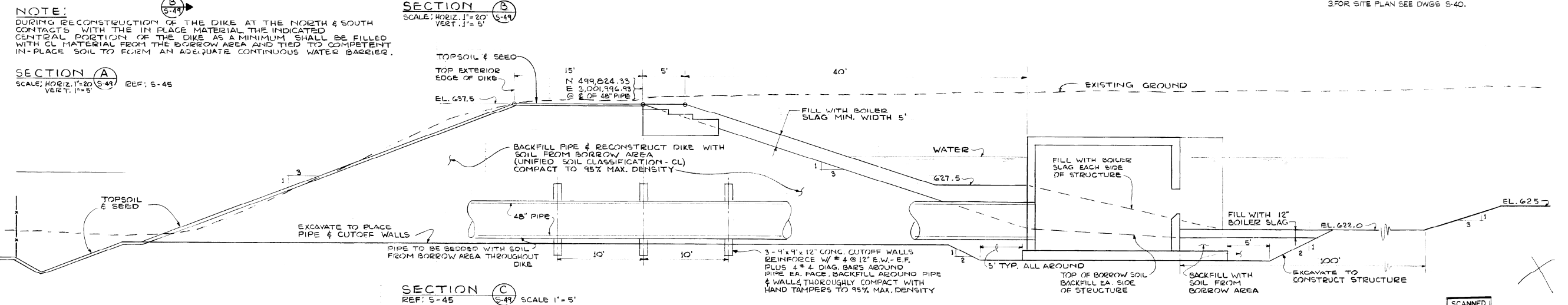
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SECTION B SCALE: HORIZ. 1"=20' VERT. 1"=5'



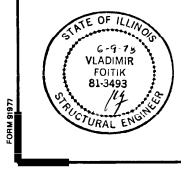
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SECTION A SCALE: HORIZ. 1"=20' VERT. 1"=5'

- NOTES:
1. FOR LEGEND SEE DWG S-40.
2. FOR ASH POND NO. 1 PLANS, SEE DWGS S-44 & S-45.
3. FOR SITE PLAN SEE DWGS S-40.

SCANNED BY S&L



NO.	REVISIONS	DATE	BY	CHKD	APPD	NO.
1	GENERAL REVISIONS	4/1/78	WJS			
2	NOTE CORRECTION, APPROVED FOR CONSTRUCTION	5/4/78	JKS			
3	FIELD REVISION	6-2-81	JFD			
4	SCANNED BY S&L	7/2/86				

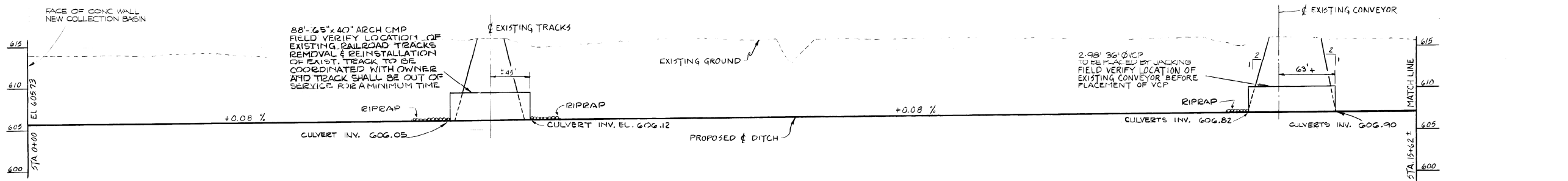
DATE ISSUED	REVISION NO.	FOR	CUSTOMER	FIELD	INTRA CO.	Q201-200
3/2/78	0	BID	4	-	7/15	6
4/1/78	1	BID	4	-	7/15	6
5/4/78	2	CONST	6	-	7/15	6

DATE	BY	CHKD	APPD	NO.
3/2/78	JKS			
4/1/78	JKS			
5/4/78	JKS			
6-2-81	JFD			
7/2/86				

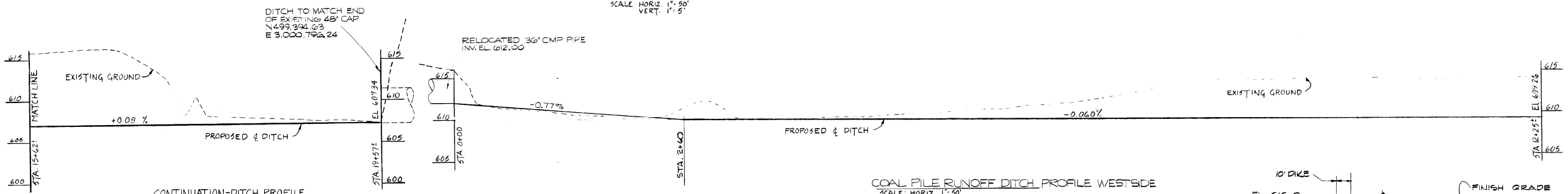
DATE	BY	CHKD	APPD	NO.
3/2/78	JKS			
4/1/78	JKS			
5/4/78	JKS			
6-2-81	JFD			
7/2/86				

DATE	BY	CHKD	APPD	NO.
3/2/78	JKS			
4/1/78	JKS			
5/4/78	JKS			
6-2-81	JFD			
7/2/86				

SR NO Y2-9
CIVIL
MISCELLANEOUS SECTIONS & DETAILS SHEET 4
WASTEWATER MANAGEMENT FACILITIES
CENTRAL ILLINOIS PUBLIC SERVICE COMPANY
COFFEE POWER STATION
SCALE AS SHOWN
Stearns-Roger
ORDER NO. C-20000
9201
SHEET NO. S-49
REV.

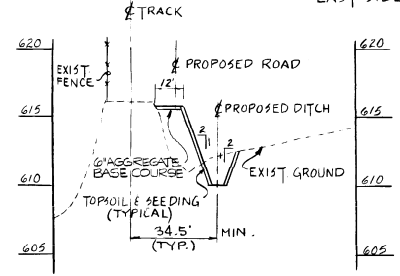


COAL PILE RUNOFF DITCH PROFILE EASTSIDE
SCALE: HORIZ. 1"=50'
VERT. 1"=5'

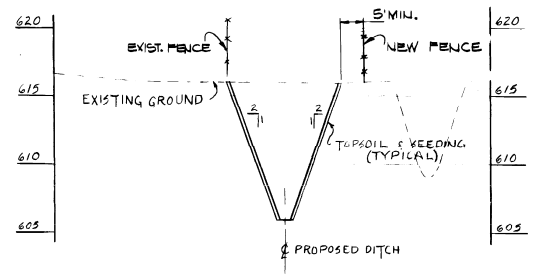


CONTINUATION-DITCH PROFILE EAST SIDE

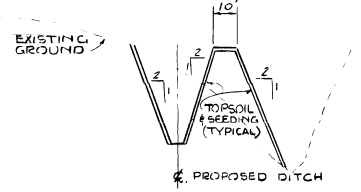
COAL PILE RUNOFF DITCH PROFILE WESTSIDE
SCALE: HORIZ. 1"=50'
VERT. 1"=5'



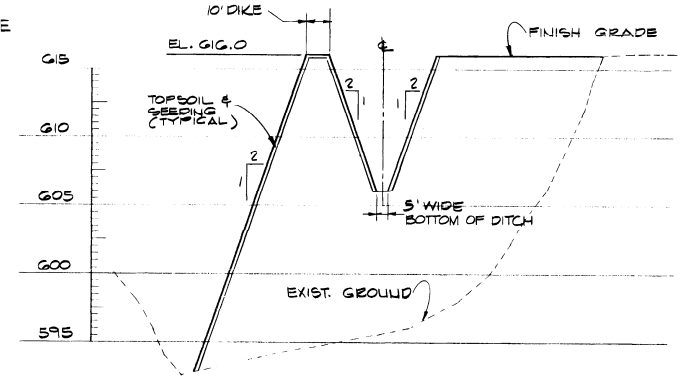
SECTION B
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: S-42



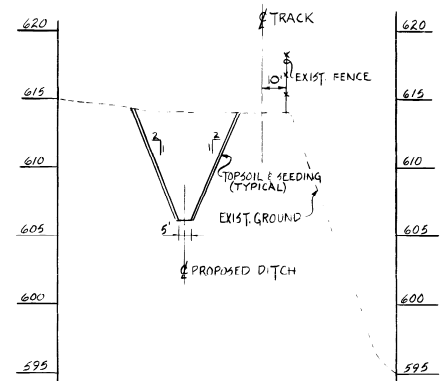
SECTION C
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: S-42



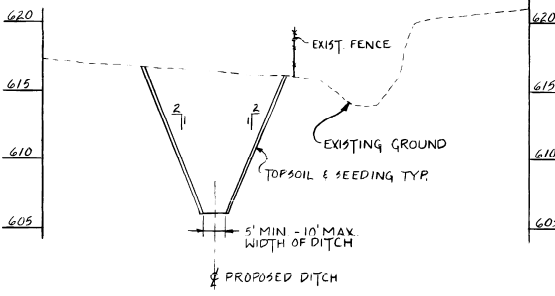
SECTION E
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: S-42



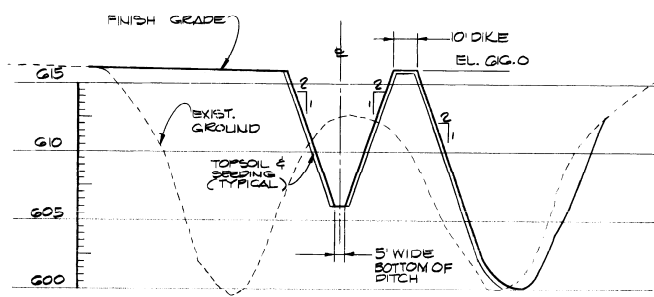
SECTION G
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: DWG. S-43



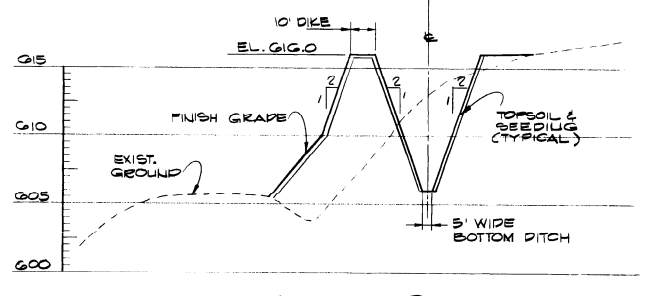
SECTION D
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: S-43



SECTION A
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: S-42



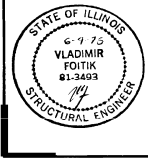
SECTION F
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: DWG. S-43



SECTION H
SCALE: HORIZ. 1"=30'
VERT. 1"=5' REF: DWG. S-43

NOTE:
THE WIDTH OF BOTTOM OF DITCH IS 5'-0" EXCEPT WHERE NOTED OTHERWISE.

NOTES:
1. FOR DITCH PLAN SEE DWGS S-42 & S-43.
2. FOR SITE PLAN SEE DWG S-40.



REVISIONS				REFERENCE DRAWINGS				PRINT RECORD				ENG. RECORD		DRAWING STATUS	
NO.	DATE	BY	CHKD	APPD	NO.	DATE	ISSUED	DATE	DATE	DATE	DATE	DATE	DATE		
1	4/10/78	WSB													
2	4/12/78	KS													
3	4/12/78	KS													
4	4/12/78	KS													
5	4/12/78	KS													
6	4/12/78	KS													
7	4/12/78	KS													
8	4/12/78	KS													
9	4/12/78	KS													
10	4/12/78	KS													
11	4/12/78	KS													
12	4/12/78	KS													
13	4/12/78	KS													
14	4/12/78	KS													
15	4/12/78	KS													
16	4/12/78	KS													
17	4/12/78	KS													
18	4/12/78	KS													
19	4/12/78	KS													

SCANNED BY S&L

S.R. NO Y2-10

CIVIL
MISCELLANEOUS SECTIONS & DETAILS SHEET

WASTEWATER MANAGEMENT FACILITIES
CENTRAL ILLINOIS PUBLIC SERVICE COMPANY
COFFEE POWER STATION

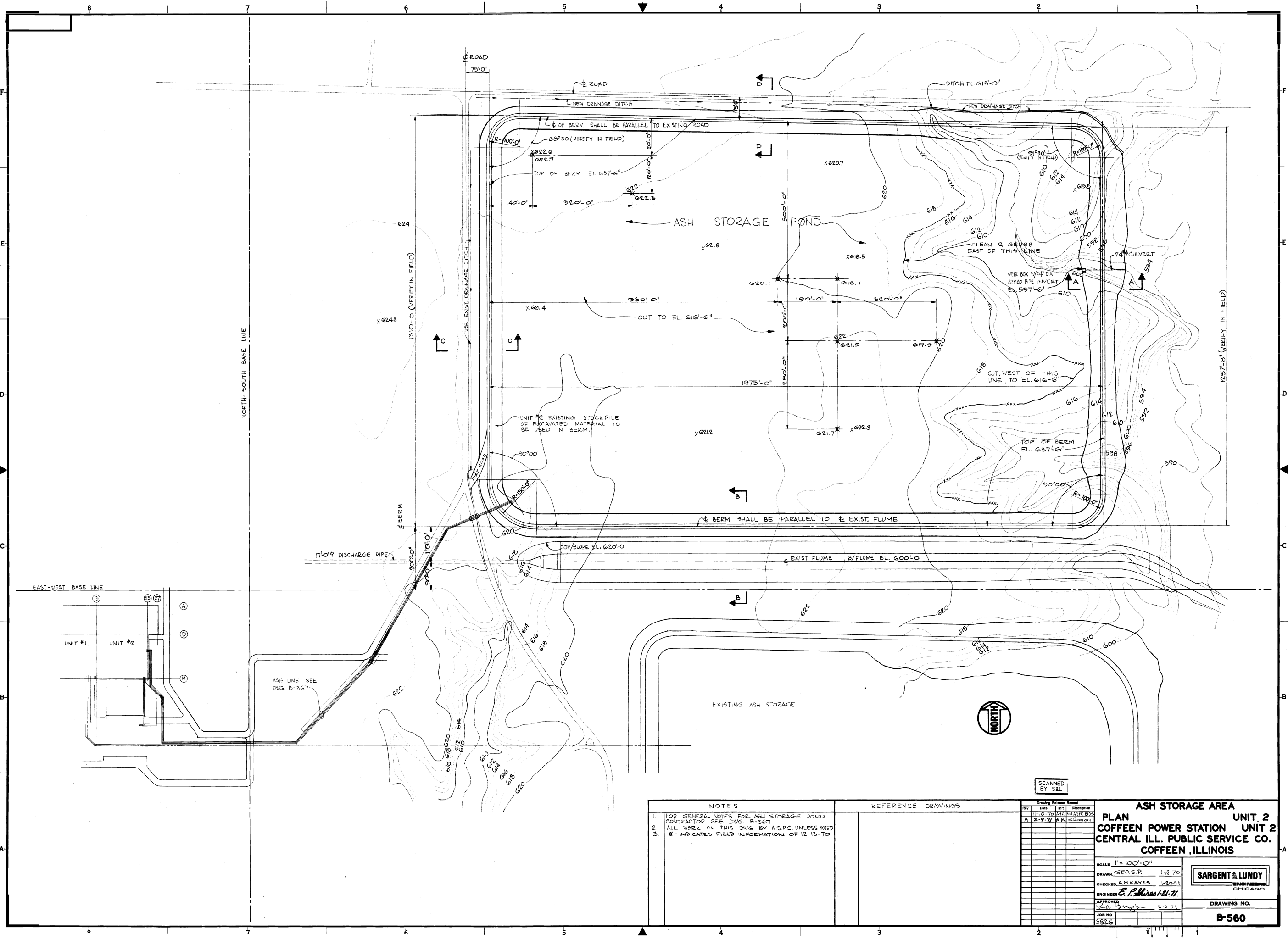
SCALE: AS SHOWN

Stearns-Roger
INCORPORATED

ORDER NO. C-23000

DWG. NO. L-22886

0201
SHEET NO. S-50



- NOTES**
1. FOR GENERAL NOTES FOR ASH STORAGE POND CONTRACTOR SEE DWG. B-367
 2. ALL WORK ON THIS DWG. BY A.S.P.C. UNLESS NOTED
 3. * - INDICATES FIELD INFORMATION OF 12-15-70

REFERENCE DRAWINGS

Drawing Release Record			
Rev.	Date	By	Description
A	11-10-70	AHL	PER A.S.P.C. BIDS
A	2-9-71	A.K.	FOR CONSTRUCTION

ASH STORAGE AREA UNIT 2

COFFEEN POWER STATION UNIT 2

CENTRAL ILL. PUBLIC SERVICE CO.

COFFEEN, ILLINOIS

SCALE 1" = 100'-0"

DRAWN S.E.O.S.P. 1-12-70

CHECKED A.M.KAYES 1-20-71

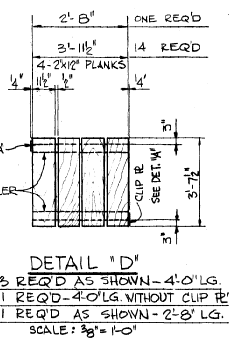
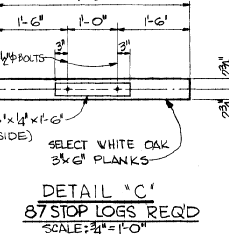
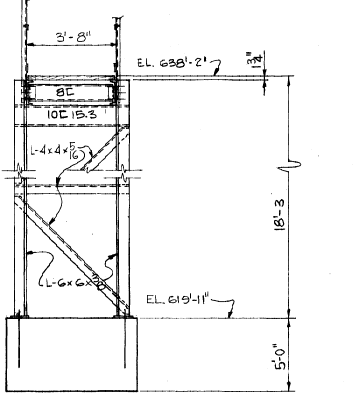
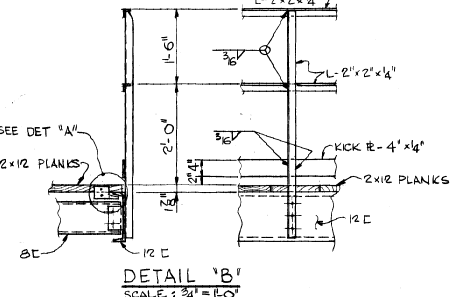
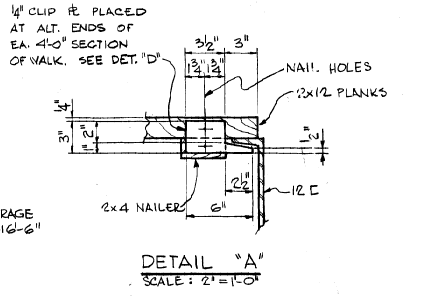
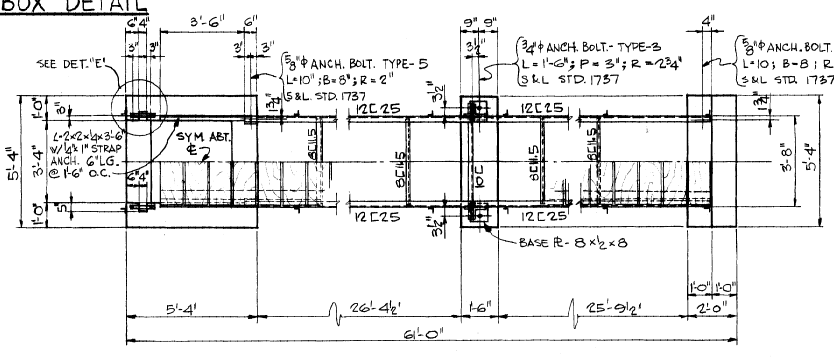
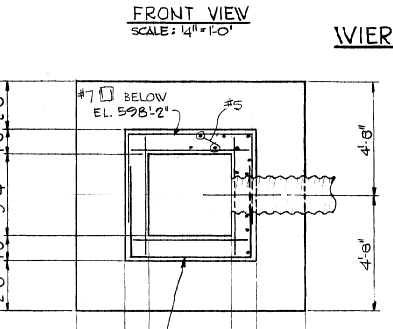
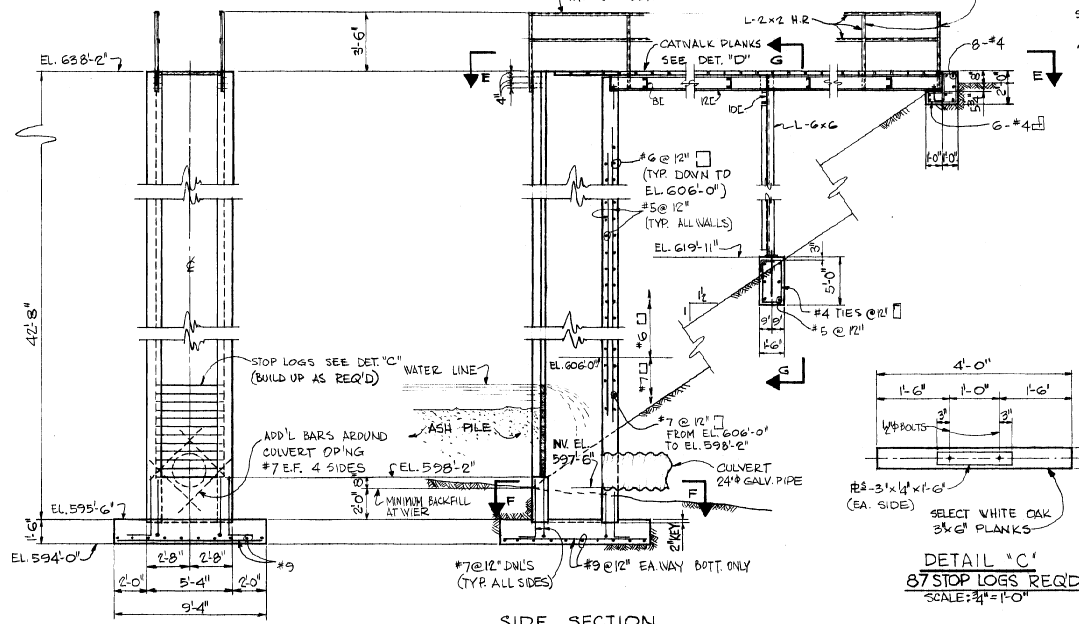
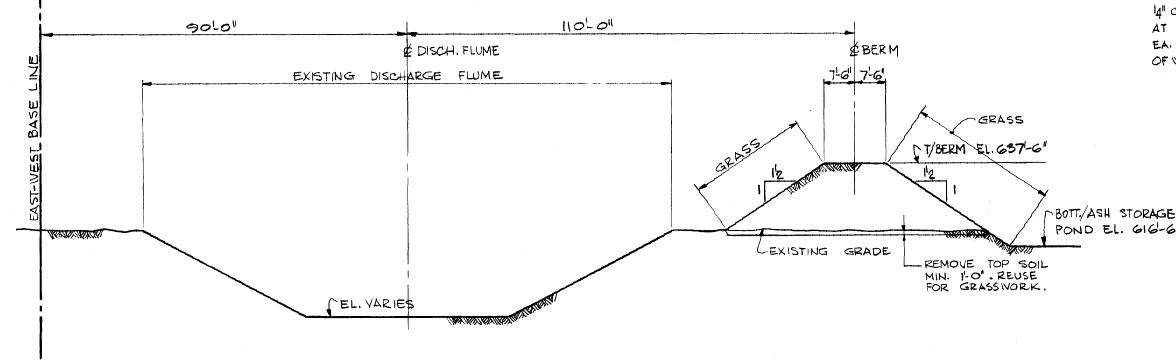
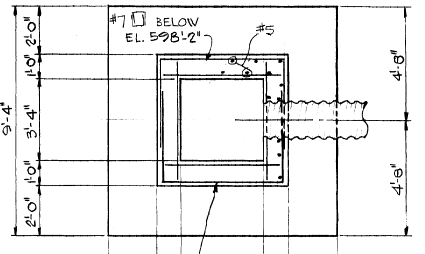
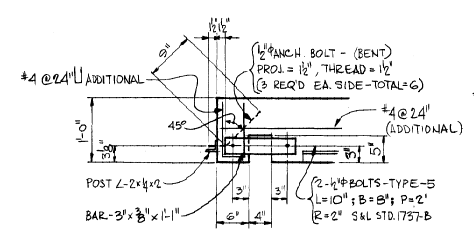
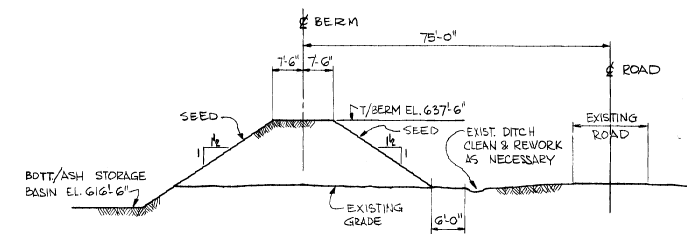
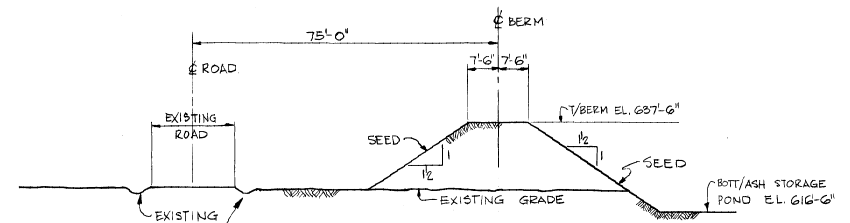
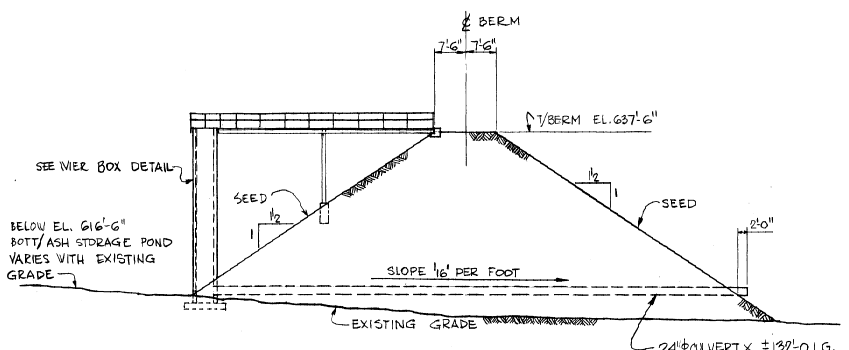
ENGINEER E.P. Williams 1-21-71

APPROVER [Signature] 2-2-71

JOB NO. 3826

SARGENT & LUNDY
ENGINEERS
CHICAGO

DRAWING NO.
B-560



NOTES		REFERENCE DRAWINGS	
1.	FOR GENERAL NOTES FOR ASH STORAGE POND CONTRACTOR SEE DWG. B-567	B-560	ASH STORAGE AREA
2.	ALL WORK ON THIS DWG. BY A.S.P.C.		
3.	ALL ANCHOR BOLTS SHALL BE GALVANIZED.		

SCANNED BY: SBL	
Rev	Description
1	ISSUED FOR CONSTRUCTION
2	REVISED

ASH STORAGE AREA
SECTIONS & DETAILS UNIT 2
COFFEEN POWER STATION UNIT 2
CENTRAL ILL. PUBLIC SERVICE CO.
COFFEEN, ILLINOIS

SCALE: AS NOTED

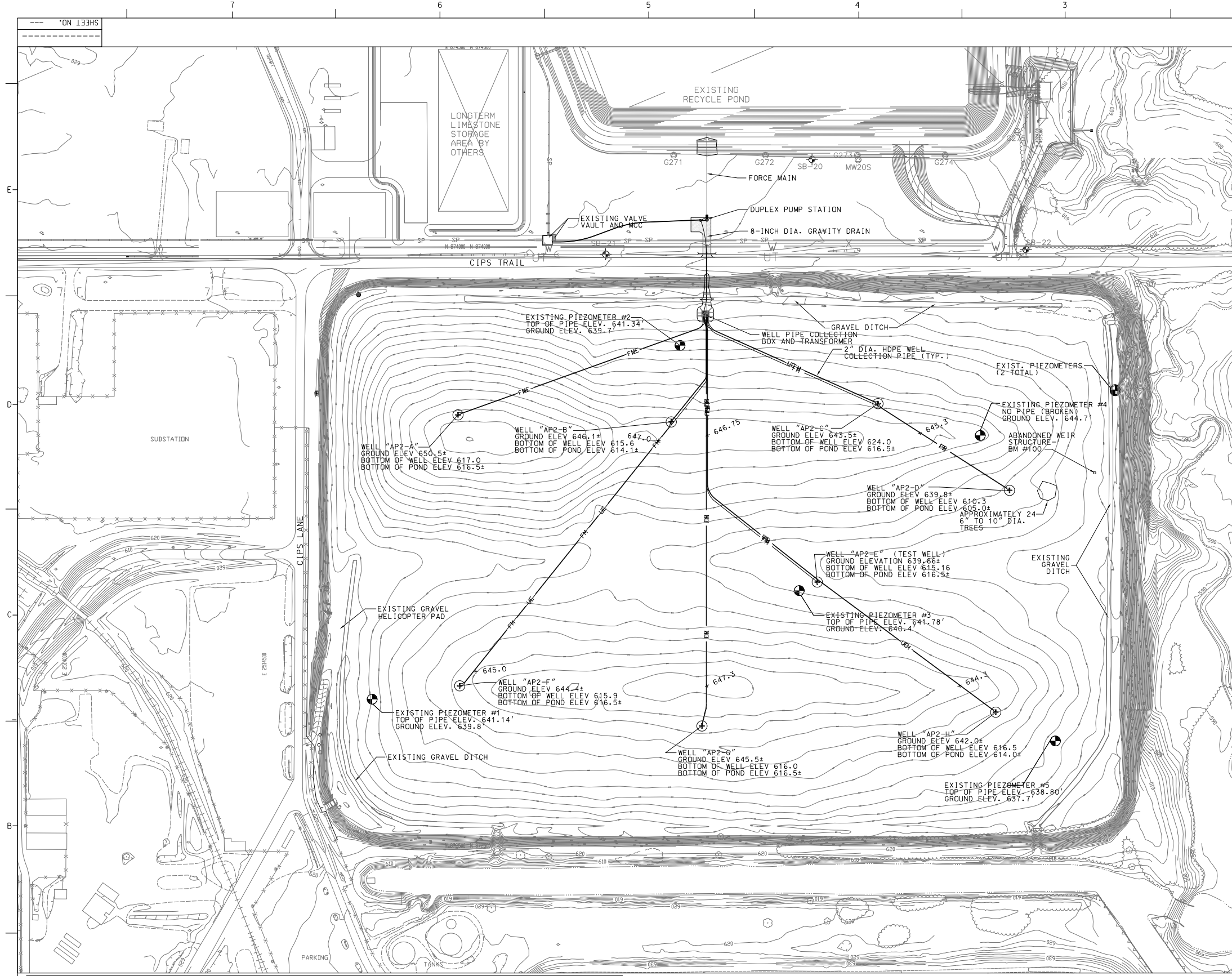
DRAWN: G.E.S.P. 1-27-70
 CHECKED: A.M.K.A.Y.E.S. 1-28-71
 ENGINEER: E.P. ... 1-27-71

APPROVED: ... 1-27-71

JOB NO: 3026

SARGENT & LUNDY
 ENGINEERS
 CHICAGO

DRAWING NO. **B-561**



NOTES:

1. THE LOCATION, SIZE AND/OR TYPE OF MATERIAL OF EXISTING UNDERGROUND OR OVERHEAD UTILITIES AS MAY BE INDICATED ON THESE CONSTRUCTION PLANS IS NOT REPRESENTED AS BEING ACCURATE, SUFFICIENT OR COMPLETE. THE OWNER AND THE PROJECT ENGINEER HAVE NOT INDEPENDENTLY VERIFIED THIS INFORMATION AND DO NOT ASSUME ANY RESPONSIBILITY WHATSOEVER IN RESPECT TO THE ACCURACY, SUFFICIENCY OR COMPLETENESS OF THE INFORMATION AND GIVE NO EXPRESSED OR IMPLIED GUARANTEE THAT ANY CONDITIONS INDICATED ARE REPRESENTATIVE OF ACTUAL CONDITIONS TO BE ENCOUNTERED.
2. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE THE ACTUAL LOCATION OF ALL SUCH FACILITIES, INCLUDING SERVICE CONNECTIONS TO UNDERGROUND UTILITIES. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL NOTIFY ALL UTILITY COMPANIES AND AGENCIES OF HIS CONSTRUCTION PLANS AND SHALL OBTAIN FROM EACH PARTY DETAILED INFORMATION AND ASSISTANCE RELATIVE TO THE LOCATION OF ALL UTILITIES AND THE SCHEDULE OF ANY REMOVALS AND ADJUSTMENTS REQUIRED OF THE UTILITY. THE CONTRACTOR SHALL CONTACT J.U.L.I.E. (1-800-892-0123 OR 811) TO ASSIST IN COMPLETING THIS RESPONSIBILITY.
3. THE CONTRACTOR SHALL PROTECT ANY FACILITIES TO THE SATISFACTION OF THE UTILITY OR OWNING-AGENCY WITH THE COST OF ANY REQUIRED PROTECTION TO BE INCIDENTAL TO THE CONTRACT. IN THE EVENT A UTILITY LINE OR SERVICE IS UNEXPECTEDLY ENCOUNTERED DURING CONSTRUCTION, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE OWNER AND THE UTILITY COMPANY OR AGENCY OF JURISDICTION. ANY SUCH UTILITIES DISTURBED BY THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED TO SERVICE AT ONCE.

COORDINATE TABLE

POINT	STATE PLANE COORDINATES			DESCRIPTION	PLANT COORDINATES		
	NORTH	EAST	ELEV.		NORTH	EAST	ELEV.
100	873,444.5	2,516,566.2	636.78'	BENCHMK: CHISELED "C" IN N.E. COR. OF WEIR BOX	3610.958	1357.406	616.31'
1	872,940.7900	2,513,222.5010	615.74'	CONTROL PT #1 BRASS MONUMENT NEAR N.W. ENTRANCE	3610.440	2430.490	623.44'
2	872,942.4450	2,514,295.5440	622.92'	CONTROL PT #2 BRASS MONUMENT NEAR EAST PKG AREA	4561.951	2430.945	N/A
3	873,893.9360	2,514,294.3340	624.63'	CONTROL PT #3 BRASS MONUMENT NEAR MAIN ENTRANCE			
5001	873,587.59	2,514,969.63	.	WELL AP2-A			
5002	873,569.87	2,515,503.76	.	WELL AP2-B			
5003	873,615.67	2,516,021.68	.	WELL AP2-C			
5004	873,398.41	2,516,351.29	.	WELL AP2-D			
5005	873,168.81	2,515,869.61	.	WELL AP2-E			
5006	872,908.89	2,514,973.83	.	WELL AP2-F			
5007	872,808.37	2,515,980.73	.	WELL AP2-G			
5008	872,842.91	2,516,316.70	.	WELL AP2-H			

LEGEND

- ELECTRICAL SPLICE BOX
- TELEPHONE SPLICE BOX
- GUARDPOST
- EXISTING FENCE
- TREE AREA
- GRAVEL EDGE
- PAVED OUTLET EDGE
- FM FORCE MAIN
- SS SANITARY SEWER
- UE UNDERGROUND ELECTRIC
- ▭ GRAVEL SURFACING
- PIEZOMETER
- IRON PIN

100' 50' 0 50' 100'
SCALE IN FEET

AS BUILT

**OVERALL SITE PLAN
DEWATERING SYSTEM
ASH POND #2
AMEREN COFFEEN POWER STATION**

SITE: ASH POND #2

LOC. NO.		DRAWING NO.	
CLASS		REV.	
A1000		0	
SHEET NO. 1 OF 5			

Ameren Energy
Resources
Generating

NOTICE OF LIMITED RESPONSIBILITY
THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN OF THE PROJECT AS SHOWN ON THESE PLANS. THE ENGINEER DOES NOT ASSUME ANY RESPONSIBILITY FOR THE DESIGN OF ANY OTHER WORK OR FOR THE CONSTRUCTION OF THE PROJECT OR FOR THE PERFORMANCE OF THE PROJECT OR FOR THE SAFETY OF THE PROJECT OR FOR THE SAFETY OF THE PUBLIC.

REV.	DATE	PROJECT NO.	AMEREN NO.	SUPV ENGR	DRAFTING	ENGR APPROVAL	DESCRIPTION
A	10-12-09	#1314	SDB	MJW	SDB	MJW	

NOTES

REFERENCE DRAWINGS

STANDARD LEGEND

(ALL SYMBOLS MAY NOT BE USED ON EACH PLAN)

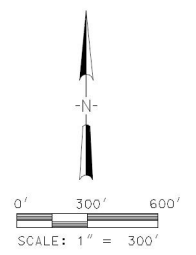
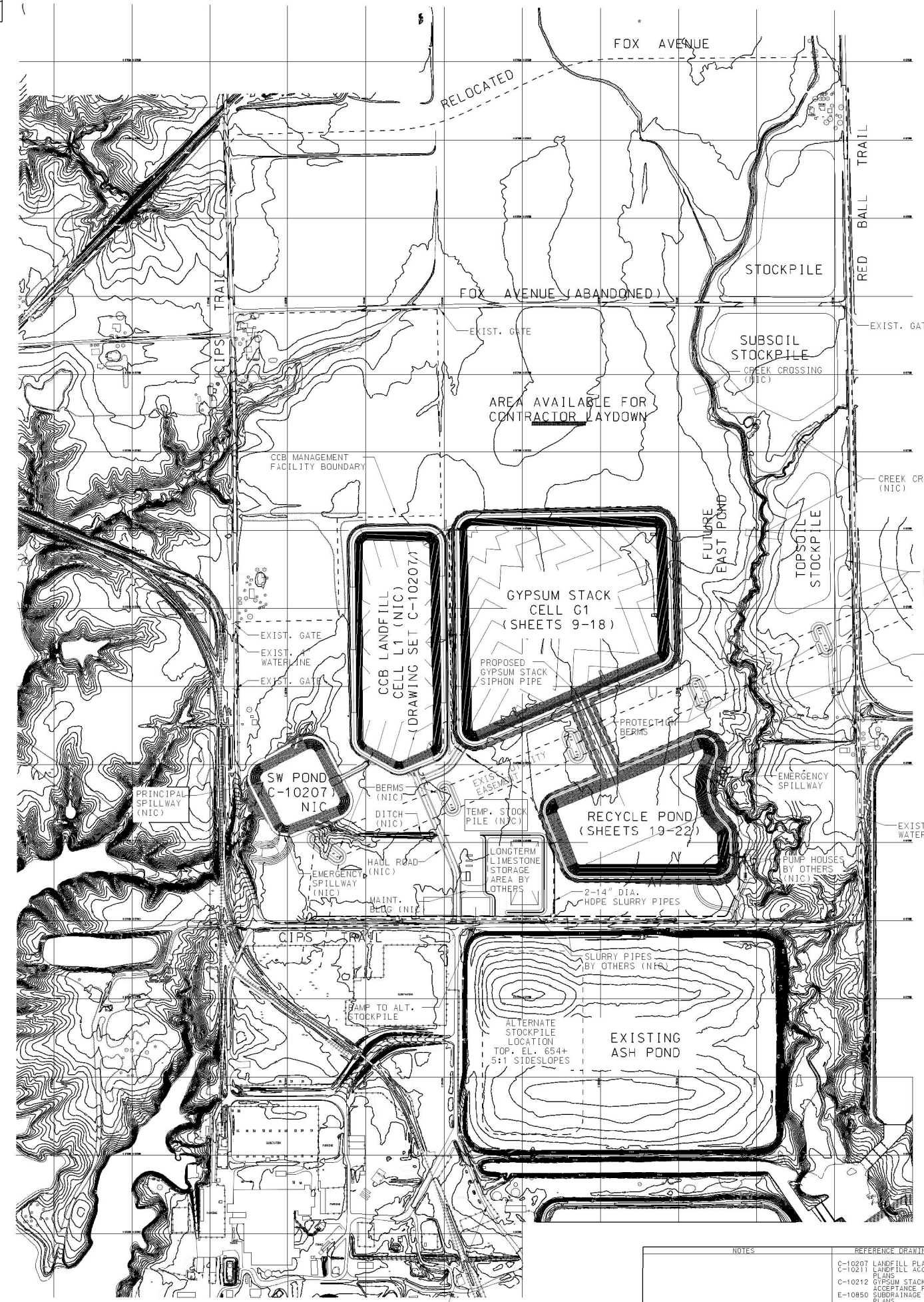
- ORIGINAL CONTOURS
- FENCE LINE
- TRANSMISSION TOWERS
- OVERHEAD ELECTRIC
- WATER MAIN
- TELEPHONE LINE
- LIMITS OF VEGETATION
- GRAVEL ROAD
- RAILROAD
- CP-1000 △ CONTROL POINT - HORIZ. & VERT.
- A — A' GEOLOGICAL CROSS SECTION LINE
- LANDFILL CONTOURS
- SB-55 ■ BORING
- GXXX ⊕ PREVIOUSLY INSTALLED GROUNDWATER MONITORING WELL
- GXXX ⊕ GROUNDWATER MONITORING WELL
- MW13S ⊕ WELLS - NOT PART OF MONITORING PLAN

NOTES:

1. SEE NOTES ON SHEET 2 REGARDING THE RESPONSIBILITY FOR EXISTING UTILITIES.
2. (NIC) INDICATES SITE FEATURES WHICH ARE NOT PART OF THIS CONTRACT. THEY ARE SHOWN FOR INFORMATIONAL PURPOSES ONLY.
3. HORIZONTAL AND VERTICAL CONTROL ARE BASED ON NAD 83, ILLINOIS STATE PLANE COORDINATE SYSTEM WEST ZONE, NAVD 88, US SURVEY FEET AND THE FOLLOWING PROJECT CONTROL POINTS WERE ESTABLISHED BY HANSON PROFESSIONAL SERVICES:

HORIZONTAL CONTROL

POINT	NORTHING	EASTING	ELEV.	DESCRIPTION
CP-1000	873998.37	2515334.17	620.81	#8 REBAR IN 6" DIA. CONCRETE MONUMENT
CP-1001	873985.17	2513180.64	621.96	#8 REBAR IN 6" DIA. CONCRETE MONUMENT
CP-1002	877937.14	2514533.34	623.15	#8 REBAR IN 6" DIA. CONCRETE MONUMENT
CP-1003	877949.62	2516989.49	622.10	#8 REBAR IN 6" DIA. CONCRETE MONUMENT



NOTICE OF LIMITED RESPONSIBILITY
 THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN AND CONSTRUCTION OF THE FACILITY SHOWN ON THIS DRAWING. THE ENGINEER DOES NOT GUARANTEE THE ACCURACY OF THE DATA PROVIDED TO HIM BY OTHERS. THE ENGINEER'S LIABILITY IS LIMITED TO THE DESIGN AND CONSTRUCTION OF THE FACILITY SHOWN ON THIS DRAWING. THE ENGINEER DOES NOT GUARANTEE THE ACCURACY OF THE DATA PROVIDED TO HIM BY OTHERS.

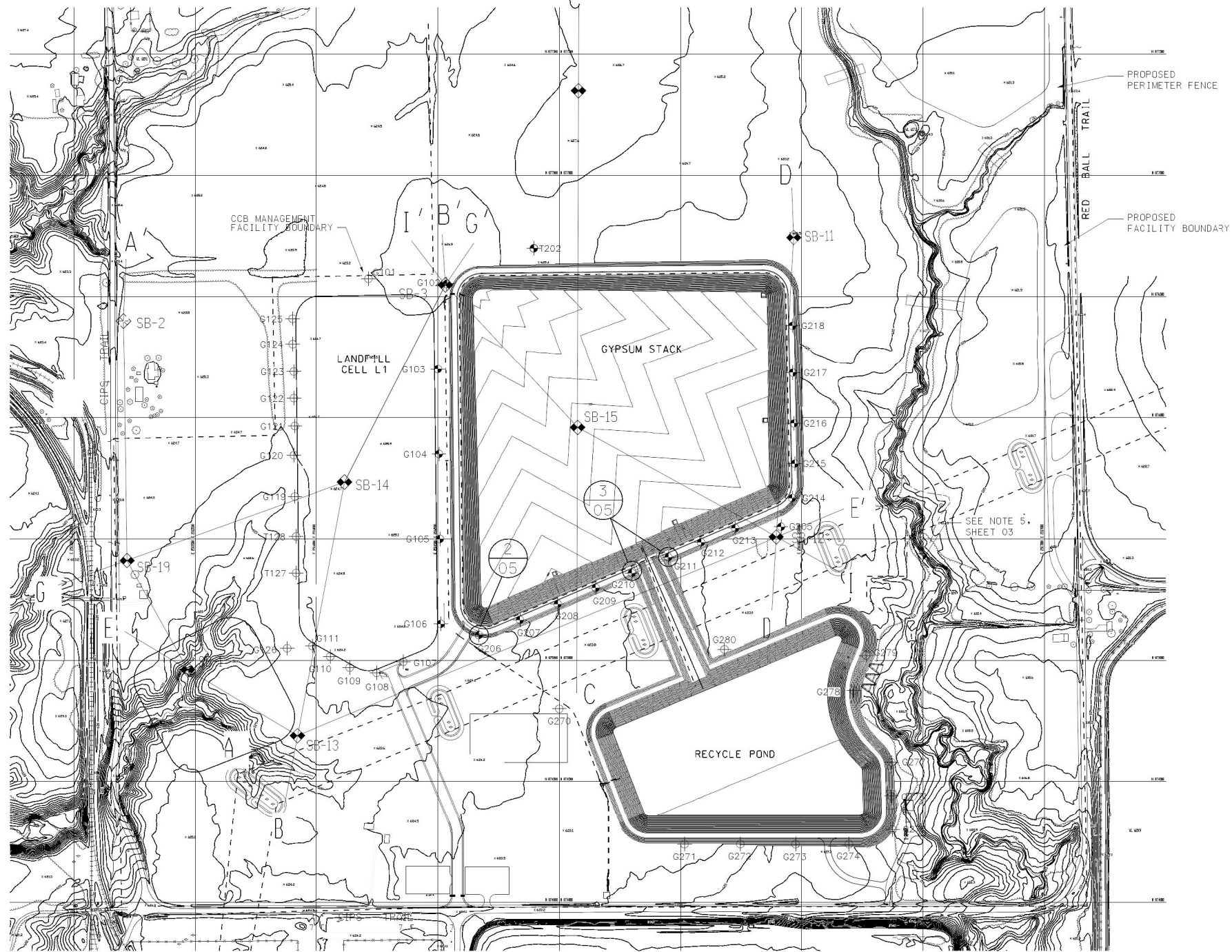
REV.	DATE	PROJECT NO.	AMEREN SUPPLY ENGR.	AMEREN DRAFTING	AMEREN ENGR. APPROVAL	DESCRIPTION
0	01-05-11					ISSUED FOR PEOPLE DRAWING

PROPOSED SITE PLAN
CCB MANAGEMENT FACILITY
 SITE: COFFEEN

LOC. NO.	DRAWING NO.	REV.
	C-10206	0
CLASS.	SHEET NO. 04	

Ameren Energy
Generating

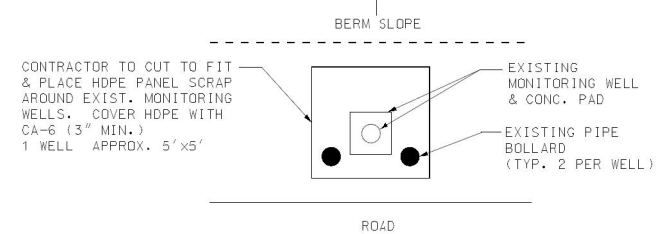
EXISTING HYDRANT TO BE USED FOR DUST CONTROL WATER SOURCE



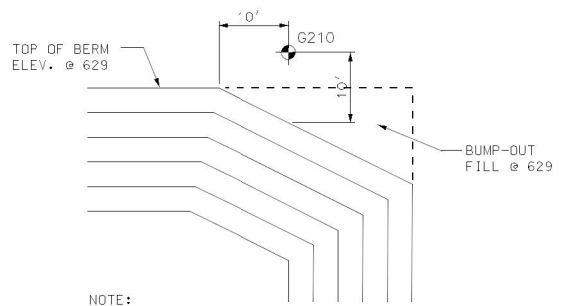
STANDARD LEGEND
(ALL SYMBOLS MAY NOT BE USED ON EACH PLAN)

	ORIGINAL CONTOURS
	FENCELINE
	TRANSMISSION TOWERS
	OVERHEAD ELECTRIC
	WATER MAIN
	TELEPHONE LINE
	LIMITS OF VEGETATION
	GRAVEL ROAD
	RAILROAD
	CONTROL POINT - HORIZ. & VERT.
	GEOLOGICAL CROSS SECTION LINE
	LANDFILL CONTOURS
	BORING
	GYPSUM STACK GROUNDWATER MONITORING WELL
	OTHER MONITORING WELL

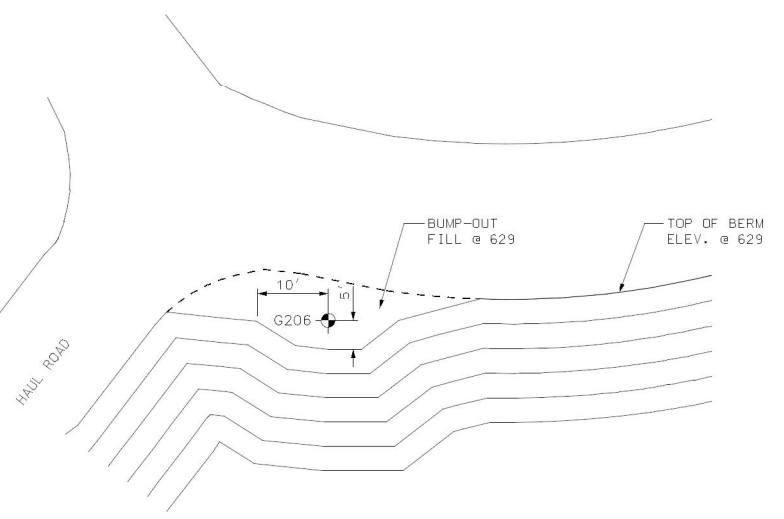
- NOTES:**
- CONTOURS SHOWN IN GYPSUM STACK ARE TOP OF CLAY LINER. (NOTE: THE UPPER PORTION OF THE PROCESS WATER RECOVERY SYSTEM LINER WAS ELIMINATED AS PART OF THE CLEAN CLOSURE REVISION)
 - THE PROPERTY IS LOCATED IN SECTIONS 10 & 11, TOWNSHIP 7 NORTH, RANGE 3 WEST, 3RD P.M., MONTGOMERY COUNTY, ILLINOIS
 - THE DESIGN ELEMENTS INCLUDED IN THESE DRAWINGS WERE BASED UPON EXISTING AERIAL MAPPING AS OBTAINED FROM AMEREN.
 - ALL DISTURBED AREAS, INCLUDING THE CREST AND EXTERIOR FACE OF BOTH DAMS WILL BE SEEDED AND MULCHED WITH IDOT CLASS 1A SEED MIXTURE.
 - FILL MATERIAL FOR DAM EMBANKMENT WILL BE PLACED IN MAXIMUM UNCOMPACTED LIFTS OF 9-INCHES AND COMPACTED TO 95% OF THE STANDARD PROCTOR DENSITY.
 - SURFICIAL CONTOURS WERE DEVELOPED USING AERIAL PHOTOGRAMMETRIC METHODS WITH PHOTOGRAPHS TAKEN BY SURDEX.
 - CURRENT SURFACE TOPOGRAPHY MAY DIFFER FROM THAT SHOWN DUE TO ON GOING OPERATIONS AT THE SITE.
 - FOR CLARITY, NOT ALL EXISTING SITE FEATURES ARE SHOWN ON THIS DRAWING.



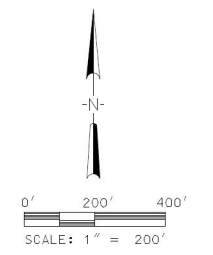
WEED CONTROL AT EXIST. SINGLE MONITORING WELLS



MONITORING WELL BUMP-OUT NEAR TRANSFER CHANNEL



MONITORING WELL BUMP-OUT AT HAUL ROAD



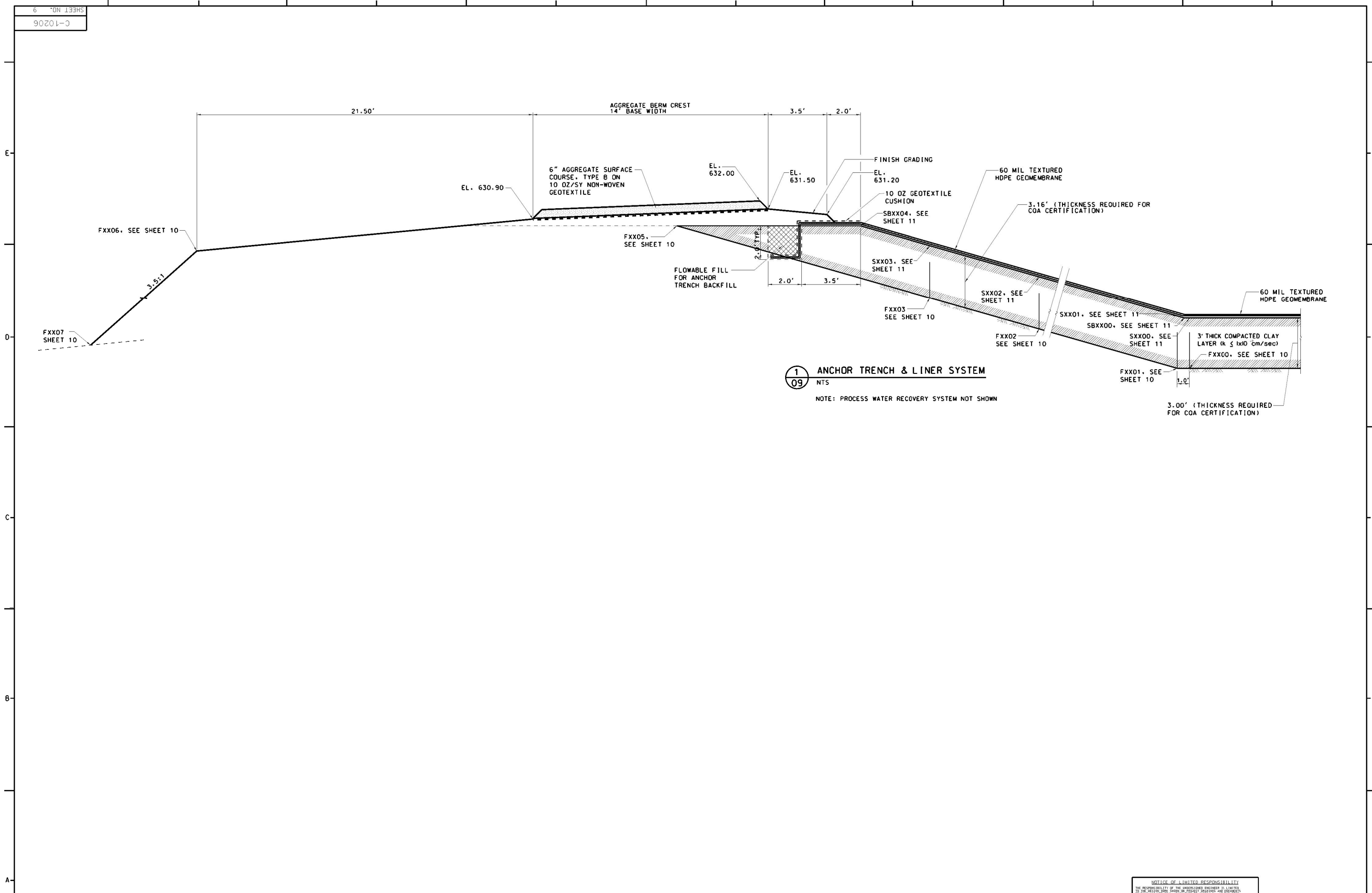
REV.	DATE	PROJECT NO.	AMEREN SUPPLY ENGR.	DRAFTING	ENGR. APPROVAL	DESCRIPTION
0	01-05-11					

NOTICE OF LIMITED RESPONSIBILITY
THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN AND CONSTRUCTION OF THE PROJECT DESCRIBED IN THESE PLANS. THE ENGINEER DOES NOT WARRANT THE ACCURACY OF THE DATA OR THE RESULTS OF THE DESIGN. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY OTHER LIABILITY FOR ANY OTHER PARTY'S NEGLIGENCE OR MISFEASANCE.

SCALE: 1" = 200'

GROUNDWATER MONITORING & BORING PLAN
CCB MANAGEMENT FACILITY
SITE: COFFEEN

LOC. NO.	AMEREN	DRAWING NO.	REV.
CLASS	Generating	C-10206	0
		SHEET NO.	05



1
09 ANCHOR TRENCH & LINER SYSTEM
NTS

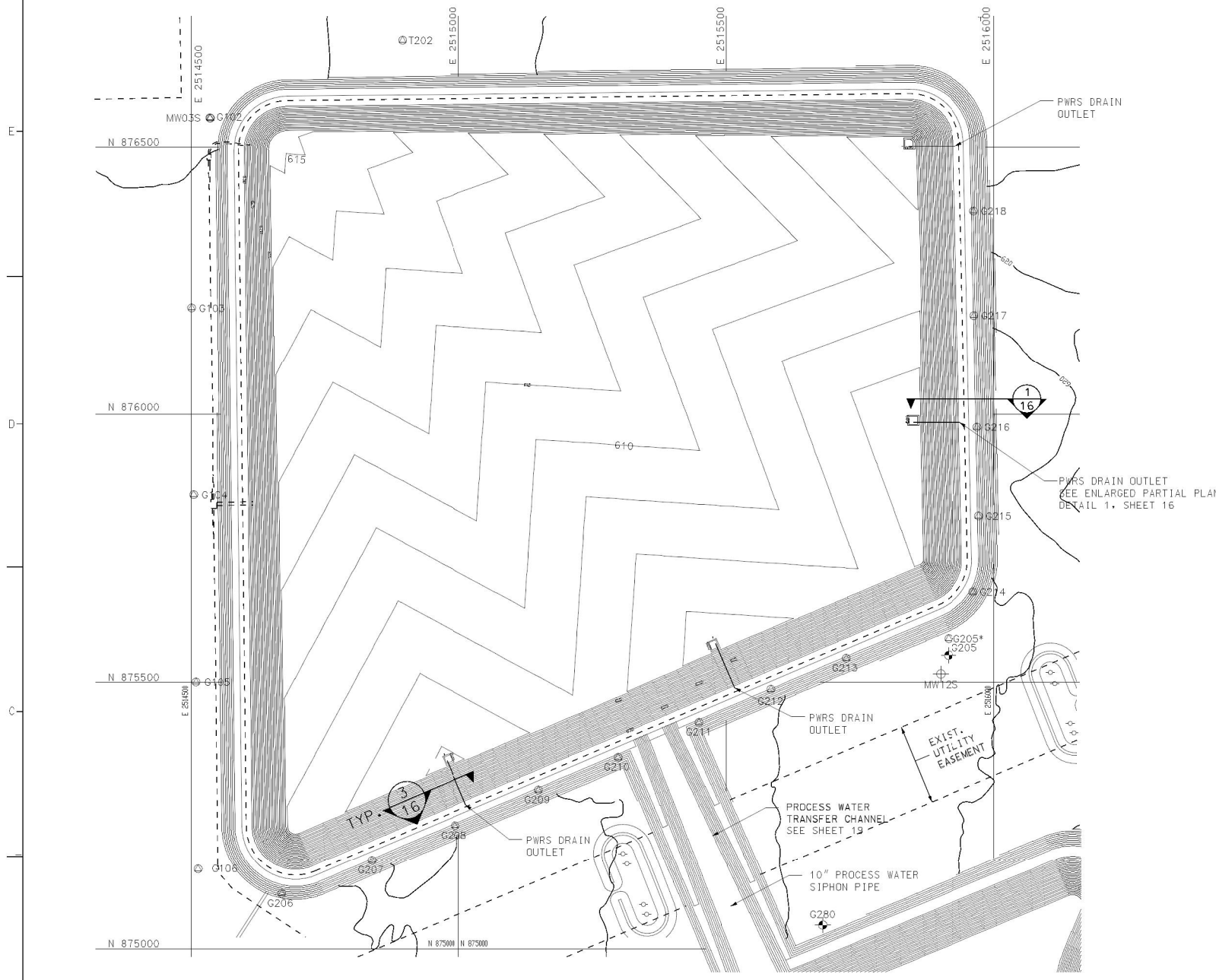
NOTE: PROCESS WATER RECOVERY SYSTEM NOT SHOWN

NOTICE OF LIMITED RESPONSIBILITY
THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN AND CONSTRUCTION OF THE PROJECT AS SHOWN ON THESE PLANS. THE UNDERSIGNED ENGINEER DOES NOT GUARANTEE THE ACCURACY OF THE INFORMATION PROVIDED TO HIM OR HER AND DOES NOT ASSUME RESPONSIBILITY FOR ANY OTHER DESIGN OR CONSTRUCTION OF THE PROJECT WHICH IS NOT SHOWN ON THESE PLANS.

REV.	DATE	PROJECT NO.	DRAWING RECORD				DESCRIPTION
			AMEREN SUPV. ENGR.	AMEREN DRAFTING	AMEREN ENGR. APPROVAL	OTHER	
0	01-05-11		SVB	MWR			ISSUED FOR PERIOD BIDDING

NOTES
C-10207 LANDFILL PLANS
C-10211 LANDFILL ACCEPTANCE PLANS
C-10212 GYPSUM STACK ACCEPTANCE PLANS
E-10850 SUBDRAINAGE POWER PLANS
E-10855 LEACHATE COLLECTION POWER PLANS

SCALE		ANCHOR TRENCH AND LINER SYSTEM	
SITE: COFFEEN		CCB MANAGEMENT FACILITY	
LOC. NO.	DRAWING NO.	REV.	
CLASS	C-10206	0	
SHEET NO. 09		AMEREN ENERGY GENERATING	



NOTICE OF LIMITED RESPONSIBILITY
 THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN WORK SHOWN ON THESE DRAWINGS AND COMMENTS HEREON. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF ANY OTHER WORK NOT SHOWN ON THESE DRAWINGS OR COMMENTS HEREON. THE ENGINEER'S LIABILITY IS LIMITED TO THE DESIGN WORK SHOWN ON THESE DRAWINGS AND COMMENTS HEREON. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE DESIGN OR CONSTRUCTION OF ANY OTHER WORK NOT SHOWN ON THESE DRAWINGS OR COMMENTS HEREON.

0' 100' 200'
 SCALE: 1" = 100'

N

REV.	DATE	PROJECT NO.	AMEREN SUPV. ENGR.	AMEREN DRAFTING	AMEREN ENGR. APPROVAL	DESCRIPTION
0	01-05-11					ISSUED FOR PEOPLE DRIVING

SCALE

CELL G1-PROCESS WATER RECOVERY SYSTEM
 CCB MANAGEMENT FACILITY
 SITE: COFFEEN

LOC. NO. CLASS

AMEREN Generating

DRAWING NO. C-10206
 REV. 0
 SHEET NO. 15

NOTES

REFERENCE DRAWINGS

C-10207	LANDFILL PLANS
C-10211	LANDFILL ACCEPTANCE PLANS
C-10212	GYPSPUM STACK ACCEPTANCE PLANS
E-10850	SUBDRAINAGE POWER PLANS
E-10855	LEACHATE COLLECTION POWER PLANS

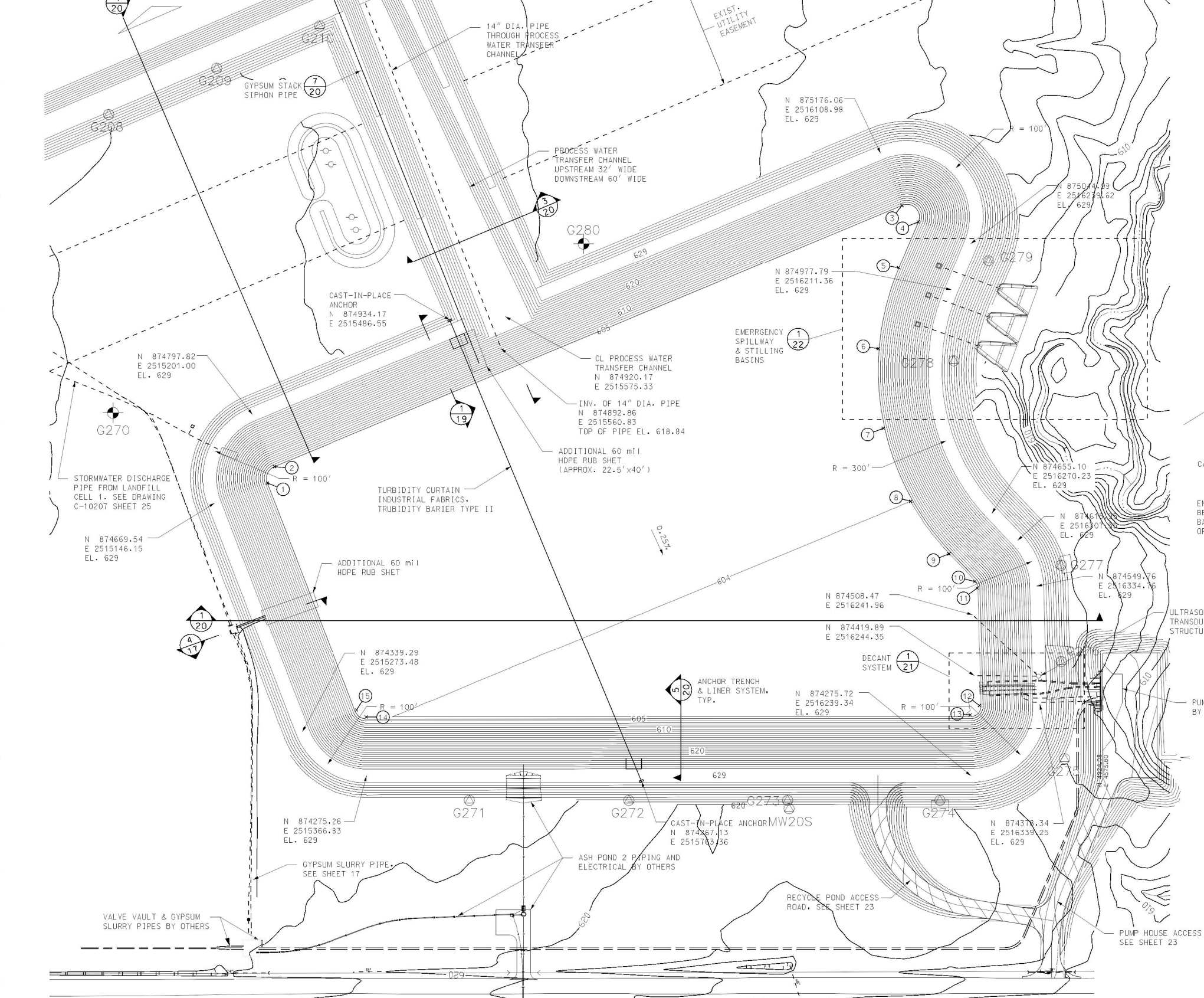
GYPHUM STACK CELL G1

INV. OF 14" DIA. PIPE
N 875464.88
E 2515336.44
EL. 619.00

CL PROCESS WATER
TRANSFER CHANNEL
N 875451.52
E 2515348.42

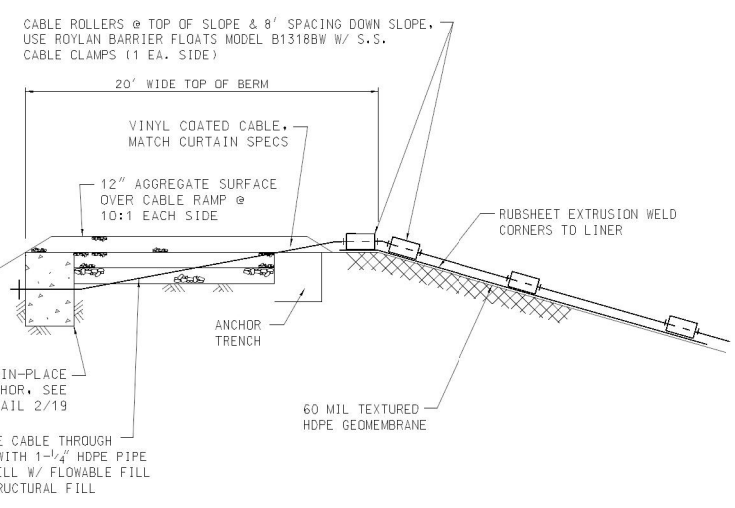
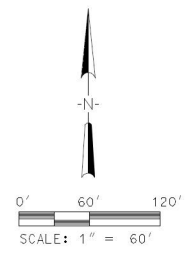
MW12S

PROTECTION
BERMS, TYP.



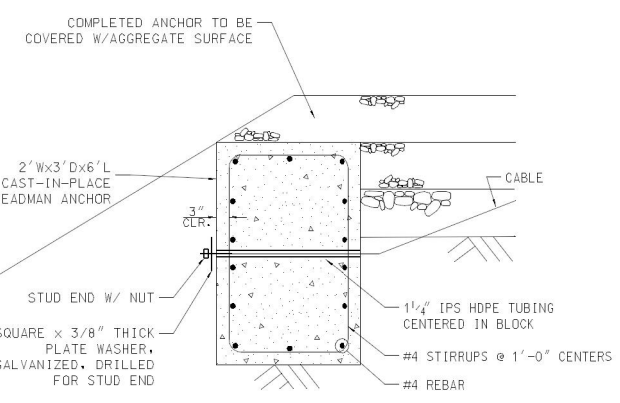
RECYCLE POND - INTERIOR TOE

POINT	N	E	ELEV.
1	874706.97	2515227.14	605.24
2	874723.70	2515241.70	605.02
3	875095.89	2516132.29	605.05
4	875069.26	2516159.88	604.92
5	875005.90	2516132.91	604.95
6	874909.29	2516107.54	604.91
7	874811.81	2516106.37	604.65
8	874717.55	2516129.35	604.25
9	874633.41	2516175.61	604.17
10	874555.24	2516246.12	604.22
11	874549.81	2516248.01	604.07
12	874377.99	2516248.77	602.44
13	874365.11	2516237.06	604.00
14	874359.95	2515366.65	604.25
15	874372.65	2515351.72	604.49



1 TURBIDITY CURTAIN ANCHOR/CONNECTION
NTS

NOTE:
ALL CABLE CONNECTIONS SHALL BE 316 S.S.
WITH A STRENGTH RATING GREATER THAN CABLE.



2 CAST-IN-PLACE ANCHOR
NTS

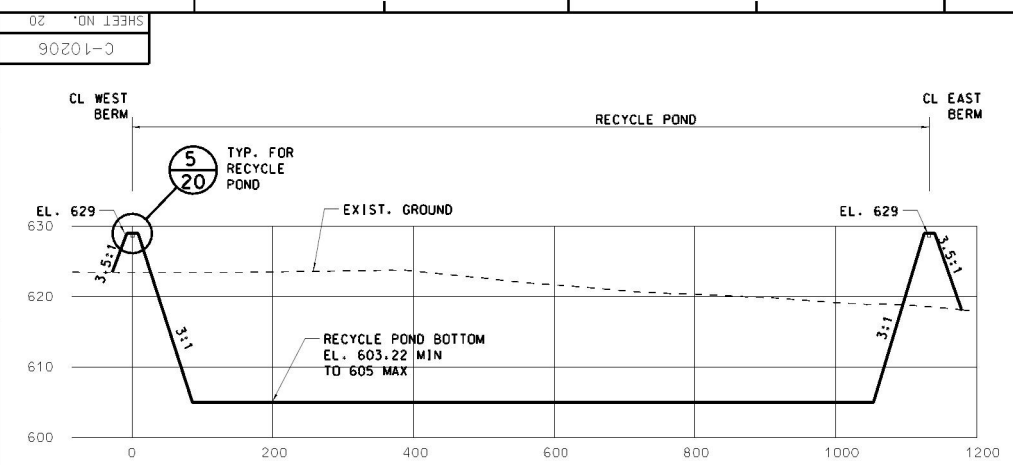
NOTICE OF LIMITED RESPONSIBILITY
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TO THE DESIGN AND CONSTRUCTION OF THE PROJECT AS SHOWN
HEREON. THE ENGINEER DOES NOT GUARANTEE THE ACCURACY
OF THE INFORMATION PROVIDED TO HIM BY OTHERS FOR THE DESIGN
OR CONSTRUCTION OF THE PROJECT. THE ENGINEER SHALL NOT
BE HELD RESPONSIBLE FOR ANY OTHER CAUSES OF FAILURE OF THE
PROJECT OR FOR ANY OTHER CAUSES OF FAILURE OF THE PROJECT.

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0	01-05-11					

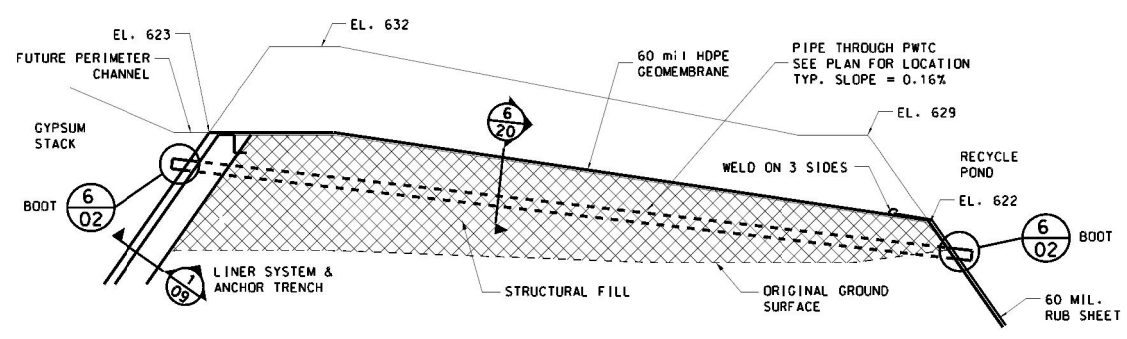
LOC. NO.	DRAWING NO.	REV.
	C-10206	0



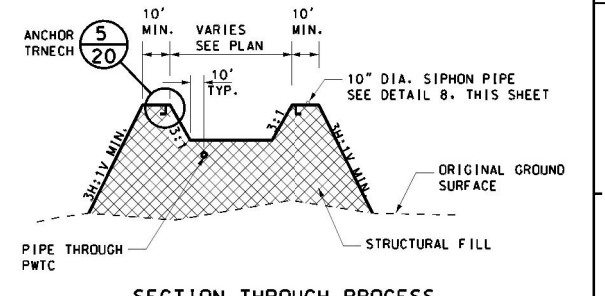
RECYCLE POND PLAN &
CONTROL DATA
CCB MANAGEMENT FACILITY
SITE: COFFEEEN



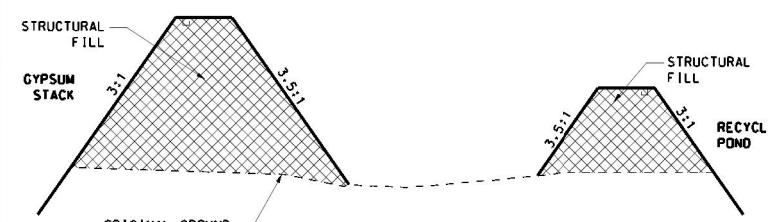
1 TYPICAL SECTION THROUGH RECYCLE POND
NTS



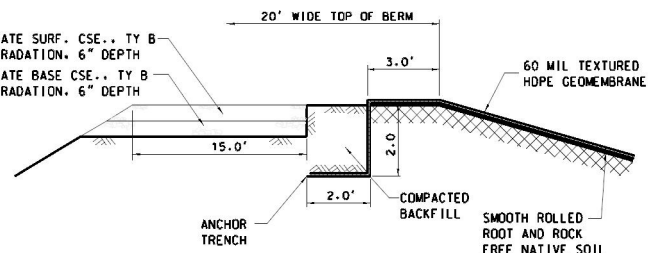
2 SECTION THROUGH PROCESS WATER TRANSFER CHANNEL
NTS



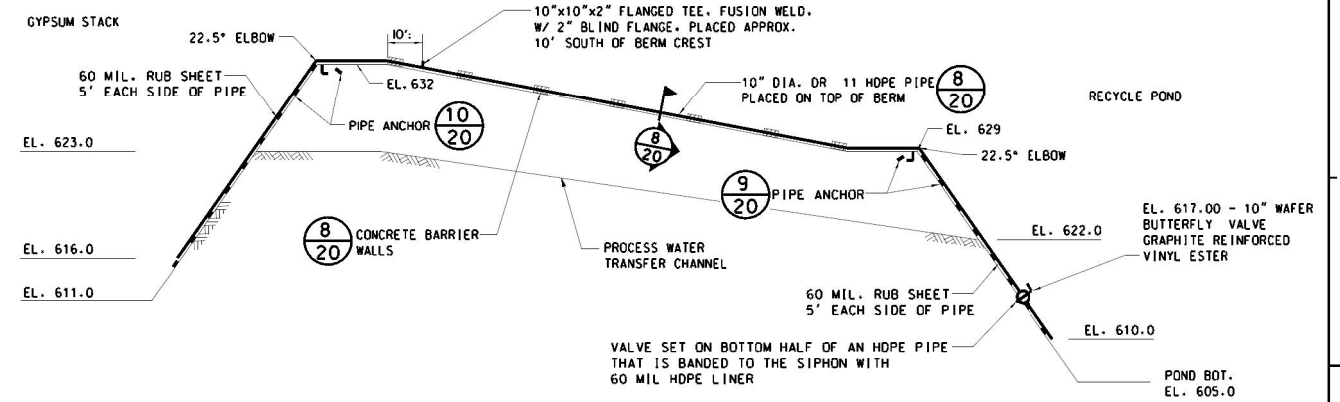
3 SECTION THROUGH PROCESS WATER TRANSFER CHANNEL
NTS



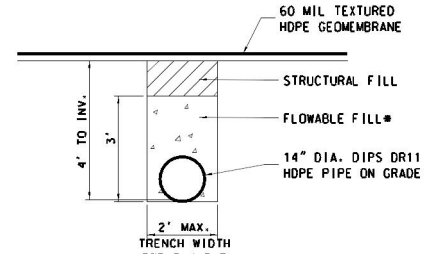
4 SECTION THROUGH BERMS
NTS



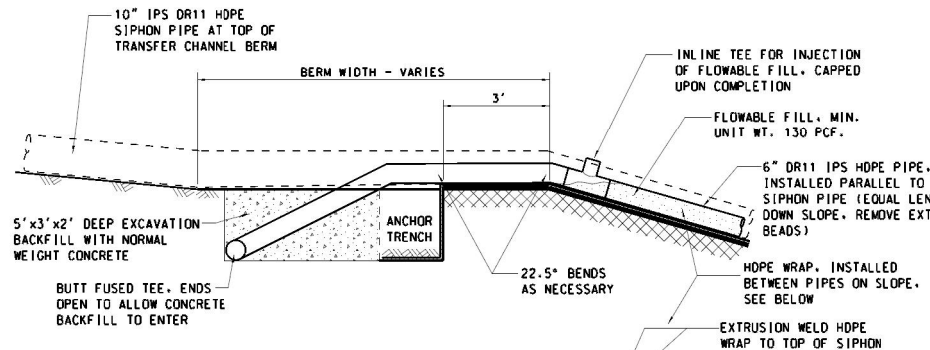
5 RECYCLE POND - LINER SYSTEM
NTS



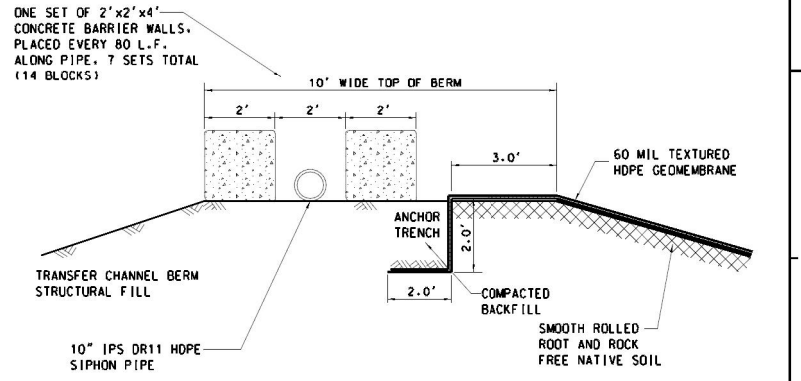
7 GYPSUM STACK SIPHON SYSTEM SECTION THROUGH PROCESS WATER TRANSFER CHANNEL
NTS



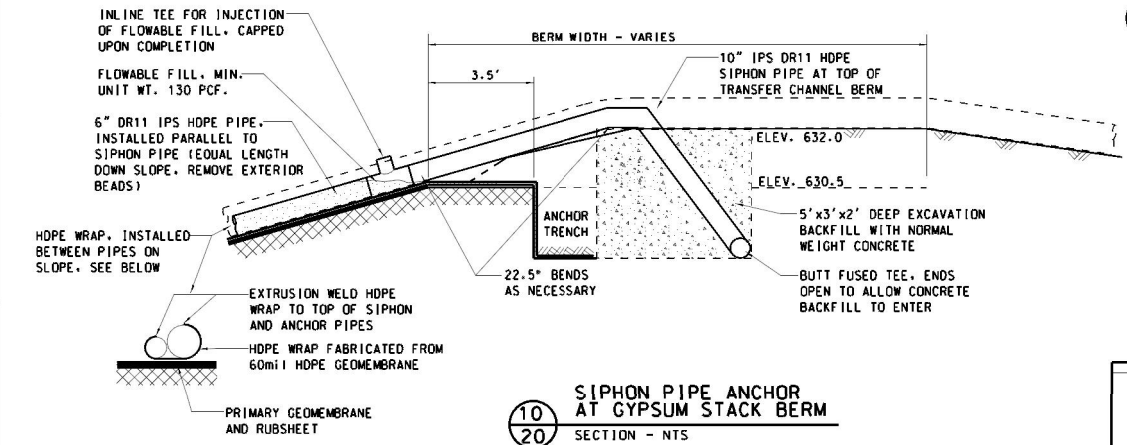
6 TYPICAL TRENCH SECTION
NTS



9 SIPHON PIPE ANCHOR AT RECYCLE POND BERM SECTION - NTS



8 SIPHON PIPE INSTALLATION SECTION - NTS

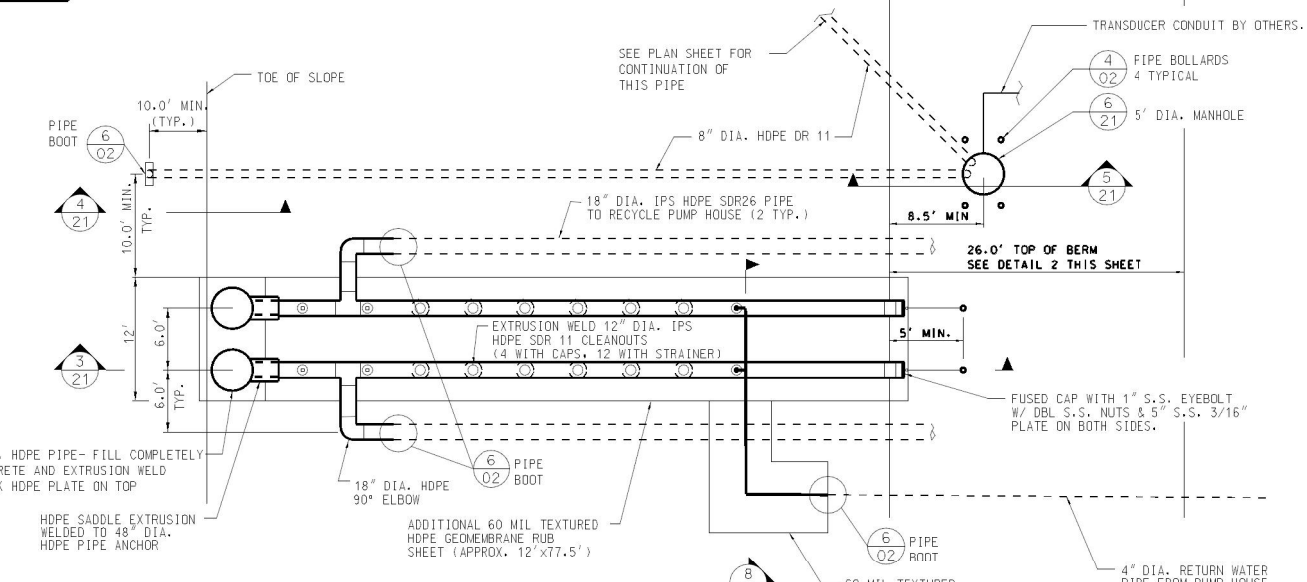


10 SIPHON PIPE ANCHOR AT GYPSUM STACK BERM SECTION - NTS

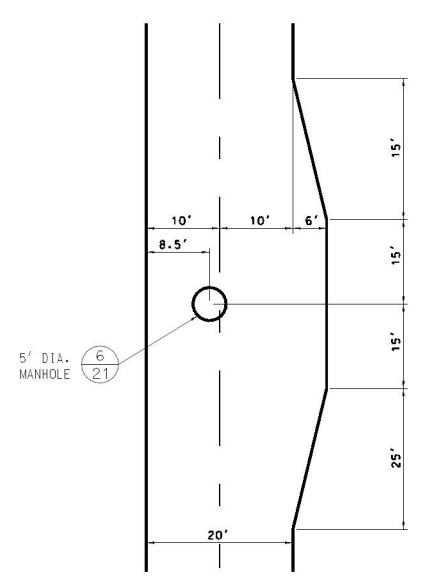
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0	01-05-11						

NOTICE OF LIMITED LIABILITY
THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN AND CONSTRUCTION OF THE PROJECT DESCRIBED IN THESE PLANS AND NOT TO ANY OTHER MATTER. THE ENGINEER DOES NOT WARRANT THE ACCURACY OF THE INFORMATION PROVIDED TO HIM BY OTHERS. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR ANY OTHER MATTER OR FOR ANY OTHER MATTER NOT DESCRIBED IN THESE PLANS.

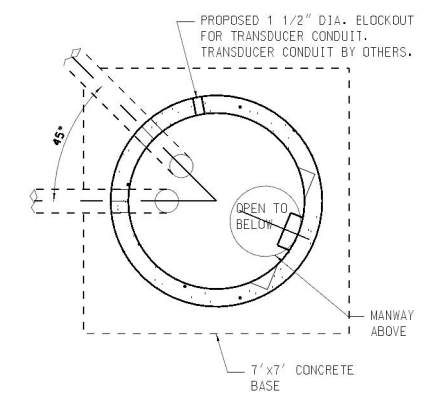
RECYCLE POND - PROCESS WATER TRANSFER CHANNEL DETAILS		
CCB MANAGEMENT FACILITY		
SITE: COFFEEN		
LOC. NO.	DRAWING NO.	REV.
CLASS	C-10206	0
AmereEnergy Generating		SHEET NO. 20



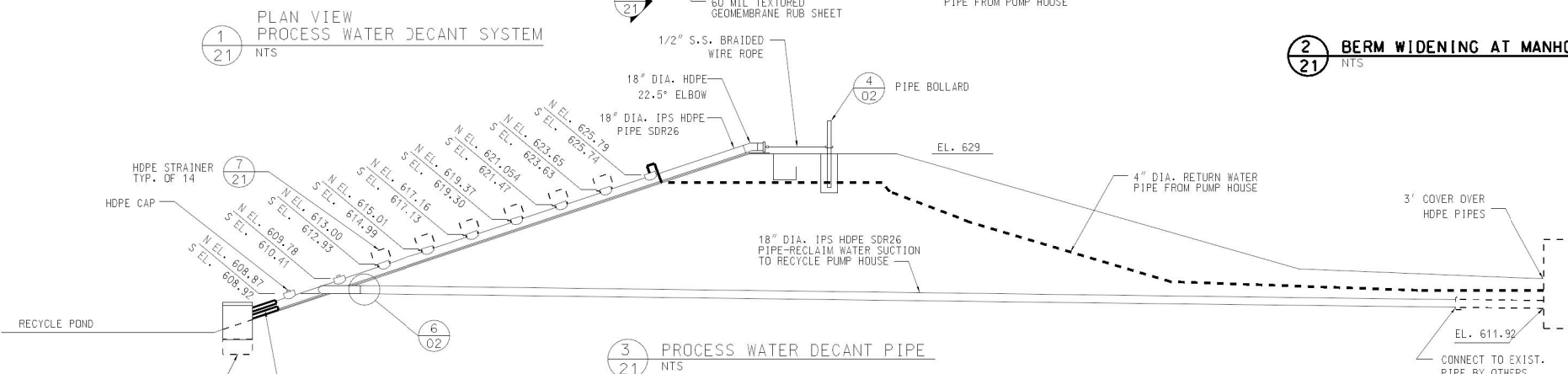
1 PLAN VIEW PROCESS WATER DECANT SYSTEM
NTS



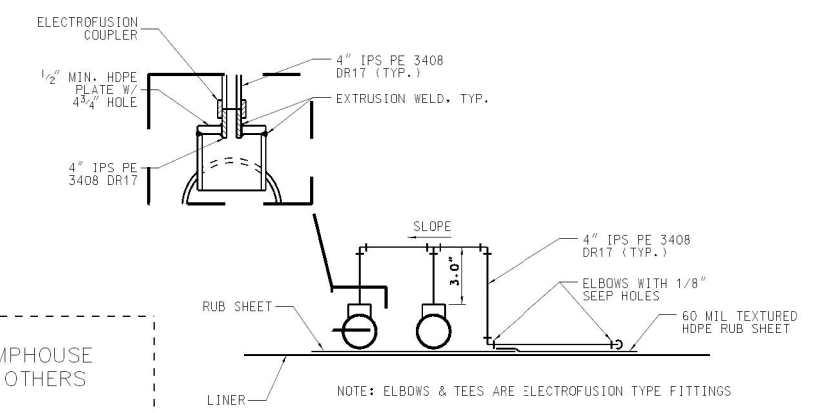
2 BERM WIDENING AT MANHOLE
NTS



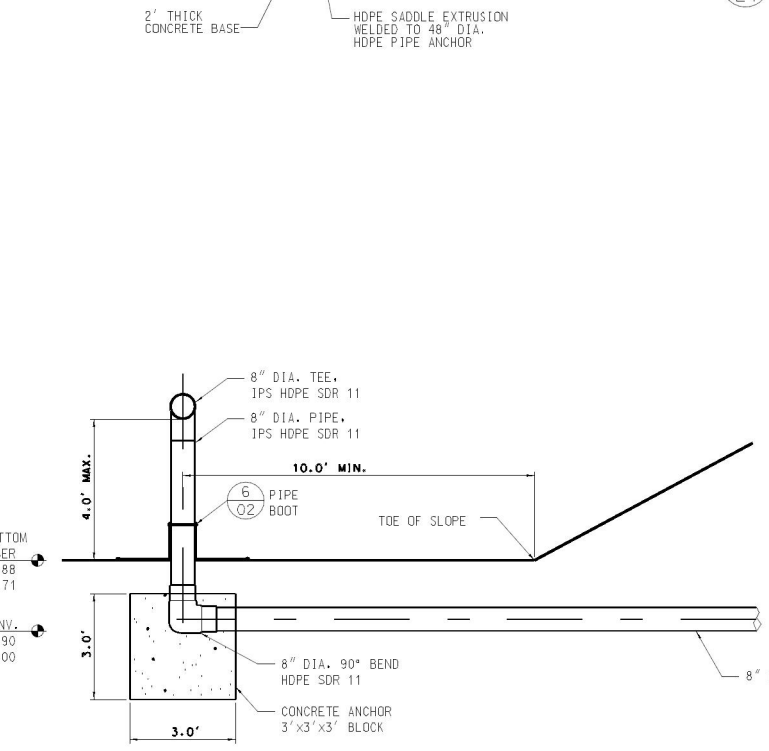
6 MANHOLE PLAN
NTS



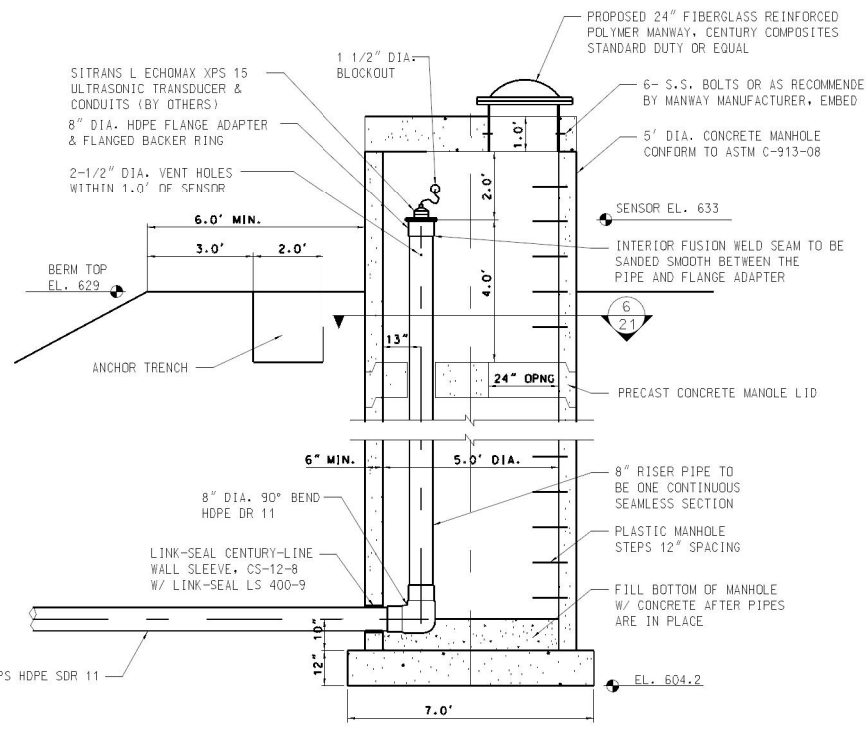
3 PROCESS WATER DECANT PIPE
NTS



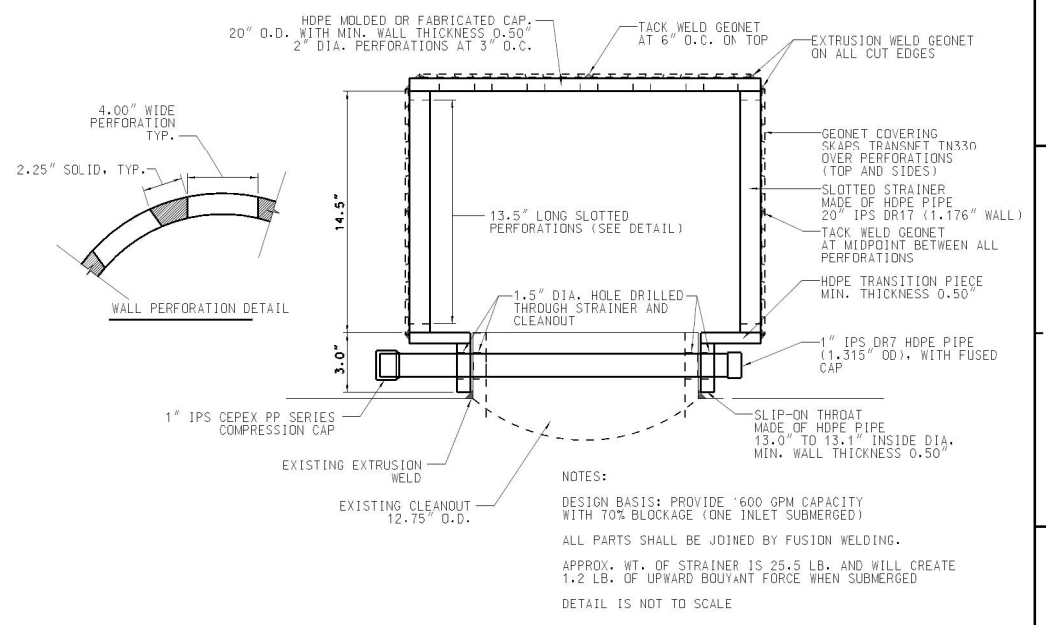
8 RECIRCULATING LINE CONNECTION
NTS



4 POND RISER PIPE FOR ULTRASONIC TRANSDUCER
NTS



5 MANHOLE FOR ULTRASONIC TRANSDUCER
NTS



7 HDPE STRAINER DETAIL
NTS

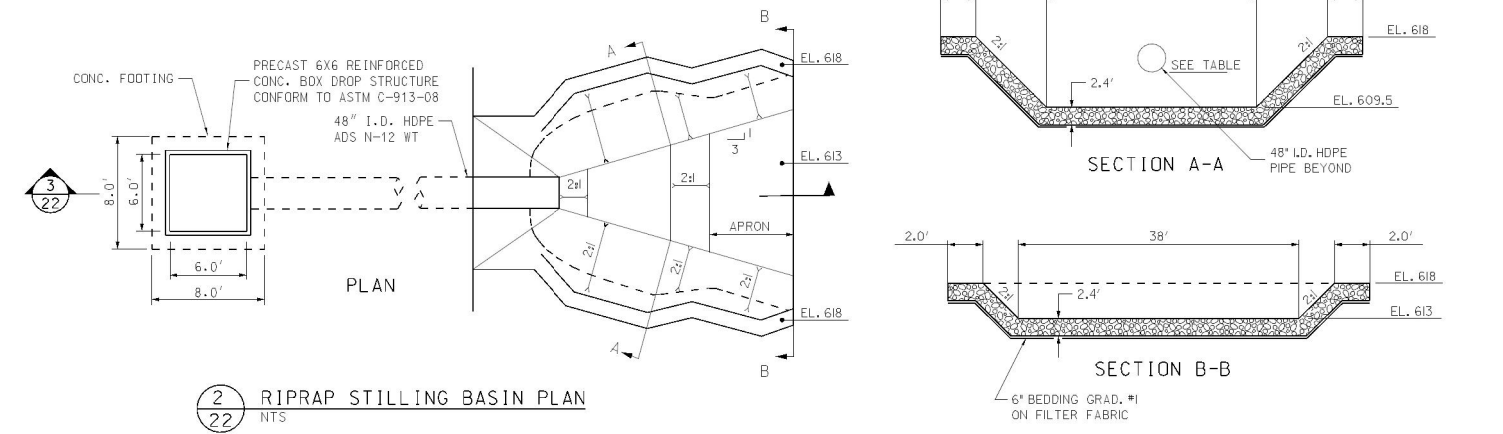
NOTICE OF LIMITED LIABILITY
THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN OF THE PROJECT DESCRIBED IN THESE PLANS. THE ENGINEER DOES NOT WARRANT, REPRESENT OR GUARANTEE THE ACCURACY OF THE INFORMATION PROVIDED TO HIM BY OTHERS, NOR DOES HE WARRANT, REPRESENT OR GUARANTEE THE ACCURACY OF THE INFORMATION PROVIDED TO HIM BY HIS OWN EMPLOYEES OR AGENTS.

NO.	DATE	PROJECT	BY	CHKD	APP'D	DESCRIPTION
1	01-05-11	C-10207 LANDFILL PLANS	SVB	MWR	MWR	ISSUED FOR PERMITS
2		C-10211 LANDFILL ACCEPTANCE PLANS	SVB	MWR	MWR	
3		C-10212 GYPSUM STACK ACCEPTANCE PLANS	SVB	MWR	MWR	
4		E-10850 SUBDRAINAGE POWER PLANS	SVB	MWR	MWR	
5		E-10855 LEACHATE COLLECTION POWER PLANS	SVB	MWR	MWR	

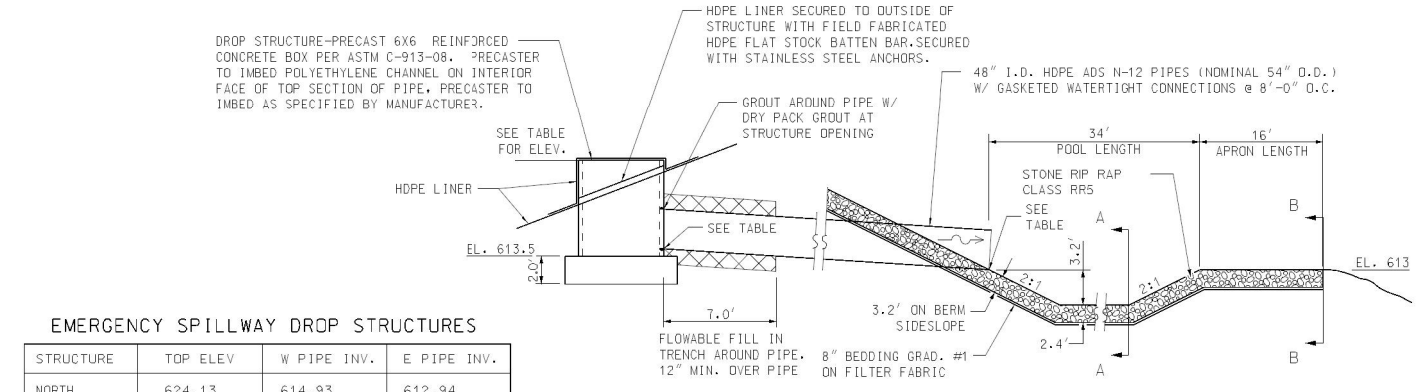
SCALE	RECYCLE POND - PROCESS WATER DECANT SECTIONS & DETAILS
CLASS	CCB MANAGEMENT FACILITY
LOC. NO.	SITE: COFFEEN
DRAWING NO.	C-10206
REV.	0
SHEET NO.	21



1 STILLING BASIN PLAN AT EMERGENCY SPILLWAY
22 NTS



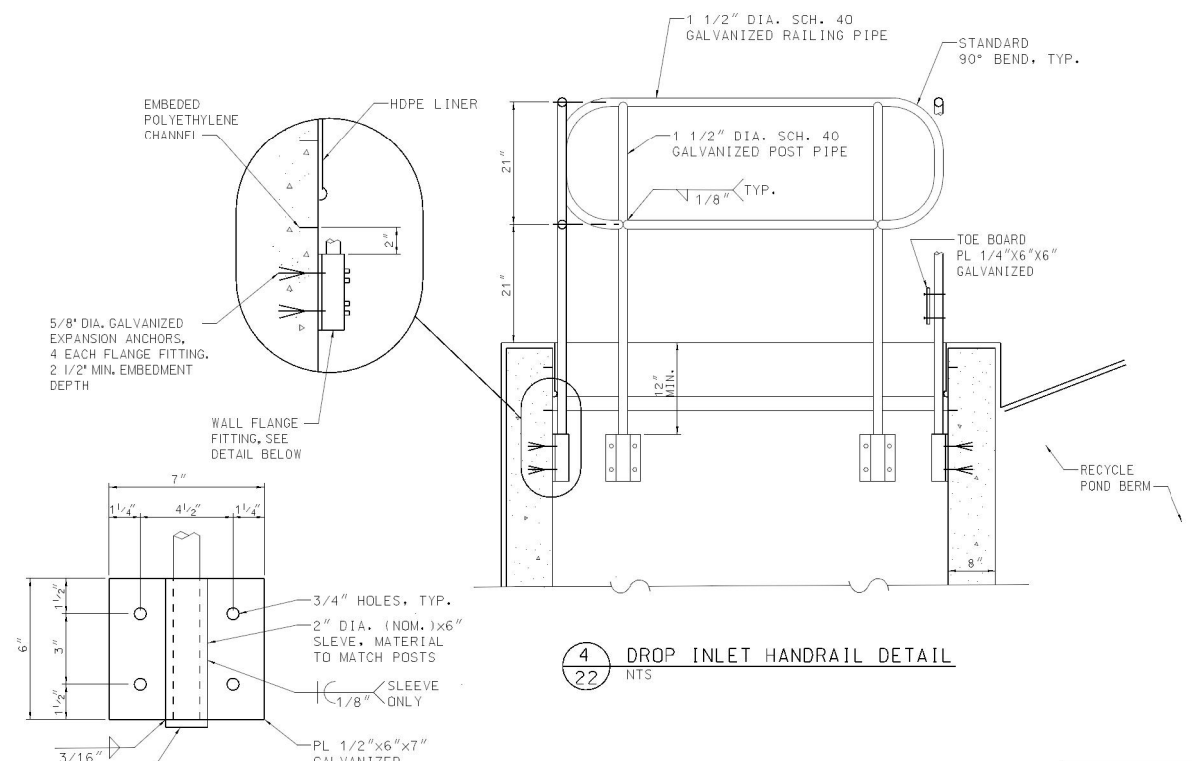
2 RIPRAP STILLING BASIN PLAN
22 NTS



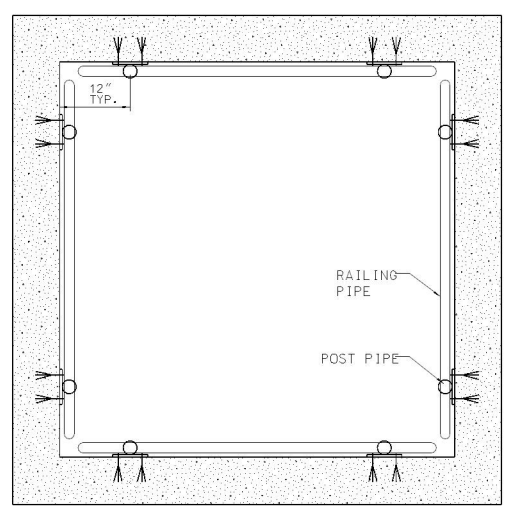
EMERGENCY SPILLWAY DROP STRUCTURES

STRUCTURE	TOP ELEV	W PIPE INV.	E PIPE INV.
NORTH	624.13	614.93	612.94
MIDDLE	624.15	614.95	613.00
SOUTH	624.11	614.03	613.06

3 RIPRAP STILLING BASIN SECTION
22 NTS



4 DROP INLET HANDRAIL DETAIL
22 NTS



5 DROP INLET HANDRAIL PLAN
22 NTS

HANDRAIL NOTES

- GT-1 COPE ALL INTERSECTIONS OF RAILS AND POSTS, WELD JOINTS, AND GRIND SMOOTH TO A PLEASING APPEARANCE, TAKING CARE TO NOT REMOVE EXCESSIVE AMOUNT OF WELDED MATERIALS.
- GT-2 BUTT WELD END-TO-END JOINTS, OR USE WELDING CONNECTIONS.
- GT-3 CLEAN WELDED AREAS USING A WIRE BRUSH TO REMOVE SLAG AND LOOSE PARTICULATE. WIPE AWAY DUST WITH A CLEAN DRY RAG AND CLEAN THE WELDED SURFACE IN ACCORDANCE WITH SSPC-SP 1.
- GT-4 TREAT ALL WELD ZONES WITH TWO COATS OF COLD GALVANIZING COMPOUND.
- GT-5 COAT THE WELDED HANDRAIL ASSEMBLY PER THE FOLLOWING:
 - 1 - 4 MIL COAT OF KRYLON INDUSTRIAL GALVANIZED METAL PRIMER
 - 2 - 3 MIL COATS OF KRYLON INDUSTRIAL RUST TOUGH DTM ALKYD ENAMEL - COLOR: OSHA SAFETY YELLOW

NOTICE OF LIMITED LIABILITY
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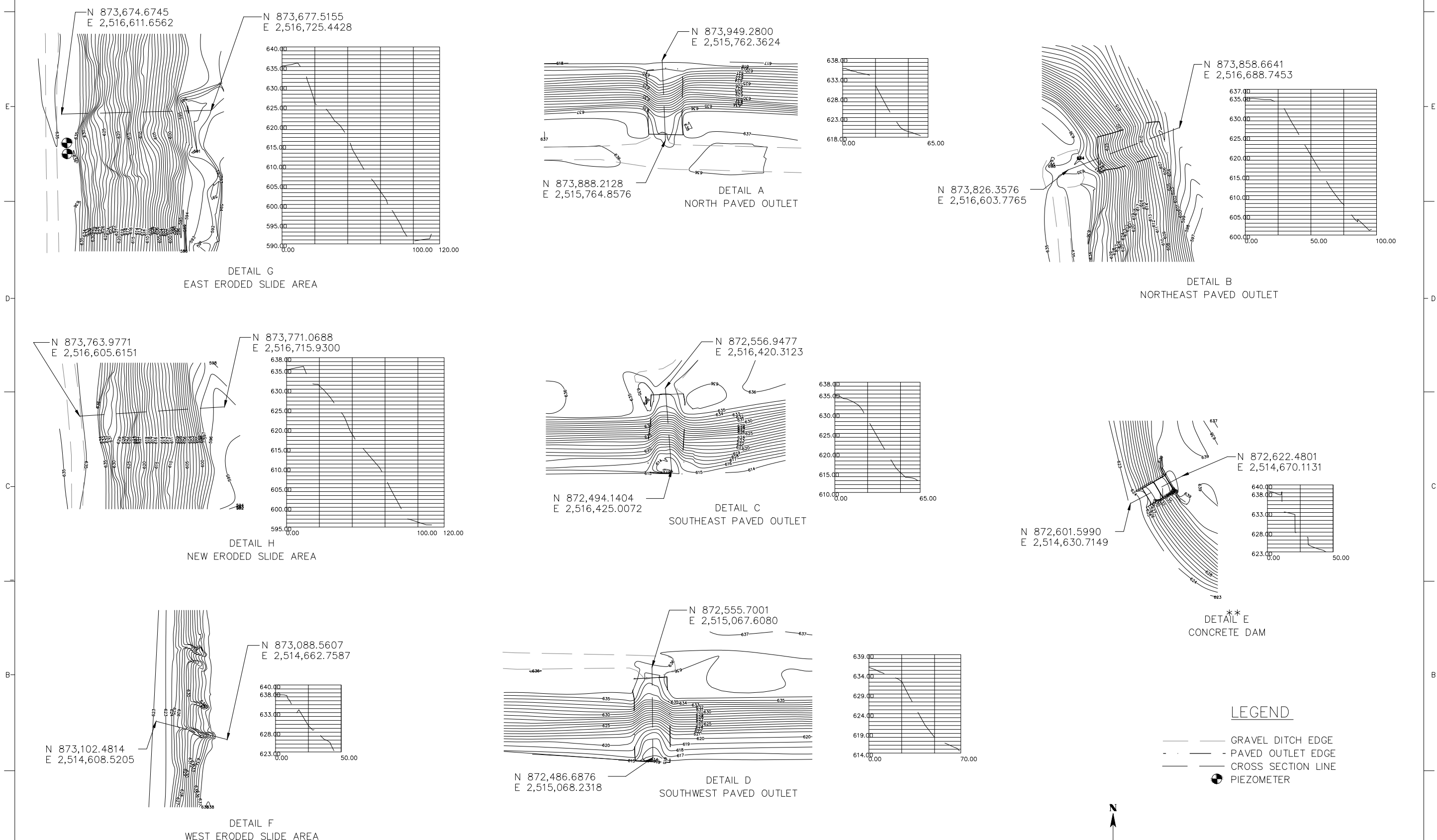
SCALE: _____
 RECYCLE POND - EMERGENCY SPILLWAY SECTIONS & DETAILS
 CCB MANAGEMENT FACILITY
 SITE: COFFEE
 AMEREN ENERGY logo
 DRAWING NO. C-10206
 REV. 0
 SHEET NO. 22

REV.	DATE	PROJECT NO.	AMEREN SUPPLY ENGR.	DESIGNER	DRAFTING	ENGR. APPROVAL	DESCRIPTION
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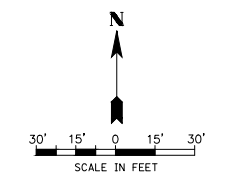
REFERENCE DRAWINGS:
 C-10207 LANDFILL PLANS
 C-10211 LANDFILL ACCEPTANCE PLANS
 C-10212 GYPSUM STACK ACCEPTANCE PLANS
 E-10850 SUBDRAINAGE POWER PLANS
 E-10855 LEACHATE COLLECTION POWER PLANS

WALL FLANGE FITTING
NTS

COFFEEN POWER STATION CLOSED ASH POND TOPO SURVEY



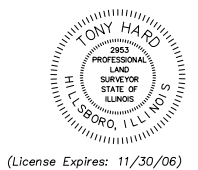
- LEGEND**
- GRAVEL DITCH EDGE
 - - - PAVED OUTLET EDGE
 - CROSS SECTION LINE
 - ⊙ PIEZOMETER



NOTICE OF LIMITED RESPONSIBILITY
 THE RESPONSIBILITY OF THE UNDERSIGNED ENGINEER IS LIMITED TO THE DESIGN AND CONSTRUCTION OF THE PROJECT AS SHOWN ON THESE PLANS. THE ENGINEER HAS NOT CONDUCTED A VISUAL INSPECTION OF THE PROJECT AS SHOWN ON THESE PLANS. THE ENGINEER HAS NOT CONDUCTED A VISUAL INSPECTION OF THE PROJECT AS SHOWN ON THESE PLANS.

AS BUILT

McDonough-Whitlow, P.C.
 Consulting Engineers & Land Surveyors
 138 East Wood Street
 Hillsboro, IL 62049
 Phone: 217.532.9233
 Fax: 217.532.6300
 PROFESSIONAL DESIGN NO. 184-002754



THIS IS TO CERTIFY TO AMEREN ENERGY THAT A TOPOGRAPHIC SURVEY SHOWN HEREON HAS BEEN PERFORMED BY ME OR UNDER MY SUPERVISION AND THIS PLAT IS A REPRESENTATION OF MY PROFESSIONAL OPINION OF SAID SURVEY.
 ILLINOIS PROFESSIONAL LAND SURVEYOR NO. 035-2953 DATE
 FIELD WORK FOR THIS SURVEY WAS COMPLETED ON MAY 28, 2009

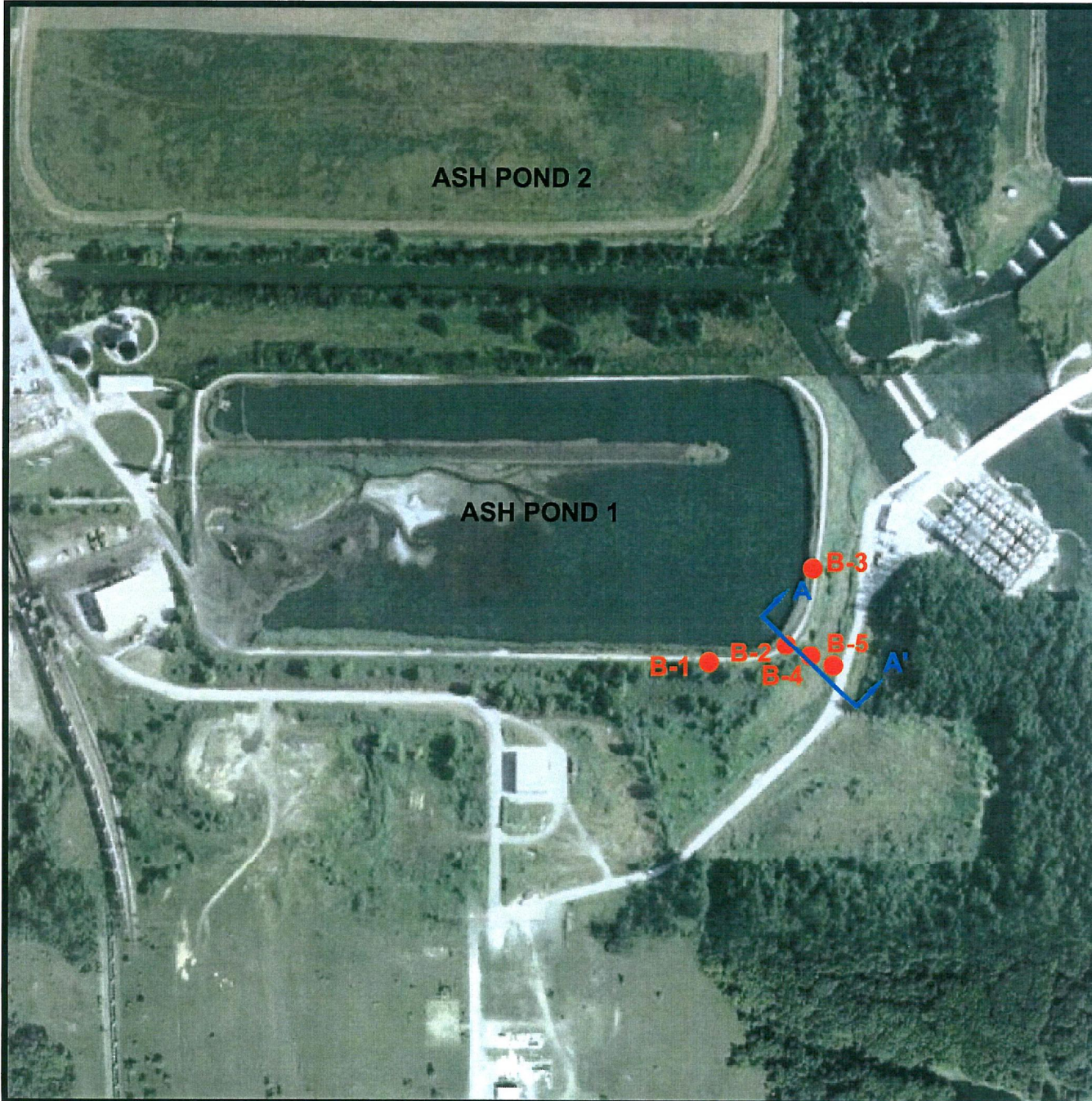
NOTES		REFERENCE DRAWINGS		DRAWING RECORD				DESCRIPTION
REV.	DATE	PROJECT NO.	AMEREN SUPV ENGR	DRAFTING	ENGR APPROVAL	AMEREN OTHER		

SCALE	1" = 30'
PROJECT	COFFEEN POWER STATION ASH POND #2 DRAINAGE MODIFICATIONS
SITE	AMEREN CLOSED ASH POND
LOC. NO.	
CLASS	
DRAWING NO.	W1008
REV.	
SHEET NO.	2 OF 2





Appendix C: Coffeen Power Station Boring and Piezometer Locations



NOTES

1. Plan adapted from an aerial photograph courtesy of Google Earth.

LEGEND

- Boring Location
- Slope Stability Cross Section



Drawn By: SLC	Ck'd By: <i>SA</i>	App'vd By: <i>DW</i>
Date: 11-04-10	Date: <i>12/21/10</i>	Date: <i>1/4/11</i>



Coffeen Power Station
Coffeen, Illinois

**AERIAL PHOTOGRAPH OF SITE
AND BORING LOCATIONS**

Project Number
J017150.01

PLATE 2

File: P:\PROJECTS\GEOTECH\60428794_DYNEGYCCR\047TASKS\00 PROGRAM TASKS\1.0 TASK 1 INITIAL UNIT ASSESSMENT\CCR FACT SHEETS\SITE MAPS\FIGURE 2A PIEZOMETER LOCATION PLAN (COFFEEN).DWG Last edited: NOV. 04. 15 @ 3:03 p.m. by: david_dequire



COFFEEN
ASH POND
NO. 1

COF-P008

COF-P003

COF-P007

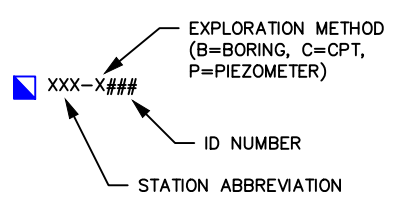
COF-P005

COF-P006

COF-P000

COF-P001

COF-P002



LEGEND

PIEZOMETER LOCATION



CCR UNIT BERM ALIGNMENT



APPROXIMATE SCALE FEET



Illinois Power Generating Company	PROJECT NO. 60440742
-----------------------------------	-------------------------



DRN. BY:djd October 2015 DSGN. BY:eg CHKD. BY:eg	Coffeen Ash Pond No.1 Piezometer Locations	FIG. NO. 2A
--	---	----------------

SOURCE:
MAP PROVIDED BY GOOGLE EARTH PRO 2015

File: P:\PROJECTS\GEOTECH\60428794_DYNEGYCCR\04\TASKS\00 PROGRAM TASKS\1.0 TASK 1 INITIAL UNIT ASSESSMENT\CCR FACT SHEETS\SITE MAPS\FIGURE 2A PIEZOMETER LOCATION PLAN (COFFEEN).DWG Last edited: NOV. 04. 15 @ 3:03 p.m. by: david_dequire



COFFEEN
ASH POND
NO. 2

LEGEND

- AECOM PIEZOMETER LOCATION
- HISTORICAL PIEZOMETER LOCATION

— — — CCR UNIT BERM ALIGNMENT



APPROXIMATE SCALE FEET

SOURCE:
MAP PROVIDED BY GOOGLE EARTH PRO 2015

Illinois Power Generating Company		PROJECT NO. 60440742
AECOM		
DRN. BY:djd October 2015 DSGN. BY:eg CHKD. BY:eg	Coffeen Ash Pond No.2 Piezometer Locations	FIG. NO. 2B



Appendix D: Project Specifications, Gypsum Stack and Recycle Pond Construction (Hanson 2008)

PROJECT SPECIFICATIONS
GYPSUM STACK AND RECYCLE POND CONSTRUCTION
GYPSUM MANAGEMENT FACILITY
COFFEEN POWER STATION
MONTGOMERY COUNTY, ILLINOIS

Prepared For:

AMEREN ENERGY GENERATING COMPANY

Prepared By:

HANSON PROFESSIONAL SERVICES INC.
1525 South Sixth Street
Springfield, Illinois 62703

January 2008

GYPSUM STACK AND RECYCLE POND CONSTRUCTION
GYPSUM MANAGEMENT FACILITY
COFFEEN POWER STATION
MONTGOMERY COUNTY, ILLINOIS

TABLE OF CONTENTS

Sheet No.

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Division 2 – Site Work

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02275 - Riprap	02275-1 to 02275-2
02315 - Granular Drainage Materials	02315-1 to 02315-8
02373 - Geotextiles	02373-1 to 02373-8
02376 - Geosynthetic Clay Liner	02376-1 to 02376-9
Attachment 1 – Bentomat [®] Certified Properties	End of Section
02640 - HDPE Piping	02640-1 to 02640-4
02800 - HDPE Geomembrane	02800-1 to 02800-12
02936 – Topsoil Seeding and Mulching	02936-1 to 02936-3
03100 – Concrete Formwork	03100-1 to 03100-5
03200 – Concrete Reinforcement	03200-1 to 03200-3
03300 – Cast-In-Place Concrete	03300-1 to 03300-6
03400 – Concrete Embedment Liner	03400-1 to 03400-4

DIVISION1-GENERAL REQUIREMENTS
Section 01356 – Storm Water Pollution Prevention
Measures

PART 1. GENERAL

1.01 DESCRIPTION

- A. This section pertains to the construction and maintenance of temporary erosion control systems to control erosion and sediment damage to adjacent properties and water resources, and the removal of erosion control devices when they are no longer required.

1.02 RELATED SECTIONS

The following sections contain items which are related to the work in this section:

02936 - Topsoil, Seeding, and Mulching.

1.03 REFERENCES

The following reference, or cited portions thereof, governs the work:

Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.

1.04 SUBMITTALS

- A. Submittals shall follow the provisions of Section 01010.
- B. Preconstruction Submittals: A storm water best management practices (BMP) plan shall be submitted that includes the following items:
1. Site drawing showing anticipated locations of structural erosion controls, areas of disturbed soils, and drainage patterns;
 2. Inspection and record-keeping procedures; and
 3. Maintenance procedures for erosion controls.

PART 2. PRODUCTS

2.01 EROSION CONTROL SYSTEMS

Materials for erosion control systems shall be in accordance with Article 280.02 of the IDOT Standard Specifications.

PART 3. EXECUTION

3.01 EXAMINATION

The site shall be examined to determine the extent of work required.

3.02 PRECONSTRUCTION JOBSITE INSPECTION

- A. The person who shall be at the jobsite during construction and who shall be responsible for insuring that erosion control work is completed in a timely manner shall be identified at the preconstruction meeting.
- B. A jobsite inspection shall be conducted with the Owner's Representative to review and designate the locations and types of erosion protection to be placed. The inspection shall be scheduled at the preconstruction conference and carried out on the job site before beginning any work that will disturb existing drainage or potentially create erodible conditions.

3.03 CONSTRUCTION

- A. Temporary erosion control systems shall be constructed in accordance with IDOT Standard 280001 and Article 280.04 of the Standard Specifications and as directed by the Owner's Representative. Erosion control devices shall be in place and approved by the Owner's Representative prior to beginning other work.
- B. Incorporate permanent erosion control features into the project at the earliest practicable time to minimize the need for temporary erosion controls.

3.04 MAINTENANCE

- A. Temporary erosion control systems shall be maintained in accordance with Article 280.05 of the Standard Specifications, except that measurement and payment provisions shall not apply.
- B. Temporary erosion control systems for unprotected disturbed areas shall be cleaned of trapped sediment and repaired immediately prior to project close out.
- C. Temporary seeding shall be applied to all disturbed areas except the gypsum stack excavation and the future fill and topsoil stockpiles.

3.05 REMOVAL AND DISPOSAL

When the Owner's Representative deems that temporary erosion control systems are no longer needed, they shall be removed and properly disposed, and silt deposits shall be removed or regarded as directed by the Owner's Representative, and the area seeded. Non-biodegradable temporary erosion control materials shall be disposed of off site. Biodegradable erosion control devices may be disposed of in spoil areas designated by the Owner's Representative. All laws and regulations in disposing of the materials shall be obeyed.

END OF SECTION 01356

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PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to stripping of topsoil and vegetation from areas of the site that are to be excavated.

1.02 RELATED SECTIONS

No related sections.

PART 2. PRODUCTS

No products used.

PART 3. EXECUTION

3.01 EXAMINATION

The Contractor shall examine the site to determine the extent of work required.

3.02 SITE PREPARATION - STRIPPING

- A. All vegetation and topsoil encountered within the Gypsum Stack grading limits shall be stripped. Topsoil shall be kept clean and free of all foreign material, and stored in separate stockpiles from vegetation and common excavations. Stockpiles shall be located as indicated on the drawings or as directed by the Owner's Representative.
- B. Payment for stripping shall be based upon removal of 24 inches of topsoil in areas that require stripping.

3.03 DISPOSAL

All materials resulting from site preparation operations shall be stockpiled in the designated spoil area. Contractor shall obey all laws and regulations when disposing of the materials.

END OF SECTION 02100

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PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to excavation, fill, and backfill required for foundation preparation, construction of low-permeability soil layer, anchor trench construction, miscellaneous site grading and berm construction.

1.02 RELATED SECTIONS

- A. The following sections contain items which are related to the work in this section:
1. 01356 – Storm Water Pollution Prevention Measures
 2. 02100 - Site Preparation
 3. 02373 – Geotextiles
 4. 02936 - Topsoil, Seeding, and Mulching

1.03 REFERENCES

The following references, or cited portions thereof, govern the work:

1. Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007
2. Department of Sustainable Natural Resources, Soil Survey Standard Test Method, Unified Soil Classification System: Field Method (USCS).

1.04 MEASUREMENT AND PAYMENT

- A. The Contractor shall be responsible for estimating the extent of excavation and fill required to complete the work, including, but not limited to, excavation to required elevations; loading, transporting, placing, and compacting low permeability soil; excavation and backfill of anchor trench; and miscellaneous site grading and berm construction. The Contractor shall include the dollar amount associated with all earthwork in his Lump Sum Bid amount.
- B. Removal and replacement of unsuitable foundation material and subgrade stabilization measures directed by the Owner's Representative will be paid for on a time and material basis.

1.06 COORDINATION

Existing utilities or other plant facilities shall not be interrupted, except when permitted in writing by the Owner's Representative and then only after acceptable

temporary services have been provided. A minimum 48-hour notice shall be provided prior to proceeding with an approved temporary interruption.

1.08 SUBMITTALS

A. Materials Handling Plan.

A materials handling plan shall be submitted for construction and protection of the low permeability soil liner. The plan shall describe the following:

1. Processing and placement of the low permeability soil type, model number, weight, and critical dimensions of equipment to be used for soil processing, compaction, scarification, and smooth rolling;
2. Method of protecting low permeability soil from changes in moisture content and freezing after placement.

B. Construction Access Ramp Layout.

Layout drawings shall be submitted showing alignment, profile, and typical section of the construction access ramps from the haul road into the bottom of the Gypsum Management Facility excavation. The minimum width of the ramp shall be 50 ft., and the longitudinal grade shall not exceed 8 percent.

PART 2. PRODUCTS

2.01 MATERIALS

A. Earth Fill Material

Earth Fill Material shall consist of a mixture of clay, silt, sand, and gravel-sized particles obtained from previously constructed subsoil stockpiles. These materials can be used separately or mixed as required for best results. When placed, Embankment Material shall have a USCS classification of SM, ML, or CL and shall be uniform. This material shall be free of ice, snow, organic matter, rubbish, and debris. Coarse-grained particles shall be well dispersed to prevent the development of segregated pockets or zones with insufficient fine material to fill the interstices.

B. Soil Liners

The Soil Liner for the Gypsum Management Facility is considered a Clay Liner, and shall be soil classification CL, CL-ML, or CH. The material shall be free of roots, debris, organic or frozen material, and shall have a maximum clod size no greater than the length of the compactor foot for the compaction equipment proposed by the Contractor. When compacted, the material shall have a hydraulic conductivity of less than 1×10^{-4} cm/sec.

C. Soil Stabilizers and Moisture Conditioning Agents

Additives to accelerate drying or to improve stability and workability of soil shall not be permitted unless approved in writing by the Owner's Representative.

2.02 EQUIPMENT

A. Compaction Equipment

1. Tamping foot rollers

Compaction equipment shall consist of tamping foot rollers which have a minimum weight of 40,000 pounds. At least one tamping foot shall be provided for each 110 square in. of drum surface. The length of each tamping foot, measured from the outside surface of the drum, shall be at least 1 in. longer than the loose lift thickness.

2. Steel-Wheeled Rollers

Equipment used to produce a smooth compacted surface shall be a smooth, non-vibratory steel wheeled roller weighing not less than 1,000 lb. per lineal ft.

B. Scarification Equipment

Discs, rotor tillers, or other equipment used to scarify the surface shall be capable of uniformly disturbing the upper 6 in. of surface to provide good bonding between lifts.

C. Mixing and Spreading Equipment

Discs, harrows, and motor graders or other similar equipment shall be available at the site for use in spreading, mixing, and drying Compacted Subsoil Stockpile Material.

PART 3. EXECUTION

3.01 PREPARATION

A. Control of Work

Benchmarks, monuments, and other reference points shall be maintained throughout the work area.

B. Utility Location

Before starting excavation, the location and extent of underground utilities in the work area shall be established.

3.02 EXCAVATION

A. General

Excavation consists of removal and redistribution of material encountered when establishing required grade and subgrade elevations. The Contractor shall be responsible for dewatering, protection, shoring, and disposal of excavated materials as necessary to complete the excavation.

B. Procedures

Excavation may be accomplished by any method and by use of any equipment that is suitable to the work, except that blasting will not be permitted. Based on previous construction experience at the site, it is recommended that excavation to the foundation grade be completed as far in advance of low permeability soil placement as possible to allow the foundation surface to dry and form a "crust" capable of sustaining compactive effort.

C. Overexcavation

All excavation shall be performed to the lines and grades indicated on the plans. Any overexcavation or excess excavation not requested by the Owner's Representative shall be at the expense of the Contractor.

D. Disposal of Excavated Materials

Contractor shall utilize excavated material as stockpile materials for future use as specified in paragraph E.

E. Stockpile Requirements

1. Excavated clay and silty clay materials are to be stockpiled in the short-term subsoil stockpile area.
2. Excess excavated materials are to be stockpile in the areas designated on the drawings.
3. Materials not suitable for use as fill or backfill shall be disposed of onsite in the locations specified by the Owner's Representative.
4. Spread fill material for use by others, topsoil, and low permeability soil are to be stockpiled in layers not to exceed 1 ft loose thickness.
5. Tops of stockpiles are to be graded to ensure positive drainage. Side slopes for stockpiles shall be no steeper than 3H:1V.
6. Perimeter ditches are to be excavated to intercept runoff flowing toward stockpile areas and to route it to outlet locations approved by the Owner's Representative.

3.03 SUBGRADE PREPARATION

- A. Areas to receive fill shall be proof rolled under the observation of the Owner's Representative. Soft, loose, weak, or wet materials shall be removed and replaced with compacted fill or stabilized with geotechnical fabric or geogrid as directed by the Owner's Representative. Joints, fractures, and moisture seeps shall be repaired, and local sand deposits, if present at foundation grade, shall be removed and backfilled with compacted fill material as directed by the Owner's Representative.
- B. The Owner's Representative may recommend additional drying time for soft, wet subgrade that has not been exposed long enough to permit "crust" formation. If approved by the Owner's Representative, the Contractor may install, at his own expense, geotechnical fabric or geogrid to stabilize the wet subgrade and expedite construction.
- C. No fill shall be placed until the subgrade has been examined and approved.

3.04 GENERAL FILL

- A. Placement
 - 1. Unless otherwise indicated on the plans, all fill shall be composed of Earth Fill Material.
 - 2. Fill materials used in embankment construction shall normally be placed in lanes parallel to the embankment axis and shall be placed in conformance with the lines, grades, and slopes as indicated on the plans. Placement of fill materials in lanes which are not parallel to the embankment will be allowed only where working room is too restricted for normal placement as determined by the Owner's Representative.
 - 3. Fill shall be spread in approximately flat layers in such a manner as to obtain lifts of relatively uniform thickness without spaces between successively deposited loads. Segregation shall be prevented during placing and spreading. Hauling equipment shall be routed across the fill in such a way as to promote uniform compaction and to prevent the formation of ruts.
 - 4. The maximum compacted thickness of each lift shall not exceed 8 in. where heavy compaction equipment will be used. The maximum compacted thickness shall not exceed 3 in. where power tampers or similar smaller equipment will be used. It may be necessary to reduce the thickness of lifts in order to obtain the required minimum density.
 - 5. Where compacted earth fill is to be placed against existing slopes, each lift shall be keyed into existing slope by removing existing slope material in steps as each new lift is placed.
 - 6. The surface of the fill shall be kept reasonably smooth. The fill surface shall be sloped transverse to the axis of the embankments to allow drainage. If the compacted surface is, in the opinion of the Owner's Representative, too smooth or too dry to bond properly with the

succeeding lift, it shall be roughened by scarifying, light discing, or other acceptable means, and it shall be sprinkled before the succeeding lift is placed thereon. If the surface becomes rutted or uneven subsequent to compaction, it shall be flattened and leveled before placing the next lift. This extra work shall be at the Contractor's expense.

7. Fill operations shall be suspended during periods of extended wet weather. Upon resuming operations, all fill materials that are excessively wet or soft shall be reprocessed in place or removed and stockpiled for reprocessing. The removal of soft material shall be carried to such depth as is necessary to expose firm materials. Fill shall not be placed on frozen surfaces.
8. When filling operations at any section will be suspended for any period in excess of 12 hours or in wet weather, the surface of the fill shall be rolled smooth to seal it against excessive absorption of moisture and to facilitate runoff. Prior to resuming fill placement and compaction, the fill surface shall be scarified and/or disced and moisture conditioned as required.
9. The Contractor will receive no additional compensation for any removal, reprocessing, stockpiling, recompaction, wasting, or similar operation related to suspensions or conditions due to weather or other causes unless caused by the Owner.
10. Earth fill access ramps shall not be constructed within the limits of the compacted embankments without prior approval. When such ramps are approved, they shall be constructed of low permeability soil (in-board of the perimeter berm) or compacted fill (out-board of the perimeter berm).

B. Compaction -

1. Fill materials shall be compacted to a dry density equal to or greater than the following:
 - a. The Gypsum Management Facility: 95 percent of the maximum dry density obtained from the Standard Proctor Test, ASTM D698.

In order to insure uniform coverage and to facilitate construction inspection and control, the compaction of each layer shall proceed in a systematic, orderly, and continuous manner. Rolling shall be parallel to the embankment axis, except where there is insufficient working room for such operations.

2. The moisture content of all earth fill materials shall be as uniform as practicable throughout each lift. Fill shall be compacted at a moisture content that is no more than 2 percent below and no more than 2 percent above optimum moisture content.
3. Moisture conditioning of fill materials shall be performed by discing, harrowing, plowing, blading, or other suitable means prior to excavation. Moisture conditioning where the fill is placed shall be limited to minor adjustments prior to compaction. Addition of moisture shall be by using a

- pressure spray bar mounted in front of or to one side of a water tanker so that water will not collect in the tracks of the truck.
4. Compaction of fill materials shall not commence if the moisture content is not within the specified limits. Any materials that are placed but not compacted prior to drying out or becoming too wet shall be removed and replaced or reprocessed at the Contractor's expense.
 5. No admixtures as drying agents or to improve the workability of the soil will be allowed.

3.05 SOIL LINERS

A. Sources

The Soil Liners for the Gypsum Management Facility shall be constructed from Soil Liner Material as described in paragraph 2.01(B) above.

B. Test Liner

A compacted low permeability soil test liner of the actual full scale liner shall be constructed in accordance with the following requirements:

1. Test liner will be constructed from the same soil material sources, to the same design specifications, and with similar equipment and procedures as are proposed for the full scale liner.
2. Test liner will be at least four times the width of the widest piece of equipment to be used.
3. Test liner will be no less than 100 ft long to allow equipment to reach normal operating speed before reaching a central 40-ft test area.
4. Test liner will be constructed with maximum 8-in. compacted lifts for a total liner thickness of 3 ft.
5. Test liner will be tested by the Owner's Testing Consultant as described below for each of the following physical properties:
 - a. Multiple two-stage Boutwell permeameter tests will be used on the test liner to determine the hydraulic conductivity. The two-stage field hydraulic conductivity test is a falling head infiltration test conducted in a cased borehole, typically 4 in. in diameter. The test is cited in the U.S. EPA Technical Guidance Document: Quality Assurance and Quality Control for Waste Containment Facilities, September 1993 (EPA/600/R-93/182).
 - b. Undisturbed samples (Shelby tubes) will be tested in the laboratory for hydraulic conductivity to determine if there is a statistical correlation to the field testing results.
 - c. Other engineering parameters including, but not limited to, particle size analysis, liquid limits, plasticity, water content, and in-place density that are needed to evaluate the full scale liner will be determined.

6. Additional test fills will be constructed for each new soil type or for each change in equipment or procedures.

C. Full Scale Liner Construction:

1. Full scale liner construction shall not be commenced until the results of the in-place compaction testing and Boutwell permeameter tests on the test liner confirm that the construction procedures and specified compaction requirements produce a in-situ hydraulic conductivities as specified in Section 2.01(B) above.
2. The liner shall be constructed according to the placement and compaction requirements for general fill, except the material shall be compacted to a density of no less than 95 percent of maximum dry density at a moisture content between 100 percent and 105 percent of optimum. The same compaction procedures, such as number of passes, speed, and compaction equipment used on construction of the test liner shall be used. Grade stakes shall not be driven into the clay liner.
3. The completed liner shall be smooth rolled to limit moisture loss and promote run-off of surface water. Moisture content shall be maintained within the specified range and erosion or other damage that occurs in the soil liner shall be repaired as directed by the Owner's Representative until the geosynthetic liner is placed.
4. Repair of any rutting or other damage caused by the installation of the geosynthetic liner will be paid for on a time and material basis.
5. Voids created in the clay barrier layer during construction (including, but not limited to, penetrations for test samples, and other penetrations necessary for construction) shall be repaired by removing material that does not meet the requirements for low permeability soil, placing low permeability soil backfill, granular or pelletized bentonite, or a mixture of bentonite and low permeability soil in lifts no thicker than 2 in. and tamping each lift with a steel rod. Each lift shall be tamped a minimum of 25 times altering the location of the rod within the void for each blow. Other ruts and depressions in the surface of the lifts shall be scarified, filled, and then compacted to grade.

3.06 CUSHION DIRT

Cushion Dirt to be placed beneath the upper High Density Polyethylene (HDPE) Geomembrane is to be placed to the specifications for General Fill in Section 3.04 above, except fill materials for Cushion Dirt shall be compacted to a dry density equal to or greater than 90 percent of the maximum dry density obtained from the Standard Proctor Test, ASTM D698.

3.07 ANCHOR TRENCH CONSTRUCTION

A. Gypsum Management Facility

1. A ledge at the bottom of the anchor trench elevation shall be excavated. Low permeability soil shall be placed and compacted on the ledge as shown on the anchor trench details in the plans.
2. The anchor trench shall be excavated to the depth and width shown on the anchor trench details. The front edge of the trench shall be rounded to eliminate any sharp corners that could cause excessive stress to the geosynthetic liners. Loose soil shall be removed or compacted into the floor of the trench.
3. Subsequent to Geosynthetic Clay Liner (GCL), Bottom HDPE Geomembrane and Geotextile Cushion installation, it shall be verified that the liners cover the entire trench floor, but do not extend up the back of the trench wall. After the liner installation in the trench has been inspected and approved by the Owner's Representative, the trench shall be backfilled with 1 ft. of low permeability soil. The backfill shall be deposited and compacted according to the requirements for general fill in such a manner as to prevent damage to the GCL and liner materials.
4. Subsequent to installation of separation geotextile on top of drainage layer, it shall be verified that the fabric extends across the top of the initial 1 ft layer of trench backfill, but does not extend up the back of the trench wall. After the fabric installation in the trench has been inspected and approved by the Owner's Representative, the trench shall be backfilled with 1 ft of low permeability soil. The backfill shall be deposited and compacted according to the requirements for general fill in such a manner as to prevent damage to the geotextile fabric.
5. Subsequent to installation of the upper HDPE Geomembrane, verify that the liner extends across the top of the initial 1 ft layer of trench backfill, but does not extend up the back of the trench wall. After the liner installation in the trench has been inspected and approved by the Owner's Representative, backfill the remainder of the trench to the top of the low permeability soil layer. Deposit and compact the backfill according to the requirements for general fill in such a manner as to prevent damage to the HDPE Geomembrane.

3.08 TESTING

- A. Construction Quality Assurance (CQA) compaction and permeability tests will be made by the Owner's Testing Consultant during the progress of the work as indicated in Appendix 2. The Contractor shall cooperate with the Testing Consultant and allow such tests to be performed.

- B. If tests indicate that an area of fill or low permeability soil liner does not meet the specified requirements, additional tests shall be performed to determine the extent of non-compliance. The Contractor shall moisture condition and recompact that area until a passing test result is obtained.

3.08 FINISH GRADING

All excavated and filled areas shall be fine graded and leveled to provide a smooth finish free of debris, foreign matter, objectionable stones, clods, lumps, pockets, or high spots, properly drained and true to indicated elevations. Finish grading shall be only near completion of work or when requested. Any portions of the berm damaged by construction shall be restored. The berm ditch shall be finished to design grade, and the ditch side slopes shaped and trimmed to provide a uniform ditch cross section.

3.09 CONSTRUCTION TOLERANCES

- A. The foundation grade and finished earthwork grades shall be no more than 0.4 ft below and not above plan grade.
- B. The minimum thickness of low permeability soil layer shall be 3 ft.

END OF SECTION 02200

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PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to the placement of riprap for erosion control.

1.02 RELATED SECTIONS

The following section contains items which are related to the work in this section:

02200 - Earthwork

1.03 REFERENCES

Specified references or cited portions thereof, current at date of bidding documents unless otherwise specified, govern the work.

- A. Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.

1.04 SUBMITTALS

Product Data: Provide quarry name and material type prior to delivery.

PART 2. PRODUCTS

2.01 MATERIALS

- A. Stone Riprap and Bedding materials according to Article 1005.01 of the Illinois Standard Specifications for Road and Bridge Construction.
- B. Filter Fabric material for Stone Riprap according to Article 1080.03, with an AOS (Apparent Opening Size) as indicated on the plans.
- C. Supplier shall be listed on the current IDOT Approved Aggregate Source List.
- D. Gradation as indicated in the drawings. Quality shall be Class A.

PART 3. EXECUTION

3.01 CONSTRUCTION REQUIREMENTS

- A. Stone Riprap and Bedding shall be installed in accordance with Section 281 of the Illinois Standard Specifications for Road and Bridge Construction for the placement of Stone Riprap. Measurement and payment provisions of Section 281 shall not apply.
- B. Filter Fabric for Stone Riprap shall be installed in accordance with Section 282 of the Illinois Standard Specifications for Road and Bridge Construction.
- C. The Owner's Representative shall be allowed to visually inspect Riprap for compliance with specifications prior to placement.

END OF SECTION 02275

PART 1. GENERAL

1.01 DESCRIPTION

A. Gypsum Management Facility

This section pertains to the following:

1. Furnishing and placing granular drainage materials for the drainage layer and leachate collection system.
2. Furnishing and placing coarse aggregate for encasement of the ring drain collection piping.
3. Furnishing and installing materials for roadbed construction related to the Gypsum Management Facility access roads and the McKinley Road relocation.
4. Recycle Pond Drain.

1.02 RELATED SECTIONS

The following sections contain items which are related to the work in this section:

1. 02300 - Earthwork
2. 02373 – Geotextiles
3. 02640 - HDPE Piping

1.03 REFERENCES

The following references, or cited portions thereof, govern the work:

1. Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.
2. American Society for Testing and Materials (ASTM):
 - a. ASTM D 75 (2003) Practice for Sampling Aggregates.
 - b. ASTM D 422 (1963; R 2002) Test Method for Particle-Size Analysis of Soils.
 - c. ASTM D 2434 (1968, R 2000) Test Method for Permeability of Granular Soils (Constant Head).
 - d. ASTM D 3042 (2003) Test Method for Insoluble Residue in Carbonate Aggregates.
 - e. ASTM C 1260 (2005) Standard Test Method for Potential Alkali Reactivity of Aggregates (Mortar-Bar Method).

3. American Geological Institute (AGI). Geoscience Handbook AGI Data Sheets, 4th Edition.

1.04 MEASUREMENT AND PAYMENT

The Contractor shall be responsible for estimating the extent of granular materials required to complete the work including, but not limited to, construction of drainage layer, encasement of leachate collection piping, and road-bed construction. The Contractor shall include the dollar amount associated with furnishing and placing all granular materials in his Lump Sum Bid amount.

1.05 COORDINATION

- A. The geosynthetic liner shall be covered with granular materials as soon as practicable after a section of liner has been approved by the Owner's Representative.

1.06 SUBMITTALS

- A. Product Data:
 1. Aggregate source list: Submit a list of proposed aggregate sources.
 2. Shipping Tickets: Submit shipping tickets for the granular materials delivered to the site. Shipping tickets shall be according to paragraph 1004.01f of the IDOT Standard Specifications.
- B. Test Reports.
 1. Submit results of grain size analysis (ASTM D422) and hydraulic conductivity testing (ASTM D2434) for gradations established by the Contractor that provide the specified hydraulic conductivity. Test results are required for each proposed source and gradation. Submit test results for each source demonstrating compliance with reactivity, soundness, and abrasion requirements specified herein.
- C. Samples:
 1. Submit one sample per source for each gradation proposed for use on the project. Samples shall be at least one pound and shall be obtained and shipped according to ASTM D75. Submit samples at least 15 days prior to starting construction of the drainage layer and coarse aggregate encasement for leachate piping.

1.07 STORAGE AND HANDLING

- A. Storage and handling of granular materials shall be according to paragraph 1004.01e of the IDOT Standard Specifications.

1.01

PART 2. PRODUCTS

2.01 MATERIALS

A. General

1. Unless otherwise approved by the Owner's Representative, granular materials shall be obtained from sources listed on the current IDOT Approved Aggregate Source List (www.dot.il.gov/materials/approvedaggregatesources.pdf).
2. Coarse Granular materials shall meet the Description of Gravel, as described in Section 1004.01(a)(1) of the IDOT Standard Specifications, and shall be spherical to sub-discoidal, sub-rounded to well rounded particles as defined by AGI Data Sheet, 4th Edition, Sheet 8.4 – Comparison Charts for Estimating Roundness and Sphericity.
3. Granular materials shall experience no more than 15 percent carbonate loss per ASTM D3042.
4. Granular materials shall be free of deleterious material, and shall meet the Na₂SO₄ soundness and Los Angeles Abrasion Specifications for Class B quality aggregate per paragraph 1004.01 of the IDOT Standard Specifications.
5. All material shall pass the 2 in. sieve, and no greater than 5 percent shall be retained on the No. 200 sieve.
6. Granular materials shall be innocuous to alkali-silica reactivity, and shall exhibit internal expansions of less than 0.10 percent at 16 days after casting as determined by ASTM C 1260.

B. Gypsum Management Facility Granular Materials

1. Granular Materials for Drainage Layer

Gradation for granular material for drainage layer shall be as required to provide a minimum hydraulic conductivity (ASTM D2434) of 1×10^{-3} cm/sec.

2. Coarse Aggregate around Ring Drain Collection Piping

Coarse Aggregate used to encase the ring drain collection piping shall be IDOT Gradation CA 7 material as outlined in Article 1004.01 of the IDOT Standard Specifications for Road and Bridge Construction.

3. Filter Sand

Filter Sand used for protective cover over the ring drain collection system shall be IDOT Gradation FA 1, Class B or better according to Article 1003 of the IDOT Standard Specifications for Road and Bridge Construction.

4. Aggregate Base Course, Type B

Aggregate Base Course, Type B used for base material for all new access roads and shall be IDOT Gradation CA 2, in accordance with Section 1004.04 of the IDOT Standard Specifications for Road and Bridge Construction. The material shall originate from an IDOT approved source. The Na_2SO_4 soundness and Los Angeles Abrasion Specifications for Class B quality aggregate per paragraph 1004.01 of the IDOT Standard Specifications shall not apply.

5. Aggregate Surface Course, Type B

Aggregate Surface Course, Type B used for surface material for all new access roads and the McKinley Road relocation shall be IDOT Gradation CA 6, in accordance with Section 1004.04 of the IDOT Standard Specifications for Road and Bridge Construction. The material shall originate from an IDOT approved source. The Na_2SO_4 soundness and Los Angeles Abrasion Specifications for Class B quality aggregate per paragraph 1004.01 of the IDOT Standard Specifications shall not apply.

2.02 EQUIPMENT

Equipment for spreading and compacting granular materials shall be low ground pressure equipment to prevent damage to the underlying geosynthetic liners.

PART 3. EXECUTION

3.01 PROTECTION OF GEOSYNTHETICS

- A. Protection of the geosynthetic liners is critically important. Approved geosynthetic liner shall be covered by granular material as soon as practicable. Granular material shall be placed to a minimum thickness of 1 ft before any heavy equipment or loaded trucks are allowed on the lined area.
- B. No equipment will be permitted directly on the geosynthetic liner.
- C. Any damage to the geosynthetic liner system shall be repaired, as directed by the Owner's Representative, at the expense of the Contractor.

3.02 GRANULAR DRAINAGE LAYER (GYPSUM MANAGEMENT FACILITY)

- A. Placement on Cell Floor
 - 1. The granular material shall be back-dumped on the geotextile cushion fabric in a sequence of operations beginning at the perimeter of the liner on the cell floor.
 - 2. Placement of material on the fabric shall be accomplished by spreading dumped material off of previously placed material with a bulldozer blade or endloader, in such a manner as to prevent tearing or shoving of the cloth. Dumping of material directly on the fabric will only be permitted to establish an initial working platform. No vehicles or construction equipment shall be allowed on the fabric prior to placement of the granular blanket to a minimum thickness of 1 ft.
- B. Placement on Cell Side Slopes
 - 1. Placement of granular material on cell side slopes shall be accomplished using methods and equipment similar to that specified for placement of material on cell floor.
 - 2. The Contractor may place gypsum underlain with separation geotextile fabric to buttress the granular material on the slope:
 - a. The Construction Quality Assurance (CQA) survey to certify thickness of drainage material shall be completed within the footprint of the gypsum stack before gypsum placement.

- b. Separation geotextile fabric shall extend beyond the toe of gypsum buttress a sufficient distance to prevent contamination of the granular drainage layer. See Sections 02373 and 02320 for construction of separation geotextile fabric and gypsum, respectively.

3.04 COARSE AGGREGATE FOR ENCASEMENT OF RING DRAIN COLLECTION PIPING (GYPSUM MANAGEMENT FACILITY)

- A. The geotextile filter fabric for encasement of leachate collection piping shall be placed on the approved cushion geotextile fabric according to Section 02373 – Geotextiles.
- B. The coarse aggregate shall be placed on the encasement fabric to the width shown on the plans to the level of the bottom of the ring drain collection piping.
- C. Course aggregate shall be placed and tamped along the pipe during pipe installation. The coarse aggregate shall be placed longitudinally along the pipe in lifts not to exceed 8 in. thick to a height of at least the center of the pipe. The aggregate shall be maintained at equal elevation on each side of the pipe, and the first lift of material shall be mechanically tamped to ensure that the space under the pipe is completely filled. The top of pipe shall not be covered until the CQA survey certifies leachate piping grade has been completed.
- C. After the CQA survey has been completed, coarse aggregate material shall continue to be placed in lifts not to exceed 8 in. thick, as specified in the previous paragraph until the minimum cover height shown in the plans is attained.
- D. The running of trucks or heavy equipment over leachate piping shall be avoided until there is at least a 12 in. cover of Filter Sand over the completed geotextile envelop. Temporary ramps no steeper than 10H:1V transverse to the piping shall be provided for temporary equipment crossings until the first lift of gypsum is placed.

3.03 ROADWAY CONSTRUCTION

- A. Prepare the roadway subgrade as shown on the plans, in accordance with Section 02200 – Earthwork.
- B. Furnish Geotechnical Fabric for Ground Stabilization in accordance with Section 02373 – Geotextiles.
- C. Furnish Aggregate Base Course, Type B in accordance with Article 351 of the IDOT Standard Specifications for Road and Bridge Construction.

- D. Furnish Aggregate Surface Course, Type B in accordance with Article 402 of the IDOT Standard Specifications for Road and Bridge Construction.

3.04 TESTING

- A. CQA gradation and permeability tests will be made by the Owner's Testing Consultant during the progress of the work as indicated in Appendix 2. The Contractor shall cooperate with the Testing Consultant and allow such tests to be performed.
- B. If tests indicate that an area of granular material or coarse aggregate does not meet the specified requirements, then the Contractor shall remove the material and replace it with suitable material.

3.05 FINISH GRADING

The granular drainage layer shall be fine graded to provide a smooth finish before a CQA survey of the completed portion of the drainage layer is requested. Ruts or erosion damage shall be repaired before placement of the separation geotextile fabric.

3.06 CONSTRUCTION TOLERANCES

The minimum thickness of drainage layer shall be 1 ft.

END OF SECTION 02315

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to furnishing and installing geotextile fabrics on prepared surfaces.

1.02 RELATED SECTIONS

The following sections contain items which are related to the work in this section:

1. 02300 - Earthwork
2. 02315 - Granular Materials
3. 02800 – HDPE Geomembrane

1.03 REFERENCES

The following references, or cited portions thereof, govern the work:

1. Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.
2. American Society for Testing and Materials (ASTM):
 - a. ASTM 3776 (1996; R 2002) Standard Test Method for Mass per Unit Area (Weight) of Fabric;
 - b. ASTM D 3786 (2001) Test Method for Hydraulic Bursting Strength of Textile Fabrics – Diaphragm Bursting Strength Tester Method;
 - c. ASTM D 4533 (2004) Test Method for Trapezoid Tearing Strength of Geotextiles;
 - d. ASTM D 4632 (1991; R 2003) Test Method for Grab Breaking Load and Elongation of Geotextiles;
 - e. ASTM D 4751 (2004) Test Method for Determining Apparent Opening Size of Geotextile;
 - f. ASTM D 4833 (2000) Test Method for Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products;
 - g. ASTM D 4873 (2002) Guide for Identification, Storage, and Handling of Geosynthetic Rolls;
 - h. ASTM D 4884 (1996; R 2003) Test Method for Strength of Sewn or Thermally Bonded Seams of Geotextiles;
 - i. ASTM D5261-92(2003) Standard Test Method for Measuring Mass per Unit Area of Geotextiles

- j. ASTM D6241-04 Standard Test Method for the Static Puncture Strength of Geotextiles and Geotextile-Related Products Using a 50-mm Probe

1.04 MEASUREMENT AND PAYMENT

- A. The Contractor shall be responsible for estimating the extent of geotextile fabric required to complete the work including fabric for laps, anchorage, repairs, and samples for Construction Quality Assurance (CQA) testing. The Contractor shall include the dollar amount associated with all geotextile construction in his Lump Sum Bid amount, except as specified in paragraph B.
- B. Geotextile fabric for ground stabilization, when directed by the Owner's Representative, will be paid for on a time and materials basis.
- C. No additional payment will be made for geotextile fabric for ground stabilization installed at the Contractor's discretion.

1.05 SUBMITTALS

- A. Product Data
 - 1. The manufacturer's list of guaranteed properties for each geotextile fabric or geogrid proposed for use on the project shall be submitted.
 - 2. The manufacturer's installation guidelines shall be submitted.
- B. Samples

Samples of geotextile fabrics shall be submitted for CQA prequalification testing. Sample size and sampling frequency are specified in Appendix 2.
- C. Inventory

A copy of the roll inventory that identifies, as a minimum, manufacturer or supplier, product or style number, roll number, width, and length of roll as identified on the roll label shall be submitted.

1.06 STORAGE AND HANDLING

Geotextiles shall be stored and handled according to ASTM D4873.

PART 2. PRODUCTS

2.01 MATERIALS

A. Geotextile Fabric for Liner System

Geotextile fabrics for use in the cell liner system shall consist of non-woven filaments of polypropylene, polyester, or polyethylene. Stabilizers and/or inhibitors shall be added to the base polymer if necessary to make the filaments resistant to deterioration caused by ultraviolet light and heat exposure. Reclaimed or recycled fibers or polymer shall not be added to the formulation. Non-woven fabric may be needle-punched, heat-bonded, or a combination thereof. The filaments shall be dimensionally stable (i.e., filaments shall maintain their relative position with respect to each other) and resistant to delamination. The edges of the geotextile shall be finished to prevent the outer fiber from pulling away from the geotextile. The filaments shall be free from any chemical treatment or coating that might significantly reduce porosity and permeability.

Fabric shall have the following physical properties:

Physical Properties⁽¹⁾	4 oz. (Separation)	6 oz. (PWRS)	16 oz. (CA Envelope)
Mass/Unit Area (oz./yd ²) ASTM D5261	4.0	6.0	16.0
Grab Tensile Strength (lb.) ASTM D4632	115	160	380
Grab Elongation (%) ASTM D4632	50	50	50
Puncture Strength (lb.) ASTM D4833	65	85	240
Puncture (CBR) Strength (lb.) ASTM D6241	310	410	1025
Mullen Burst Strength (psi) ASTM D3786	210	280	750
Trapezoidal Tear Strength (lb.) ASTM D4533	50	60	150
Width (ft.)	15	15	15
Apparent Opening Size (AOS) Max. US Std. Sieve No. ASTM D4751	70	70	100
UV Resistance ⁽²⁾ (%) ASTM D4355	70	70	70
Roll Width (ft.)	15	15	15

Notes:

- (1) All Values listed are Minimum Average Roll Values (MARV) unless otherwise noted, calculated as the typical minus two standard deviations..
- (2) UV Resistance is a minimum value and not a MARV. Evaluation to be on 2.0 inch strip tensile specimens after 500 hours exposure.

A. Cushion Geotextile Fabric.

Cushion geotextile fabric shall consist of non-woven filaments of polypropylene, polyester, or polyethylene. Stabilizers and/or inhibitors shall be added to the base polymer if necessary to make the filaments resistant to deterioration caused by ultraviolet light and heat exposure. Reclaimed or recycled fibers or polymer shall not be added to the formulation. Non-woven fabric may be needle-punched, heat-bonded, or a combination thereof. The filaments shall be dimensionally stable (i.e., filaments shall maintain their relative position with respect to each other) and resistant to delamination. The edges of the geotextile shall be finished to prevent the outer fiber from pulling away from the geotextile. The filaments shall be free from any chemical treatment or coating that might significantly reduce porosity and permeability.

Fabric shall have the following physical properties:

Physical Properties⁽¹⁾	10 oz. (Cushion)
Mass per unit area (oz/yd ²) ASTM D5261	10
Grab Tensile Strength (lb.) ASTM D4632	230
Grab Tensile Elongation (%) ASTM D4632	50
Trapezoidal Tear Strength (lb.) ASTM D4533	95
Puncture (CBR) Strength (lb.) ASTM D6241	700
Puncture (CBR) Elongation (in.) ASTM D6241	1.5
UV Resistance ⁽²⁾ (%) ASTM D4355	70
Apparent Opening Size (Max.) (AOS) Sieve No. - ASTM D4751	---
Roll Width (ft.)	15

Notes:

- (1) All Values listed are Minimum Average Roll Values (MARV) unless otherwise noted, calculated as the typical minus two standard deviations..
- (2) UV Resistance is a minimum value and not a MARV. Evaluation to be on 2.0 inch strip tensile specimens after 500 hours exposure.

B. Geotechnical Fabric for Ground Stabilization

Geotechnical fabric for ground stabilization shall conform to Article 1080.02 of the IDOT Standard Specifications for Road and Bridge Construction.

C. Thread for Seams

High strength thread should be used such that seam test should conform to ASTM D4884. The thread shall meet the chemical, ultraviolet, and physical requirements of the geotextile, and the color shall be different from that of the geotextile.

D. Securing Devices

Pins, staples, and other devices that project through the geotextile fabric are not permitted for fabrics installed above the geomembrane. Sandbags, stone, or other appropriate means approved by the Owner's Representative shall be used to prevent movement of the geotextile.

2.02 EQUIPMENT

- A. Equipment for spreading and compacting granular materials shall be low ground pressure equipment to prevent damage to the underlying geosynthetic liners.

PART 3. EXECUTION

3.01 SAMPLES FOR CQA TESTING

- A. Geotextile fabric samples shall be obtained, identified and packaged from rolls designated by the Owner's Representative according to ASTM D4873.
- B. Samples shall be 3 ft. wide by the full roll width.

3.02 BASE PREPARATION

- A. Surface on which the geotextile will be placed shall be prepared to a relatively smooth surface condition, and shall be free from obstruction, debris, depressions, erosion features, or any irregularities that would prevent continuous, intimate contact of the geotextile with the entire surface. Rills, gullies, and ruts must be graded out of the surface before geotextile placement. Areas on which geotextile are to be placed shall be graded and/or dressed in accordance with Section 02200 – Earthwork and Section 02315 – Granular Drainage Materials. Immediately prior to placing the geotextile, the prepared base will be inspected by the Owner's Representative, and no material shall be placed thereon until that area has been approved.
- B. Geotextile cushion fabric will be installed directly on the geosynthetic liner. Jointly inspect the liner with the Owner's Representative before commencing fabric installation each day. Notify the Owner's Representative promptly of any damage or defects observed in the liner as fabric installation progresses. Do not place fabric in the damaged or defective area until the liner has been repaired and

approved by the Owner's Representative. Submit a daily inspection report identifying the area of fabric placement and certifying that there were no visible defects in the area of fabric placement.

- C. Do not run heavy vehicle traffic directly on the geosynthetic liner or cushion geotextile. Use vehicles and equipment as specified in paragraph 2.02 to transport and deploy fabric on the liner. Operate the equipment with care, and place protective cover over the geomembrane, if necessary, to avoid damaging the liner. Route traffic and personnel over installed cushion fabric and use the installed fabric as a working platform to the greatest extent possible.

3.03 INSTALLATION

A. General Requirements:

- 1. Geotextile fabric shall be unrolled and laid out following these requirements to the greatest extent practical:
 - a. Orient panels with the longest dimension parallel to the slope.
 - b. Minimize the number of seams in corners and odd-shaped areas.
 - c. Extend panels on slopes a minimum of 5 ft onto a horizontal surface.

Geotextile panels shall be unrolled using methods that will not damage the fabric and will protect underlying surface from damage. While unrolling, the geotextile fabric shall be visually inspected for imperfections and faulty or suspect areas marked. Ballast shall be placed on fabric to prevent wind uplift. Expansion and contraction should be allowed for by leaving slack.

Heavy vehicle traffic shall not be run directly on geotextile fabric. Fabric in areas of heavy traffic shall be protected with protective cover over the fabric.

2. Laps

Individual panels of geotextile fabric shall be lapped according to manufacturer's instructions and as specified herein. Provide a minimum overlap of 3 in. unless otherwise specified herein or in the plans. Shingle overlaps so that water or other material cannot run down the slope between the two layers of fabric.

3. Field Seams

Continuously sew all laps on slopes steeper than 10H:1V. This requirement does not apply to the heavy geotextile fabric for envelopment of coarse aggregate around leachate piping.

4. Defects and Repairs

Examine the installed geotextile fabric for defects, holes discontinuous seams, puckered or separated laps, etc. Repair defective laps and seams. Patch holes and defects according to manufacturer's recommendations and as directed by the Owner's Representative. Do not cover suspect or patched areas until they have been inspected and approved by the Owner's Representative.

B. Geotextile Fabric for Separation

1. Use low ground pressure equipment to avoid rutting the granular material.
2. Horizontal seams (parallel to top of slope) will be permitted on cell side slopes to facilitate staged construction of the drainage layer on the side slope.
3. Extend separation geotextile fabric into and across the bottom of the anchor trench and complete backfill of the trench according to Section 02200.

C. Geotextile Fabric for Coarse Aggregate Envelope

1. Geotextile for coarse aggregate envelope will be installed directly on the cushion fabric. Remove any foreign materials from the cushion fabric within the footprint of the coarse aggregate leachate piping encasement before installing the geotextile envelope. Place sufficient width to completely envelop the coarse aggregate and provide a longitudinal lap of at least 6 in.
2. After the coarse aggregate encasement has been completed, according to Section 02315, wrap the geotextile around the mounded aggregate, and cover the lap with at least 6 in. of material before permitting vehicle or equipment on the fabric.
3. Any ballast material other than coarse aggregate, according to Section 02315, that is placed within the envelope will require removal during coarse aggregate construction.

D. Geotechnical Fabric for Ground Stabilization

1. Install Geotechnical Fabric for Ground Stabilization in accordance with Section 210 of the IDOT Standard Specifications for Road and Bridge Construction.

2. If approved by the Owner's Representative, the Contractor may, at his own expense, install geotextile or geogrid for ground stabilization outside the limits designated by the Owner's Representative.
3. Submit as-built drawings that clearly delineate limits and type of ground stabilization.

3.04 PROTECTION

- A. Protect installed fabric until it is covered by at least 1 ft. of overlying material.
- B. Any damage to the geotextile during its installation or during placement of overlying materials shall be replaced by the Contractor at no cost to the Owner. Unless otherwise noted, the work shall be scheduled so that the covering of the geotextile with a layer of the specified material is accomplished within 14 calendar days after placement of the geotextile. Failure to comply shall require replacement of geotextile. The geotextile shall be protected from damage prior to and during the placement of overlying materials. Before placement of overlying materials, the Contractor shall demonstrate that the placement technique will not cause damage to the geotextile.

3.05 TESTING AND INSPECTION

- A. Prequalification Testing

Geotextiles are subject to CQA testing by the Owner's Testing Consultant to verify conformance with the manufacturer's list of guaranteed properties according to Appendix 2. The Contractor shall provide samples as specified herein. If tests indicate nonconformance to the list of guaranteed properties, provide additional samples as directed by the Owner's Representative to determine the extent of the non-conformance. Any fabric that does not conform to the list of guaranteed properties shall be removed from the site.

- B. Installed fabric shall be inspected by the Owner's Representative. No material shall be placed on the fabric, other than ballast, until the installation has been approved by the Owner's Representative. Ballast shall not obscure seams or significant length of unseamed laps. The Owner's Representative may require removal of ballast to inspect suspect areas.
- C. If the Owner's Representative suspects that completed work has been damaged by construction methods that do not conform to the specifications, he may require removal of completed work to verify the integrity of the underlying materials. The Contractor shall bear the cost of removal and subsequent repair as directed by the Owner's Representative.

END OF SECTION 02373

PART 1. GENERAL

1.01 DESCRIPTION

- A. This section covers furnishing and installation of a reinforced needlepunched Geosynthetic Clay Liner (GCL) at the Gypsum Management Facility and the CCB Management Facility.
- B. The work includes furnishing all equipment and materials, providing all labor, supervision, administration and management necessary to perform the work as specified herein and as shown on the plans.

1.02 RELATED SECTIONS

None.

1.03 REFERENCES

The following references, or cited portions thereof, govern the work

- 1. American Society for Testing and Materials (ASTM):
 - a. ASTM D 4632 (1991; R 2003), Standard Test Method for Grab Breaking Load and Elongation of Geotextiles;
 - b. ASTM D 4643 (2000), Test Method for Determination of Water (Moisture) Content of Soil by the Microwave Oven Method;
 - c. ASTM D 5084 (2003), Test Method for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter;
 - d. ASTM D 5261 (1992; R 2003), Test Method for Measuring Mass Per Unit Area of Geotextiles;
 - e. ASTM D 5321 (2002), Test Method for Determining the Coefficient of Soil and Geosynthetic or Geosynthetic and Geosynthetic Friction by the Direct Shear Method;
 - f. ASTM D 5887 (2004), Test Method for Measurement of Index Flux Through Saturated Geosynthetic Clay Liner Specimens Using a Flexible Wall Permeameter;
 - g. ASTM D 5888 (1995; R 2002), Practice for Storage and Handling of Geosynthetic Clay Liners;
 - h. ASTM D 5889 (1997; R 2003), Practice for Quality Control of Geosynthetic Clay Liners;

- i. ASTM D 5890 (2002), Test Method for Swell Index of Clay Mineral Component of Geosynthetic Clay Liners;
- j. ASTM D 5891 (2002), Test Method for Fluid Loss of Clay Component of Geosynthetic Clay Liners.

1.04 SUBMITTALS

- A. With the bid, the Contractor shall furnish the following information:
 1. Conceptual description of the proposed plan for placement of the GCL panels over the areas of installation.
 2. GCL Manufacturer's Quality Control (MQC) Plan for documenting compliance with Sections 2.01 and 2.02 of these specifications.
 3. GCL manufacturer's historical data for reinforced GCL of a) 10,000-hour creep shear testing per Section 2.01 D, and b) seam flow data at 2 psi confining pressure per Section 2.01 E.
 4. A copy of GCL manufacturer's International Standards Organization (ISO) Quality Certificate of Registration.
 5. Statement of experience from the proposed GCL supplier.
 6. Statement of experience from the proposed GCL installer.

- B. At the Owner Representative's or Owner's request, the Contractor shall furnish:
 1. A representative sample of the GCLs.
 2. A project reference list for the GCL(s) consisting of the principal details of at least ten projects totaling at least 10 million sq. ft (100,000 sq. meters) in size.

- C. Upon shipment, the Contractor shall furnish:
 1. The GCL manufacturer's Quality Assurance/Quality Control (QA/QC) certifications to verify that the materials supplied for the project are in accordance with the requirements of this specification.
 2. Inventory of materials received.

- D. As installation proceeds, the Contractor shall submit certificates of subgrade acceptance, signed by the Contractor and Construction Quality Assurance (CQA) Inspector (see Sections 1.06 and 3.03) for each area that is covered by the GCL.

- E. Warranty

After construction, the contractor shall submit material and installation warranty certificates.

1.05 QUALIFICATIONS

- A. GCL Manufacturer must have produced at least 10 million sq. ft. (1 million sq. meters) of GCL, with at least 8 million sq. ft. (800,000 sq. meters) installed.
- B. The GCL Installer must either have installed at least 1 million sq. ft. (100,000 sq. meters) of GCL, **or** must provide to the Engineer satisfactory evidence, through similar experience in the installation of other types of geosynthetics, that the GCL will be installed in a competent, professional manner.

1.06 CONSTRUCTION QUALITY ASSURANCE (CQA)

- A. The Owner shall provide a third-party inspector for CQA of the GCL installation. The inspector shall be an individual or company who is independent from the manufacturer and installer and who shall be responsible for monitoring and documenting activities, related to the CQA of the GCL throughout installation.
- B. Testing of the GCL as necessary to support the CQA effort shall be performed by a third party laboratory retained by the Owner and independent from the GCL manufacturer and installer.

WARRANTY

The geomembrane material shall be warranted, on a pro-rata basis against manufacturer's defects for a period of five (5) years from the date of liner installation. The installation shall be warranted against defects in workmanship for a period of (1) year from the date of liner completion.

PART 2. PRODUCTS

2.01 MATERIALS

- A. Acceptable products for the GCL are GCL Bentomat[®] SDN, as manufactured by CETCO, 1350 West Shure Drive, Arlington Heights, Illinois 60004 USA (847-392-5800), or an engineer-approved reinforced needlepunched GCL material equal to Bentomat SDN.
- B. The delineation of areas to receive GCL shall be agreed by the Installer and the Engineer prior to installation.
- C. The GCL and its components shall have the properties shown in the GCL Certified Properties table at the end of this section.
- D. The reinforced GCL shall have 10,000 hour test data for large-scale constant-load (creep) shear testing for related products under hydrated conditions. The

displacement shall be 0.13 in. (3.3 mm) or less at a constant shear load of 250 psf (12 kPa) and a normal load of 500 psf (24 kPa).

- E. The reinforced GCL shall have seam test data from an independent laboratory showing that the seam flow with a grooved cut in one of the nonwoven geotextiles is less than $1 \times 10^{-8} \text{ m}^3/\text{m}^2/\text{s}$ at 2 psi hydraulic pressure.
- F. The minimum acceptable dimensions of full-size GCL panels shall be 150 ft. (45.7 m) in length. Short rolls [(those manufactured to a length greater than 70 ft. (21 meters) but less than a full-length roll)] may be supplied at a rate no greater than three (3) per truckload or three (3) rolls every 36,000 sq. ft. (3,500 sq. meters) of GCL, whichever is less.
- G. A 6-inch (150 mm) overlap guideline shall be imprinted on both edges of the upper geotextile component of the GCL as a means for providing quality assurance of the overlap dimension. Lines shall be printed in easily visible, non-toxic ink.

2.02 PRODUCT QUALITY DOCUMENTATION

The GCL manufacturer shall provide the Contractor or other designated party with manufacturing QA/QC certifications for each shipment of GCL. The certifications shall be signed by a responsible party employed by the GCL manufacturer and shall include:

- A. Certificates of analysis for the bentonite clay used in GCL production demonstrating compliance with the swell index and fluid loss parameters shown in the GCL Certified Properties tables.
- B. Manufacturer's test data for finished GCL product(s) of bentonite mass/area, GCL tensile strength and GCL peel strength (reinforced only) demonstrating compliance with the index parameters shown in the GCL Certified Properties tables.
- C. GCL lot and roll numbers supplied for the project (with corresponding shipping information).

2.03 PRODUCT LABELING

- A. Prior to shipment, the GCL manufacturer shall label each roll, identifying:
 - 1. Product identification information (Manufacturer's name and address, brand product code).
 - 2. Lot number and roll number.
 - 3. Roll length, width and weight.

2.04 PACKAGING

- A. The GCL shall be wound around a rigid core whose diameter is sufficient to facilitate handling. The core is not necessarily intended to support the roll for lifting but should be sufficiently strong to prevent collapse during transit.
- B. All rolls shall be labeled and bagged in packaging that is resistant to photodegradation by ultraviolet (UV) light.

2.05 ACCESSORY BENTONITE

- A. The granular bentonite sealing clay used for overlap seaming, penetration sealing and repairs shall be made from the same natural sodium bentonite as used in the GCL and shall be as recommended by the GCL manufacturer. Seaming of GCLs shall be conducted in accordance with the manufacturer's guidelines for each particular GCL. Please refer to the installation guidelines for Bentomat /Claymax GCLs.

PART 3. EXECUTION

3.01 SHIPPING AND HANDLING

- A. The rolls of GCL shall be packaged and shipped by appropriate means to prevent damage to the material and to facilitate off-loading.
- B. The Installation Supervisor shall be present during delivery and unloading of the GCL. A visual inspection of each roll should be made during unloading to identify if any packaging has been damaged. Rolls with damaged packaging should be marked and set aside for further inspection. The packaging should be repaired prior to being placed in storage. The Installation Supervisor shall prepare and submit an inventory that includes lot and roll number for materials received.
- C. The Installer is responsible for unloading the GCL. The Owner will make available equipment and operators employed at the site to assist with unloading. The Installer shall coordinate with the Owner to determine equipment availability and should contact the Manufacturer prior to shipment to ascertain the appropriateness of the proposed unloading methods and equipment.

3.02 STORAGE

- A. Storage of the GCL rolls shall be the responsibility of the Installer. A dedicated storage area shall be provided by the Owner at the job site. Submit storage area requirements (size and preferred location) with bid documents.

- B. Rolls should be stored in a manner that prevents sliding or rolling from the stacks and may be accomplished by the use of chock blocks. Rolls should be stacked at a height no higher than that at which the lifting apparatus can be safely handled (typically no higher than four).
- C. All stored GCL materials and the accessory bentonite must be covered with a plastic sheet or tarpaulin until their installation.
- D. The integrity and legibility of the labels shall be preserved during storage.

3.03 EARTHWORK

- A. The low permeability soil layer upon which the GCL is installed shall be prepared and compacted prior to installation. The surface shall be smooth, firm, and unyielding, and free of:
 - 1. Vegetation.
 - 2. Construction debris.
 - 3. Sticks.
 - 4. Sharp rocks.
 - 5. Void spaces.
 - 6. Ice.
 - 7. Abrupt elevation changes.
 - 8. Standing water.
 - 9. Cracks larger than 0.25 in. (6 mm) in width.
 - 10. Any other foreign matter that could contact the GCL.
- B. Immediately prior to GCL deployment, the low permeability soil layer shall be final-graded by the contractor to fill in all voids or cracks and then smooth-rolled to provide the best practicable surface for the GCL. At completion of this activity, no wheel ruts, footprints or other irregularities shall exist in the subgrade. Furthermore, all protrusions extending more than 0.5 in. (12 mm) from the surface shall either be removed, crushed or pushed into the surface with a smooth-drum compactor.
- C. On a continuing basis, the project CQA inspector shall certify acceptance of the subgrade before GCL placement.
- D. It shall be the Installer's responsibility thereafter to indicate to the Owner's Representative any change in the condition of the low permeability soil layer that could cause the subgrade to be out of compliance with any of the requirements listed in this Section. The Installation Supervisor shall certify in the daily report that no GCL was placed over visibly defective low permeability soil surface.
- E. At the top of sloped areas of the job site, an anchor trench for the GCL shall be excavated by the contractor in accordance with the project plans. The trench shall

be excavated and approved by the CQA Inspector prior to GCL placement. No loose soil shall be allowed at the bottom of the trench and no sharp corners or protrusions shall exist anywhere within the trench.

3.04 GCL PLACEMENT

- A. GCL rolls shall be delivered to the working area of the site in their original packaging. Immediately prior to deployment, the packaging shall be carefully removed without damaging the GCL. The orientation of the GCL (i.e., which side faces up) shall be in accordance with the Owner Representative's recommendations.
- B. Equipment which could damage the GCL, shall not be allowed to travel directly on it. If the installation equipment causes rutting of the subgrade, the subgrade must be restored to its originally accepted condition before placement continues.
- C. Care must be taken to minimize the extent to which the GCL is dragged across the subgrade in order to avoid damage to the bottom surface of the GCL. A temporary geosynthetic subgrade covering commonly known as a slip sheet or rub sheet may be used to reduce friction damage during placement.
- D. The GCL panels shall be placed parallel to the direction of the slope.
- E. All GCL panels shall lie flat on the underlying surface, with no wrinkles or folds, especially at the exposed edges of the panels.
- F. Only as much GCL shall be deployed as can be covered at the end of the working day with soil, a geomembrane, or a temporary waterproof tarpaulin. The GCL shall not be left uncovered overnight. If the GCL is hydrated when no confining stress is present, the Installer shall remove and replace the hydrated material as directed by the Owner Representative.

3.05 ANCHORAGE

- A. As directed by the project drawings and specifications, the end of the GCL roll shall be placed in an anchor trench at the top of the slope. The front edge of the trench shall be rounded so as to eliminate any sharp corners. Loose soil shall be removed from the floor of the trench. The GCL shall cover the entire trench floor, but shall not extend up the rear trench wall.

3.06 SEAMING

- A. The GCL seams shall be constructed by overlapping their adjacent edges according to the manufacturer's recommendations. Care should be taken to ensure that the overlap zone is not contaminated with loose soil or other debris.

- B. The minimum dimension of the longitudinal overlap should be 6 in. (150 mm) for Bentomat SDN. If the GCL is manufactured with a grooved cut in the nonwoven geotextile that allows bentonite to freely extrude into the longitudinal overlap then no bentonite-enhanced seam is required for this overlap. If the GCL does not have a grooved cut in one of the nonwoven geotextiles in the longitudinal overlap, then bentonite-enhanced seams are required as described below.
- C. End-of-roll overlapped seams shall be constructed with a minimum overlap of 24 in. (600 mm) for Bentomat SDN. Seams at the ends of the panels should be constructed such that they are shingled in the direction of the grade to prevent the potential for runoff flow to enter the overlap zone. End-of-roll overlapped seams for all reinforced GCL seams require bentonite-enhanced seams as described below.
- D. Bentonite-enhanced seams shall be constructed between the overlapping adjacent panels as follows. The underlying edge of the longitudinal overlap is exposed and then a continuous bead of granular sodium bentonite is applied along a zone defined by the edge of the underlying panel and the 6-inch (150 mm) line. The granular bentonite shall be applied at a minimum application rate of one quarter pound per lineal ft. (0.4 kg/m). A similar bead of granular sodium bentonite is applied at the end-of-roll overlap.

3.07 DETAIL WORK

- A. There shall be no penetrations through the GCL.
- B. Cutting the GCL should be performed using a sharp utility knife. Frequent blade changes are recommended to avoid damage to the geotextile components of the GCL during the cutting process.

3.08 DAMAGE REPAIR

- A. If the GCL is damaged (torn, punctured, perforated, etc.) during installation, it may be possible, if approved by the Owner's Representative, to repair it by cutting a patch to fit over the damaged area. The patch shall be obtained from a new GCL roll and shall be cut to size such that a minimum overlap of 12 in. (300 mm) is achieved around all of the damaged area. Granular bentonite or bentonite mastic shall be applied around the damaged area prior to placement of the patch. It may be desirable to use an adhesive to affix the patch in place so that it is not displaced during cover placement. Patching shall be observed and approved by the Owner's Representative.

GCL CERTIFIED PROPERTIES

MATERIAL PROPERTY	TEST METHOD	TEST FREQUENCY ft ² (m ²)	REQUIRED VALUES
Bentonite Swell Index ¹	ASTM D 5890	1 per 50 tonnes	24 mL/2g min.
Bentonite Fluid Loss ¹	ASTM D 5891	1 per 50 tonnes	18 mL max.
Bentonite Mass/Area ²	ASTM D 5993	40,000 ft ² (4,000 m ²)	0.75 lb/ft ² (3.6 kg/m ²) min
GCL Grab Strength ³	ASTM D 6768	200,000 ft ² (20,000 m ²)	30 lbs/in (53 N/cm) MARV
GCL Peel Strength ³	ASTM D 6496	40,000 ft ² (4,000 m ²)	2.5 lbs/in (4.4 N/cm) min
GCL Index Flux ⁴	ASTM D 5887	Weekly	1 x 10 ⁻⁸ m ³ /m ² /sec max
GCL Hydraulic Conductivity ⁴	ASTM D 5887	Weekly	5 x 10 ⁻⁹ cm/sec max
GCL Hydrated Internal Shear Strength ⁵	ASTM D 5321 ASTM D 6243	Periodic	500 psf (24 kPa) typ @ 200 psf

Notes

¹ Bentonite property tests performed at a bentonite processing facility before shipment the manufacturer's production facilities.

² Bentonite mass/area reported at 0 percent moisture content.

³ All tensile strength testing is performed in the machine direction using ASTM D 6768. All peel strength testing is performed using ASTM D 6496. Upon request, tensile and peel results can be reported per modified ASTM D 4632 using 4 inch grips.

⁴ Index flux and permeability testing with deaired distilled/deionized water at 80 psi (551kPa) cell pressure, 77 psi (531 kPa) headwater pressure and 75 psi (517 kPa) tailwater pressure. Reported value is equivalent to 925 gal/acre/day. This flux value is equivalent to a permeability of 5x10⁻⁹ cm/sec for typical GCL thickness. Actual flux values vary with field condition pressures. The last 20 weekly values prior the end of the production date of the supplied GCL may be provided.

⁵ Peak values measured at 200 psf (10 kPa) normal stress for a specimen hydrated for 48 hours. Site-specific materials, GCL products, and test conditions must be used to verify internal and interface strength of the proposed design.

END OF SECTION 02376

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DIVISION 2 - SITE WORK
Section 02640 – HDPE Piping

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to construction of the HDPE (High Density Polyethylene) Piping at the Gypsum Management Facility and the CCB Management Facility.

1.02 RELATED SECTIONS

None.

1.03 REFERENCES

The following references, or cited portions thereof, govern the work

A. American Society of Testing and Materials:

1. ASTM D 2683 (2004); Specification for Socket-Type Polyethylene Fittings for Outside Diameter-Controlled Polyethylene Pipe and Tubing.
2. ASTM D 3261 (2003); Specification for Butt Heat Fusion Polyethylene (PE) Plastic Fittings for Polyethylene (PE) Plastic Pipe and Tubing.
3. ASTM D 3350 (2005); Specification for Polyethylene Plastics Pipe and Fittings Materials.
4. ASTM F 412 (2001a); Terminology Relating to Plastic Piping System.
5. ASTM F 1055 (1998); Specification for Electrofusion Type Polyethylene Fittings for Outside Diameter Controlled Polyethylene Pipe and Tubing.
6. ASTM F 1056 (2004); Specification for Socket Fusion Tools for Use in Socket Fusion Joining Polyethylene Pipe or Tubing and Fittings.

1.04 SUBMITTALS

A. Qualifications

Submit qualifications of the Welding Supervisor who will be responsible for construction quality control of the pipe joining process.

B. Material Certifications

Submit manufacturer certifications that the pipe provided complies with the requirements herein.

C. Product Data

1. Submit product data and operating instructions for pipe joining equipment.
2. Submit pipe manufacturer's recommended procedures for storing, handling, and installing pipe and fittings.

1.05 QUALIFICATIONS

- A. The Contractor or Subcontractor performing the work under this section shall have in his employ a Welding Supervisor who has completed a minimum of 1,000 ft of pipe joining work using the type of equipment proposed for use in this work. The Welding Supervisor shall be on site at all times during pipe line installation, and shall provide direct supervision over other employees.

1.06 WARRANTY

- A. The pipe and fittings shall be warranted, on a pro-rata basis, against manufacturer's defects for a period of five (5) years from the date of pipe installation. The installation shall be warranted against defects in workmanship for a period of one (1) year from the date of completion of the leachate collection piping system.

PART 2. PRODUCTS

2.01 MATERIALS

A. Pipe

1. Pipe material shall be High Density Polyethylene (HDPE) PE 3408, according to ASTM F412, with a cell class designation of 345464C, according to ASTM D3350. Iron pipe size (IPS) and standard dimension ratio (SDR) shall be as indicated in the plans.
2. Size and spacing of holes in perforated pipe shall be as indicated in the plans.

B. Fittings

1. Fittings shall be made of the same material, and shall have a pressure rating no less than 160 psi. Butt fusion, socket, or electrofusion fittings, according to ASTM D3261, ASTM D2683, and ASTM F1055, respectively, are acceptable.

2.02 EQUIPMENT

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A. Butt Fusion Machine

The butt fusion machine shall include the following features:

1. Facer with rotating planer block design.
2. Heater faces coated by the manufacturer to prevent molten plastic from adhering to the heater face.
3. Hydraulic-operated jaws suitable for use with the pipe sizes indicated in the plans.

B. Socket Fusion Equipment

Socket fusion heating tools and depth gauges shall be of the same manufacturer, unless they are all marked F1056, indicating compliance with ASTM F1056.

- C. All equipment shall conform to any requirements specified in the pipe and socket manufacturer's installation instructions, and shall be approved by the Owner's Representative.

PART 3. EXECUTION

3.01 MATERIAL DELIVERY, STORAGE, AND HANDLING

- A. HDPE pipe and fittings shall be packaged and shipped by appropriate means to prevent damage to the material and to facilitate off-loading. The Owner will provide an on-site storage site. Storage site requirements (size and preferred location) shall be submitted with the bid documents.
- B. Storage and handling shall be according to manufacturer's recommendations.

3.02 BASE PREPARATION

All HDPE piping shall be installed on a layer of coarse aggregate placed by the Contractor in accordance with the plans. The grade of the coarse aggregate base shall be verified before installing the piping.

3.03 INSTALLATION

All pipe and fittings shall be installed according to the manufacturer's recommendations. Removal of weld beads is not required. Contractor shall place coarse aggregate along the pipe to provide lateral stability. Welds shall not be obscured until they have been approved by the Owner's Representative, the top of pipe shall not be covered until the Construction Quality Assurance (CQA) survey has been completed to verify conformance with specified tolerances.

3.04 INSPECTIONS

- A. The Owner's Representative shall be visually inspect pipe materials to verify that each pipe material is properly stamped (by the manufacturer) for ASTM acceptance before installation. Defective or damaged materials shall be removed from the site.
- B. Each weld and connection shall be visually inspected by the Owner's Representative. Defective welds shall be repaired as directed by the Owner's Representative and according to manufacturer's recommendations. Welds and connections shall not be covered until they have been approved by the Owner's Representative.

3.05 TOLERANCES

- A. HDPE piping shall be located within 0.5 ft. of plan location, and elevation shall be within 0.1 ft. of plan elevation with no adverse slopes.

END OF SECTION 02936

PART 1. GENERAL

1.01 DESCRIPTION

- A. This section includes manufacturing, furnishing, and installing High Density Polyethylene (HDPE) Geomembranes for the Gypsum Management Facility and the Gypsum Management Facility Recycle Pond.
- B. The work includes furnishing all equipment and materials and providing all labor, supervision, administration and management necessary to perform the work as shown on the plans.

1.02 RELATED SECTIONS

- A. The following sections contain items which are related to the work in this section:
 - 1. 02373 – Geotextiles
 - 2. 02376 – Geosynthetic Clay Liner

1.03 REFERENCES

- A. The following references, or cited portions thereof, govern the work:
 - 1. American Society for Testing and Materials (ASTM):
 - a. D 638, Standard Test Method for Tensile Properties of Plastics.
 - b. D 751, Standard Test Methods for Coated Fabrics.
 - c. D 792, Standard Test Methods for Density and Specific Gravity (Relative Density) of Plastics by Displacement.
 - d. D 1004, Standard Test Method for Initial Tear Resistance of Plastic Film and Sheeting.
 - e. D 1204, Standard Test Method for Linear Dimensional Changes of Non Rigid Thermoplastic Sheeting or Film at Elevated Temperature.
 - f. D 1238, Standard Test Method for Flow Rates of Thermoplastics by Extrusion Plastometer.
 - g. D 1505, Standard Test Method for Density of Plastics by Density-Gradient Technique.
 - h. D 1603, Standard Test Method for Carbon Black in Olefin Plastics.
 - i. D 3895, Test Method for Oxidative Induction Time of Polyolefins by Thermal Analysis.
 - j. D 4218, Test Method for Determination of Carbon Black Content in Polyethylene Compounds by the Muffle-Furnace Technique.

- k. D 4437, Standard Practice for Determining the Integrity of Field Seams Used in Joining Flexible Polymeric Sheet Geomembranes.
- l. D 4833, Test Method for Index Puncture Resistance of Geotextiles, Geomembranes and Related Products.
- m. D 5199, Standard Test Method for Measuring Nominal Thickness of Smooth Geomembranes.
- n. D 5397, Standard Test Method for Evaluation of Stress Crack Resistance of Polyolefins using Notched Constant Tensile Load Test.
- o. D 5596, Standard Practice for Microscopical Examination of Pigment Dispersion in Plastic Compounds.
- p. D 5641, Standard Practice for Geomembrane Seam Evaluation by Vacuum Chamber.
- q. D 5721, Practice for Air-Oven Aging of Polyolefin Geomembranes.
- r. D 5820, Test Method for Air Testing.
- s. D 5885, Test Method for Oxidative Induction Time of Polyolefin Geosynthetics by High Pressure Differential Scanning Calorimetry.
- t. D 5994, Standard Test Method for Measuring Nominal Thickness of Textured Geomembranes
- u. D 6365, Standard Practice for the Nondestructive Testing of Geomembrane Seams using The Spark Test

2. Geosynthetic Research Institute (GRI):

- a. GRI GM 6, Pressurized Air Channel Test for Dual Seamed Geomembranes
- b. GRI GM 9, Cold Weather Seaming of Geomembranes
- c. GRI GM 10, Specification for Stress Crack Resistance of HDPE Geomembrane Sheet
- d. GRI GM 13, Test Properties, Testing Frequency and Recommended Warranty for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes
- e. GRI GM 14, Test Frequencies for Destructive Seam Testing

1.04 SUBMITTALS

- A. Submit the following to the Engineer or Owner, for review and approval, within a reasonable time so as to expedite shipment or installation of the Geomembrane:
 - 1. Documentation of manufacturer's qualifications as specified in subsection 1.05A of this Section.
 - 2. Manufacturer's Quality Control program manual or descriptive documentation.
 - 3. A material properties sheet, including at a minimum all properties specified in GRI GM 13, including test methods used.
 - 4. Sample of the material.

5. Documentation of Installer's qualifications, as specified below and in subsection 1.05B of this Section.
 - a. Submit a list of at least ten completed facilities. For each name and type of facility; its location; the date of installation; number of contact at the facility; type and thickness of geomembrane and; surface area of the installed geomembrane.
 - b. Submit resumes or qualifications of the Installation Supervisor, Master Seamer and Technicians to be assigned to this project.
 - c. Quality Control Program.
6. Example Material Warranty and Liner Installation Warranty complying with subsections 1.07 and 1.08 of this Section.
7. Resin Supplier's name, resin production plant identification, resin brand name and number, production date of the resin, resin Manufacturer's quality control certificates, and certification that the properties of the resin meet the requirements

B. Shop Drawings

1. Submit copies of shop drawings for engineer's approval within a reasonable time so as not to delay the start of geomembrane installation. Shop drawings shall show the proposed panel layout identifying seams and details. Seams should generally follow direction of the slope. Butt seams or roll-end seams should not occur on a slope unless approved by the Owner's Representative. Butt seams on a slope, if allowed, should be staggered.
2. Placement of geomembrane will not be allowed to proceed until Owner's Representative has received and approved the shop drawings.

C. Additional Submittals (In-Progress and at Completion)

1. Manufacturer's warranty (refer to subsection 1.08).
2. Geomembrane installation warranty (refer to subsection 1.09).
3. Daily written acceptance of subgrade surface (refer to subsection 3.01.C).
4. Low-temperature seaming procedures if applicable (refer to subsection 3.03.A)
5. Prequalification test seam samples (refer to subsection 3.05.A.6).
6. Field seam non-destructive test results (refer to subsection 3.05.B.1).
7. Field seam destructive test results (refer to subsection 3.05.C.6).
8. Daily field installation reports (refer to subsection 3.05.G).
9. Installation record drawing, as discussed in subsection 3.05.G).

1.05 QUALITY CONTROL

A. Manufacturer's Qualifications:

The manufacturer of geomembrane of the type specified or similar product shall have at least five years experience in the manufacture of such geomembrane. In addition, the geomembrane manufacturer shall have manufactured at least

10,000,000 sq. ft. of the specified type of geomembrane or similar product during the last five years.

B. Installer's Qualifications:

- 1 The Geomembrane Installer shall be the Manufacturer, approved Manufacturer's Installer or a contractor approved by the Owner's Representative to install the geomembrane.
- 2 The Geomembrane Installer shall have at least three years experience in the installation of the specified geomembrane or similar. The Geomembrane Installer shall have installed at least 10 projects involving a total of 5,000,000 sq. ft. of the specified type of geomembrane or similar during the last three years.
- 3 Installation shall be performed under the direction of a field Installation Supervisor who shall be responsible throughout the geomembrane installation, for geomembrane panel layout, seaming, patching, testing, repairs, and all other activities of the Geomembrane Installer. The Field Installation Supervisor shall have installed or supervised the installation and seaming of a minimum of 10 projects involving a total of 5,000,000 sq. ft. of geomembrane of the type specified or similar product.
- 4 Seaming shall be performed under the direction of a Master Seamer (who may also be the Field Installation Supervisor or Crew Foreman) who has seamed a minimum of 3,000,000 sq. ft. of geomembrane of the type specified or similar product, using the same type of seaming apparatus to be used in the current project. The Field Installation Supervisor and/or Master Seamer shall be present whenever seaming, patching, other welding operations, and testing is performed.
- 5 All seaming, patching, other welding operations, and testing shall be performed by qualified technicians employed by the Geomembrane Installer.

1.06 DELIVERY, STORAGE AND HANDLING

- A. Each roll of geomembrane delivered to the site shall be labeled by the manufacturer. The label shall be firmly affixed and shall clearly state the manufacturer's name, product identification, material thickness, roll number, roll dimensions and roll weight.
- B. Geomembrane shall be protected from mud, dirt, dust, puncture, cutting or any other damaging or deleterious conditions.
- C. Rolls shall be stored away from high traffic areas. Continuously and uniformly support rolls on a smooth, level prepared surface.
- D. Rolls shall not be stacked more than three high.

1.07 PROJECT CONDITIONS

Geomembrane shall not be installed in the presence of standing water, while precipitation is occurring, during excessive winds, or when material temperatures are outside the limits specified in Section 3.03.

1.08 MATERIAL WARRANTY

As required by specification, or as required in GRI GM 13 (attachment A)

1.09 GEOMEMBRANE INSTALLATION WARRANTY

The Geomembrane Installer shall guarantee the geomembrane installation against defects in the installation and workmanship for 1 year commencing with the date of final acceptance.

1.10 GEOMEMBRANE PRE-CONSTRUCTION MEETING

- A. Geomembrane Pre-Construction Meeting shall be held at the site prior to installation of the geomembrane. At a minimum, the meeting shall be attended by the Geomembrane Installer, Owner, Owner's representative (Engineer and/or CQA Firm), and the General Contractor.
- B. Topics for this meeting shall include:
 - 1. Responsibilities of each party.
 - 2. Lines of authority and communication. Resolution of any project document ambiguity.
 - 3. Methods for documenting, reporting and distributing documents and reports.
 - 4. Procedures for packaging and storing archive samples.
 - 5. Review of time schedule for all installation and testing.
 - 6. Review of panel layout and numbering systems for panels and seams including details for marking on geomembrane.
 - 7. Procedures and responsibilities for preparation and submission of as-built panel and seam drawings.
 - 8. Temperature and weather limitations. Installation procedures for adverse weather conditions. Defining acceptable subgrade, geomembrane, or ambient moisture and temperature conditions for working during liner installation.
 - 9. Subgrade conditions, dewatering responsibilities and subgrade maintenance plan.
 - 10. Deployment techniques including allowable subgrade for the geomembrane.
 - 11. Plan for controlling expansion/contraction and wrinkling of the geomembrane.
 - 12. Covering of the geomembrane and cover soil placement.
 - 13. Measurement and payment schedules.
 - 14. Health and safety.
- C. The meeting shall be documented by the Owner's Representative and minutes shall be transmitted to all parties.

PART 2. PRODUCTS

2.01 SOURCE QUALITY CONTROL

Manufacturing Quality Control

- A. The test methods and frequencies used by the manufacturer for quality control/quality assurance of the above geomembrane prior to delivery, shall be in accordance with GRI GM 13, or modified as required for project specific conditions.
- B. The manufacturer's geomembrane quality control certifications, including results of quality control testing of the products, as specified in subsection 2.01.C of this Section, must be supplied to the Owner's Representative. The certification shall be signed by a responsible party employed by the manufacturer, such as the QA/QC Manager, Production Manager, or Technical Services Manager. Certifications shall include lot and roll numbers and corresponding shipping information.
- C. The Manufacturer will provide Certification that the geomembrane and welding rod supplied for the project have the same base resin and material properties.

2.02 GEOMEMBRANE

- A. The geomembrane shall consist of new, first quality products designed and manufactured specifically for the purpose of this work which shall have been satisfactorily demonstrated by prior testing to be suitable and durable for such purposes. The geomembrane rolls shall be seamless, high density polyethylene (HDPE- Density >0.94g/cm) containing no plasticizers, fillers or extenders and shall be free of holes, blisters or contaminants, and leak free verified by 100% in line spark or equivalent testing. The geomembrane shall be supplied as a continuous sheet with no factory seams in rolls. The geomembrane will meet the property requirements as shown in Table A. (GRI GM 13)
- B. Material shall be reviewed for conformance to the project specifications by the Owner's Representative
- C. The geomembrane seams shall meet the property requirements as shown in Table 2, (Attachment B).

PART 3. EXECUTION

3.01 SUBGRADE PREPARATION

- A. Geomembrane installed over geosynthetic clay liner (GCL).

The area of GCL to be covered with geomembrane shall be jointly inspected daily with the Owner's Representative before commencing geomembrane installation for the day, and the condition of the GCL shall be continuously observed as geomembrane installation progresses. Rocks, stones, sticks, sharp objects and debris of any kind shall be removed from the surface of the GCL. The Owner's Representative shall be notified of any discontinuities, premature hydration, or

otherwise defective GCL. Geomembrane shall not be placed over suspect areas until they have been repaired to the satisfaction of the Owner's Representative. The Installation Supervisor shall certify daily in writing that the GCL surface was acceptable at the time of geomembrane installation.

B. Geomembrane installed over cushion dirt.

The area of cushion dirt to be covered with geomembrane shall be prepared in accordance with the Section 02200 – Earthwork. The surface shall be smooth and free of ruts and holes, rocks, stones, sticks, sharp objects and debris of any kind.

C. The Geomembrane installer shall provide daily written acceptance for the surface to be covered by the geomembrane in that day's operations. The surface shall be maintained in a manner, during geomembrane installation, to ensure subgrade suitability.

D. All subgrade damaged by construction equipment and deemed unsuitable by the Owner's Representative for geomembrane deployment shall be repaired prior to placement of the geomembrane. All repairs shall be reviewed by the Owner's Representative and approved by the Geomembrane Installer. This damage, repair, and the responsibilities of the contractor and Geomembrane Installer shall be defined in the preconstruction meeting.

3.02 GEOMEMBRANE PLACEMENT

A. No geomembrane shall be deployed until the applicable certifications and quality control certificates listed in subsection 1.04 of this Section are submitted to and approved by the Owner's Representative. Should geomembrane material be deployed prior to approval by the Owner's Representative it will be at the sole risk of the Geomembrane Installer and/or Contractor. If the material does not meet project specifications it shall be removed from the work area at no cost to the owner.

B. The geomembrane shall be installed to the limits shown on the project drawings and essentially as shown on approved panel layout drawings.

C. No geomembrane material shall be unrolled and deployed if the material temperatures are lower than 0 degrees C (32 degrees F). Temperature limitations should be defined in the preconstruction meeting. Typically, only the quantity of geomembrane that will be anchored and seamed together in one day should be deployed.

D. No vehicular traffic shall travel on the geomembrane other than an approved low ground pressure All Terrain Vehicle or equivalent.

E. Sand bags or equivalent ballast shall be used as necessary to temporarily hold the geomembrane material in position under the foreseeable and reasonably - expected wind conditions. Sand bag material shall be sufficiently close-knit to prevent soil fines from working through the bags and discharging on the geomembrane.

F. Geomembrane placement shall not be done if moisture prevents proper subgrade preparation, panel placement, or panel seaming. Moisture limitations

- should be defined in the preconstruction meeting.
- G. Damaged panels or portions of the damaged panels which have been rejected shall be marked and their removal from the work area recorded.
 - H. The geomembrane shall not be allowed to "bridge over" voids or low areas in the subgrade. In these areas, the subgrade shall be prepared to allow the geomembrane to rest in intimate contact with the subgrade.
 - I. Wrinkles caused by panel placement or thermal expansion should be minimized in accordance with section 1.10 B. 11.
 - J. Considerations on Site Geometry: In general, seams shall be oriented parallel to the line of the maximum slope. In corners and odd shaped geometric locations, the total length of field seams shall be minimized. Seams shall not be located at low points in the subgrade.
 - K. Overlapping: The panels shall be overlapped prior to seaming to whatever extent is necessary to effect a good weld and allow for proper testing. In no case shall this overlap be less than 75mm (3 in.).

3.03 SEAMING PROCEDURES

- A. Cold weather installations should follow guidelines as outlined in GRI GM9.
- B. No geomembrane material shall be seamed when liner temperatures are less than 0 degrees C (32 degrees F).
- C. No geomembrane material shall be seamed when the sheet temperature is above 75 degrees C (170 degrees F) as measured by an infrared thermometer or surface thermocouple.
- D. Seaming shall primarily be performed using automatic fusion welding equipment and techniques. Extrusion welding shall be used where fusion welding is not possible such as at pipe penetrations, patches, repairs and short (less than a roll width) runs of seams.
- E. Fishmouths or excessive wrinkles at the seam overlaps, shall be minimized and when necessary cut along the ridge of the wrinkles back into the panel so as to effect a flat overlap. The cut shall be terminated with a keyhole cut (nominal 10 mm (1/2 in) diameter hole) so as to minimize crack/tear propagation. The overlay shall subsequently be seamed. The key hole cut shall be patched with an oval or round patch of the same base geomembrane material extending a minimum of 150 mm (6 in.) beyond the cut in all directions.

3.04 PIPE AND STRUCTURE PENETRATION SEALING SYSTEM

- A. Provide penetration sealing system as shown in the Project Drawings.
- B. Penetrations shall be constructed from the base geomembrane material, flat stock, prefabricated boots and accessories as shown on the Project Drawings. The prefabricated or field fabricated assembly shall be field welded to the geomembrane as shown on the Project Drawings so as to prevent leakage. This assembly shall be tested as outlined in section 3.05.B. Alternatively, where field non destructive testing can not be performed, attachments will be field spark tested by standard holiday leak detectors in accordance with ASTM 6365 Spark testing should be done in areas where both air pressure testing and vacuum testing are not possible.
 - 1. Equipment for Spark testing shall be comprised of but not limited to: A

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- hand held holiday spark tester and conductive wand that generates a high voltage.
2. The testing activities shall be performed by the Geomembrane Installer by placing an electrically conductive tape or wire beneath the seam prior to welding. A trial seam containing a non welded segment shall be subject to a calibration test to ensure that such a defect (non welded segment) will be identified under the planned machine settings and procedures. Upon completion of the weld, enable the spark tester and hold approximately 25mm (1 in) above the weld moving slowly over the entire length of the weld in accordance with ASTM 6365. If there is no spark the weld is considered to be leak free.
 3. A spark indicates a hole in the seam. The faulty area shall be located, repaired and retested by the Geomembrane Installer.
 4. Care should be taken if flammable gases are present in the area to be tested.

3.05 FIELD QUALITY CONTROL

The Owner's Representative shall be notified prior to all pre qualification and production welding and testing, or as agreed upon in the pre construction meeting.

A. Prequalification Test Seams

1. Test seams shall be prepared and tested by the Geomembrane Installer to verify that seaming parameters (speed, temperature and pressure of welding equipment) are adequate.
2. Test seams shall be made by each welding technician and tested in accordance with ASTM D 4437 at the beginning of each seaming period. Test seaming shall be performed under the same conditions and with the same equipment and operator combination as production seaming. The test seam shall be approximately 3.3 meters (10 feet) long for fusion welding and 1 meter (3 feet) long for extrusion welding with the seam centered lengthwise. At a minimum, tests seams should be made by each technician 1 time every 4-6 hours; additional tests may be required with changes in environmental conditions.
3. Two 25 mm (1 in) wide specimens shall be die-cut by the Geomembrane Installer from each end of the test seam. These specimens shall be tested by the Geomembrane Installer using a field tensiometer testing both tracks for peel strength and also for shear strength. Each specimen shall fail in the parent material and not in the weld, "Film Tear Bond"(F.T.D. failure). Seam separation equal to or greater than 10% of the track width shall be considered a failing test.
4. The minimum acceptable seam strength values to be obtained for all specimens tested are listed in Subsection 3.05.C.4 of this Section. All four specimens shall pass for the test seam to be a passing seam.
5. If a test seam fails, an additional test seam shall be immediately conducted. If the additional test seam fails, the seaming apparatus shall be rejected and not used for production seaming until the deficiencies are

- corrected and a successful test seam can be produced.
6. A sample from each test seam shall be labeled. The label shall indicate the date, geomembrane temperature, number of the seaming unit, technician performing the test seam and pass or fail description. The sample shall then be given to the Owner's Representative for archiving.

B. Field Seam Non-destructive Testing

1. All field seams shall be non-destructively tested by the Geomembrane Installer over the full seam length before the seams are covered. Each seam shall be numbered or otherwise designated. The location, date, test unit, name of tester and outcome of all non-destructive testing shall be recorded and submitted to the Owner's Representative.
2. Testing should be done as the seaming work progresses, not at the completion of all field seaming. All defects found during testing shall be numbered and marked immediately after detection. All defects found should be repaired, retested and remarked to indicate acceptable completion of the repair.
3. Non-destructive testing shall be performed using vacuum box, air pressure or spark testing equipment.
4. Non-destructive tests shall be performed by experienced technicians familiar with the specified test methods. The Geomembrane Installer shall demonstrate to the Owner's Representative all test methods to verify the test procedures are valid.
5. Extrusion seams shall be vacuum box tested by the Geomembrane Installer in accordance with ASTM D 4437 and ASTM D 5641 with the following equipment and procedures:
 - a. Equipment for testing extrusion seams shall be comprised of but not limited to: a vacuum box assembly consisting of a rigid housing, a transparent viewing window, a soft rubber gasket attached to the base, port hole or valve assembly and a vacuum gauge; a vacuum pump assembly equipped with a pressure controller and pipe connections; a rubber pressure/vacuum hose with fittings and connections; a plastic bucket; wide paint brush or mop; and a soapy solution.
 - b. The vacuum pump shall be charged and the tank pressure adjusted to approximately 35 kPa (5 psig).
 - c. The Geomembrane Installer shall create a leak tight seal between the gasket and geomembrane interface by wetting a strip of geomembrane approximately 0.3m (12 in) by 1.2m (48 in) (length and width of box) with a soapy solution, placing the box over the wetted area, and then compressing the box against the geomembrane. The Geomembrane Installer shall then close the bleed valve, open the vacuum valve, maintain initial pressure of approximately 35 kPa (5 psig) for approximately 5 seconds. The geomembrane should be continuously examined through the viewing window for the presence of soap bubbles, indicating a leak. If no bubbles appear after 5 seconds, the area shall be

- considered leak free. The box shall be depressurized and moved over the next adjoining area with an appropriate overlap and the process repeated.
- d. All areas where soap bubbles appear shall be marked, repaired and then retested.
 - e. At locations where seams cannot be non destructively tested, such as pipe penetrations, alternate nondestructive spark testing (as outlined in section 3.04.B) or equivalent should be substituted.
 - f. All seams that are vacuum tested shall be marked with the date tested, the name of the technician performing the test and the results of the test.
6. Double Fusion seams with an enclosed channel shall be air pressure tested by the Geomembrane Installer in accordance with ASTM D 5820 and ASTM D 4437 and the following equipment and procedures:
- a. Equipment for testing double fusion seams shall be comprised of but not limited to: an air pump equipped with a pressure gauge capable of generating and sustaining a pressure of 210 kPa (30 psig), mounted on a cushion to protect the geomembrane; and a manometer equipped with a sharp hollow needle or other approved pressure feed device.
 - b. The Testing activities shall be performed by the Geomembrane Installer. Both ends of the seam to be tested shall be sealed and a needle or other approved pressure feed device inserted into the tunnel created by the double wedge fusion weld. The air pump shall be adjusted to a pressure of 210 kPa (30 psig), and the valve closed,. Allow 2 minutes for the injected air to come to equilibrium in the channel, and sustain pressure for 5 minutes. If pressure loss does not exceed 28 kPa (4 psig) after this five minute period the seam shall be considered leak tight. Release pressure from the opposite end verifying pressure drop on needle to ensure testing of the entire seam. The needle or other approved pressure feed device shall be removed and the feed hole sealed.
 - c. If loss of pressure exceeds 28 kPa (4 psig) during the testing period or pressure does not stabilize, the faulty area shall be located, repaired and retested by the Geomembrane Installer.
 - d. Results of the pressure testing shall be recorded on the liner at the seam tested and on a pressure testing record.

C. Destructive Field Seam Testing

1. One destructive test sample per 150 linear m (500 linear ft) seam length or another predetermined length in accordance with GRI GM 14 shall be taken by the Geomembrane Installer from a location specified by the Owner's Representative. The Geomembrane Installer shall not be informed in advance of the sample location. In order to obtain test results prior to completion of geomembrane installation, samples shall be cut by the Geomembrane Installer as directed by the Owner's Representative as seaming progresses.

2. All field samples shall be marked with their sample number and seam number. The sample number, date, time, location, and seam number shall be recorded. The Geomembrane Installer shall repair all holes in the geomembrane resulting from obtaining the seam samples. All patches shall be vacuum box tested or spark tested. If a patch cannot be permanently installed over the test location the same day of sample collection, a temporary patch shall be tack welded or hot air welded over the opening until a permanent patch can be affixed.
3. The destructive sample size shall be 300 mm (12 in) wide by 1 m (36 in) long with the seam centered lengthwise. The sample shall be cut into three equal sections and distributed as follows: one section given to the Owner's Representative as an archive sample; one section given to the Owner's Representative for laboratory testing as specified in paragraph 5 below; and one section retained by the Geomembrane Installer for field testing as specified in paragraph 4 below.
4. For field testing, the Geomembrane Installer shall cut 10 identical 25 mm (1 in) wide replicate specimens from his sample. The Geomembrane Installer shall test five specimens for seam shear strength and five for peel strength. Peel tests will be performed on both inside and outside weld tracks. To be acceptable, 4 of 5 test specimens must pass the stated criteria in section 2.02 with less than 10% separation. If 4 of 5 specimens pass, the sample qualifies for testing by the testing laboratory if required.
5. If independent seam testing is required by the specifications it shall be conducted in accordance with ASTM 5820 or ASTM D4437 or GRI GM 6.
6. Reports of the results of examinations and testing shall be prepared and submitted to the Owner's Representative.
7. For field seams, if a laboratory test fails, that shall be considered as an indicator of the possible inadequacy of the entire seamed length corresponding to the test sample. Additional destructive test portions shall then be taken by the Geomembrane Installer at locations indicated by the Engineer, typically 3 m (10 ft) on either side of the failed sample and laboratory seam tests shall be performed. Passing tests shall be an indicator of adequate seams. Failing tests shall be an indicator of non-adequate seams and all seams represented by the destructive test location shall be repaired with a cap-strip extrusion welded to all sides of the capped area. All cap-strip seams shall be non-destructively vacuum box tested until adequacy of the seams is achieved. Cap strip seams exceeding 50 M in length (150 FT) shall be destructively tested.

D. Identification of Defects

1. Panels and seams shall be inspected by the Installer and Owner's Representative during and after panel deployment to identify all defects, including holes, blisters, undispersed raw materials and signs of contamination by foreign matter.

E. Evaluation of Defects: Each suspect location on the liner (both in geomembrane
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seam and non-seam areas) shall be non-destructively tested using one of the methods described in Section 3.05.B. Each location which fails non-destructive testing shall be marked, numbered, measured and posted on the daily "installation" drawings and subsequently repaired.

1. If a destructive sample fails the field or laboratory test, the Geomembrane Installer shall repair the seam between the two nearest passed locations on both sides of the failed destructive sample location.
 2. Defective seams, tears or holes shall be repaired by reseaming or applying a extrusion welded cap strip.
 3. Reseaming may consist of either:
 - a. Removing the defective weld area and rewelding the parent material using the original welding equipment; or
 - b. Reseaming by extrusion welding along the overlap at the outside seam edge left by the fusion welding process.
 4. Blisters, larger holes, and contamination by foreign matter shall be repaired by patches and/or extrusion weld beads as required. Each patch shall extend a minimum of 150 mm (6 in) beyond all edges of the defects.
 5. All repairs shall be measured, located and recorded.
- F. Verification of Repairs on Seams: Each repair shall be non-destructively tested using either vacuum box or spark testing methods. Tests which pass the non-destructive test shall be taken as an indication of a successful repair. Failed tests shall be reseamed and retested until a passing test results. The number, date, location, technician and test outcome of each patch shall be recorded.
- G. Daily Field Installation Reports: At the beginning of each day's work, the Installer shall provide the Engineer with daily reports for all work accomplished on the previous work day. Reports shall include the following:
1. Total amount and location of geomembrane placed;
 2. Total length and location of seams completed, name of technicians doing seaming and welding unit numbers;
 3. Drawings of the previous day's installed geomembrane showing panel numbers, seam numbers and locations of non-destructive and destructive testing;
 4. Results of pre-qualification test seams;
 5. Results of non-destructive testing; and
 6. Results of vacuum testing of repairs.
- H. Destructive test results shall be reported prior to covering of liner or within 48 hours.

3.06 LINER ACCEPTANCE

- A. Geomembrane liner will be accepted by the Owner's Representative when:
1. The entire installation is finished or an agreed upon subsection of the installation is finished;

2. All Installer's QC documentation is completed and submitted to the owner
3. Verification of the adequacy of all field seams and repairs and associated geomembrane testing is complete.

3.07 ANCHOR TRENCH

- A. Construct as specified on the project drawings.

3.08 DISPOSAL OF SCRAP MATERIALS

- A. On completion of installation, the Geomembrane Installer shall dispose of all trash and scrap material in a location approved by the Owner, remove equipment used in connection with the work herein, and shall leave the premises in a neat acceptable manner. No scrap material shall be allowed to remain on the geomembrane surface.

PART 4. GRI GM13 SPECIFICATIONS

Geosynthetics Research Institute (GRI) Test Method GM13 – “Test Methods, Test Properties and Testing Frequency for High Density Polyethylene (HDPE) Smooth and Textured Geomembranes”, Revision 8, Dated July 10, 2006.

ATTACHMENT A:

Minimum Average Weld Properties for Smooth and Textured HDPE Geomembranes (English units)								
Property	Test Method	30 mil	40 mil	50 mil	60 mil	80 mil	100 mil	120 mil
Peel strength (fusion & extrusion) lb/in.	ASTM 4437	39	52	65	78	104	130	156
Shear strength (fusion & extrusion) lb/in.	ASTM 4437	60	80	100	120	160	200	239

END OF SECTION 02800

DIVISION 2 - SITE WORK

Section 02936 - Topsoil, Seeding, and Mulching

PART 1. GENERAL

1.01 DESCRIPTION

This section pertains to seeding and placing mulch or erosion control blanket over seeded areas.

1.02 RELATED SECTIONS

A. Specified elsewhere:

1. 02200 - Earthwork

1.03 REFERENCES

The following reference or cited portions thereof, current at date of bidding documents unless otherwise specified, governs the work.

A. Illinois Department of Transportation (IDOT): Standard Specifications for Road and Bridge Construction, adopted January 1, 2007.

1.04 SPECIFICATIONS

A. Work shall conform to the applicable requirements of Sections 250 and 251 of Standard Specifications for Road and Bridge Construction and to the requirements hereinafter specified.

B. Exceptions: All references in the IDOT specifications to methods of measurement and payment shall not apply.

1.05 WARRANTY

A. Warranty for one (1) year plus one growing season from date of substantial completion shall be provided.

PART 2. PRODUCTS

2.01 MATERIALS

A. Seed: Seed shall conform to Article 1081.04 of the IDOT Standard Specifications. The composition of the Ameren Energy Resources Generating hay seeding mix shall

be as follows:

<u>Seed Type</u>	<u>Pounds/Acre</u>
Vernal Alfalfa	12
Wrangler Alfalfa	8
Medium Red Clover	6
Timothy	4

- B. Mulch Material and Erosion Control Blanket: Mulch material shall conform to Article 1081.06 and the excelsior blanket/knitted straw mat shall conform to Article 1081.10 of the IDOT Standard Specifications.
- C. Fertilizer and agricultural ground limestone will not be permitted.

PART 3. EXECUTION

3.01 CONSTRUCTION

- A. Seed bed preparation and seeding methods shall conform to Section 250 of the IDOT Specifications. Seeding of areas disturbed by construction activities after September 30, 2008 may be deferred until Spring 2009.
- B. Seed shall be applied to the perimeter berm ditch, to disturbed portions of the perimeter berm, and to all disturbed earth surfaces outside of the existing perimeter berm. IDOT seeding mixture 7 shall be used on stockpiles. IDOT seeding mixture 1A shall be used on the gypsum stack perimeter earthen berm, the recycle pond dam embankment and on slopes that are 4H:1V or steeper. The Ameren hay seed mix shall be used on slopes flatter than 4H:1V.
- C. Application rates for IDOT seed mixtures shall be as specified in Section 250 of the IDOT Specifications. The application rate for the Ameren Energy Resources Generating's seed mix shall be as specified in the Ameren Energy Resources Generating's hay seeding mix.
- D. Seeded areas shall be mulched in accordance with Article 251.03. The Contractor may use either Method 2 or Method 3.

3.02 MAINTENANCE OF COMPLETED WORK

- A. All areas seeded by the Contractor shall be maintained by the Contractor during the period between completion of such work and final completion and acceptance of the Contractor's work by the Owner. This maintenance shall be such that the completed work, at time of acceptance, complies in all respects with the requirements herein specified.

- B. The areas seeded will be required to germinate. If the seed does not germinate, the Contractor will be required to regrade and reseed at no additional cost.

END OF SECTION 02936

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DIVISION 3 - CONCRETE

Section 03100 - Concrete Formwork

PART 1. GENERAL

1.01 WORK INCLUDES

- A. The complete installation of the formwork for cast-in-place concrete, with shoring, bracing and anchorage, openings for other work, form accessories, form stripping.

1.02 RELATED SECTIONS

- A. Section 03200 - Concrete Reinforcement.
- B. Section 03300 - Cast-In-Place Concrete.
- C. Section 03400 – Concrete Embedment Liner.

1.03 REFERENCES

- A. ACI 347 - Recommended Practice For Concrete Formwork.
- B. ACI 301 - Specifications For Structural Concrete For Buildings.

1.04 DESIGN REQUIREMENTS

- A. Design, engineer and construct formwork, shoring and bracing to conform to design and code requirements; resultant concrete to conform to required shape, line and dimension.

1.05 QUALITY ASSURANCE

- A. Perform Work in accordance with ACI 347 and 301.

1.06 REGULATORY REQUIREMENTS

- A. Conform to applicable code for design, fabrication, erection and removal of formwork.

1.07 DELIVERY, STORAGE, AND HANDLING

- A. Store off ground in ventilated and protected manner to prevent deterioration from moisture.

1.08 COORDINATION

- A. Coordinate this Section with other Sections of work which require attachment of components of formwork.
- B. If formwork is placed which results in insufficient concrete cover over reinforcement, request instructions from Owner's Representative before proceeding.

PART 2. PRODUCTS

2.01 WOOD FORM MATERIALS

- A. Softwood Plywood: 3/4 in. PS 1-83 "B-B" (concrete form) plywood, Class I, exterior grade or better, mill-oiled and edge sealed with each piece bearing legible inspection trademark.
- B. Architectural Plywood: 3/4 in. PS 1-83 "B-B" plyform, Class I, with High Density smooth overlay, 1 surface, edge sealed with each piece bearing legible inspection trademark.

2.02 MANUFACTURERS - PREFABRICATED FORMS

- A. Weyerhaeuser Concrete Form.
- B. Georgia Pacific, G-P Exterior Soft Wood Plywood Product.
- C. Plywood and Door Corporation's Finn-Form.

2.03 PREFABRICATED FORMS

- A. Preformed Steel Forms: Minimum 16 gage matched, tight fitting, stiffened to support weight of concrete without deflection detrimental to tolerances and appearance of finished surfaces.
- B. Glass Fiber Fabric Reinforced Plastic Forms: Matched, tight fitting, stiffened to support weight of concrete without deflection detrimental to tolerances and appearance of finished concrete surfaces.

2.04 FORMWORK ACCESSORIES

- A. Form Ties: Snap-off type, galvanized metal, adjustable length, 1 in. back break dimension, free of defects that could leave holes larger than 1 in. in concrete surface; Dayton-Sure Grip snap-in-form ties, as manufactured by Dayton Superior

Corp., Symons Ties as manufactured by Symons Corporation, Snap-Tys as manufactured by Richmond Corporation. Ties shall be removed after forms are removed, and holes filled with mortar that matches the adjacent surfaces.

- B. Form Release Agent: Colorless mineral oil which will not stain concrete, or absorb moisture; by Magic Kote manufactured by Symons Manufacturing Co., Form Coat manufactured by Concrete Services Co., Formcel manufactured by Lambert Corp.
- C. Corners: Chamfered, wood strip type; 3/4 x 3/4 in. size on all exterior corners, 3 x 3 in. size where shown on the drawings; maximum possible lengths.
- D. Nails, Spikes, Lag Bolts, Through Bolts, Anchorages: Sized as required, of sufficient strength and character to maintain formwork in place while placing concrete.
- E. Concrete Embedment Liner, where required, shall be installed in accordance with Section 03400 – Concrete Embedment Liner.

PART 3. EXECUTION

3.01 EXAMINATION

- A. Verify lines, levels and centers before proceeding with formwork. Ensure that dimensions agree with drawings.

3.02 EARTH FORMS

- A. Earth forms are not permitted, except for footings.

3.03 ERECTION - FORMWORK

- A. Erect formwork, shoring and bracing to achieve design requirements, in accordance with requirements of ACI 301. Metal forms shall be installed in strict accordance with manufacturer's directions and specifications.
- B. Provide bracing to ensure stability of formwork. Shore or strengthen formwork subject to overstressing by construction loads.
- C. Arrange and assemble formwork to permit dismantling and stripping. Do not damage concrete during stripping. Permit removal of remaining principal shores.
- D. Align joints and make watertight. Keep form joints to a minimum.
- E. Obtain approval before framing openings in structural members which are not indicated on drawings.

3.04 APPLICATION - FORM RELEASE AGENT

- A. Apply form release agent on formwork in accordance with manufacturer's recommendations.
- B. Apply prior to placement of reinforcing steel, anchoring devices, and embedded items.
- C. Do not apply form release agent where concrete surfaces will receive special finishes or applied coverings which are affected by agent.

3.05 INSERTS, EMBEDDED PARTS, AND OPENINGS

- A. Provide formed openings where required for items to be embedded in or passing through concrete work.
- B. Locate and set in place items which will be cast directly into concrete.
- C. Coordinate work of other Sections in forming and placing openings, slots, reglets, recesses, chases, sleeves, bolts, anchors, and other inserts.
- D. Install accessories in accordance with manufacturer's instructions, straight, level, and plumb. Ensure items are not disturbed during concrete placement.
- E. Provide temporary ports or openings in formwork where required to facilitate cleaning and inspection. Locate openings at bottom of forms to allow flushing water to drain.
- F. Close temporary openings with tight fitting panels, flush with inside face of forms, and neatly fitted so joints will not be apparent in exposed concrete surfaces.

3.06 FORM CLEANING

- A. Clean and remove foreign matter within forms as erection proceeds.
- B. Clean formed cavities of debris prior to placing concrete.
- C. Flush with water or use compressed air to remove remaining foreign matter. Ensure that water and debris drain to exterior through clean-out ports.
- D. During cold weather, remove ice and snow from within forms. Do not use de-icing salts or water to clean out forms. Use compressed air or other means to remove foreign matter.

3.07 FORMWORK TOLERANCES

- A. Construct formwork to maintain tolerances required by ACI 301.

3.08 FIELD QUALITY CONTROL

- A. Inspect erected formwork, shoring, and bracing to ensure that work is in accordance with formwork design, and that supports, fastenings, wedges, ties, and items are secure.
- B. Do not reuse wood formwork more than three times for concrete surfaces to be exposed to view. Do no patch formwork.

3.09 FORM REMOVAL

- A. Do not remove forms or bracing until concrete has gained sufficient strength to carry its own weight and imposed loads.
- B. Loosen forms carefully. Do not wedge pry bars, hammers, or tools against finished concrete surfaces scheduled for exposure to view.
- C. Store removed forms in manner that surfaces to be in contact with fresh concrete will not be damaged. Discard damaged forms.

END OF SECTION 03100

DIVISION 3 - CONCRETE
Section 03200 - Concrete Reinforcement

PART 1. GENERAL

1.01 WORK INCLUDES

- A. The complete installation of the reinforcing steel bars and accessories for cast-in-place concrete.

1.02 RELATED SECTIONS

- A. Section 03100 - Concrete Formwork.
- B. Section 03300 - Cast-in-Place Concrete.

1.03 REFERENCES

- A. ACI 301 - Structural Concrete for Buildings.
- B. ACI 318 - Building Code Requirements For Reinforced Concrete.
- C. ACI SP-66 - American Concrete Institute - Detailing Manual.
- D. ASTM A615 - Deformed and Plain Billet Steel Bars for Concrete Reinforcement.
- E. CRSI - Concrete Reinforcing Steel Institute Manual of Practice.

1.04 SUBMITTALS

- A. Submit under provisions of Section 01010.
- B. Shop Drawings: Indicate bar sizes, spacings, locations, and quantities of reinforcing steel, and bending and cutting schedules. Contract drawings shall not be reproduced as the basis for shop drawings.
- C. Manufacturer's Certificate: Certify that products meet or exceed specified requirements.

1.05 QUALITY ASSURANCE

- A. Perform Work in accordance with CRSI Manual of Standard Practice.
- B. Submit certified copies of mill test report of reinforcement materials analysis.

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1.06 COORDINATION

- A. Coordinate with placement of formwork, formed openings and other work.

PART 2. PRODUCTS

2.01 REINFORCEMENT

- A. Reinforcing Steel: ASTM A615, 60 ksi yield grade; deformed billet steel bars.

2.02 ACCESSORY MATERIALS

- A. Tie Wire: Minimum 16 gage, annealed steel wire, epoxy coated when used with epoxy-coated reinforcement.
- B. Chairs, Bolsters, Bar Supports, Spacers: Sized and shaped for strength and support of reinforcement during concrete placement conditions.
- C. Special Chairs, Bolsters, Bar Supports, Spacers Adjacent to Weather Exposed Concrete Surfaces: Plastic coated steel type; size and shape as required.

2.03 FABRICATION

- A. Fabricate concrete reinforcing in accordance with CRSI Manual of Standard Practice and ACI SP-66.
- B. Splice reinforcement on at locations indicated on drawings. Indicate location of splices on shop drawings.

PART 3. EXECUTION

3.01 PLACEMENT

- A. Place, support and secure reinforcement against displacement. Do not deviate from required position. Clean reinforcement of foreign particles or coatings.
- B. Accommodate placement of formed openings.
- C. Conform to ACI 318 code for concrete cover over reinforcement.

3.02 FIELD QUALITY CONTROL

- A. Contractor shall notify the Owner's Representative at least 24 hrs. in advance of concrete placement. Placement of reinforcing shall occur in such sequence that the Owner's Representative has sufficient time to inspect the correctness of the reinforcing within the placement area. The Owner's Representative retains the right to require necessary revisions be made before concrete is placed.

END OF SECTION 03200

DIVISION 3 - CONCRETE
Section 03300 - Cast-In-Place Concrete

PART 1. GENERAL

1.01 WORK INCLUDES

- A. The complete installation of cast-in-place concrete structures, including joint sealants.

1.02 RELATED SECTIONS

- A. Section 03100 - Concrete Formwork: Formwork and accessories.
- B. Section 03200 - Concrete Reinforcement.
- C. Section 03400 – Concrete Embedment Liner

1.03 REFERENCES

- A. ACI 301 – Structural Concrete for Buildings.
- B. ACI 302 - Guide for Concrete Floor and Slab Construction.
- C. ACI 304 - Recommended Practice for Measuring, Mixing, Transporting and Placing Concrete.
- D. ACI 305R - Hot Weather Concreting.
- E. ACI 306R - Cold Weather Concreting.
- F. ACI 308 - Standard Practice for Curing Concrete.
- G. ACI 318 - Building Code Requirements for Reinforced Concrete.
- H. ASTM C31 - Concrete Test Specimens.
- I. ASTM C33 - Concrete Aggregates.
- J. ASTM C94 - Ready-Mixed Concrete.
- K. ASTM C150 - Portland Cement.
- L. ASTM C260 - Air Entraining Admixtures for Concrete.

M. ASTM C494 - Chemical Admixtures for Concrete.

1.04 SUBMITTALS

A. Product Data: Provide data on joint devices, attachment accessories, admixtures.

1.05 QUALITY ASSURANCE

A. Perform Work in accordance with ACI 301.

B. Acquire cement and aggregate from same source for all work.

C. Conform to ACI 305R when concreting during hot weather.

D. Conform to ACI 306R when concreting during cold weather.

1.06 COORDINATION

A. Coordinate this Section with other Sections which require embedment of components in cast-in-place concrete.

1.07 PRODUCT DATA

A. Submit proposed mix design to Owner's Representative for review prior to commencement of work. Identify source and provide material certificates for cement, fine and coarse aggregates. Provide recent laboratory gradation for fine and coarse aggregates and mix design information in accordance with ACI 301.

B. Submit Construction joint plan.

PART 2. PRODUCTS

2.01 CONCRETE MATERIALS

A. Cement: ASTM C150, Type I - Normal Portland Type, Gray Color.

B. Fine and Coarse Aggregates: ASTM C33.

C. Water: Potable.

2.02 ADMIXTURES

A. Air Entrainment: ASTM C260.

- B. Chemical: ASTM C494. Maximum 0.05% Chloride Ion Contents.
- C. The use of calcium chloride in any concrete is not permitted.

2.03 ACCESSORIES

- A. Non-Shrink Grout: Premixed compound consisting of non-metallic aggregate, cement, water reducing and plasticizing agents; capable of developing minimum compressive strength of 2,400 psi in 48 hours and 7,000 psi in 28 days.
- B. Curing Compound: Dress and Seal No. 18 by L&M Construction Chemicals, MB-429 by Master Builders, or Sikagard Cure/Hard by the Sika Corporation.
- C. Epoxy Grouted Adhesive Anchors: Hilti, Red Head, Simpson, or Rawl.

2.04 CONCRETE MIX

- A. Mix concrete in accordance with ACI 304. Deliver concrete in accordance with ASTM C94.
- B. Select proportions for normal weight concrete in accordance with ACI 301.
- C. Provide normal weight concrete of the following characteristics:
 - 1. Compressive strength at 28 days: 4,000 psi.
 - 2. Slump: 4 in. - A tolerance of up to 1 in. above the maximum shall be allowed for one batch in any five consecutive batches tested.
 - 3. Water/cement ratios: 0.4 (max).
- D. Use accelerating admixtures in cold weather only when approved by Owner's Representative. Use of admixtures will not relax cold weather placement requirements.
- E. Use set-retarding admixtures during hot weather only when approved by Owner's Representative.
- F. Water-reducing admixtures may be used in all concrete except footings and in strict compliance with the manufacturer's directions.
- G. Add air-entraining agent to concrete mix for air content of 6% ($\pm 1\%$).

PART 3. EXECUTION

3.01 EXAMINATION

- A. Verify requirements for concrete cover over reinforcement.
- B. Verify that anchors, seats, plates, reinforcement and other items to be cast into concrete are accurately placed, positioned securely, and will not cause hardship in placing concrete.

3.02 PLACING CONCRETE

- A. Place concrete in accordance with ACI 301.
- B. Notify Owner's Representative minimum of 24 hours prior to commencement of operations.
- C. Ensure reinforcement, inserts, and embedded parts are not disturbed during concrete placement.
- D. Maintain records of concrete placement. Record date, location, quantity, air temperature, and test samples taken.
- E. Place concrete continuously between predetermined expansion, control, and construction joints.
- F. When air temperature is between 80°F and 90°F, reduce the mixing and delivery time specified in ASTM C94 from 1-1/2 hours to 75 minutes. When the air temperature is above 90°F, reduce the mixing and delivery time to 60 minutes.
- G. Cold weather concreting. Comply with ACI 306 except as follows:
 - 1. In freezing weather, provide suitable means for maintaining concrete temperature at a minimum of 70°F for three days, or 50°F for five days after placing.
 - 2. Cooling of concrete to outside temperature: Not faster than 1° per hour for first day and 2° per hour thereafter until outside temperature is reached.
 - 3. Maximum temperature of concrete produced with heated aggregated, heated water, or both, at any time during its production or transportation: 90°F.
 - 4. Do not mix chemicals or other foreign materials in concrete to prevent freezing or to accelerate hardening of concrete, unless approved in writing by Owner's Representative.

- H. Hot weather concreting. Comply with ACI 305R.
 - 1. ACI recommendations shall be observed when any combination of high air temperature, low relative humidity and wind velocity tend to impair the quality of fresh or hardened concrete.
 - 2. Retarding and water reducing admixtures shall be approved in writing for each concrete mix design prior to placement.

3.03 CONCRETE FINISHING

- A. Provide exterior concrete formed surfaces to be left exposed with smooth rubbed finish in accord with ACI 301. All other formed surfaces shall have fins, projections and offsets removed.
- B. Provide Class A tolerances to exterior concrete slabs according to ACI 301.
 - 1. Broom finish all exterior slabs. Broom out all tool marks.
- C. Pitch slabs to drain.

3.04 CURING AND PROTECTION

- A. Immediately after placement, protect concrete from premature drying, excessively hot or cold temperatures, and mechanical injury.
- B. Maintain concrete with minimal moisture loss at relatively constant temperature for a period necessary for hydration of cement and hardening of concrete in accordance with ACI 308.
- C. Cure and protect finished concrete slabs in accordance with ACI 308.

3.05 FIELD QUALITY CONTROL

- A. Field inspection and testing will be performed in accordance with ACI 301 and under provisions of Section 01010, paragraph 8.0.
- B. Testing firm will take cylinders, perform slump and air entrainment tests in accordance with ACI 301.
- C. Provide free access to Work and cooperate with appointed firm.
- D. Submit proposed concrete mix design to Owner's Representative firm for review 14 days prior to commencement of Work.

- E. Testing frequency shall be as specified in Section 01010, paragraph 8, except that one additional test cylinder will be taken during cold weather concreting, cured on job site under same conditions as concrete it represents.

3.06 PATCHING

- A. Defective Concrete: Concrete not conforming to required lines, details, dimensions, tolerances or specified requirements.
- B. Repair or replacement of defective concrete will be determined by Owner's Representative and performed by the Contractor at no additional cost to the project.
- C. Do not patch, fill, touch-up, repair, or replace exposed concrete except upon express direction of Owner's Representative for each individual area.

END OF SECTION 03300

DIVISION 3 - CONCRETE

Section 03400 - Concrete Embedment Liner

PART 1. GENERAL

1.01 WORK INCLUDES

- A. Specifications and guidelines for manufacturing and installing high-density polyethylene embedment liners.

1.02 RELATED SECTIONS

- A. Section 03100 - Concrete Formwork.
- B. Section 03300 - Cast-in-Place Concrete.

1.03 REFERENCES

A. American Society for Testing and Materials (ASTM)

1. D 1505 Test Method for Density of Plastics by the Density-Gradient Technique
2. D 1603 Test Method for Carbon Black in Olefin Plastics
3. D 5199 Standard Test Method for Measuring Nominal Thickness of Geotextiles and Geomembranes
4. D 5596 Standard Test Method for Microscopic Evaluation of the Dispersion of Carbon Black in Polyolefin Geosynthetics
5. D 6693 Standard Test Method for Determining Tensile Properties of Nonreinforced Polyethylene and Nonreinforced Flexible Polypropylene Geomembranes
6. D 1204 Standard Test Method for Linear Dimensional Changes of Nongrid Thermoplastic Sheeting or Film at Elevated Temperature
7. D 696 Standard Test Method for Coefficient of Linear Thermal Expansion of Plastics Between -30°C and 30°C With a Vitreous Silica Dilatometer
8. D 746 Standard Test Method for Brittleness Temperature of Plastics and Elastomers by Impact
9. D 570 Standard Test Method for Water Absorption of Plastics
10. E 96 Standard Test Method for Water Vapor Transmission of Material

1.04 SUBMITTALS

- A. All work for and in connection with the installation of the lining, field seaming and welding joints shall be completed in strict conformity with all applicable instructions and recommendations of the liner manufacturer.

- B. Included with the shipment of liner, submit certified test reports that the liner and material are manufactured in accordance with standards specified herein.

1.05 QUALIFICATIONS

- A. The HDPE liner specified in this section shall be furnished by a manufacturer who is fully experienced, reputable and qualified in the manufacturing of the materials. The manufacturer must at least 10 years of manufacturing experience.
- B. Locking devices must be extruded to the sheet as a one step process.
- C. Liner shall be GSE StudLiner as manufactured by GSE Lining Technology, Inc.
- D. Liner shall be 8 feet in width.
- E. Liner shall demonstrate a minimum pull-out strength of 14,000 psf.

1.06 COORDINATION

- A. Coordinate with placement of formwork, formed openings and other work.

PART 2. PRODUCTS

2.01 ROLL DIMENSIONS

- A. Embedment sheets shall be produced in rolls that are 8.0 ft (2.4 m) in width and a thickness range of 80 mils (2.0 mm) to 200 mils (5.0 mm) in thickness. Roll lengths vary according to thickness.
- B. Locking studs of the same material as that of the liner shall be integrally extruded with the sheet. Stud spacing shall be on approximate 1.25 in (30 mm) centers, such that there are approximately 110 studs per square foot (1200 per square meter).

2.02 MATERIAL PROPERTIES

- A. The material used in the embedment liner and in all welding strips shall be made from 97-98% virgin high density polyethylene and 1.5-3% carbon black or pigmentation for the purpose of an otherwise specified color.
- B. Plasticizer shall not be added to the resin formulation.

- C. Embedment sheet and welding strips shall be free of holes, pinholes, bubbles, blisters, excessive contamination by foreign matter, and nicks and cuts on roll edges.
- D. The HDPE cap strips shall be made from HDPE, have good impact resistance and have an elongation sufficient to bridge up to 1/4 inch settling cracks.
- E. Cap strips shall be approximately 4 inches wide or greater and shall be equivalent to that of the liner.
- F. Material shall maintain a repairable state through it's lifecycle by methods approved and recommended by the manufacturer.
- G. Embedment sheets shall have the following physical properties when tested in accordance with Table 1.
- H. Raw resin shall have the following properties when tested in accordance with Table 2.

Table 1: Material Properties

Property	Test Method	Nominal Value				Testing Frequency
Thickness, mm (mil)	ASTM D 5199	2.00 (80)	3.00 (120)	4.00 (160)	5.00 (200)	Every 5 th roll
Density, g/cm ³	ASTM D 1505	0.94	0.94	0.94	0.94	1/100,000 ft ²
Tensile Properties Strength@Yield, lb/in ² (MPa) Elongation @ Break, %	ASTM D 6693 Type IV, Dumbbell G.L.= 2.0in.	2,200 (14.5) 500	2,200 (14.5) 500	2,200 (14.5) 500	2,200 (14.5) 500	1/100,000 ft ²
Stud Pull-Out Strength ¹ , lb/ft ² (kN/m ²)		>14,000 (669.89)	>14,000 (669.89)	>14,000 (669.89)	>14,000 (669.89)	1/ product
Carbon Black Content/ Pigment Content, % Black Liner Gray Liner	ASTM D 1603, mod. ASTM D 5630, mod.	2-3 1.5 – 2.5	2-3 1.5 – 2.5	2-3 1.5 – 2.5	2-3 1.5 – 2.5	1/100,000 ft ²
Carbon Black Dispersion ²	ASTM D 5596	Note 2	Note 2	Note 2	Note 2	1/100,000 ft ²
Notched Constant Tensile Load, hours	ASTM D 5397	400	400	400	400	1/ formulation
Coefficient of Linear Thermal Expansion, per °C	ASTM D 696	1.20E-04	1.20E-04	1.20E-04	1.20E-04	1/ product
Low Temperature Brittleness, °C	ASTM D 746	-77	-77	-77	-77	1/ product
Dimensional Stability, % (each direction)	ASTM D 1204	± 1.0	± 1.0	± 1.0	± 1.0	1/ product
Water Absorption, %	ASTM D 570	0.1	0.1	0.1	0.1	1/ product
Water Vapor Transmission, (g/m ² /day)	ASTM E 96	<0.01	<0.01	<0.01	<0.01	1/ product

¹Note: Concrete must have a compressive strength of at least 5,000 lb/in² (34,500 kPa).

²Note: Dispersion only applies to near spherical agglomerates. 9 of 10 views shall be Category 1 or 2. No more than 1 view for category 3.

03200-4

Table 2: Raw Material Properties

Property	Test Method	Value	Testing Frequency
Density, g/cm ³	ASTM D 1505	0.932	1/ resin lot
Melt Flow, g/10 min	ASTM D 1238 (190/2.16)	≤ 1.0	1/ resin lot
OIT, minutes	ASTM D 3895 (1atm/200°C)	100	1/ formulation

2.03 MATERIAL SUPPLY

- A. Embedment sheets shall be supplied in roll form, sheets, pre-fabricated tubes or panels.
- B. Cap strips shall be supplied in 4 inch widths or greater.

PART 3. EXECUTION

3.01 PLACEMENT

- A. Place, support and secure reinforcement against displacement. Do not deviate from required position. Clean reinforcement of foreign particles or coatings.
- B. Accommodate placement of formed openings.
- C. Conform to ACI 318 code for concrete cover over reinforcement.

3.02 FIELD QUALITY CONTROL

- A. Contractor shall notify the Owner's Representative at least 24 hrs. in advance of concrete placement. Placement of the Concrete Embedment Liner shall occur in such sequence that the Owner's Representative has sufficient time to inspect the correctness of the placement within the concrete formwork area. The Owner's Representative retains the right to require necessary revisions be made before concrete is placed.

END OF SECTION 03200



Appendix E: Operation & Maintenance Manual for #1 Ash Pond



Coffeen Power Station

Operational Procedure

X-XXX-XXXX--XXX

Operation & Maintenance Manual for #1 Ash Pond
(Bottom Ash Recycle Pond)

Effective Date: xx/xx/xxxx

Reason for Change: New Procedure

Approved By: _____ x _____ Date: _____ xx/xx/xxxx

x
John Romang

Responsible Department: Coffeen Power Station, Technical Services Department

- This entire document shall be in the field during procedure performance.
 - The following portions of this procedure shall be in the field during procedure performance: _____
 - _____ from this procedure shall be in the field during procedure performance.
 - No part of this procedure is required to be in the field during procedure performance.
-

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- 1.0 Purpose
 - 1.1 This procedure is intended to ensure the safe and environmentally responsible operation and use of the #1 Ash Pond (Bottom Ash Recycle Pond) at the Coffeen Power Station. The primary purpose of the #1 Ash Pond is for the removal of bottom ash by settling and the recirculation of slag tank water. The pond is used to supply water to the Unit 1 and Unit 2 ash handling systems via the recycle pumps.
- 2.0 Scope
 - 2.1 This procedure applies to all onsite personnel and the Dam Safety Group staff.
- 3.0 Responsibilities
 - 3.1 Outside Unit Operator – Checks the pond level and screens once a shift. Operates the facilities as described in this Operational Procedure. Reports any conditions noted during routine activities to the Shift Supervisor and Chemistry Department. Writes job requests if a problem is identified.
 - 3.2 Shift Supervisor (SS) - Calls the Chemistry Department when structural concerns or overflow conditions are reported. Make entries into the shift electronic log book (e-log) indicating the concern and actions taken.
 - 3.3 Dam Safety Inspector - Conducts weekly detailed dam safety inspections and provides a report with findings and recommendations. Make entries in e-log indicating the concern and actions taken.
- 4.0 Historical Information
 - 4.1 The #1 Ash Pond was initially constructed to be a mixed ash deposition pond and was put in service in the mid-1960's. It is located east of the Main Building. It is a 23 acre pond with a maximum outer berm height of 41.5 feet above ground surface level (approximately elevation 637.5'). The pond overflow was located on the north east corner of the pond and discharged into the flume.
 - 4.2 The #1 Ash Pond was converted to act as a closed loop system in the late 1970's when the dewatering bins were installed. The mixed ash was removed and deposited into the #2 ash pond during the closure of #2 pond. The #1 Ash Pond berms were modified and an inner berms was added to the pond to aid in dropping out bottom ash solids. Exterior berm elevation is approximately 637.5 feet.

- 4.3 The #1 Ash Pond was equipped with an emergency overflow at the outlet structure. When the pond level reaches approximately 6.5 feet from the top of the berm, it will overflow into the flume. Overflow will be reported to the EPA. In 2011 there was an assessment of the overflow pipe which showed no obstructions or damage.
- 4.4 In 2006, the bottom ash system was modified to directly sluice bottom ash into the pond, bypassing the retired dewatering bins. Bottom Ash is removed from the pond via an outside contractor on an as needed (typically daily) basis.

5.0 Water Supply

All water inlets to the pond are located on the west side of the pond.

The ash sluice lines (from the valve house) discharge to the pond. These lines are used to convey ash from the slag handling system to the #1 Ash Pond. These lines are the southern most of the pond inlets. HPSW system is routed to the pond (valve house sparger valves, floor drains at Unit 1 cyclone level).

The Slag Tank Overflow sump pumps discharge into the pond at the concrete culvert located directly east of the of the lime/soda ash silo. Also in this area, a small stainless line extends thru the concrete. This is the discharge of the sludge pumps at the Waste Treatment System in the Recycle Pump House building.

The recycle pump flow control valves discharge to the pond through a line located at the northwest corner of the pond. Also in the vicinity of this line is the discharge pipe of the recycle pump house sump pumps.

Water from the Unit 1 and Unit 2 oil water separators are typically routed to the pond via the Slag Tank Overflow Pump (STOP) House sumps. Water entering these sumps are floor and roof drains in the plant and the yard area immediately to the north of the main building.

6.0 Operations Requirements

Pond Level - Plant personnel shall monitor the level of the #1 Ash Pond on a daily basis. Pond level is maintained at approximately 1.0' to 1.5' at the water level staff gauge located on the pond side of the screens. The staff gauge has elevation 629.0' as the 0 elevation.

At 2.0' water level (elevation 631.0 feet), the pond overflows resulting in a sampling and analysis requirement for Total Suspended Solids and Oil and Grease with reporting of the results to the IEPA. If the pond is found at or above 2.0' on the pump side staff gauge, contact Chemistry immediately.

Water can be added to the pond from either the Unit 1 or Unit 2 Low Pressure Service Water (LPSW) headers via piping that discharges to the slag tank overflow trench.

Water can be drained from the pond via the water supply pipe to the dewatering bins. Opening this valve drains water from the recycle header which will remove water from the #1 Ash Pond.

Recycle Pump Intake Structure – Suction to the recycle pumps is supplied from the intake structure located at the west end of the north leg of the pond. This is the only water discharge point from the pond. Water level staff gauges are located upstream and downstream of the trash screens for determination of the screen differential. At 0.5' differential, the screens should be cleaned. Level sensors are also installed upstream and downstream of the screens. Digital displays of the upstream and downstream levels are located along the north side of the catwalk leading out to the screen enclosure. These level sensors will generate a high screen differential alarm in the Control Room DCS. Check screen differential (should clean screens at 6 inches differential.) When the screens become plugged, suction to the recycle pumps is reduced. Call shift supervisor to report if screens needs to be cleaned.

Oil Boom – Plant personnel shall monitor the oil boom that is provided upstream of the intake structure. Check condition of oil booms across pond, at discharge, and across pond inlet. Booms should be replaced when they become oil saturated or damaged. Also check that booms have not come unattached from one another. Write JR to change out booms or to reconnect booms when required.

Emergency Conditions – If a condition arises where there is a possibility of an embankment failure, then the following procedures will be followed:

1. Notify the Supervising Engineer Dam Safety immediately.

7.0 Dam Safety Requirements

- 7.1 Dam Safety Inspections - The plant's impoundment and flood prevention structures shall be inspected and maintained in a manner to ensure safe and environmentally responsible operations. A regular maintenance program shall be performed and shall consist of the following inspection items:

1. Earth embankments: Walk the crest, side slopes, and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides, and animal burrows. Frequency of inspection: Weekly.
2. Vegetation: Grass should be a thick vigorous growth to stabilize the earth embankment soils and prevent erosion from occurring. There should be NO trees on the earth embankment and none within a minimum of 20 feet of the embankment toe or other structures. Mowing frequency: Semiannually.
3. Well Readings: Record level of wells on the crest and toe of the berm. Frequency: Quarterly.
4. Special Inspections – Special inspections of the levees and ash pond berms shall be performed after earthquakes, floods, water level exceedance in the ponds, or heavy rainfall events. Inspection and report shall be equal to an annual inspection level of detail. Water level in the pond should be noted after a heavy rainfall. Dam Safety staff shall accompany plant personnel on special inspections. Frequency: As required.

8.0 Maintenance Log

- 8.1 Dam & Berm Inspector shall enter on e-log under the Dam Safety tab all weekly inspections, any usual occurrences, and maintenance performed.

9.0 Contact Numbers

Plant Environmental Supervisor: John Romang / 217-534-7629

Plant Dam & Berm Inspector: Vito Passariello / 217-534-7664

Plant Control Room: 217-534-7668 / 217-534-7669

Supervising Engineer Dam Safety: Steve Bluemner / 314-554-6298

Dam Safety Staff Contact: Mike Wagstaff / 314-554-6296

10.0 References



**Appendix F: Operation and Maintenance Manual, Gypsum Management Facility Operation
(2015)**

Operation and Maintenance Manual
Coffeen Energy Center
Gypsum Management Facility
Montgomery County, Illinois

IDNR Permit # DS2014019
Dam Permit # IL50578 & IL50579

Prepared For:

ILLINOIS POWER GENERATING COMPANY
Coffeen Energy Center
134 CIPS Lane
Coffeen, Illinois 62017

Prepared By:

HANSON PROFESSIONAL SERVICES INC.
1525 South Sixth Street
Springfield, Illinois 62703

Amended By:

DYNEGY OPERATING COMPANY
1500 Eastport Plaza Drive
Collinsville, Illinois 62234

Original: February 2008
Amended: March 2015

**OPERATION AND MAINTENANCE MANUAL
COFFEEN ENERGY CENTER
GYPSUM MANAGEMENT FACILITY
MONTGOMERY COUNTY, ILLINOIS**

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SECTION 1.0 **GENERAL**

This operation and maintenance (O&M) manual outlines objectives, proposed policies, responsibilities, and procedures for Coffeen Energy Center personnel who are responsible for the management of the Coffeen Energy Center Gypsum Management Facility (GMF). The GMF incorporates two reservoirs, the Gypsum Pond and the Recycle Pond, for processing and storing gypsum.

1.1 REASONS FOR DEVELOPMENT AND DISSEMINATION OF THE O&M MANUAL

The State of Illinois Rivers, Lakes and Streams Act, (615 ILCS 5) Paragraph 23a includes the statement "The Department is authorized to carry out inspections of any dam within the State, and to establish standards and issue permits for the safe construction of new dams and the reconstruction, repair, operation and maintenance of all existing dams." (emphasis added).

Part 3702 of Section 17 of the Illinois Administrative Code, Chapter I entitled the "Construction and Maintenance of Dams" details the requirements to obtain a permit for the construction, operation, and maintenance of a dam. Section 3702.40 b) includes the following statements:

"4) An applicant for a Class I or II dam shall submit an operational plan specifying the method and schedule for the operation of the dam and the routine operating procedures to keep the dam in good working order, including an emergency warning plan." and

"5) As a condition of each permit, the dam owner shall submit a maintenance plan detailing the procedures and schedules to be followed to maintain the dam and its appurtenances in a reasonable state of repair."

Thus, it is a requirement of all dam owners who have dams which fall under the jurisdiction of the Illinois Department of Natural Resources Office of Water Resources (IDNR-OWR) to operate and maintain them safely.

As a dam owner, Illinois Power Generating Company (IPGC) Coffeen Energy Center is responsible for the safety of the public and for maintaining the structures at the facility for both safety and economy. The overall public interest is served by providing a document to serve as a basis for the safe and economical operation and maintenance of the dam during both emergency and day-to-day conditions.

1.2 GENERAL RESPONSIBILITIES CONCERNING DAMS

IPGC is responsible for the operation and maintenance of the Gypsum Pond Dam and the Recycle Pond Dam. These responsibilities include general maintenance (mowing, removing debris from decants, placing riprap where needed, etc.), operation, inspection and emergency action decisions.

SECTION 2.0 **DEFINITIONS**

Appurtenant Works - The structures or machinery auxiliary to dams which are built to operate and maintain dams; such as outlet works, spillways, gates, valves, channels, etc.

Boil - A stream of water discharging from the ground surface downstream of the dam carrying with it a volume of soil which is distributed around the hole formed by the discharging water.

Berm - A horizontal step or bench in the sloping profile of an embankment dam.

Breach - A break, gap, or opening (failure) in a dam which releases impoundment water.

Dam - A barrier built for impounding or diverting the flow of water.

Dike (Levee) - An embankment, usually applied to embankments or structures built to protect land from flooding.

Drain, Layer or Blanket - A layer of pervious material in a dam to facilitate the drainage of the embankment including such items as a toe drain, a weephole, and a chimney drain.

Drawdown - The resultant lowering of the water surface level due to the release of water from the impoundment.

Embankment - Fill material, usually rock or earth, placed with sloping sides.

Earthen Dam - Any dam constructed of excavated natural materials.

Emergency Action Plan - A predetermined plan of action to be taken to reduce the potential for property damage and loss of lives.

Failure - An incident resulting in the uncontrolled release of water from the dam.

Freeboard - The vertical distance between a stated water level and the top of the dam.

Gate or Valve - In general, a device in which a leaf or member is moved across the waterway to control or stop the flow.

Groin - The junction of the upstream or downstream face of the dam with the valley wall.

Maintenance - The upkeep, involving labor and materials, necessary for efficient operation of dams and their appurtenant works.

Operation - The administration, management, and performance needed to operate the dam and appurtenant works.

Operation and Maintenance Inspection - Inspections conducted by the dam operator. These inspections are frequent visual "Walk-around" inspections of the dam surface and appurtenant works.

Outlet - An opening through which water can freely discharge for a particular purpose from an impoundment.

Phreatic Surface - The upper surface of saturation in an embankment.

Piping - The progressive development of internal erosion by seepage, appearing downstream as a hole or seam, discharging water that contains soil particles.

Riprap - A layer of large stones, broken rock or precast blocks placed in a random fashion usually on the upstream slope of an embankment dam, on a reservoir shore, or on the sides of a channel as a protection against wave and ice action.

Silt/Sediment - Soil particles and debris in an impoundment.

Slump/Slide Area - A portion of earth embankment which moves downslope, sometimes suddenly, often with cracks developing.

Spillway System - A structure or structures over or through which flows are discharged. If the flow is controlled by gates, it is considered a controlled spillway. If the elevation of the spillway crest is the only control of the flows, it is considered an uncontrolled spillway.

Emergency Spillway - A spillway designed to operate very infrequently, only during exceptionally large floods, usually constructed of materials expected to erode slowly.

Principal Spillway - The main spillway which controls both normal and flood flows and is usually constructed of non-erodable materials.

Auxiliary Spillway - A spillway which works in conjunction with the principal spillway to control flood flows and is usually constructed of non-erodable materials.

Stilling Basin - A basin constructed to dissipate the energy of fast flowing water, such as from a spillway, and to protect the streambed from erosion.

Toe of Embankment - The junction of the face of the dam with the ground surface in the floodplain upstream or downstream of the dam.

SECTION 3.0 **INFORMATION ABOUT THE DAMS**

3.1 LOCATION

The Gypsum Pond Dam and Recycle Pond Dam are located in the NW 1/4 of Section 11, Township 7 North, Range 3 West of the Third Principal Meridian in Montgomery County, Illinois. More specifically, the dams are located approximately 1.5 miles south of Coffeen, Illinois. A map showing the location of the dams is included in Appendix A.

3.2 DESCRIPTION OF DAM AND APPURTENANCES

The gypsum pond perimeter earthen dam, the gypsum pond “gypsum” dam, and the recycle pond dam will all be regulated in accordance with 17 Illinois Administrative Code (IAC) Part 3702, Construction and Maintenance of Dams. The gypsum pond perimeter earthen dam, which will be lined with a dual high density polyethylene (HDPE) geomembrane system, will have a maximum embankment height of 13 ft and a maximum impounding capacity of 442 acre-ft (measured at the top of earthen dam elevation 632 ft). There will be an additional 123 acre-ft of incised storage. The total volume of gypsum stored within the completed gypsum pond dams will be approximately 2,478 acre-ft.

The dam for the recycle pond, which will be lined with a 60 mil HDPE geomembrane, will have a maximum embankment height of 16 ft and a maximum impounding capacity of 243 acre-ft (measured at the top of dam elevation 629 ft). There will be an additional 99 acre-ft of incised storage.

The gypsum pond will be divided into two sub-cells for the containment of scrubber sludge (gypsum). Discharges to the site will switch back and forth between the two sub-cells so that one sub-cell can be dewatered and raised while the other is in use. There will be two fixed decant pipes constructed in the gypsum stack – one for each sub-cell - which will discharge to stilling wells located adjacent to the perimeter ditches. The control elevation on the decant pipes will be maintained 5.0 ft below the lowest point on the stack cell crest. The decant pipes will enable the cells to be dewatered after storm events so that a minimum of 5.0 ft of freeboard will be maintained in each cell. A minimum of 4.7 ft of freeboard is required above the decant inlet to contain the Probable Maximum Flood (PMF) storm event in addition to peak wind generated waves.

The gypsum pond dam perimeter ditches will be located on the interior sides of the earthen dam. Runoff from the stack will be conveyed through the ditches to a transfer channel which will discharge into the recycle pond. The ditches will be trapezoidal in shape with a 15 ft bottom width, a maximum depth of 9 ft and a longitudinal slope of 0.0005 ft/ft. Side slopes will be 3H:1V. During operation, the ditches will be monitored for erosion. If erosion of the designed ditch geometry occurs, a geogrid will be used for stabilization.

The transfer channel between the gypsum pond dam and the recycle pond have a trapezoidal cross-section with 3H:1V side slopes will be lined with HDPE. The 500 ft long transfer channel will transition from a 32-ft bottom width at an invert elevation of 623.0 ft at the upstream end to a 60-ft bottom width at an invert elevation of 622.0 ft at the downstream end. The transfer channel will be fitted with stop logs capable of raising the discharge control elevation to 625.0 ft. To prevent degradation of the HDPE liner due to flow velocities, the transfer channel and a portion of the recycle pond dam will incorporate an additional sacrificial layer of HDPE.

The emergency spillway for the recycle pond will consist of three 6 ft by 6 ft precast reinforced concrete risers (drop inlets) with a top elevation of 624 ft (5 ft below the top of the dam). The recycle pond's HDPE liner will attach to the exterior sides of each riser. A 4-ft diameter HDPE outlet conduit will be constructed at each riser with an upstream invert of 615.0 ft and a downstream invert of 613.0 ft. Assuming a normal pool elevation of 624 ft (control elevation of the risers), the emergency spillway has been designed to pass the 24-hour PMF storm event with adequate freeboard to prevent overtopping of the recycle pond crest by wind generated waves. The emergency spillway has been provided in the event of accident or catastrophic rainfall only. It is not expected to be activated during the life of the facility. As designed, all discharges from the system will be through the pump house located on the southeast corner of the recycle pond.

3.3 SIZE AND HAZARD CLASSIFICATION

If a worst case failure of the gypsum pond dam were to occur, and the entire volume of the stack is released easterly into Coffeen Lake, the Coffeen Lake reservoir has adequate freeboard to accept this additional volume without overtopping the dam during flood events up to and including the 60 percent PMF. However, the power plant and several residences could potentially be impacted if the gypsum stack dam were to fail in a westerly direction. Considering the regulatory criteria established in Part 3702, the gypsum stack perimeter earthen dam and the gypsum stack "gypsum" dam are classified as intermediate-size Class I (high hazard potential) dams.

A failure of the recycle pond dam would discharge water to Coffeen Lake but it is not anticipated to result in loss of life or any significant economic damage. Breach analyses indicate that a failure of the recycle pond dam during a PMF event would be expected to result in an increase in the Coffeen Lake water surface elevation of not more than ½ inch. Accordingly, the recycle pond dam is classified as a small-size Class III (low hazard potential) dam.

3.4 PURPOSE OF THE DAMS

The dams will be used to dewater, store and dispose of flue gas desulphurization sludge (gypsum) from the Coffeen Power Station (the Plant). Gypsum will be transported to the Gypsum Pond Dam in slurry form (approximately 20 percent solids) and allowed to settle. Clarified process water will then be decanted to the recycle pond and returned to the Plant for reuse via a pipeline.

3.5 PERTINENT DATA

Pertinent data about the dams, appurtenant works, and reservoirs are presented in Table 3-1 and Table 3-2.

Table 3-1 Pertinent Data for the Gypsum Pond Earthen Dam
(Based on the Construction of 2 Gypsum Cells)

Perimeter Ditches			Transfer Channel		
Bottom Width	15.00	feet	Bottom Width	32.00	feet
Top Width	73.50	feet	Top Width	86.00	feet
Depth	9.00	feet	Depth	9.00	feet
Outer Side Slope	3:1	H:V	Upstream Invert	623.00	feet
Inner Side Slope	3:1	H:V	Downstream Invert	622.00	feet
Upstream Invert	624.85	feet	Weir Elevation	625.00	feet
Downstream Invert	623.00	feet	Weir Length (at 2 ft height)	44.00	feet
Ditch slope	0.00050	ft/ft			
Bank Full Cross-sectional Area	378.00	sf	Dam		
Length of Each Ditch (Centerline)	3710.00	feet	Top of Dam Elevation	632	feet
Bank Full Volume of Each Ditch	32.19	acre-ft	Reservoir Surface Area	77.29	acres
Total Ditch length (Centerline)	7420.00	feet	Total Watershed Area	77.29	acres
Total Ditch Bank Full Volume	64.39	acre-ft	Dam Length	7720	feet
			Dam Height	13	feet
1.0 PMF Storm Event			0.5 PMF Storm Event		
Storm Duration	24	hours	Storm Duration	24	hours
Peak Outflow Discharge	1100.7	cfs	Peak Outflow Discharge	541.1	cfs
Total Discharge Volume	228.83	acre-ft	Total Discharge Volume	122.41	acre-ft
Peak WSEL in Perimeter Ditches	629.89	feet	Peak WSEL in Perimeter Ditches	628.23	feet
Freeboard over Max WSEL	2.11	feet	Freeboard over Max WSEL	3.77	feet
Wave Runup/Wind Setup	2.06	feet	Wave Runup/Wind Setup	2.06	feet
Adequate Freeboard?	YES		Adequate Freeboard?	YES	
100-yr Storm Event - Critical Duration			100-yr Storm Event - 24 Hour Duration		
Storm Duration	12	hours	Storm Duration	24	hours
Peak Outflow Discharge	92.6	cfs	Peak Outflow Discharge	62.9	cfs
Total Discharge Volume	50.91	acre-ft	Total Discharge Volume	57.01	acre-ft
Peak WSEL in Perimeter Ditches	626.07	feet	Peak WSEL in Perimeter Ditches	625.84	feet
Freeboard over Max WSEL	5.93	feet	Freeboard over Max WSEL	6.16	feet
Wave Runup/Wind Setup	2.06	feet	Wave Runup/Wind Setup	2.06	feet
Adequate Freeboard?	YES		Adequate Freeboard?	YES	

Note: The Critical Storm Duration is the duration of the rainfall event which produces the highest reservoir water surface elevation in the Gypsum Stack Perimeter Ditches for the given storm frequency. In each case, the starting normal pool elevation of the Recycle Pond is considered to be at elevation 624 ft.

Table 3-2 Pertinent Data for the Recycle Pond Dam
(Based on the Construction of 2 Gypsum Cells)

Dam			3 Spillways- 6ft x 6ft inlet w/ 4ft dia outlet pipe		
Top of Dam Elevation	629	feet	Weir Length	22	feet
Invert of Reservoir Elevation	605	feet	Weir Elevation	624.00	feet
Reservoir Area at Invert	11.55	acres	Outlet Conduit Length	120	feet
Reservoir Area at Top of Dam	17.07	acres	Outlet Conduit Diameter (Inside)	48	inch
Total Reservoir Volume	341.91	acre-ft	Upstream Invert	615	feet
Volume at Elevation 624 ft	259.60	acre-ft	Downstream Invert	614	feet
Total Watershed Area	94.36	acres	Outlet Conduit Slope	0.00833	
Dam Length	3600	feet			
Dam Height	16	feet			
1.0 PMF Storm Event - Normal Pool at Elev. 624 ft			1.0 PMF Storm Event - Normal Pool at Elev. 609 ft		
Storm Duration	24	hours	Critical Storm Duration	24	hours
Peak Inflow	1261.6	cfs	Peak Inflow	1261.6	cfs
Peak Outflow	586.9	cfs	Peak Outflow	289.7	cfs
Peak Storage	315.47	acre-ft	Peak Storage	280.65	acre-ft
Peak WSEL (HEC-HMS)	627.45	feet	Peak WSEL (HEC-HMS)	625.34	feet
Freeboard over Peak WSEL	1.55	feet	Freeboard over Peak WSEL	3.66	feet
Wave Runup/Wind Setup	1.20	feet	Wave Runup/Wind Setup	1.20	feet
Adequate Freeboard?	YES		Water Released from Dam?	YES	
0.5 PMF Storm Event - Normal Pool at Elev. 624 ft			0.5 PMF Storm Event - Normal Pool at Elev. 613 ft		
Storm Duration	24	hours	Critical Storm Duration	24	hours
Peak Inflow	608.4	cfs	Peak Inflow	608.4	cfs
Peak Outflow	413.6	cfs	Peak Outflow	0	cfs
Peak Storage	286.48	acre-ft	Peak Storage	255.83	acre-ft
Peak WSEL (HEC-HMS)	625.69	feet	Peak WSEL (HEC-HMS)	623.75	feet
Freeboard over Peak WSEL	3.31	feet	Freeboard over Peak WSEL	5.25	feet
Wave Runup/Wind Setup	1.20	feet	Wave Runup/Wind Setup	1.20	feet
Adequate Freeboard?	YES		Water Released from Dam?	NO	
100-yr Storm Event - Normal Pool at Elev. 624 ft			100-yr Storm Event - Normal Pool at Elev. 619 ft		
Critical Storm Duration	12	hours	Critical Storm Duration	24	hours
Peak Inflow	113.2	cfs	Peak Inflow	76.6	cfs
Peak Outflow	95.8	cfs	Peak Outflow	0	cfs
Peak Storage	269.36	acre-ft	Peak Storage	258.48	acre-ft
Peak WSEL (HEC-HMS)	624.63	feet	Peak WSEL (HEC-HMS)	623.94	feet
Freeboard over Peak WSEL	4.37	feet	Freeboard over Peak WSEL	5.06	feet
Wave Runup/Wind Setup	1.20	feet	Wave Runup/Wind Setup	1.20	feet
Adequate Freeboard?	YES		Water Released from Dam?	NO	

Note: The above variation in normal pool elevations for the Recycle Pond is for the purpose of documenting the water surface elevation which must be maintained in the recycle pond in order to prevent the release of water from the GMF for the above described storm events.

SECTION 4.0 **OPERATIONS ACTIVITIES**

4.1 INTRODUCTION

The operations plan describes the proposed operation of the Coffeen Gypsum Management Facility (GMF) which includes the gypsum pond and the recycle pond.

4.2 SITE OPERATIONS AND PERSONNEL

4.2.1 Site Operations

The GMF will receive gypsum slurry 24 hours per day, seven days per week. Routine facility maintenance and construction activities will generally be conducted during day shift hours. The crest widths for both the gypsum stack earthen dam and the recycle pond dam are 20 ft. In addition, multi-directional ramps are being provided for both structures so that they are readily accessible by inspection, maintenance and gypsum recovery equipment.

The Plant is a restricted access location. Additional fencing around the perimeter of the active sedimentation cells of the gypsum stack and the recycle pond will be erected to prevent unauthorized access to the GMF, which is also under surveillance by security personnel.

4.2.2 Personnel

The proposed GMF will be owned and operated by Ameren Energy Generating Company (Ameren). Corporate offices are located in St. Louis, Missouri. Overall responsibility for the GMF operation lies with Ameren management personnel.

4.3 GYPSUM MANAGEMENT FACILITY STARTUP

The major components of the proposed GMF consist of:

- The gypsum stack dam/impoundment;
- The recycle pond;
- The earthen transfer channel that connects the two structures and through which process water will be decanted from the gypsum stack into the recycle pond; and
- The recycle pond decant and pumphouse through which process water will be returned to the Plant for reuse.

Both the recycle pond and the gypsum stack dam will be constructed before gypsum is placed within the gypsum stack dam/impoundment.

Upon startup, it is likely that the gypsum stack impoundment will have no more than a few feet of water in the bottom to prevent the high density polyethylene (HDPE) geomembrane from moving. The gypsum slurry (approximately 20 percent solids) will be pumped from the

Plant to the gypsum stack via piping. The piping will be HDPE with a suitable pressure rating for the intended hydraulic and static head. The HDPE pipe will discharge the slurry into the impoundment, and gypsum will settle by gravity.

It will take approximately 10 months before the gypsum stack impoundment is filled to elevation 623 ft, the point where process water may begin flowing into the recycle pond via the HDPE-lined earthen channel connecting the two structures. As soon as water begins to fill the recycle pond, it will be pumped back to the Plant for reuse.

4.4 WATER BALANCE

The capacity of the recycle pond has been designed to accommodate all precipitation runoff from the entire gypsum pond/recycle pond area during a 2-week complete maintenance outage at the Coffeen Power Station (the Plant) followed by a 12-week outage of one of the two units. The runoff and excess water accumulated during this time can be stored within the recycle pond without discharging. The design is based on the maximum 3.5 month precipitation that has occurred in the area since 1950. This occurred in April, May, June and half of July, 1957 and consisted of 28.83 inches of rainfall.

The water balance has been carried out for the expected life of the Site. During the first nine or ten months of operation, the water balance is positive, meaning that there is more water entering the gypsum stack/recycle pond system through process water and precipitation than is leaving the system through process water return and evaporation. However, there is 15 ft of freeboard between the pump discharge and the emergency spillway. With proper water-level management, the water surface will remain well below discharge elevation. After this initial startup period, the water balance is negative, meaning that other water sources will need to be continually added to the process water makeup stream to maintain the volume necessary for transport of the gypsum slurry.

The water balance is of particular concern since the entire system is designed to be a closed loop with no discharges. (*As previously noted, the recycle pond has been designed with an emergency spillway, but this is only to protect the structures in the event of an unforeseen accident or catastrophic rainfall event.*) Table 3.5-2 lists the maximum water surface elevation allowed in the recycle pond in order to prevent the discharge of water for the 100-year storm event and the 0.5 PMF storm event.

4.5 GYPSUM MANAGEMENT FACILITY OPERATION

4.5.1 Routine Operations

Gypsum slurry will initially be discharged at the southwestern corner of the gypsum pond impoundment. Settled gypsum will gradually create a plane of material sloping gently towards the north end of the impoundment. Depending on the slope of the settled gypsum, the discharge pipe may be moved to other corners of the impoundment to evenly distribute the material. Care must be taken during the initial filling period so to ensure that the sand layer covering the ring drains is not disturbed. If necessary, the sand may be armored with larger washed aggregate or

the impoundment may be gradually filled with water to cover the sand prior to the discharge of gypsum slurry into the impoundment.

Once the gypsum plane reaches approximately elevation 627 ft (5 ft below the earthen dam crest), a track excavator or similar piece of equipment will be used to create the first gypsum berm and to form the perimeter ditch. Each gypsum berm will be approximately 10 ft in height and will effectively create a two-compartment impoundment within its perimeter. Gypsum for construction of the gypsum berm will be obtained from the settled material on the inside of the berm, creating an inner ditch. Gypsum slurry will then be discharged alternately into the inner ditch of each compartment. Gypsum will settle out into the inner ditch and clarified process water will flood the compartment to a depth of several feet. This water will be decanted to the perimeter ditch by way of an HDPE decant pipe which will discharge to a stilling well located at the toe of the gypsum stack.

As each compartment fills with settled gypsum, the discharge piping will be moved to the alternate compartment. The compartment, or sub-cell, that is not in service will be allowed to dewater and another gypsum berm will be constructed on top of the previous gypsum berm, effectively raising the gypsum stack another 10 ft. This alternating cycle of gypsum discharge, compartment dewatering and berm construction will continue. Gypsum will be deposited in the stack with an average dry density of approximately 74 lb/ft³. Drawing No. C-10201-25 provides a visual description of this process.

4.5.2 Piezometer Installation and Monitoring

The side slopes of the gypsum pond will be constructed with 3:1 side slopes. After consolidation of the settled gypsum over time, the final slopes should approach 3.75:1. The stability of each gypsum pond slope is critically dependent on the location of the phreatic surface which is anticipated to develop within the stack. Ring drains are intended to lower the phreatic surface so that it is located an adequate distance from the surface of the slope in order to maintain slope stability. In order to monitor the phreatic surface within the stack, piezometers will be installed on each side of the gypsum pond. The piezometers will be installed every 15 vertical feet up the slope (45 horizontal feet based on 3:1 side slopes) and will extend to a depth of at least 15 feet below the anticipated phreatic water surface elevation as shown in Figure 4-1. At the time of installation, each piezometer will be labeled with the “critical elevation” corresponding to the anticipated phreatic surface elevation at that location. The anticipated phreatic surface elevation is the water surface elevation which was used in the slope stability analysis of the gypsum pond. The water level in each piezometer will be read and recorded on a monthly basis. If at any time a reading is recorded higher than “critical elevation” for that specific peizometer, the design engineer must be contacted immediately for evaluation of the reading. Any readings above the “critical elevation” may be indicative of improper ring drain function and/or slope instability which could lead to a failure of the gypsum stack. Therefore, it is critical that the piezometers are installed in accordance with the construction plans and specifications and monitored in accordance with this manual. It may be necessary to install additional subdrainage to maintain the phreatic surface at the desired level within the gypsum stack.

Figure 4-1 Anticipated Phreatic Surface in Gypsum Pond

Refer to figure at the end of the report text.

4.6 DAM INSPECTIONS

The inspection program includes two types of dam inspections. The first is regularly conducted by the dam operator and is referred to as an Operation and Maintenance Inspection. The second type of inspection, referred to as the Engineering Inspection, is conducted by a qualified engineer approved by IPGC. All engineering inspection reports must be signed and sealed by an Illinois Registered Professional Engineer.

The dam operator will perform monthly Operation and Maintenance Inspections of the gypsum pond perimeter earthen dam and the gypsum berms and side slopes during the operating life of the structure. During these inspections, the gypsum stack ditches and the transfer channel will also be examined for signs of erosion and liner degradation. The “operating life of the structure” will be considered to cease upon covering of the gypsum with an HDPE/soil cover. Engineering Inspections will be conducted on an annual basis during the operating life of the structure and will continue after covering of the gypsum pond until authorization to abandon the structure is received from IDNR/OWR.

4.6.1 Operation and Maintenance Inspection

Occasional "walk-around" inspections of the dams and appurtenant works are to be made by the dam operator. During these inspections, a checklist of items to be maintained and items to be observed should be recorded. Appendix A provides an example of the Operation and Maintenance Inspection Checklist to be utilized for these inspections. **If any of the following items are found to be unusual or are cause for concern, the Shift Supervisor should be notified and the Emergency Action Plan should be immediately consulted for guidance on an appropriate course of action.**

Frequency: Operation and maintenance inspections will be performed by the dam operator on a monthly basis and also during and after unusual events such as heavy rainfall or an earthquake.

Inspection Items: During each inspection the following items should be noted in particular.

1. Water Level - Maximum reservoir levels as a result of heavy rainfall should be recorded.
2. Earth Embankment - Walk the crest, side slopes and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides, and animal burrows. These are described as follows:
 - Surface Erosion - Removal of vegetative cover by water action or pedestrian or vehicle usage forming deep ruts or gullies.
 - Seepage - The passage of water through and/or underneath the earth embankment abutment and natural groundline or at the contact between the embankment and

outlet works. It can be indicated by cattails or other wet environmental vegetation, erosion, channelization, or slumping on the embankment face.

- Cracks - Deep cracks usually indicate the movement of the dam and/or the foundation and can be in either the longitudinal (along the length of the dam) or transverse (across the dam) directions. Cracking can be an indicator of the beginning of slumps. Shallow cracks may develop during the summer when the surface soils of the embankment become severely dried and are usually of no concern in regard to the safety of the dam.
 - Settlement - Settlement is indicated by depressions or low spots and can be signs of consolidation of the dam or foundation or the loss of material beneath the settlement area.
 - Slumps/Slides - A slow or sudden movement of the earth embankment slope on either face toward the toe of the dam.
 - If seepage indicates the presence of soil particles, or if deep cracks, settlement, slumps, or slides are noticed, a qualified engineer should be contacted immediately for consultation.
 - Animal Burrows - Animal burrows result in a loss of earth embankment material and can provide seepage paths for water through the embankment.
3. Gypsum Embankment - Walk the crest, side slopes and downstream toe of the dam concentrating on surface erosion, seepage, cracks, settlement, slumps, slides and animal burrows. The descriptions for these are the same as for earth embankment.
 4. Vegetation - Grass should be a thick vigorous growth to stabilize the earth embankment soils and prevent erosion from occurring. Note the height of the grass; if greater than 1 foot a mowing of the area should be scheduled before the next inspection. There should be NO trees on the earth embankment and NONE within a minimum of 20 feet of the embankment toes or other structures. The gypsum embankment will not be seeded and is not expected to have any vegetation.
 5. Gypsum Stack piezometers should be inspected for any damage or loss of function. Damaged piezometers must be promptly repaired or replaced since their function is critical to ensuring stability of the gypsum stack.
 6. The water level in each Gypsum Stack piezometer must be measured and recorded during each monthly inspection. If the water level in any piezometer is above the “critical elevation” as discussed in Section 4.5.2 of this plan, the Ameren Technical Services Superintendent should be notified and the **Emergency Action Plan should be immediately consulted for guidance on an appropriate course of action.**

7. Gypsum Pond LD/LCRS Drains - The change in location or amount of flows discharging from the Leak Detection/Leachate Collection Recovery System (LD/LCRS) should be recorded. If a significant change has occurred, a qualified engineer should be contacted for consultation.
8. Gypsum Stack Ring Drains - The change in location or amount of flows discharging from the Ring Drains should be recorded. If a significant change has occurred, a qualified engineer should be contacted for consultation.
9. Gypsum Stack Fixed Decant – Check the alignment and supports for the pipe. Record the amount of flows discharging from the pipe and any erosion or scour around the discharge point.
10. Gypsum Stack Perimeter Ditch – The perimeter ditch should have a consistent prismatic shape for the entire length. Inspect the perimeter ditch for evidence of erosion, sediment deposition and irregularity in channel geometry, especially in the vicinity of siphon, decant or ring drain outfall structures. If irregularities are noted, repairs should be scheduled and completed.
11. Drawdown Facilities - Check to make sure that the drawdown stop logs in the transfer ditch are undamaged, operating well and allowing for the free flow of water over them. Confirm during inspections the valves are opened and closed at least quarterly.
12. Transfer Channel - Check for any debris or other obstructions which may block or restrict the free flow of water. Check for any pools or undulation of the floor of the channel.
13. Recycle Pond Decant - Check for any debris or other obstructions around the Recycle Pond decant which may block or restrict the free flow of water. The emergency dewatering valve should be lubricated. If there is no return water in the pipe, the emergency dewatering valve should be exercised. Record the physical and operating conditions of the system.
14. Recycle Pond Drop Inlet Spillways - Check for any debris or other obstructions around the inlet crest and at the bottom of the drop inlet which may block or restrict the free flow of water. Check for the development of any rusty areas on the concrete, and seepage, cracking, breaking, or spalling of the concrete. Check for settlement or cracking of the crest. Check for any debris in the pipes which may restrict the flow of water. Check for any tears or leaks in the HDPE liner covering the concrete.
15. Recycle Pond Rip Rap Basin - Check for any debris or other obstructions in the riprap basin which may block or restrict the free flow of water. Check to make sure that the rip rap is remaining in a uniform position. Freeze/thaw action or flow over the rip rap may tend to lift or fracture, thus requiring replacement or leveling to maintain the necessary level of protection. NO trees or woody vegetation should be growing through the rip rap.

16. Fences - Check for damage, accumulated debris, operation of gates and locks, and adequacy of locations (this may change with time as people access the area or development occurs in the area).
17. Perimeter - Check the perimeter of the dams for a distance of at least 100 feet beyond the toe for signs of seepage or boils.
18. HDPE Liner – Wherever exposed, the HDPE Liner should be inspected for tears, gouges, protrusions under the liner and abrasion.

Records: A log book of activities occurring at the dam is to be kept current by the dam operator. The log book should be reviewed during the Engineering Inspection. This book should contain at the least the following documentation:

1. Completed operation and maintenance inspection checklists
2. Readings from all piezometers on the Gypsum Stack
3. Additional visual observations
4. A list of maintenance performed
5. A list of any unusual occurrences at the dam
6. Copies of the engineering inspection reports

4.6.2 Engineering Inspection

The engineering inspection is to be conducted by a qualified engineer approved by Ameren. The inspection will provide a thorough evaluation of the dam condition and appurtenances. Appendix B is an example of the inspection report form which is to be utilized for these inspections.

Frequency: The Gypsum Pond Dam is a Class I, High Hazard Potential dam and is to be inspected by an Illinois Registered Professional Engineer at least once per year. The Recycle Pond Dam is classified as a Class III, Low Hazard Potential dams and is to be inspected by an Illinois Registered Professional Engineer at least once every five years.

Inspection Items: The engineer will thoroughly inspect all of the items noted in Section 4.6.1 Operation and Maintenance Inspection.

Records: The Dam Inspection Report form from IDNR-OWR “Guidelines and Forms for Inspection of Illinois Dams” (a copy of which is included in Appendix B), will be completed by the inspecting engineer and will be signed and sealed by an Illinois Registered Professional Engineer. This report will document problem areas and deficiencies; recommend remedial actions for problem areas; and establish time requirements for dealing with the problems. The original report will be retained in Dynegy Operating Company (DOC) files, and a copy of the report will be submitted to the Illinois Department of Natural Resources, Office of Water Resources.

4.6.3 Review of Emergency Action Plan

The emergency action plan should be reviewed annually to assure that all contacts, addresses and telephone numbers are current. Changes in the adjacent land use should also be noted and may dictate the need for revisions to the plan. Changes to the plan should be made as appropriate but only with the concurrence of the Montgomery County Emergency Services and Disaster Agency and of the Illinois Department of Natural Resources, Office of Water Resources. Copies of any revisions should also be forwarded to all personnel and known emergency responders that possess previous versions the plan.

SECTION 5.0
MAINTENANCE ACTIVITIES

Timely repairs are a must after problem areas have been identified. The dam operator is to perform the work required to correct items noted in the operation and maintenance inspections and engineering inspections. Such items include repairing erosion of the gypsum slopes, mowing, seeding, tree and brush removal, replacing rip rap, repairing fences and locks, clearing debris, etc. The maintenance activities specified in the following sections are minimum requirements. NOTE: NO alterations or repairs to structural elements should be made without the assistance of the Ameren Chief Dam Safety Engineer and the concurrence of the Illinois Department of Natural Resources, Office of Water Resources.

Debris: Remove all trash, logs and other debris which may obstruct flow into the principal spillway pipes and drop inlets, or block passage from their discharge channels.

Rip Rap: Replenish rip rap as needed to provide adequate protection against erosion.

Vegetation Control

1. Maintain a good grass cover on the embankment by seeding, fertilizing and mulching areas which are refilled, barren, or thinly vegetated. Seeding mixtures used for maintenance reseeding shall result in a cover compatible with adjacent cover. The seeding mixture specified at the time of the dam's construction was IDOT Standard Specifications Class 1A (Salt Tolerant Lawn Mixture) as follows:

IDOT Class 1A Salt Tolerant Lawn Mixture

Bluegrass	60 lb/acre
Perennial Ryegrass	20 lb/acre
Dawsons Red Fescue.....	20 lb/acre
Scaldis Hard Fescue	20 lb/acre
Fults Salt Grass	60 lb/acre

2. Grassed areas such as the embankment and the areas beyond the embankment toes for a distance of at least 20 feet should be mowed at least twice annually or at any time the height of the grass exceeds 1 foot.
3. All erosion areas will be filled and compacted, reseeded, fertilized and mulched to establish a thick erosion resistant cover.
4. Remove all trees and brush growing on the dam embankment to prevent development of a root system which could provide seepage paths. Herbicides utilized for tree and brush control are discussed in Appendix D.
5. Keep the riprap basin clear of weeds, brush, and trees.

6. Clear all brush and trees for a distance of approximately 20 feet beyond the toe of each dam.

Animal Damage: Fill rodent holes and other animal burrows with compacted clayey soil and reseed. If rodents become a nuisance, an effective rodent control program as approved by the Illinois Department of Natural Resources District Wildlife Biologist should be implemented.

Signs: All warning signs shall be maintained (repaired, painted, or replaced) as needed.

Gypsum Slopes: Erosion of the gypsum slopes will be evident with the presence of erosion rills. Erosion rills should be filled with additional gypsum material and graded to conform with the design slope.

Piezometers: All piezometers on the gypsum stack shall be inspected for signs of damage or displacement. Non-functioning piezometers shall immediately be replaced.

APPENDIX A
LOCATION MAP

APPENDIX B
OPERATION AND MAINTENANCE INSPECTION CHECKLIST

OPERATION AND MAINTENANCE INSPECTION CHECKLIST

Dam Name (circle one): Gypsum Pond Dam Recycle Pond Dam

Date: _____ Time: _____

Name of Inspector: _____

Reservoir Elevation: _____ feet

<u>ITEM</u>	<u>NO</u>	<u>YES</u>	<u>IF YES</u>
Record Piezometer Readings for Gypsum Stack. Are any readings above the critical level? (see section 4.5.2 of O&M Manual)			Contact Manager, Environment & Chemistry and notify Hanson Professional Services
Note the condition of the Piezometers on the Gypsum Stack. Any damage?			Contact Manager, Environment & Chemistry
Deep Surface Cracks			Contact Manager, Environment & Chemistry
Slump or Slide on the upstream or downstream face			Contact Manager, Environment & Chemistry
Erosion from runoff, wave action or traffic			Repair and stabilize
Embankment, abutment or spillway seepage			Contact Manager, Environment & Chemistry
Seepage or flows of muddy water			Contact Manager, Environment & Chemistry
Uneven settlement			Contact Manager, Environment & Chemistry
Trees, brush or burrow holes on the embankment or in the riprap basin			Remove trees and brush, fill holes
Transfer channel or Spillway pipes blocked			Clear immediately
Damage to stop logs			Repair or replace
Damage to HDPE Liner			Repair and schedule engineer inspection
Settlement or displacement of Gypsum Pond fixed decant pipes or outlets			Schedule engineer inspection
Discharge from Gypsum Pond LD/LCRS Drains?			Record discharge rate for each outlet (time to fill bucket)
Discharge from Gypsum Pond Ring Drains?			Record discharge rate for each outlet (time to fill bucket)
Gypsum Stack Perimeter Ditch erosion			Schedule repair
Problems with Recycle Pond spillways			Contact Manager, Environment & Chemistry
Problems with Recycle Pond decant			Contact Manager, Environment & Chemistry
Height of grass (inches)		inches	If more than 1 foot, schedule mowing
Damage to fencing, gates and locks or other access restriction measures			Contact Manager, Environment & Chemistry
Confirm drawdown facilities are opened and closed at least quarterly.			Contact Manager, Environment & Chemistry

Comments:

APPENDIX C
ENGINEERING INSPECTION FORMS

Dam Inspection Report

Name of Dam _____ Dam Identification Number _____

Permit Number _____ Class of Dam _____

Location NW 1/4 Section 11 Township 7N Range 3W 3rd P.M.

Owner _____
Name Telephone Number (Day)

Street _____ Telephone Number (Night)

City _____ Zip Code _____ County Montgomery

Type of Dam _____

Type of Spillway _____

Date(s) Inspected _____

Weather When Inspected _____

Temperature When Inspected _____

Pool Elevation When Inspected _____

Tailwater Elevation When Inspected _____

Inspection Personnel:

Name Title

Name Title

Name Title

Name Title

Professional Engineer's Seal

The Department of Natural Resources is requesting information that is necessary to accomplish the statutory purpose as outlined under the River, Lakes and Streams Act, 615 ILCS 5 (1994 State Bar Edition). Submittal of this information is REQUIRED. Failure to provide the required information could result in the initiation of non-compliance procedures as outlined in Section 702.160 of the "Rules for Construction and Maintenance of Dams". This form has been approved by the State Forms Management Center.

CONDITION CODES

<u>EC</u>	:	<u>Emergency Condition. A serious dam safety condition exists that needs immediate action. Emergency measures implemented as instructed by Chief Dam Safety Engineer; such as, pool draw down, work stoppage, plant stoppage.</u>
<u>NE</u>	:	<u>No evidence of a problem</u>
<u>GC</u>	:	<u>Good condition</u>
<u>MM</u>	:	<u>Item needing minor maintenance and/or repairs within the year, the safety or integrity of the item is not yet imperiled</u>
<u>IM</u>	:	<u>Item needing immediate maintenance to restore or ensure its safety or integrity. Remediation should be completed within 1 month.</u>
<u>EC</u>	:	<u>Emergency condition which if not immediately repaired or other appropriate measures taken could lead to failure of the dam</u>
<u>OB</u>	:	<u>Condition requires regular observation to ensure that the condition does not become worse</u>
<u>NA</u>	:	<u>Not applicable to this dam</u>
<u>NI</u>	:	<u>Not inspected - list the reason for non-inspection under deficiencies</u>
<u>EC</u>	:	<u>Emergency Condition. A serious dam safety condition exists that needs immediate action. Emergency measures implemented as instructed by Chief Dam Safety Engineer; such as, pool draw down, work stoppage, plant stoppage.</u>

GYP SUM STACK - EARTH EMBANKMENT

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Surface Cracks			
Vertical and Horizontal Alignment of Crest			
Unusual movement or Cracking at or Beyond Toe			
Sloughing or Erosion of Outer Embankment Slopes			
Upstream Face Slope Protection (HDPE Liner)			
Seepage			
Animal Damage			

GYP SUM STACK - EARTH EMBANKMENT

(Continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Vegetative Cover			

GYPSUM STACK - GYPSUM EMBANKMENT

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Surface Cracks			
Vertical and Horizontal Alignment of Crest			
Unusual movement or Cracking at or Beyond Toe			
Sloughing or Erosion of Outside Embankment Slopes			
Sloughing or Erosion of Inside Embankment Slopes			
Seepage			
Animal Damage			

GYPSUM STACK - GYPSUM EMBANKMENT

(Continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Condition of Piezometers on Gypsum Stack			
Piezometer Readings on Gypsum Stack Above Critical Level?			

GYPSUM STACK – PERIMETER DITCH

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Ditch Geometry (15 ft bottom width, 3:1 slopes, 8-9 ft depth)			
Concrete Apron at ring drain outlets			
Ring Drain Discharge Pipes			
Stilling Wells for Fixed Decants			

TRANSFER CHANNEL - (between gypsum stack and recycle pond)

Drop Inlet Structure

Overflow Spillway Structure

Gated

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Debris			
Side Slope Stability			
HPDE Liner			
HDPE Liner Welds			
Stop Logs			
Differential Settlement			

RECYCLE POND - EMBANKMENT

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Surface Cracks			
Vertical and Horizontal Alignment of Crest			
Unusual movement or Cracking at or Beyond Toe			
Sloughing or Erosion of Outer Embankment Slopes			
Upstream Face Slope Protection (HDPE Liner)			
Seepage			
Animal Damage			

RECYCLE POND - EMBANKMENT

(Continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Vegetative Cover			

RECYCLE POND - PRINCIPAL SPILLWAY (Left, Looking Downstream)

Drop Inlet Structure

Overflow Spillway Structure

Gated

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Alignment of Structure Walls			
Construction Joints			
Differential Settlement			
Erosion, Spalling, Cavitation			
Joint Separation			
Seepage Around or into Conduit			
Surface Cracks			

RECYCLE POND - PRINCIPAL SPILLWAY (Left, Looking Downstream)

(Continued)

Drop Inlet Structure

Overflow Spillway Structure

Gated

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Structural Cracks			

RECYCLE POND - PRINCIPAL SPILLWAY (Center)

Drop Inlet Structure

Overflow Spillway Structure

Gated

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Alignment of Structure Walls			
Construction Joints			
Differential Settlement			
Erosion, Spalling, Cavitation			
Joint Separation			
Seepage Around or into Conduit			
Surface Cracks			

RECYCLE POND - PRINCIPAL SPILLWAY (Center)

(Continued)

Drop Inlet Structure

Overflow Spillway Structure

Gated

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Structural Cracks			

RECYCLE POND - PRINCIPAL SPILLWAY (Right, Looking Downstream)

Drop Inlet Structure

Overflow Spillway Structure

Gated

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Alignment of Structure Walls			
Construction Joints			
Differential Settlement			
Erosion, Spalling, Cavitation			
Joint Separation			
Seepage Around or into Conduit			
Surface Cracks			

RECYCLE POND - PRINCIPAL SPILLWAY (Right, Looking Downstream)

(Continued)

Drop Inlet Structure

Overflow Spillway Structure

Gated

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Structural Cracks			

RECYCLE POND - ENERGY DISSIPATOR

Principal Spillway

Outlet Works

Type:

FHWA HEC-14, Riprap Basin

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Riprap			
Outlet Channel			
Debris			

RECYCLE POND - DECANT STRUCTURE

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Alignment			
Connection to Bollard			
Debris in Inlets			
Condition of Pipe			
Condition of Liner Beneath Pipe			
Connection to Ballast			
Connection of Pipe Boot to Liner			

RECYCLE POND - DECANT STRUCTURE

(continued)

ITEM	CONDITION	DEFICIENCIES	RECOMMENDED REMEDIAL MEASURES & SCHEDULE
Seepage Around or into Conduit			

APPENDIX D
HERBICIDES

HERBICIDES

Site personnel should check with the Illinois Department of Natural Resources, Regional Fisheries Biologist and the Regional Wildlife Biologist before using any herbicide. Read the product label prior to use and follow the use directions and precautions accordingly.

On March 1, 1979 the U.S. Environmental Protection Agency (U.S.E.P.A.) halted the use of the herbicide 2, 4, 5-T in parks and recreation areas. The use of silvex (2, 4, 5-TP) around water has also been banned.

The Agronomy Department at the University of Illinois and the Aquatic Biology Section of the Department of Natural Resources, Office of Scientific Research and Analysis indicate that the herbicides containing the 2, 4-D or 2, 4-DP are legal for use in parks and recreation areas and effective for controlling brush and woody growth. Some examples of approved herbicides are:

1. Tordon RTU by DOW Chemical. (Can be obtained with blue dye.)
2. WEEDONE 170 by Union Carbide
3. WEEDONE, 2, 4-DP by Union Carbide
4. A 1% to 2% solution of ROUNDUP
5. Garlon by DOW Chemical
6. Banvel by Sandoz

Your distributor may carry brand name herbicides other than those listed above. Be certain that the product does not contain the ingredients 2, 4, 5-T or 2, 4, 5-TP. An example of an unacceptable product is ESTERON 2, 4, 5 by DOW Chemical.

APPENDIX E
CONSTRUCTION DRAWINGS



Appendix G: Photos of 2015 Sloughing Repairs



Figure G.1. Photo of 2015 sloughing prior to repairs.



Figure G.2. Photo of 2015 sloughing prior to repairs.



Figure G.3. Photo of 2015 sloughing area after repairs.



Figure G.4. Photo of 2015 sloughing area after repairs.

2021 USEPA CCR Rule Periodic Certification Report
Gypsum Management Facility Gypsum Stack Pond, Coffeen Power Plant,
Coffeen, Illinois

October 11, 2021

Illinois Power Generating Company
134 Cips Lane
Coffeen, Illinois 62017

**Subject: USEPA CCR Rule and IEPA Part 845 Rule Applicability Cross-Reference
2021 USEPA CCR Rule Periodic Certification Report
GMF Gypsum Stack Pond, Coffeen Power Plant, Coffeen, Illinois**

At the request of Illinois Power Generating Company (IPGC), Geosyntec Consultants (Geosyntec) has prepared this letter to document how the attached 2021 United States Environmental Protection Agency (USEPA) CCR Rule Periodic Certification Report (Report) was prepared in accordance with both the Federal USEPA CCR Rule¹ and the state-specific Illinois Environmental Protection Agency (IEPA) Part 845 Rule². Specific sections of the report and the applicable sections of the USEPA CCR Rule and Illinois Part 845 Rule are cross-referenced in **Table 1**. A certification from a Qualified Professional Engineer for each of the CCR Rule sections listed in **Table 1** is provided in Section 9 of the attached Report. This certification statement is also applicable to each section of the Part 845 Rule listed in **Table 1**.

Table 1 – USEPA CCR Rule and Illinois Part 845 Rule Cross-Reference

Report Section	USEPA CCR Rule		Illinois Part 845 Rule	
3	§257.73 (a)(2)	Hazard Potential Classification	845.440	Hazard Potential Classification Assessment ³
4	§257.73 (c)(1)	History of Construction	845.220(a)	Design and Construction Plans (Construction History)
5	§257.73 (d)(1)	Structural Stability Assessment	845.450 (a) and (c)	Structural Stability Assessment
6	§257.73 (e)(1)	Safety Factor Assessment	845.460 (a-b)	Safety Factor Assessment
7	§257.82 (a)(1-3)	Adequacy of Inflow Design Control System Plan	845.510(a), (c)(1), (c)(3)	Hydrologic and Hydraulic Capacity Requirements / Inflow Design Flood Control System Plan
	§257.82 (b)	Discharge from CCR Unit	845.510(b)	Discharge from CCR Surface Impoundment

¹ United States Environmental Protection Agency, 2015. *40 CFR Parts 257 and 261, Hazardous and Solid Waste Management System, Disposal of Coal Combustion Residuals from Electric Utilities, Final Rule.*

² State of Illinois, Joint Committee on Administrative Rule, Administrative Code (2021). *Title 35: Environmental Protection, Subtitle G: Waste Disposal, Chapter I: Pollution Control Board, Subchapter j: Coal Combustion Waste Surface Impoundment, Part 845 Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments.*

³ “Significant” and “High” hazard, per the CCR Rule¹, are equivalent to Class II and Class I hazard potential, respectively, per Part 845².

Illinois Power Resources Generating Company

October 11, 2021

Page 2

CLOSING

This letter has been prepared to demonstrate that the content and Qualified Professional Engineer Certification of the 2021 Periodic USEPA CCR Rule Certification Report fulfills the corresponding requirements of Part 845 of Illinois Administrative Code listed in **Table 1**.

Sincerely,



Lucas P. Carr, P.E.
Senior Engineer



John Seymour, P.E.
Senior Principal

**2021 USEPA CCR RULE PERIODIC
CERTIFICATION REPORT
§257.73(a)(2), (c), (d), (e) and §257.82
GMF GYPSUM STACK POND
Coffeen Power Plant
Coffeen, Illinois**

Submitted to

Illinois Power Generating Company

**134 Cips Lane
Coffeen, Illinois 62017**

Submitted by

Geosyntec 
consultants

engineers | scientists | innovators

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October 11, 2021

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Attachment D Periodic Structural Stability and Safety Factor Assessment Analyses
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EXECUTIVE SUMMARY

This Periodic United States Environmental Protection Agency (USEPA) Coal Combustion Residuals (CCR) Rule [1] certification report (Periodic Certification Report) for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GMF GSP)¹ at the Coffeen Power Plant (CPP), also known as the Coffeen Power Station (COF), has been prepared in accordance with Rule 40, Code of Federal Regulations (CFR) §257. herein referred to as the “CCR Rule” [1]. The CCR Rule requires that initial certifications for existing CCR surface impoundment, completed in 2016 and subsequently posted on the Illinois Power Generating Company (IPGC) CCR Website ([2], [3], [4], [5], [6]) be updated on a five-year basis.

The initial certification reports developed in 2016 and 2017 were independently reviewed by Geosyntec ([2], [7], [3], [8], [4], [5], [6]). Additionally, field observations, interviews with plant staff, updated engineering analyses, and evaluations were performed to compare conditions in 2021 at the GMF GSP relative to the 2016 and 2017 initial certifications. These tasks determined that updates are not required for the Initial Hazard Potential Classification. However, due to changes at the site and technical review comments, updates were required and were performed for the:

- History of Construction Report,
- Structural Stability Assessment,
- Initial Safety Factor Assessment, and
- Initial Inflow Design Flood Control System Plan.

Geosyntec’s evaluations of the initial certification reports and updated analyses identified that the GMF GSP meets all requirements for hazard potential classification, history of construction reporting, structural stability, safety factor assessment, and hydrologic and hydraulic control. **Table 1** provides a summary of the initial 2016 certifications and the updated 2021 periodic certifications.

¹ The GMF GSP is also referred to as ID Number W13501250004-03, GMF GSP by the Illinois Environmental Protection Agency (IEPA); CCR unit ID 103 by IPGC; and IL50579 within the National Inventory of Dams (NID) maintained by the Illinois Department of Natural Resources (IDNR). Within this document it is referred to as the GMF GSP.

Table 1 – Periodic Certification Summary

	CCR Rule Reference	Requirement Summary	2016 Initial Certification		2021 Periodic Certification	
			Requirement Met?	Comments	Requirement Met?	Comments
Hazard Potential Classification						
3	§257.73(a)(2)	Document hazard potential classification	Yes	Impoundment was determined to have a High hazard potential classification [2].	Yes	The Initial Hazard Potential Classification (HPC) is conservative due to the consideration of ultimate buildout conditions relative to existing conditions. An update to the Initial HPC is not required at this time but could be performed to potentially reduce the HPC to Significant.
History of Construction						
4	§257.73(c)(1)	Compile a history of construction	Yes	A History of Construction report was prepared for the GMF GSP, Ash Pond 1, Ash Pond 2, and the GMF Recycle Pond [3].	Yes	A letter listing updates to the History of Construction report is provided in Attachment C .
Structural Stability Assessment						
5	§257.73(d)(1)(i)	Stable foundations and abutments	Yes	Foundations were found to be stable. Abutments were not present [8].	Yes	Foundations and abutments were found to be stable after performing updated slope stability analyses.
	§257.73(d)(1)(ii)	Adequate slope protection	Yes	Slope protection was adequate [8].	Yes	No changes were identified that may affect this requirement.
	§257.73(d)(1)(iii)	Sufficiency of dike compaction	Yes	Dike compaction was sufficient for expected ranges in loading conditions [8].	Yes	Dike compaction was found to be sufficient after performing updated slope stability analyses.
	§257.73(d)(1)(iv)	Presence and condition of slope vegetation	Yes	Vegetation was present on exterior slopes and was maintained. Interior slopes had alternate protection (geomembrane liner) [8].	Yes	No changes were identified that may affect this requirement.
	§257.73(d)(1)(v)(A) and (B)	Adequacy of spillway design and management	Yes	Spillways were adequately designed and constructed to adequately manage flow during the probable maximum flood (PMF) [8].	Yes	Spillways were found to be adequately designed and constructed and are expected to adequately manage flow during the PMF, after performing updated hydrologic and hydraulic analyses.
	§257.73(d)(1)(vi)	Structural integrity of hydraulic structures	Not Applicable	Hydraulic structures penetrating the dikes or underlying the base of the GMF GSP were not present. This requirement was not applicable [8].	Not Applicable	No changes were identified that may affect this requirement.
	§257.73(d)(1)(vii)	Stability of downstream slopes inundated by water body.	Not Applicable	Inundation of exterior slopes were not expected. This requirement was not applicable [8].	Not Applicable	No changes were identified that may affect this requirement.
Safety Factor Assessment						
6	§257.73(e)(1)(i)	Maximum storage pool safety factor must be at least 1.50	Yes	Safety factors were calculated to be 3.45 and higher [8].	Yes	Safety factors from updated slope stability analyses were calculated to be 3.45 and higher.
	§257.73(e)(1)(ii)	Maximum surcharge pool safety factor must be at least 1.40	Yes	Safety factors were calculated to be 3.45 and higher [8].	Yes	Safety factors from updated slope stability analyses were calculated to be 3.45 and higher.
	§257.73(e)(1)(iii)	Seismic safety factor must be at least 1.00	Yes	Safety factors were calculated to be 1.47 and higher [8].	Yes	Safety factors from updated slope stability analyses were calculated to be 1.45 and higher.
	§257.73(e)(1)(iv)	For dike construction of soils that have susceptible to liquefaction, safety factor must be at least 1.20	Not Applicable	Dike soils were not susceptible to liquefaction. This requirement was not applicable [8].	Not Applicable	No changes were identified that may affect this requirement.
Inflow Design Flood Control System Plan						
7	§257.82(a)(1), (2), (3)	Adequacy of inflow design control system plan.	Yes	Flood control system adequately managed inflow and peak discharge during the Probable Maximum Precipitation, 24-hr Inflow Design Flood [8].	Yes	The flood control system was found to adequately manage inflow and peak discharge during the Probable Maximum Precipitation, 24-hour Inflow Design Flood, after performing updated hydrologic and hydraulic analyses.
	§257.82(b)	Discharge from CCR Unit	Yes	Discharges into Waters of the United States were not expected to occur during normal and Probable Maximum Precipitation, 24-hr, Inflow Design Flood conditions [8].	Yes	Discharge into Waters of the United States were found to not be expected to occur during both normal and Probable Maximum Precipitation, 24-hour Inflow Design Flood conditions, after performing updated hydrologic and hydraulic analyses.

SECTION 1

INTRODUCTION AND BACKGROUND

This Periodic United States Environmental Protection Agency (USEPA) Coal Combustion Residual (CCR) Rule [1] Certification Report was prepared by Geosyntec Consultants (Geosyntec) for Illinois Power Generating Company (IPGC) to document the re-certification of the GMF GSP at the Coffeen Power Plant (CPP), also known as the Coffeen Power Station (COF), located at 134 Cips Lane in Coffeen, Illinois, 62017. The location of CPP is provided in **Figure 1**, and a site plan showing the location of the GMF GSP, among other closed and active CCR units and non-CCR surface impoundments, is provided in **Figure 2**.

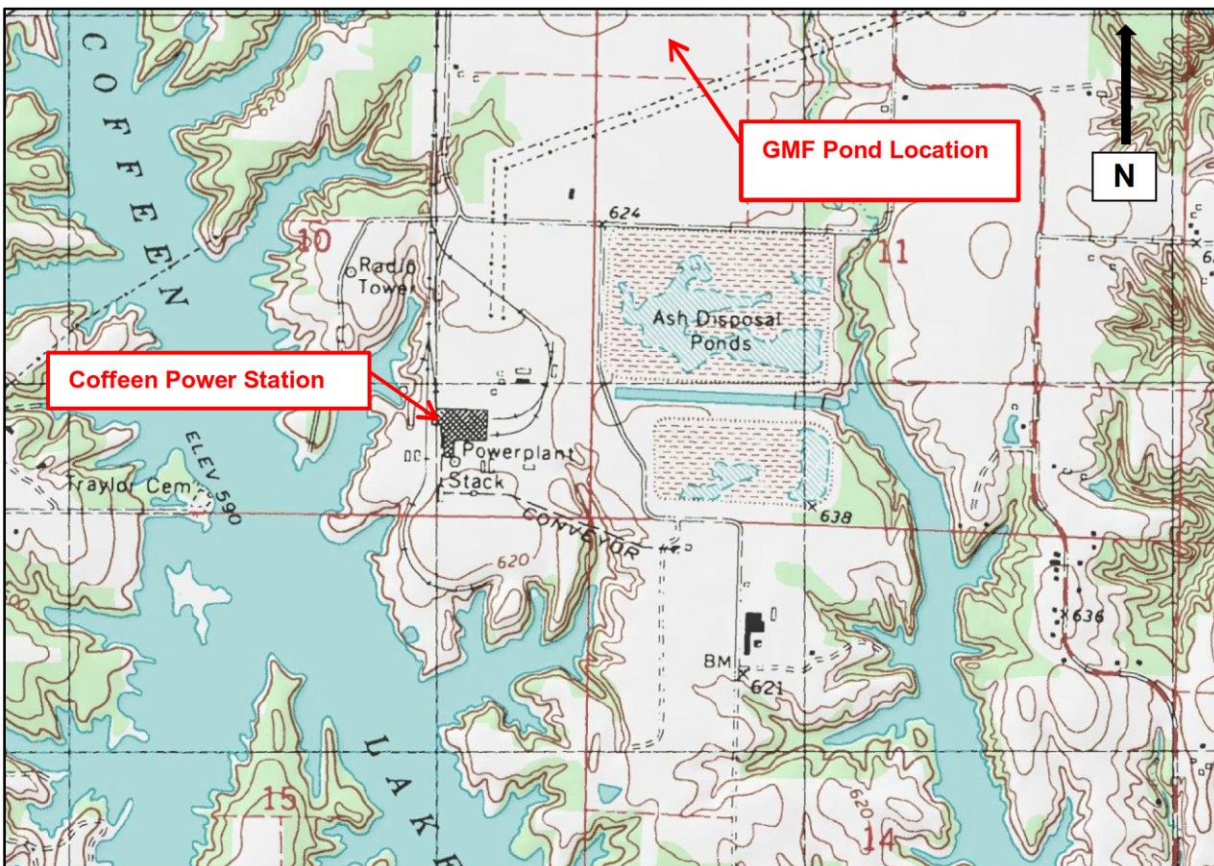


Figure 1 – Site Location Map (from AECOM, 2016)



Figure 2 – Site Plan (adapted from AECOM, 2016)

1.1 GMF GSP Description

CPP was retired in 2019. Prior to retirement, three active CCR surface impoundments – the GMF GSP, the GMF Recycle Pond, and AP1 – and one CCR landfill – were used for managing CCRs generated at CPP. This certification report only pertains to the GMF GSP. The GMF GSP has a High hazard potential, based on the initial hazard potential classification assessment performed by Stantec in 2016 in accordance with §257.73(a)(2) ([2], [7]).

The GMF GSP formerly served as the primary wet impoundment basin for gypsum produced by the wet scrubber system at CPP. The GMF GSP was constructed between 2008 and 2009 and received inflow from two pairs of high-density polyethylene (HDPE) gypsum slurry pipes. Clear water discharge from the GMF GSP flowed downstream into the GMF Recycle Pond via a lined channel (transfer channel) and a 14-in. diameter HDPE low-flow pipe buried beneath the transfer channel. The transfer channel effectively acts as the primary spillway for the GMF GSP, as the bottom elevation of the transfer channel is equal to the adjacent exterior toe elevation of the dike. The transfer channel is approximately 580 ft in length, trapezoidal in shape, lined with 60-mil HDPE, has three horizontal to one vertical (3H:1V) side slopes, and the bottom elevation² decreases from 624 ft at the upstream end to 622 ft at the downstream end.

The 14-in. diameter low-flow pipe has an invert elevation of 619.0 ft at the upstream end and 617.6 ft at the downstream end. A berm was constructed within the transfer channel in 2020 with a crest elevation of approximately elevation 627 ft [9] to retain additional water in the GMF GSP and reduce the pool level in the downstream GMF Recycle Pond. The GMF Recycle Pond formerly acted as a polishing pond, and outflow was pumped to the CPP to be recycled for use in the wet scrubber system [8].

The GMF GSP has a composite liner system that extends up to interior dike crests at elevation 630.5 ft and is present beneath the entire footprint of the pond. The liner system includes a 3-ft thick layer of compacted clay that is overlain by a 60-mil textured HDPE geomembrane. The geomembrane liner is exposed at the pond bottom and side slopes [8].

As formerly operated, the normal pool elevation of the GMF GSP was observed to be 621.2 ft in the 2015 Weaver Consultants survey of the site [10], as controlled by the 14-in. diameter low-level outlet pipe and recycle water inflow and outflow pumping rates [8]. The water elevation in the GMF GSP had increased to 625.2 ft by the time of the periodic survey in December of 2020 [9], due to the construction of the berm in the transfer channel and could rise as high as approximately El. 627 ft due to the berm that was constructed in the transfer channel.

The GMF GSP is approximately 36.2 acres in size and was formed with a continuous embankment, a ring dike, which has a total perimeter length of approximately 5,000 ft. The perimeter dike was constructed to include a crest width of between approximately 15 to 25 ft and a crest height of 5 ft at the north embankment and 9 ft at the east embankment. The interior of the GMF GSP extends deeper than the exterior natural grade, and the maximum interior slope height is approximately 25 ft in the southeast corner of the pond. The elevation of the embankment crest ranges from 631 to 632 ft. Both interior and exterior slopes have 3H:1V orientations [8].

Initial certifications for the GMF GSP for Hazard Potential Classification (§257.73(a)(2)), History of Construction (§257.73(c)), Structural Stability Assessment (§257.73(d)), Safety Factor Assessment (§257.73(e)(1)), and Inflow Design Flood Control System Plan (§257.82) were

² All elevations in the report are in the North American Vertical Datum of 1988 (NAVD88), unless otherwise noted.

completed by Stantec and AECOM in 2016 and 2017 and subsequently posted to IPGC’s CCR Website ([2], [3], [4], [5], [6]). Additional documentation for the initial certifications included detailed operating record reports containing calculations and other information prepared for the hazard potential classification by Stantec [7] and for the structural stability assessment, safety factor assessment, and inflow design flood control system plan by AECOM [8]. These operating record reports were not posted to IPGC’s CCR Website.

1.2 Report Objectives

These following objectives are associated with this report:

- Compare site conditions from 2015/2016 to site conditions in 2020/2021, and evaluate if updates are required to the:
 - §257.73(a)(2) Hazard Potential Classification [2];
 - §257.73(c) History of Construction [3];
 - §257.73(d) Structural Stability Assessment [4];
 - §257.73(e) Safety Factor Assessment [5], and/or
 - §257.82 Inflow Design Flood Control System Plan [6].
- Independently review the Hazard Potential Classification ([2], [7]), Structural Stability Assessment ([4], [8]), Safety Factor Assessment ([5], [8]), and Inflow Design Flood Control System Plan ([6], [8]) to determine if updates may be required based on technical considerations.
 - The History of Construction report [3] was not independent reviewed for technical consideration, as this report contained historical information primarily developed prior to promulgation of the CCR Rule [1] for the CCR units at CPP, and did not include calculations or other information used to certify performance and/or integrity of the impoundments under §257.73(a)(2)-(3), §257.73(c)-(e), or §257.82.
- Confirm that the GMF GSP meets all of the requirements associated with §257.73(a)(2)-(3), (c), (d), (e), and §257.82, or, if the GMF GSP does not meet any of the requirements, provide recommendations for compliance with these sections of the CCR Rule [1].

SECTION 2

COMPARISION OF INITIAL AND PEROIODIC SITE CONDITIONS

2.1 Overview

This section describes the comparison of conditions at the GMF GSP between the start of the initial CCR certification program in 2015 and subsequent collection of periodic certification site data in 2020 and 2021.

2.2 Review of Annual Inspection Reports

Annual onsite inspections of the GMF GSP were performed from 2016 to 2020 ([11], [12], [13], [14], [15]) and were certified by a licensed professional engineer in accordance with §257.83(b). Each inspection report stated the following information, relative to the previous inspection:

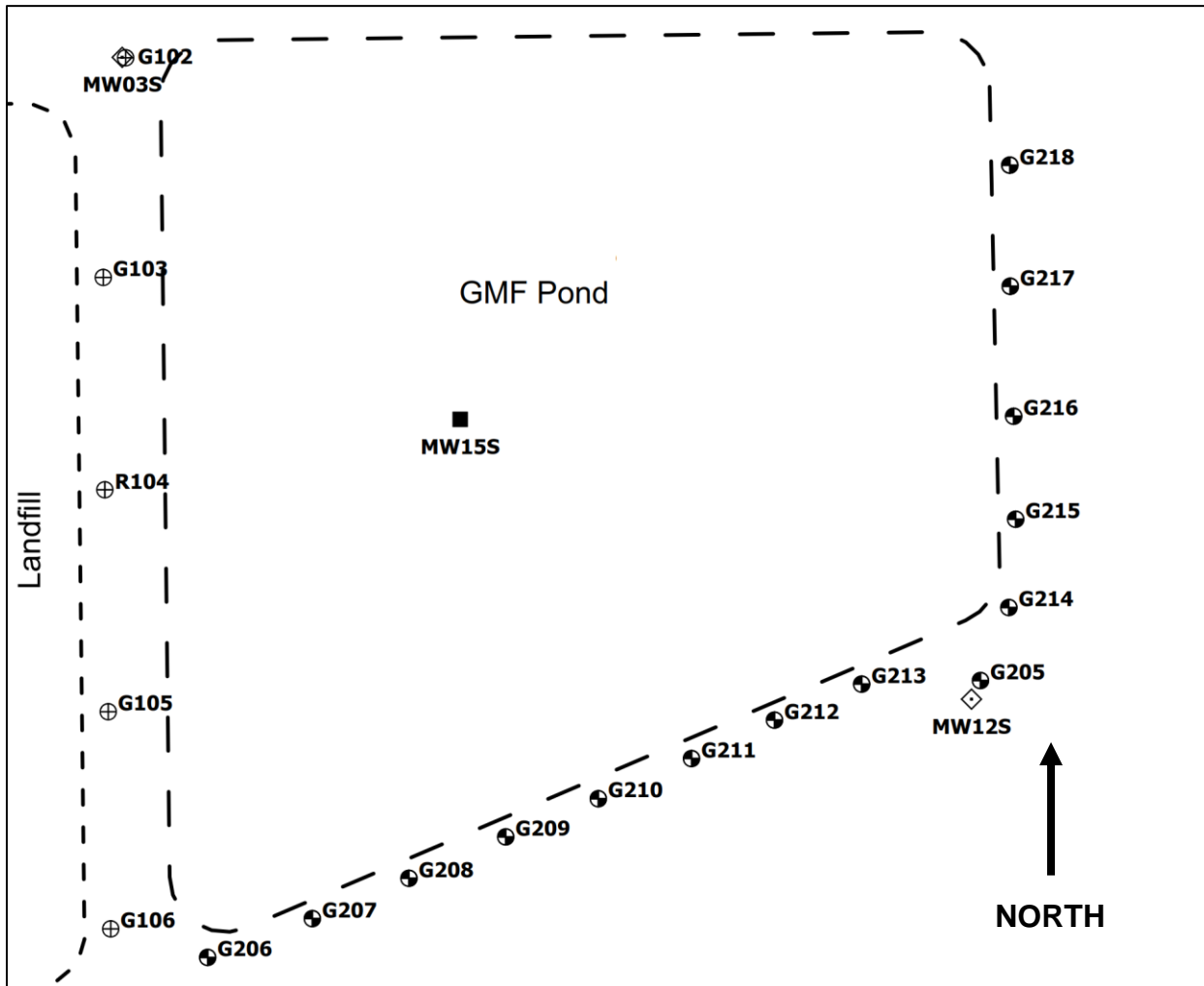
- A statement that no changes in geometry of the impounding structure were observed since the previous inspection;
- A statement that no geotechnical instrumentation was present;
- Approximate volumes of impounded water and CCR at the time of inspection;
- A statement that no appearances of actual or potential structural weakness or other disruptive conditions were observed; and
- A statement that no other changes which may have affected the stability or operation of the impounding structure were observed.

In summary, the reports did not indicate any significant changes to the GMF GSP between 2015 and 2020. No signs of instability, structural weakness, or changes which may have affected the operation or stability of the GMF GSP were noted in the inspection reports.

2.3 Review of Instrumentation Data

Nineteen groundwater monitoring wells, (G102, G103, R104, G105, G106, G205, G206, G207, G208, G209, G210, G211, G212, G213, G214, G215, G216, G217, and G218), are present at the GMF GSP. Groundwater level readings were collected generally on a quarterly basis and provided between February 17, 2016 and January 27, 2021. Geosyntec reviewed the groundwater level data to evaluate if significant fluctuations, partially increases in phreatic levels, may have occurred after development of the initial structural stability and factor of safety certifications ([4], [5], [8]), which utilized phreatic conditions estimated from cone penetration testing (CPT) data. Available water

level readings are plotted in **Attachment A** and **Figure 3** provides approximate locations of the monitoring wells.



**Figure 3 – GMF GSP Monitoring Well Locations
(Not to Scale, adapted from Hanson, 2021)**

In summary, groundwater levels in the monitoring well network were observed to be relatively consistent between individual wells. Water levels were typically no more than 1 to 4 ft different between individual wells and seasonal fluctuations were on the order of 1 to 4 ft. Water levels ranged from a low of El. 617 ft to a high of El. 627 ft, resulting in a total fluctuation of 10 ft. These water levels are approximately 1 to 3 ft higher than water levels utilized in the slope stability analyses prepared to support the initial structural stability and safety factor assessments ([4], [5], [8]).

The water levels in the initial assessments were based on cone penetration testing (CPT) pore pressure dissipation (PPD) testing collected at a discrete point in time (August 2015) and are

therefore less representative of long-term groundwater trends than the water level data collected from monitoring wells.

2.4 Comparison of Initial to Periodic Surveys

The initial survey of the GMF GSP, conducted at the site by Weaver Consultants (Weaver) in 2015 [10], was compared to the periodic survey of the GMF GSP, conducted by IngenAE, LLC (IngenAE) in 2020 [9], using AutoCAD Civil3D 2021 software. This comparison quantified changes in the volume of CCR placed within the GMF GSP and considered volumetric changes above and below the starting water surface elevation (SWSE) used for the 2016 \$257.82 inflow design flood control plan hydraulic analysis [6]. Potential changes to embankment geometry were also evaluated. This comparison is presented in a side-by-side comparison of the surveys in **Drawing 1** and a plan view isopach map denoting changes in ground surface elevation in **Drawing 2**. A summary of the water elevations and changes in CCR volumes is provided in **Table 2**.

Table 2 – Initial to Periodic Survey Comparison

Initial Surveyed Pool Elevation (ft)	621.2
Periodic Surveyed Pool Elevation (ft)	625.2
Initial \$257.82 Starting Water Surface Elevation (SWSE) (ft)	621.2
Total Change in CCR Volume (CY)	+74,294
Change in CCR Volume Above SWSE (CY)	+30,006
Change in CCR Volume Below SWSE (CY)	+44,288

The comparison indicated that approximately 74,000 CY of CCR was placed in the GMF GSP between 2015 and 2020, including approximately 30,000 CY above the SWSE, thereby leading to a potential for the peak water surface elevation (PWSE) to increase during the design 1,000-year flood event.

Furthermore, the surveyed pool elevation increased by approximately 4 ft, due to the construction of a berm in the transfer channel. A review of the 2020 survey data indicated the crest elevation of the new berm is approximately 628 ft; this is higher than the periodic surveyed pool level elevation of 625.2 ft. No other significant changes in embankment geometry or other features were noted in the comparison.

2.5 Comparison of Initial to Periodic Aerial Photography

Initial aerial photographs of the GMF GSP collected by Weaver 2015 [10] were compared to periodic aerial photographs collected by IngenAE in 2020 [10] to visually evaluate if potential site changes (i.e., changes to the embankment, outlet structures, limits of CCR, other appurtenances) may have occurred between. A comparison of these aerial photographs is provided in **Drawing 3**, and the following changes were identified:

- The berm in the transfer channel discussed in **Section 2.4** was identified in the channel.

- Minor changes in site conditions outside of the GMF GSP were identified, including the expansion of existing haul roads and the seeding of the GMF GSP exterior embankment near the transfer channel. However, these minor changes are not expected to significantly affect the design and/or operation of the GMF GSP.

2.6 Comparison of Initial to Periodic Site Visits

An initial site visit to the GMF GSP was conducted by AECOM in 2015 and documented with a Site Visit Summary and corresponding photographs [16]. A periodic site visit was conducted by Geosyntec on May 28, 2021, with Mr. Lucas P. Carr, P.E. conducting the site visit. The site visit was intended to evaluate potential changes at the site since 2015 (i.e., modification to the embankment, outlet structures or other appurtenances, limits of CCR, maintenance programs, and repairs), in addition to performing visual observations of the GMF GSP to evaluate if the structural stability requirements (§257.73(d)) were still met. The site visit included driving the perimeter of the GMF GSP, periodically stopping to exit the vehicle and visually observe conditions, recording filed notes, and collecting photographs. The site visit is documented in a photographic log provided in **Appendix B**. One significant finding was identified during the periodic site visit and is listed below:

- A berm was constructed in the transfer channel in 2020, as discussed in **Section 2.4**.

2.7 Interview with Power Plant Staff

An interview with Mr. John Romang of CPP was conducted by Mr. Lucas P. Carr, P.E. of Geosyntec on May 28, 2021. Mr. Romang had been, at the time of the interview, employed at CPP for approximately 20 years as the environmental and chemistry manager or supervisor and was responsible for general oversight and compliance for the GMF GSP, including weekly CCR inspections and identifying required repairs. The interview included a discussion of potential changes that may have occurred at the GMF GSP since the development of the initial certifications ([2], [7] [3], [8], [4], [5], [6]). A summary of the interview is provided below.

- Were any construction projects completed for the GMF GSP between 2015 and 2021, and, if so, are design drawings and/or details available?
 - A berm was constructed in the transfer channel between the GMF GSP and the GMF Recycle Pond in 2020 and excess water from the GMF Recycle Pond was pumped into the GMF GSP.
- Were there any changes to the purpose of the GMF GSP between 2015 and 2017?
 - No, outside of plant retirement.

- Were there any changes to the to the instrumentation program and/or physical instruments for the GMF GSP between 2015 and 2021?
 - No instruments are present at the GMF GSP.
- Were there any changes to spillways and/or diversion features for the GMF GSP completed between 2015 and 2021?
 - Yes, the berm was constructed within the GMF GSP transfer channel.
- Have any area-capacity curves been developed for the GMF GSP since 2015?
 - No known curves have been developed.
- Were there any changes to construction specifications, surveillance, maintenance, and repair procedures for the GMF GSP between 2015 and 2021?
 - No.
- Were there any instances of dike and/or structural instability for the GMF GSP between 2015 and 2021?
 - No known instances occurred.

SECTION 3

HAZARD POTENTIAL CLASSIFICATION - §257.73(A)(2)

3.1 Overview of Initial HPC

The Initial Hazard Potential Classification (Initial HPC) was prepared by Stantec Consulting Services, Inc. (Stantec) in 2016 ([2], [7]), following the requirements of §257.73(a)(2). The Initial HPC included the following information:

- Reviewing a breach analysis prepared by Hanson Professional Services (Hanson) in 2007 [17], as part of the permitting of obtaining a permit to construct the GMF GSP as a regulated dam through the Illinois Department of Natural Resources, Offices of Water Resources (IDNR-OWR).
 - The review indicated that 12 structures were located within an area where the inundation depth was estimated to be 5 ft, including:
 - Eight (8) occupied structures, including seven residential structures, for a breach at the northwest corner of the GMF GSP perimeter dike.
 - Two (2) residential structures for a breach at the east side of the GMF GSP perimeter dike.
 - The CPP plant building, which was frequently occupied, for a breach to at the south side of the GMF GSP perimeter dike.
 - The review also noted that the breach analyses considered the final buildout height of the GMF GSP as a gypsum stack extending approximately 100 ft above the surrounding grades, rather than the current configurations, where the level of CCR and water in the GMF GSP is approximately equal to surrounding grades.
- While a breach map is not included within the Initial HPC, it included within the §257.73(a)(3) Initial Emergency Action Plan (Initial EmAP) [18].

The breach analysis concluded that a breach of the GMF GSP, at its maximum height, would result in a probable threat to human life at multiple residential and other occupied structures. The Initial HPC therefore recommended a “High” hazard potential classification for the GMF GSP [7].

3.2 Review of Initial HPC

Geosyntec performed a review of the Initial HPC ([2], [7]), in terms of technical approach, input parameters, and assessment of results. The review included the following tasks:

- Reviewing the breach assessment inputs for appropriateness;
- Reviewing the selected HPC for appropriateness based on the results of the breach analysis, including flow velocities and depths;
- Reviewing the HPC vs. applicable requirements of the CCR Rule.

The review noted that the Initial HPC considered ultimate buildout conditions for the GMF GSP, where it extends approximately 100 ft above grade using the upstream method of construction and dikes comprised of CCR, relative to existing conditions where the GMF GSP is essentially at-grade, as discussed in **Section 3.1**. The GMF GSP is unlikely to reach ultimate buildout conditions due to closure of CPP and the cessation of CCR generation. Therefore, the Initial HPC includes a conservative volume of breach material relative to the amount of material than is currently in the pond.

No other significant technical issues were noted in the technical review, although a detailed review (e.g., check) of the calculations was not performed.

3.3 Summary of Site Changes Affecting the Initial HPC

The GMF GSP is currently considered a High hazard potential CCR surface impoundment [2]; this is the highest hazard classification within §257.53 of the CCR Rule [1]. Therefore, the hazard potential classification would not increase if new structures were to be constructed within the existing mapped breach areas, and a visual assessment of these areas was not performed.

3.4 Periodic Hazard Potential Classification

The current hazard potential classification for the GMF GSP, which is “High” per §257.73(a)(2), is considered conservative as the GMF GSP has not reached and is not expected to reach ultimate buildout conditions. The “High” hazard potential classification is conservative and could be maintained or could potentially be revised to “Significant” if a revised breach analysis is performed. However, Geosyntec recommends retaining the current “High” hazard potential classification, unless a revised breach analysis is performed to justify a “Significant” hazard potential classification.

SECTION 4

HISTORY OF CONSTRUCTION REPORT - §257.73(C)

4.1 Overview of Initial HoC

The Initial History of Construction report (Initial HoC) was prepared by AECOM in 2016 [3], following the requirements of §257.73(c), and included information on all CCR surface impoundments at CPP, including AP1, AP2, the GMF GSP, and the GMF Recycle Pond. The Initial HoC included the following information for each CCR surface impoundment:

- The name and address of the owner/operator,
- Location maps,
- Statements of purpose,
- The names and size of the surrounding watershed,
- A description of the foundation and abutment materials,
- A description of the dike materials,
- Approximate dates and stages of construction,
- Available design and engineering drawings,
- A summary of instrumentation,
- Area-capacity curves for the GMF GSP,
- Information on spillway structures,
- Construction specifications,
- Inspection and surveillance plans,
- Information on operational and maintenance procedures, and
- A statement that no known instability has occurred at the GMF GSP.

4.2 Summary of Site Changes Affecting the Initial HoC

Several significant changes at the site were identified since development of the Initial HOC and required updates to the HoC report. Each change is described below:

- A state identification number (ID) of W1350150004-03 was assigned to the GMF GSP by the Illinois Environmental Protection Agency (IEPA).
- Electricity generation at CPP ceased in 2019 and the GMF GSP is no longer being used to actively store CCR generated by CPP as CCR is no longer being generated. Additionally, the GMF GSP no longer received regular process water inflows or outflows.
- A berm was constructed within the transfer channel between the GMF GSP and GMF Recycle Pond in 2020, as discussed in **Section 2.4**.
- Revised area-capacity curves and spillway design calculations for the GMF GSP were prepared as part of the updated Periodic Inflow Design Flood Control System Plan, as described in **Section 6.3**.

A letter documenting changes to the HoC report is provided in **Attachment C**.

SECTION 5

STRUCTURAL STABILITY ASSESSMENT - §257.73(D)

5.1 Overview of Initial SSA

The Initial Structural Stability Assessment (Initial SSA) was prepared by AECOM in 2016 [4], following the requirements of §257.73(d)(1), and included the following evaluations:

- Stability of dike foundations, dike abutments, slope protection, dike compaction, and slope vegetation;
- Spillway stability including capacity, structural stability and integrity;
- An evaluation of the effects of liquefaction in the foundation soils using a slope stability analysis considering post-cyclic softening in the foundation soils; and
- An evaluation to determine if downstream water bodies that could induce a sudden drawdown condition to the exterior slopes were present.

The Initial SSA concluded that the GMF GSP met all structural stability requirements for §257.73(d)(1)(i)-(vii).

The Initial SSA referenced the results of the Initial Structural Factor Assessment (Initial SFA) ([5], [8]), to demonstrate stability of the stability of foundations and abutments (§257.73(d)(1)(i)) and sufficiency of dike compaction (§257.73(d)(1)(iii)) portions of the SSA criteria. This included stating that slope stability analyses for slip surfaces passing through the foundation met or exceeded the criteria listed in §257.73(e)(1), for the stability of foundations and abutments. For the sufficiency of dike compaction, this included stating that slope stability analyses for slip surfaces passing through the dike also met or exceeded the §257.73(e)(1) criteria.

5.2 Review of Initial SSA

Geosyntec performed a review of the Initial SSA ([4], [8]) in terms of technical approach, calculation input parameters and methodology, recommendations, and completeness. The review included the following tasks:

- Reviewing photographs collected in 2015 and used to demonstrate compliance with §257.73(d)(1)(i)-(vii).
- Reviewing geotechnical calculations used to demonstrate the stability of foundations, per §257.73(d)(1)(i) and sufficiency of dike compaction, per §257.73(d)(1)(iii), in terms of

supporting geotechnical investigation and testing data, input parameters, analysis methodology, selection of critical cross-sections, and loading conditions.

- Review of the methodology used to demonstrate that a downstream water body that could induce a sudden drawdown condition, per §257.73(d)(1)(vii), is not present.

No significant technical issues were noted within the technical review, although a detailed review (e.g., check) of the calculations was not performed.

5.3 Summary of Site Changes Affecting Initial SSA

Several changes at the site occurred after development of the Initial SSA were identified. These changes required updates to the Initial SSA. The changes and the recommend updates to the Initial SSA and are described below.

- The Initial SSA utilized the results of the Initial Inflow Design Flood Control System Plan (IDF) to demonstrate compliance with the adequacy of spillway design and management (§257.73(d)(1)(v)(A)-(B)). The Initial IDF was subsequently updated to develop a Periodic IDF, based on site changes, as discussed in **Section 7**.
- The Initial SSA utilized the slope stability analysis results of the Initial Safety Factor Assessment (SFA) as part of the compliance demonstration for the stability of foundations and abutments (§257.73(d)(1)(i)) and sufficiency of dike compaction (§257.73(d)(1)(iii)) as discussed in **Section 5.1**. The Initial SFA slope stability analyses, including the sudden drawdown analyses, were subsequently updated to develop a Periodic SFA, based on site changes, as discussed in **Section 6**.

5.4 Periodic SSA

The Periodic SFA (**Section 6**) indicated that foundations and abutments are stable and dike compaction is sufficient for expected ranges in loading conditions, as slope stability factors of safety were found to meet or exceed the requirements of §257.73(e)(1), including for post-earthquake (i.e., liquefaction) loading conditions considering seismically induced strength loss in the foundation soils. Therefore, the requirements of §257.73(d)(1)(i) and §257.73(d)(1)(iii) are still met for the Periodic SSA.

The updated Periodic IDF (**Section 7**) indicated that spillways are adequately designed and constructed to adequately manage flow during the PMF flood, as the spillway can adequately manage flow during peak discharge from the PMP storm event without overtopping of the embankments. Therefore, the requirements of §257.73(d)(1)(v)(A)-(B) are met for the Periodic SSA.

SECTION 6

SAFETY FACTOR ASSESSMENT - §257.73(E)(1)

6.1 Overview of Initial SFA

The Initial Safety Factor Assessment (Initial SFA) was prepared by AECOM in 2016 ([5], [8]), following the requirements of §257.73(e)(1). The Initial SFA included the following information:

- A geotechnical investigation program with in-situ testing;
- An assessment of the potential for liquefaction in the dike and foundation soils;
- The development of four (4) slope stability cross-sections for limit equilibrium stability analysis utilizing GeoStudio SLOPE/W software; and
- The analysis of each cross-sections for maximum storage pool, maximum surcharge pool, and seismic loading conditions.
 - Liquefaction (i.e., post-earthquake) loading conditions were analyzed due to the presence of a soft layer in the foundation material that may be susceptible to cyclic softening and/or liquefaction. However, this assessment was utilized to support the Initial SSA rather than the Initial SFA, as liquefaction-susceptible soil layers were not identified in the embankment soils.

The Initial SFA concluded that the GMF GSP met all safety factor requirements, per §257.73(e), as all calculated safety factors were equal to or higher than the minimum required values.

6.2 Review of Initial SFA

Geosyntec performed a review of the Initial SFA ([5], [8]) in terms of technical approach, calculation input parameters and methodology, recommendations, and completeness. The review included the following tasks:

- Reviewing geotechnical calculations used to demonstrate the acceptable safety factors, per §257.73(e)(1), in terms of:
 - Completeness and adequacy of supporting geotechnical investigation and testing data;
 - Completeness and approach of liquefaction triggering assessments; and
 - Input parameters, analysis methodology, selection of critical cross-sections, and loading conditions utilized for slope stability analyses.

No significant technical issues were noted within the technical review, although a detailed review (e.g., check) of the calculations was not performed.

6.3 Summary of Site Changes Affecting the Initial SFA

Several changes at the site, occurred after development of the Initial SFA ([5], [8]), were identified. These changes required updates to the Initial SFA and are described below:

- The normal pool levels within the GMF GSP increased from 621.2 ft to 625.2 ft, due to the construction of a berm in the transfer channel (**Section 7**), resulting in 4.0 ft of additional water loading on the embankment dikes for the maximum storage pool and seismic loading conditions (§257.73(e)(1)(i) and (iii)), relative to the Initial SFA.
- Peak pool levels in the GMF GSP during the PMP design flood event increased from 623.8 ft to 626.7 ft, per the updated Periodic IDF (**Section 7**), resulting in 2.9 ft of additional water loading on the embankment dikes for the maximum surcharge pool loading conditions (§257.73(e)(1)(iv)), relative to the initial SFA.
- Groundwater levels in foundation soils around the GMF GSP, as measured from the monitoring well network over a multi-year period, were observed to be approximately 1 to 3 ft higher than groundwater levels utilized in the slope stability analyses supporting the Initial SFA (see **Section 2.3**). Therefore, the groundwater levels in the slope stability analysis do not represent long-term trends at the GMF GSP.

6.4 Periodic SFA

Geosyntec revised existing slope stability analyses associated with the Initial SFA ([5], [8]), for the four cross-sections (13+50, 22+50, 46+50, and 58+00) previously evaluated to account for site changes, as described in **Section 6.3**. The following approach and input data were used to revise the analyses:

- Water levels in the GMF GSP for the maximum storage pool, and seismic slope stability analysis loading conditions were increased to El. 625.2 ft in all the cross-sections, based on the Periodic IDF (**Section 7.4**).
- Water levels in the GMF GSP for the maximum surcharge pool slope stability analysis loading conditions were increased to El. 626.7 ft in al the cross-sections based on the Periodic IDF (**Section 7.4**).
- According to updated groundwater level monitoring plot (**Section 2.3**), the phreatic level in the location of related piezometers increased for all the loading conditions from El. 621.8 to El. 623.3 ft in cross-section 22+50, from El. 623.3 to El. 624.0 ft in cross-section 46+50, and from El. 620.0 to El. 623.0 ft in cross-section 58+00.

- All other analysis input data and settings from the Initial SFA ([5], [8]), were utilized, including, but not limited to, subsurface stratigraphy and soil strengths, phreatic conditions, ground surface geometry, software package and version, slip surface search routines and methods, and input data for the seismic analyses.

Factors of safety from the Periodic SFA are summarized in **Table 3** and confirm that the GMF GSP meets the requirements of §257.73(e)(1). Slope stability analysis output associated with the Initial SFA is provided in **Attachment D**.

Table 3 – Factors of Safety from Periodic SFA

Cross-Section	Structural Stability Assessment (§257.73(d)) and Safety Factor Assessment (§257.73(e))				Structural Stability Assessment (§257.73(d))
	Maximum Storage Pool §257.73(e)(1)(i) Minimum Required = 1.50	Maximum Surcharge Pool ¹ §257.73(e)(1)(ii) Minimum Required = 1.40	Seismic §257.73(e)(1)(iii) Minimum Required = 1.00	Dike Liquefaction §257.73(e)(1)(iv) Minimum Required = 1.20	Foundation Liquefaction §257.73(d)(1)(i) Minimum Required = 1.20
13+50	3.45*	3.45*	1.6	N/A	2.46
22+50	3.48	3.48	1.45*	N/A	2.39*
46+50	4.17	4.17	1.74	N/A	3.01
58+00	3.57	3.57	1.63	N/A	2.57

Notes:

*Indicates critical cross-section (i.e., lowest calculated factor of safety out of the ten cross-sections analyzed)

N/A – Loading condition is not applicable.

SECTION 7

INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN - §257.82

7.1 Overview of 2016 Inflow Design Flood Control System Plan

The Initial Inflow Design Flood Control System Plan (Initial IDF) was prepared by AECOM in 2016 ([6], [8]), following the requirements of §257.82. The Initial IDF included the following information:

- A hydraulic and hydrologic analysis, performed for the Probable Maximum Flood design flood event because of the hazard potential classification of “High”, which corresponded to 34.25 inches of precipitation over a 24-hour period.
- The Initial IDF utilized a HydroCAD Version 10 [19] model to evaluate spillway flows and pool level increases during the design flood, with a SWSE of 621.2 ft.

The Initial IDF concluded that the GMF GSP met the requirements of §257.82, as the peak water surface estimated by the HydroCAD model was El. 623.8 ft, relative to the minimum GMF GSP dike crest elevation of 631.0 ft. Therefore, overtopping was not expected. The Initial IDF also evaluated the potential for discharge from the CCR unit and determined that discharge from the unit was not expected, as the GMF GSP does not discharge into waters of the United States and overtopping of the GMF GSP embankments was not expected during the PMF inflow design flood.

7.2 Review of Initial IDF

Geosyntec performed a review of the Initial IDF ([6], [8]) in terms of technical approach, calculation input parameters and methodology, recommendations, and completeness. The review included the following tasks:

- Reviewing the return interval used vs. the hazard potential classification.
- Reviewing the rainfall depth and distribution for appropriateness.
- Performing a high-level review of the inputs to the hydrological modeling.
- Reviewing the hydrologic model parameters for spillway parameters, starting pool elevation, and storage vs. the reference data.
- Reviewing the overall IDF vs. the applicable requirements of the CCR Rule [1].

One comment was identified during review of the Initial IDF. The comment is described below:

- The Initial IDF considered the GMF GSP, but the HydroCAD analysis supporting the Initial IDF did not explicitly consider the downstream GMF Recycle Pond (GMF RP) within the model.

7.3 Summary of Site Changes Affecting the Initial IDF

Two changes at the site that occurred after development of the Initial IDF were identified. These changes required updates to the Initial IDF and are described below:

- A berm was constructed in the transfer channel between the GMF GSP and the GMF RP, with a crest elevation of approximately 626 ft, thereby increasing the SWSE in the GMF GSP relative to the Initial IDF.
- Approximately 30,000 CY was placed above the SWSE in the GMF GSP, thereby altering the stage-storage curve relative to the Initial IDF.

7.4 Periodic IDF

Geosyntec revised the Initial IDF to account for the increase in SWSE and additional CCR placement, as described in **Section 7.2** and **7.3**. The following approach and input data were used for the revised analyses: The model was expanded to include the Gypsum Management Facility Recycle Pond (GMF RP) pond and its drainage area.

- The drainage area to the GMF RP was modeled as a subcatchment and assigned an area of 18.3 ac per the 2020 site survey [9]. It was assigned a Curve number (CN) of 98 and a time of concentration of 6 min (direct inflow).

Table 4 – GMF RP Culvert Attributes in Periodic IDF

Parameter	Value
Orifice/Grate	
Invert Elevation (ft)	624.0
Discharge Coefficient	0.6
Orifice Width (in)	60
Orifice Length (in)	60
Culvert	
Inlet Elevation (ft)	615.0
Crest Breadth (ft)	1.0
Outlet Elevation (ft)	613.0
Length (ft)	92.0
Diameter (in)	45
Manning’s n	0.013
Entrance Loss Coefficient	0.5
Contraction Coefficient	0.9

- The GMFR Pond was modeled as a pond with three identical emergency spillway outlets.
 - The outlets were modeled as horizontal orifices routed to culverts, with attributed listed in **Table 4**.
- The routing method for the model was updated to account for routing between the ponds. The Reach Routing Method was updated from “Storage Indication+ Translation” to “Dynamic Storage Indication”. The Pond Routing Method was updated from “Storage – Indication” to “Dynamic Storage Indication”.
- The stage-storage curve was updated for both the GMF GSP and GMF RP Ponds based on the 2020 site survey [9].
 - Revised stage-volume curves for the GMF RP and GMF GSP were prepared based on measuring the storage volume of the GMF RP and GMF GSP at every one-foot increment of depth from an elevation at the bottom of the ponds (621.1 ft for GMF GSP; 604.9 ft for GMF RP) to the approximate minimum perimeter dike embankment crest elevation (632 ft for GMF GSP; 629 ft for GMF RP). This analysis identified an overall decrease of 9.67 ac-ft of storage volume at the GMF GSP from the storage used in the 2016 Initial IDF Certification.
- The subcatchment area draining to the GMF GSP was updated from 33.8 ac to 36.2 ac to reflect the 2020 site survey [9].
- The time of concentration (ToC) for drainage areas to the GMF GSP was updated from 5 minutes to 6 minutes to reflect direct run-on inflow in accordance with TR-20 [20].
- The SWSE within the GMF GSP was updated from 621.2 ft to 625.2 ft to reflect the water surface elevation from 2020 site survey [9].
- The SWSE in the GMF RP was assumed to be El. 622.1 ft, based on the Updated IDF for the GMF RP [21].
- The GMF GSP and transfer channel geometry were updated to reflect the new berm at the inlet to the transfer channel.
 - The outlet invert from the GMF Pond to the transfer channel between the GMF Pond and the GMFR Pond was raised from 625 ft to 626 ft per the 2020 site survey [9]. The geometry of the outlet was updated based on the 2020 site survey, as listed in **Table 5**.

Table 5 – GMF GSP Outlet Geometry in Periodic IDF

Head (ft)	Channel Width (ft)
0	45
2	60
4	75

- The transfer channel geometry was updated based on the 2020 site survey, as listed in **Table 6**.

Table 6 – GMF GSP Transfer Channel Geometry in Periodic IDF

Parameter	Value
Bottom Width (ft)	32.7
Channel Depth (ft)	6
Left Side Slope	3
Right Side Slope	1.6
Channel Length (ft)	450

- The three outlet structures in the GMF RP were updated from 24 ft broad-crested weirs to horizontal, rectangular orifices with dimensions of 5 ft by 5 ft to reflect the riser structures existing on site. The inlet elevation of the orifices was set to 624 ft per the initial certification reports for the GMF RP ([22], [23]).

The results of the Periodic IDF are summarized in **Table 7** and confirm that the GMF GSP meets the requirements of §257.82(a)-(b), as the peak water surface elevation does not exceed the minimum perimeter dike crest elevations, as long as the SWSE in the GMF GSP is maintained at El. 625.2 ft or lower. Additionally, all discharge from the GMF GSP is routed through the existing spillway system to the GMF RP during both normal and IDF conditions. Updated area-capacity curves and HydroCAD model output are provided in **Attachment E**.

Table 7 – Water Levels from Updated Periodic IDF

Analysis	Starting Water Surface Elevation (ft)	Peak Water Surface Elevation (ft)	Minimum Dike Crest Elevation (ft)
Initial IDF	621.2	623.8	631.0
Periodic IDF Update	625.2	626.7	632.0
Initial to Periodic Change ¹	+4.0	+2.9	

Notes:

¹Postive change indicates increase in the WSE relative to the Initial IDF, negative change indicates decrease in the WSE, relative to the Initial IDF.

SECTION 8

CONCLUSIONS

The GMF GSP at CPP was evaluated relative to the USPEPA CCR Rule periodic assessment requirements for:

- Hazard potential classification (§257.73(a)(2)),
- History of Construction reporting (§257.73(d)),
- Structural stability assessment (§257.73(d)),
- Safety factor assessment (§257.73(e)), and
- Inflow design flood control system planning (§257.82).

Based on the evaluations presented herein, the referenced requirements are satisfied.

SECTION 9

CERTIFICATION STATEMENT

CCR Unit: Illinois Power Generating Company, Coffeen Power Plant, GMF Gypsum Stack Pond

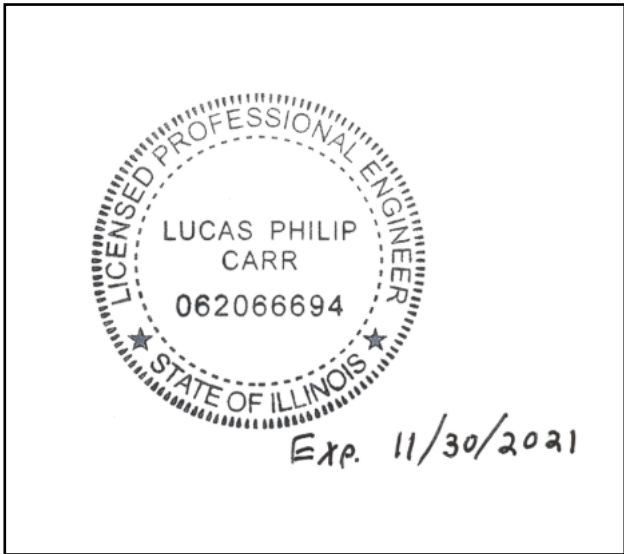
I, Lucas P. Carr, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief that the information contained in this 2021 USEPA CCR Rule Periodic Certification Report, has been prepared in accordance with the accepted practice of engineering. I certify, for the above-referenced CCR Unit, that the periodic assessment of the hazard potential classification, history of construction report, structural stability, safety factors, and inflow design flood control system planning, dated October 2021, were conducted in accordance with the requirements of 40 CFR §257.73(a)(2), (c), (d), (e), and §257.82.



Lucas P. Carr

10/11/2021

Date



SECTION 10

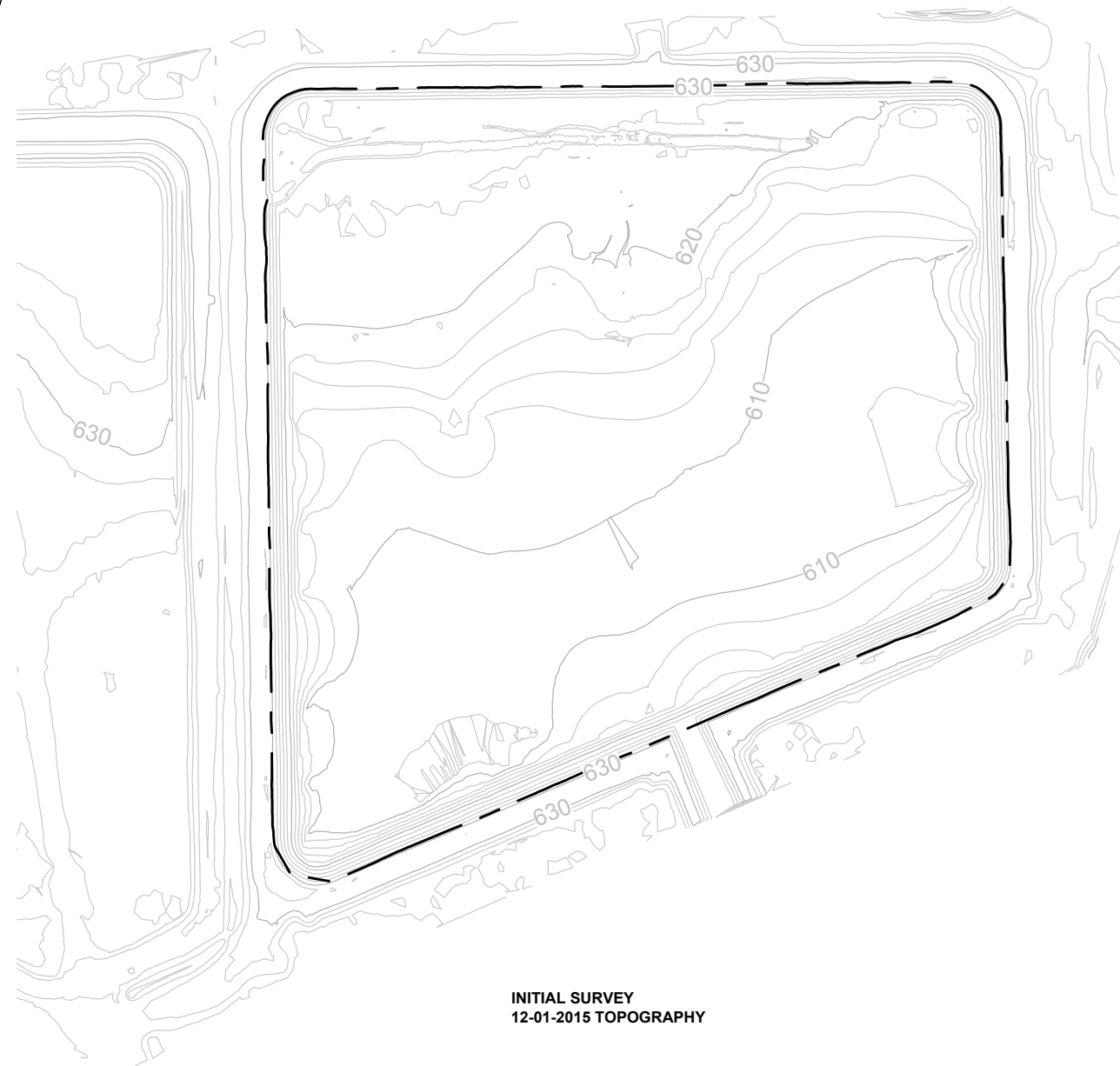
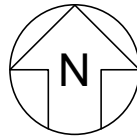
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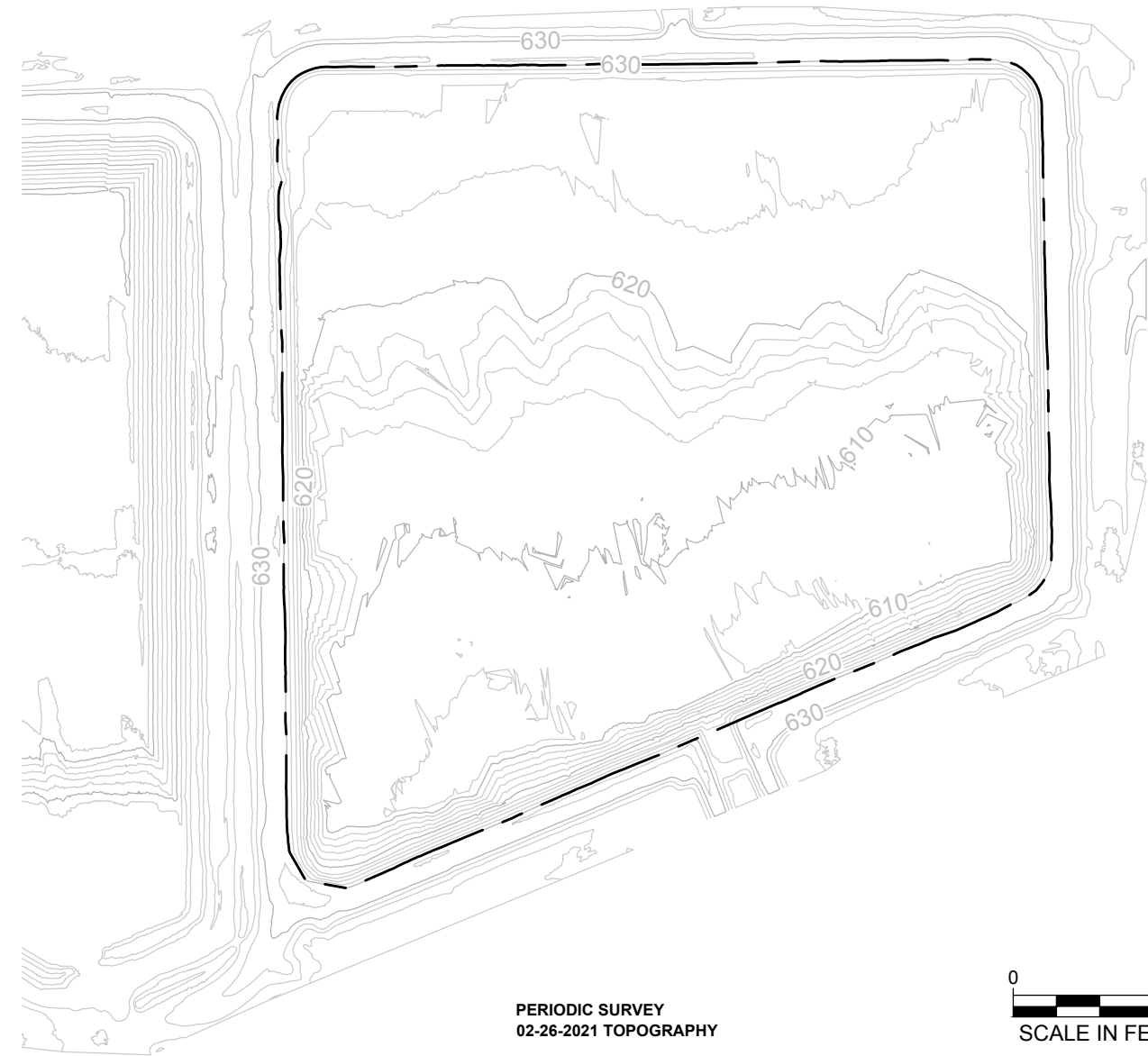
- [17] Hanson Professional Services, Inc., "Breach Analysis for Coffeen Gypsum Stack Dam," 2007.
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DRAWINGS

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INITIAL SURVEY
12-01-2015 TOPOGRAPHY



PERIODIC SURVEY
02-26-2021 TOPOGRAPHY



NOTES:

1. THE INITIAL SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "DYNEGY, COLLINSVILLE, ILLINOIS, 2015 - COFFEEN TOPOGRAPHY", PREPARED BY WEAVER CONSULTANTS GROUP, DATED DECEMBER 1, 2015.
2. THE PERIODIC SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "LUMINANT, ILLINOIS POWER GENERATING COMPANY, COFFEEN POWER STATION, DECEMBER 2020 TOPOGRAPHY", PREPARED BY INGENAE, DATED FEBRUARY 26, 2021.
3. ALL SURVEY DATA WAS COLLECTED IN THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) AND NORTH AMERICAN DATUM OF 1983 (NAD83) FOR VERTICAL AND HORIZONTAL COORDINATES, RESPECTIVELY.

INITIAL TO PERIODIC SURVEY COMPARISON
GMF GYPSUM STACK POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS



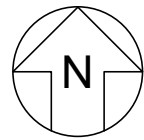
DRAWING

1

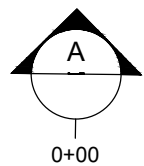
GLP8027.02

MAY 2021

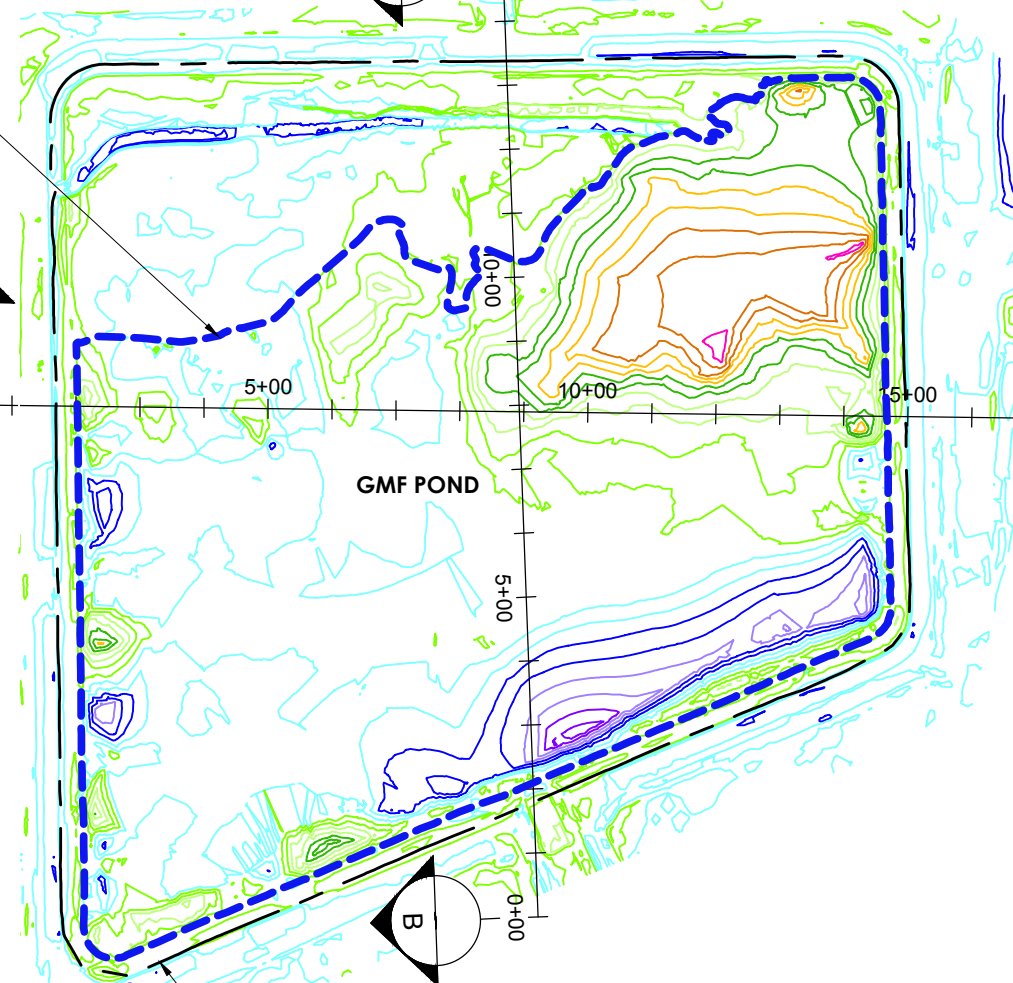
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LIMITS OF SWSE IN INITIAL SURVEY



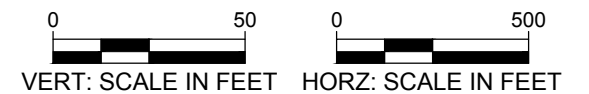
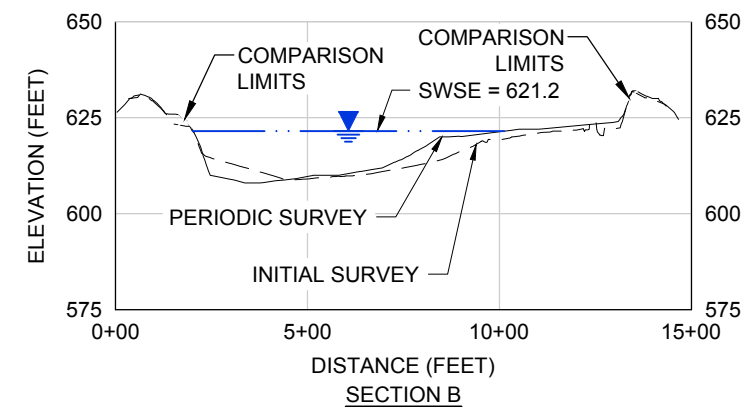
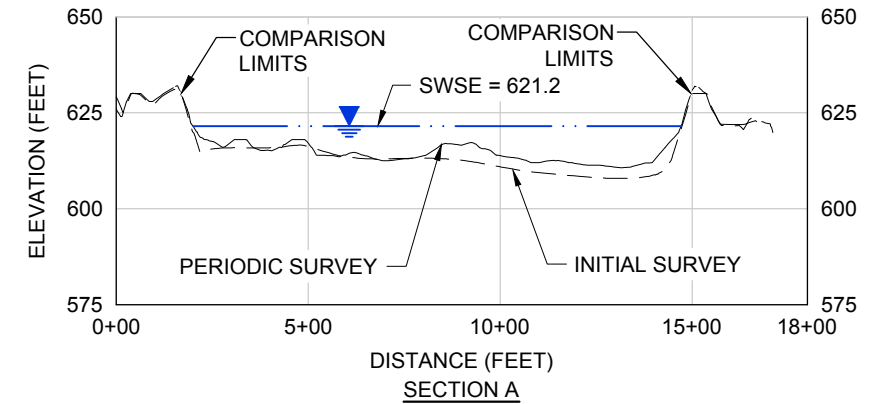
0+00 5+00 10+00 15+00 18+00



LIMITS OF INITIAL TO PERIODIC SURVEY COMPARISON, GMF POND



ISOPACH CONTOUR KEY		
COLOR	MIN ELEV	MAX ELEV
Dark Purple	-8	-6
Light Purple	-6	-4
Blue	-4	-2
Cyan	-2	0
Light Green	0	2
Green	2	4
Dark Green	4	6
Yellow	6	8
Orange	8	10
Pink	10	11



NOTES:

1. THE INITIAL SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "DYNEGY, COLLINSVILLE, ILLINOIS, 2015 - COFFEEN TOPOGRAPHY", PREPARED BY WEAVER CONSULTANTS GROUP, DATED DECEMBER 1, 2015.
2. THE PERIODIC SURVEY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "LUMINANT, ILLINOIS POWER GENERATING COMPANY, COFFEEN POWER STATION, DECEMBER 2020 TOPOGRAPHY", PREPARED BY INGENAE, DATED FEBRUARY 26, 2021.
3. ALL SURVEY DATA WAS COLLECTED IN THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) AND NORTH AMERICAN DATUM OF 1983 (NAD83) FOR VERTICAL AND HORIZONTAL COORDINATES, RESPECTIVELY.
4. THE STARTING WATER SURFACE ELEVATION (SWSE) OF THE GMF POND IS EL. 621.2 FT, AS NOTED IN THE REPORT TITLED "CCR CERTIFICATION REPORT: INITIAL STRUCTURAL STABILITY ASSESSMENT, INITIAL SAFETY FACTOR ASSESSMENT, AND INITIAL INFLOW DESIGN FLOOD CONTROL SYSTEM PLAN FOR GMF POND AT COFFEEN POWER STATION", PREPARED BY AECOM, DATED OCTOBER, 2016.

INITIAL TO PERIODIC SURVEY COMPARISON SUMMARY			
	CUT	FILL	NET (CU. YD.)
GMF POND	20,320	94,614	74,294 (FILL)
ABOVE SWSE	2,168	32,174	30,006 (FILL)
BELOW SWSE	18,152	62,440	44,288 (FILL)

SURVEY COMPARISON ISOPACH
GMF GYPSUM STACK POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

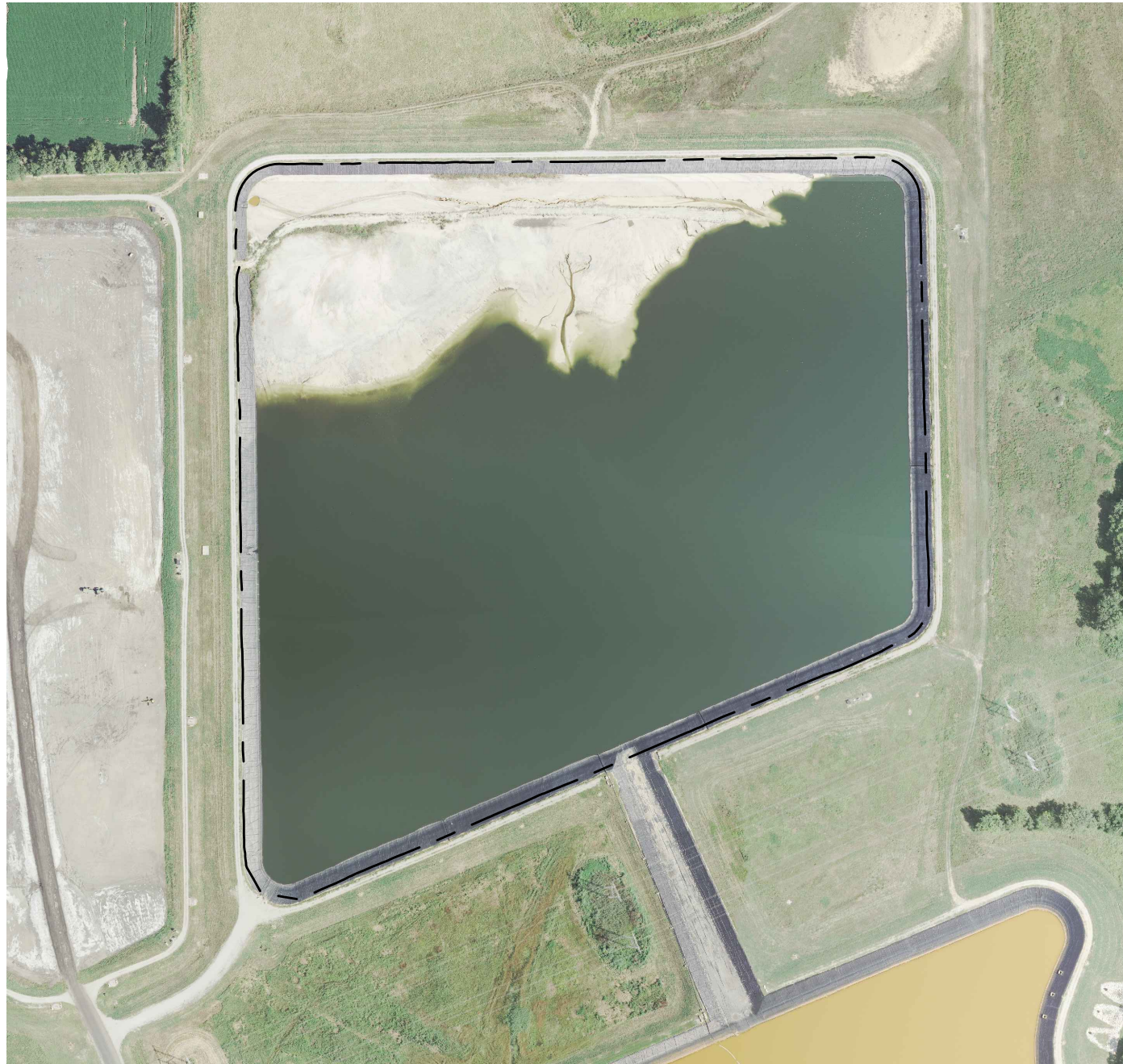
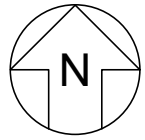


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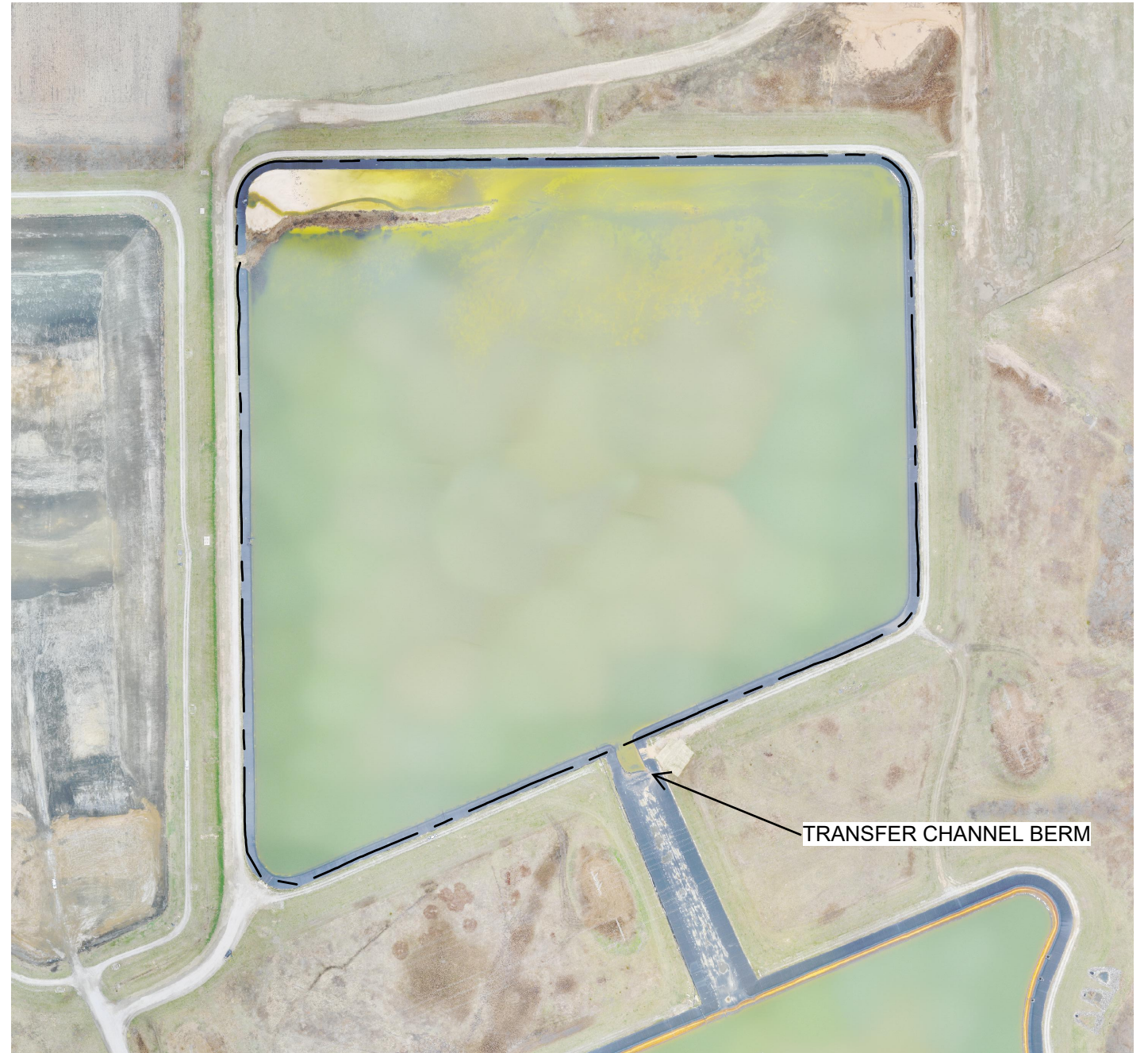
MAY 2021

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INITIAL AERIAL
12-01-2015 IMAGERY



PERIODIC AERIAL
02-26-2021 IMAGERY

TRANSFER CHANNEL BERM



NOTES:

1. THE INITIAL IMAGERY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "DYNEGY, COLLINSVILLE, ILLINOIS, 2015 - COFFEEN TOPOGRAPHY", PREPARED BY WEAVER CONSULTANTS GROUP, DATED DECEMBER 1, 2015.
2. THE PERIODIC IMAGERY WAS TAKEN FROM THE DRAWING PACKAGE TITLED "LUMINANT, ILLINOIS POWER GENERATING COMPANY, COFFEEN POWER STATION, DECEMBER 2020 TOPOGRAPHY", PREPARED BY INGENAE, DATED FEBRUARY 26, 2021.

INITIAL TO PERIODIC AERIAL IMAGERY
COMPARISON
GMF GYPSUM STACK POND
COFFEEN POWER STATION
COFFEEN, ILLINOIS



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3

GLP8027.02

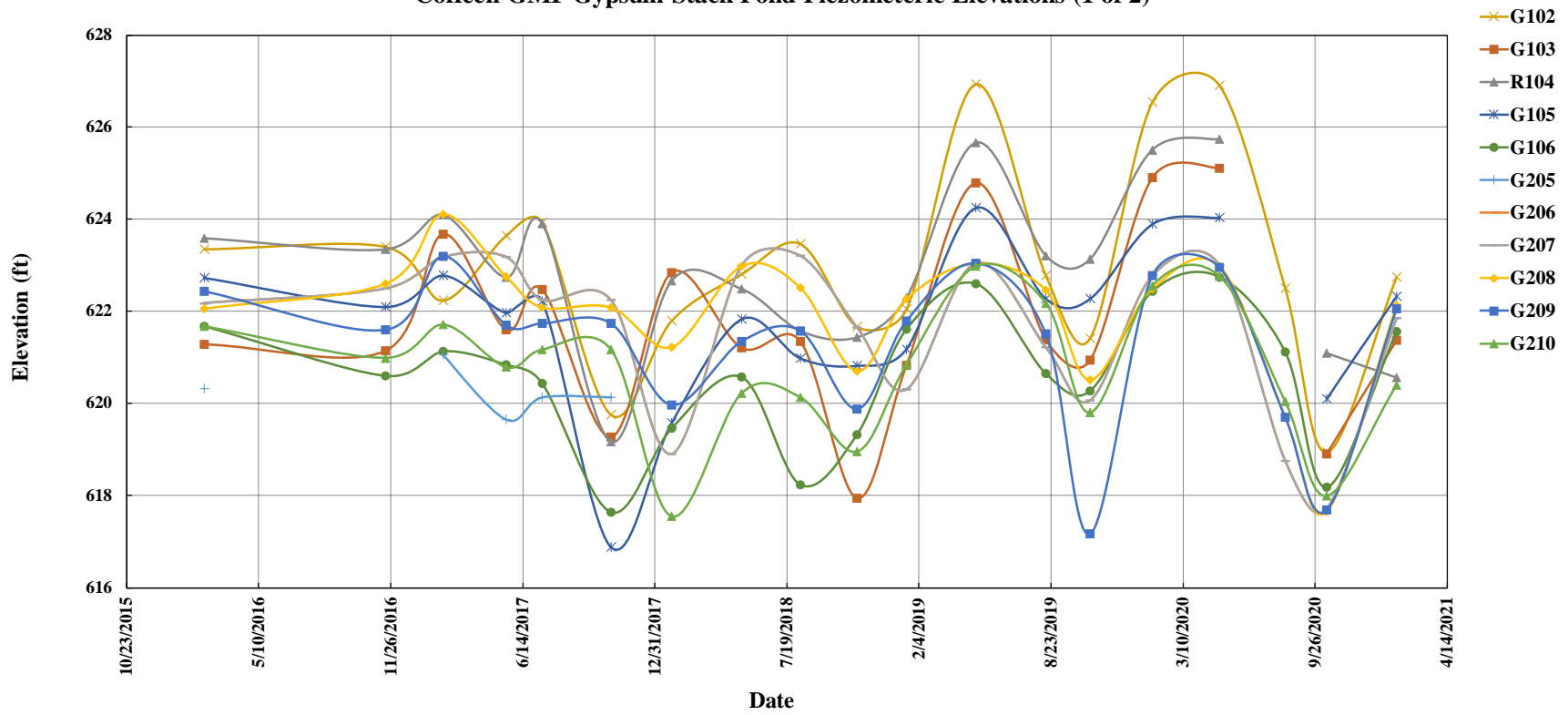
MAY 2021

ATTACHMENTS

Attachment A

GMF GSP Phreatic Data Plots

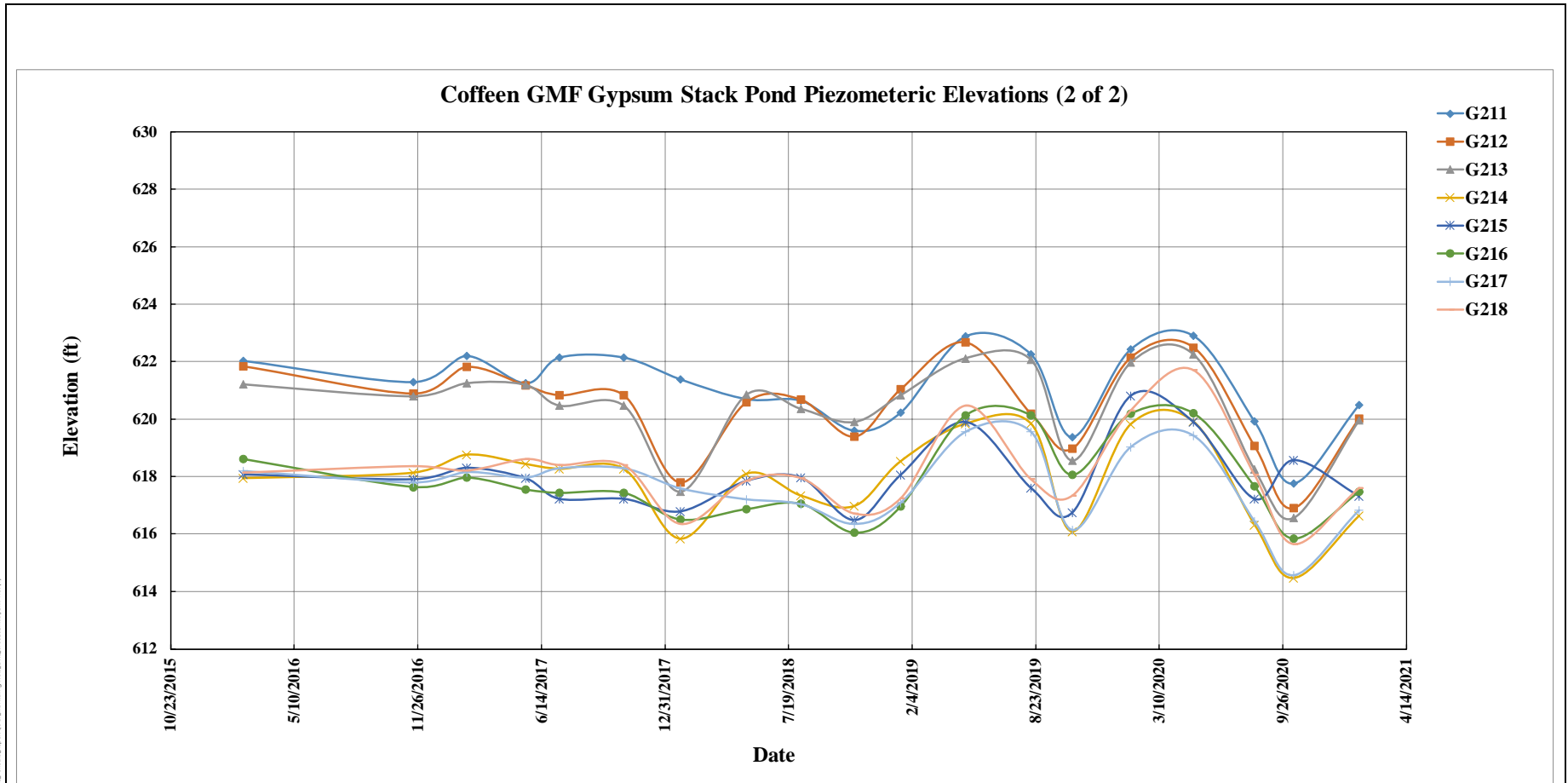
Coffeen GMF Gypsum Stack Pond Piezometric Elevations (1 of 2)



NOTES:

1. Piezometer data was taken from the spreadsheets titled " Coffeen GW 1017", " Coffeen GW 1018", " Coffeen GW 1019", " Coffeen GW 1020", " Coffeen GW 1021", provided by the Coffeen Power Plant.

PIEZOMETER DATA PERIODIC CERTIFICATION, GMF GSP COFFEEN POWER PLANT COFFEEN, ILLINOIS	
GLP8027	7/8/2021
Figure A-1	



NOTES:

1. Piezometer data was taken from the spreadsheets titled " Coffeen GW 1017", " Coffeen GW 1018", " Coffeen GW 1019", " Coffeen GW 1020", " Coffeen GW 1021", provided by the Coffeen Power Plant.

PIEZOMETER DATA PERIODIC CERTIFICATION, GMF GSP COFFEEN POWER PLANT COFFEEN, ILLINOIS	
GLP8027	7/8/2021
Figure A-2	

Attachment B

GMF Gypsum Stack Pond Site Visit Photolog

GEOSYNTEC CONSULTANTS
Photographic Record



Site Owner: Illinois Power Generating Company **Project Number:** GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) **Site:** Coffeen Power Plant

Photo: 01

Date: 05/28/2021

Direction Facing:
SE

Comments:
Berm installed in the transfer channel to reduce the water level in the downstream GMF Recycle Pond.



Photo: 02

Date: 05/28/2021

Direction Facing:
W

Comments:
Southwestern interior slopes.



GEOSYNTEC CONSULTANTS
Photographic Record



Site Owner: Illinois Power Generating Company

Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP)

Site: Coffeen Power Plant

Photo: 07

Date: 05/28/2021

Direction Facing:
N

Comments:
West dike exterior slope. Note leachate valve sump.



Photo: 08

Date: 05/28/2021

Direction Facing:
NE

Comments:
Gypsum discharge pipes.



Site Owner: Illinois Power Generating Company

Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP)

Site: Coffeen Power Plant

Photo: 09

Date: 05/28/2021

Direction Facing:
E

Comments:
North dike exterior
overview



Photo: 10

Date: 05/28/2021

Direction Facing:
E

Comments:
North dike interior
overview



GEOSYNTEC CONSULTANTS
Photographic Record



Site Owner: Illinois Power Generating Company **Project Number:** GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP) **Site:** Coffeen Power Plant

Photo: 11

Date: 05/28/2021

Direction Facing:
NE

Comments:
North dike exterior
overview



Photo: 12

Date: 05/28/2021

Direction Facing:
S

Comments:
East dike interior
overview



Site Owner: Illinois Power Generating Company

Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP)

Site: Coffeen Power Plant

Photo: 13

Date: 05/28/2021

Direction Facing:
S

Comments:
East dike exterior
overview.



Photo: 14

Date: 05/28/2021

Direction Facing:
S

Comments:
Southeast corner
interior overview



GEOSYNTEC CONSULTANTS
Photographic Record



Site Owner: Illinois Power Generating Company

Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP)

Site: Coffeen Power Plant

Photo: 15

Date: 05/28/2021

Direction Facing:
SE

Comments:
Southeast corner
exterior overview



Photo: 16

Date: 05/28/2021

Direction Facing:
N

Comments:
Area where
geomembrane has
bulged slightly due to
air.



Site Owner: Illinois Power Generating Company

Project Number: GLP8027

CCR Unit: GMF Gypsum Stack Pond (GMF GSP)

Site: Coffeen Power Plant

Photo: 17

Date: 05/28/2021

Direction Facing:
E

Comments:
Southeast embankment
exterior overview



Photo: 18

Date: 05/28/2021

Direction Facing:
E

Comments:
Southeast embankment
interior overview



Attachment C

Periodic History of Construction Report Update Letter

October 11, 2021

Illinois Power Generating Company
134 Cips Lane
Coffeen, Illinois 62017

**Subject: Periodic History of Construction Report Update Letter
USEPA Final CCR Rule, 40 CFR §257.73(c)
Coffeen Power Plant
Coffeen Illinois**

At the request of Illinois Power Resources Generation Company (IPRG), Geosyntec Consultants (Geosyntec) has prepared this Letter to documents updates to the Initial History of Construction (HoC) report for the Coffeen Power Plant (CPP), also known as the Coffeen Power Station (COF). The Initial HoC report was prepared by AECOM in October of 2016 [1] in accordance with 40 Code of Federal Regulations (CFR) §257.73(c) of the United States Environmental Protection Agency (USEPA) Coal Combustion Residuals Rule, known as the CCR Rule [2]. This letter also includes information required by Section 845.220(a)(1)(B) (Design and Construction Plans) of the state-specific Illinois Environmental Protection Agency (IEPA) Part 845 CCR Rule [3] that is not expressly required by §257.73(c).

BACKGROUND

The CCR Rule required that, by October 17, 2016, Initial HoC reports to be compiled for existing CCR surface impoundments with: (1) a height of five feet or more and a storage volume of 20 acre-feet or more, or (2) a height of 20 feet or more. The Initial HoC report was required to contain, to the extent feasible, the information specified in 40 CFR §257.73(c)(1)(i)-(xii). The Initial HoC report for CPP, which included four existing CCR surface impoundments, Ash Pond No. 1 (AP1), Ash Pond No. 2 (AP2), the GMF Gypsum Stack Pond (GMF GSP, also known as the GMF Pond), and the GMF Recycle Pond (GMF RP), was prepared and subsequently posted to IPRG's CCR Website prior to October 17, 2016.

The CCR Rule requires that Initial HoC to be updated if there is a significant change to any information compiled in the Initial HoC report, as listed below:

§ 257.73(c)(2): If there is a significant change to any information compiled under paragraph (c)(1) of this section, the owner or operator of the CCR unit must update the relevant information and place it in the facility's operating record as required by § 257.105(f)(9).

IPRG retained Geosyntec to review the Initial HoC report, review reasonably and readily available information for AP1, AP2, the GMF GSP, and the GMF RP generated since the Initial HoC report was prepared, and perform a site visit to CPP to evaluate if significant changes may have occurred since the Initial HoC report was prepared. This Letter contains the results of Geosyntec's evaluation and documents significant changes that have occurred at AP1, AP2, the GMF GSP, and the GMF RP, as they pertain the requirements of §257.73(c)(1)(i)-(xii).

UPDATES TO HISTORY OF CONSTRUCTION REPORT

Geosyntec's evaluation for the CPP AP1, AP2, GMF GSP, and GMF RP determined that no known significant changes requiring updates to the information in the Initial HoC report pertaining to §257.73(c)(1)(ii), (iv), (v), (vi), (vii), (xi), and (xii) of the CCR Rule had occurred since the Initial HoC report was developed.

However, Geosyntec's evaluation determined that significant changes at the CPP AP1, AP2, GMF GSP, and GMF RP, pertaining to §257.73(c)(1)(i), (iii), (viii), (ix), and (x) of the CCR Rule had occurred since the Initial HoC report had been developed. Additionally, information how long the CCR surface impoundments have been operating and the types of CCR in the surface impoundments, as required by Section 845.220(a)(1)(B) of the Part 845 Rule were not included in the Initial HoC report, as this information is not required by the CCR Rule. Each change and the subsequent updates to the Initial HoC report is described within this section.

Section 845.220(a)(1)(B): A statement of ... how long the CCR surface impoundment has been in operation, and the types of CCR that have been placed in the surface impoundment.

Ash Pond No. 1

The AP1 was in operation from 1964 until CPP was retired in 2019 and received CCR for approximately 55 years. As of the date of this report, the AP1 has been present for approximately 57 years [4].

CCR placed in the AP1 included bottom ash [4].

Ash Pond No. 2

The AP2 was in operation from 1971 to 1984, for a total of approximately 13 years. The AP2 was closed in 1984-1985 by installing a clay cover and has not since been active or

received CCR. As of the date of this report, AP2 has been present for approximately 50 years. [4].

CCR placed in the AP2 was used to store and dispose of fly ash and bottom ash [4].

GMF Gypsum Pond

The GMF GSP was in operation from 2010 until CPP was retired in 2019 and received CCR for approximately 9 years. As of the date of this report, the GMF GSP has been present for a total of approximately 11 years [4].

CCR placed in GMF GSP included gypsum [4].

GMF Recycle Pond

The GMF RP was in operation from 2010 until CPP was retired in 2019, for a total of 9 years [4]. As of the date of this report, the GMF RP has been present for approximately 11 years.

§ 257.73(c)(1)(i): The name and address of the person(s) owning or operating the CCR unit; the name associated with the CCR unit; and the identification number of the CCR unit if one has been assigned by the state.

State identification numbers (IDs) for AP1, AP2, the GMF GSP, and the GMF RP have been assigned by the Illinois Environmental Protection Agency (IEPA). Each ID is listed in **Table 1**.

Table 1 – IEPA ID Numbers

CCR Surface Impoundment	State ID
Ash Pond No. 1 (AP1)	W1350150004-01
Ash Pond No. 2 (AP2)	W1350150004-02
GMF Gypsum Stack Pond (GMF GSP)	W1350150004-03
GMF Recycle Pond (GMF RP)	W1350150004-04

§ 257.73(c)(1)(iii): A statement of the purpose for which the CCR unit is being used.

AP2 was closed in 2020, in substantial compliance with the written closure plan posted to IPRG’s CCR Website [5], and as documented by a certified Notification of Completion of Closures posted to DMG’s CCR Website [6].

The CPP was retired in December of 2019, with the generation of electricity ceased at that time. Therefore, AP1, the GMF GSP, and the GMF RP are no longer being used to store and dispose of new CCR that is actively generated by CPP, as CCR generation as ceased. All three impoundments still contain CCR and liquids that was present at the time of plant

retirement. The GMF RP also previously received dewatering discharge from AP2; this inflow was ceased after AP2 was closed in 202.

§ 257.73(c)(1)(viii): *A description of the type, purpose, and location of existing instrumentation.*

Instrumentation monitoring at AP2 is no longer required as the CCR surface impoundment was closed in accordance with §257.102 [6], and the instrumentation network was modified at that time. Therefore, the instrumentation locations shown in Appendix C of the Initial HoC report are no longer applicable to AP2.

§ 257.73(c)(1)(ix): *Area-capacity curves for the CCR unit.*

Updated area-capacity curves were prepared for AP1, the GMF GSP, and the GMF RP in 2021 and are provided in **Figures 1, 2, and 3**, respectively.

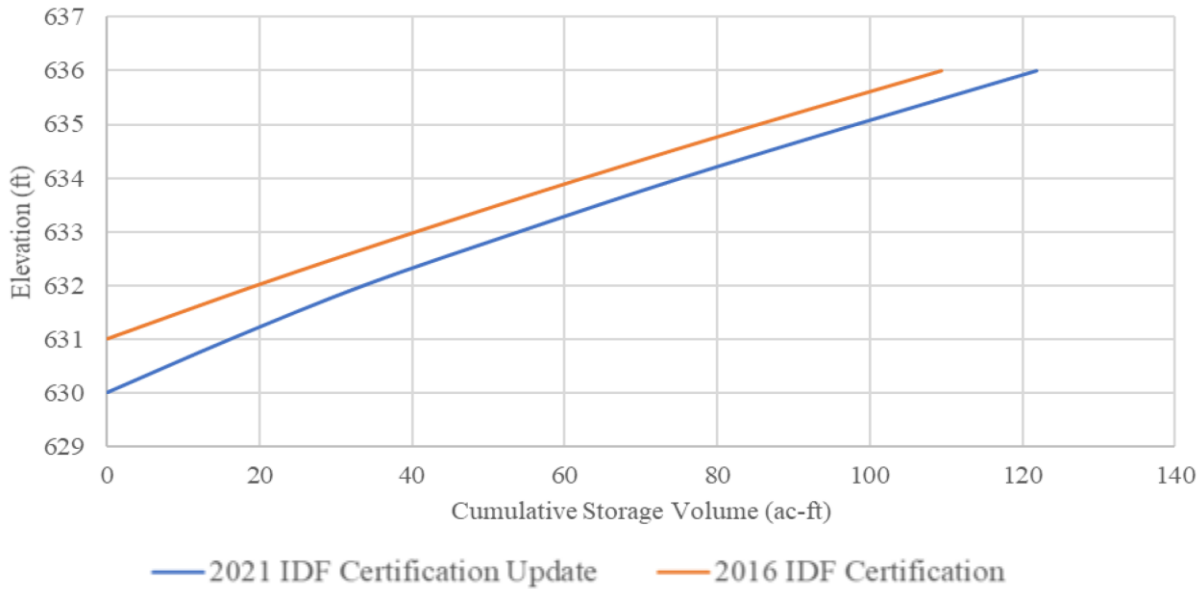


Figure 1 – Area-Capacity Curve for AP1

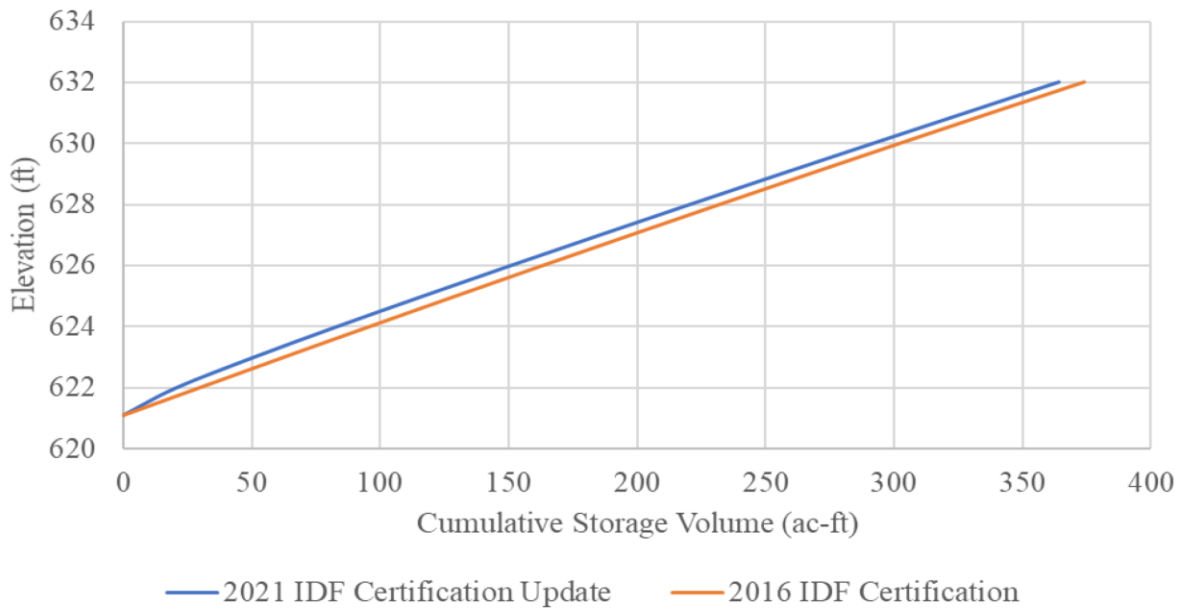


Figure 2 – Area-Capacity Curve for GMF GSP

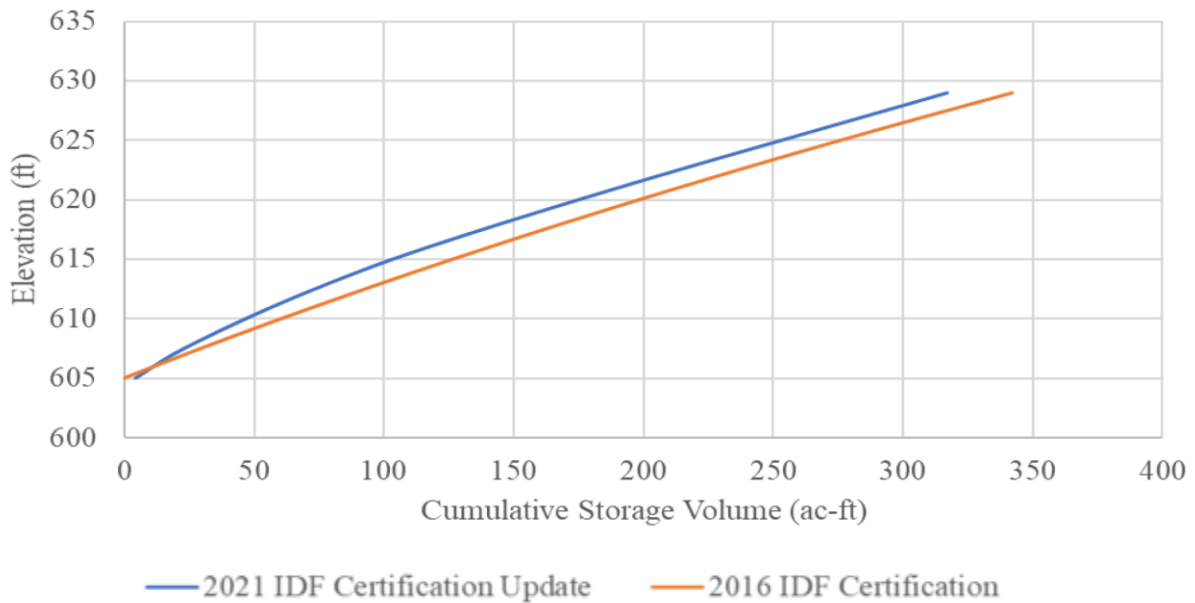


Figure 3 – Area-Capacity Curve for GMF RP

§ 257.73(c)(1)(x): A description of each spillway and diversion design features and capacities and calculations used in their determination.

The primary spillway structure for AP1 was modified in 2020 by constructing a berm of bottom ash around the entrance to the spillway, to reduce the potential for freezing around the spillway during post-CPP closure conditions, with a berm crest elevation of

approximately 630 ft. Design drawings for the bottom ash berm are not reasonably or readily available.

The transfer channel between the GMF GSP and the GMF RP was modified in 2020 by constructing a geomembrane-lined berm, in order to allow the normal pool level of the GMF GSP to be increased. Design drawings for the berm are not reasonably or readily available. However, survey data [3] indicates the berm has an elevation of approximately 628 ft, a top width (perpendicular to the flow direction) of approximately 75 ft, a total length (parallel to the flow direction) of 25 ft, and side slopes of approximately 4 horizontal to 1 vertical.

Valves were installed on the intake pipes for the GMF RP after the CPP was closed and plant process water intake pumping was ceased. Design drawings for these valves are not reasonably or readily available.

Updated discharge capacity calculations for the existing spillways of AP1, the GMF GSP, and the GMF RP were prepared in 2021 using HydroCAD 10 modeling software. The calculations indicate that the AP1 and the GMF RP have sufficient storage capacity and will not overtop the embankments during the 1,000-year, 24-hour, storm event. The calculations also indicate that the GMF GSP has sufficient storage capacity and will not overtop the embankments during the Probable Maximum Precipitation (PMP), 24-hour storm event. The results of the calculations are provided in **Table 2**.

Table 2 – Results of Updated Discharge Capacity Calculations

	AP1	GMF GSP	GMF RP
Approximate Berm Minimum Elevation ¹ , ft	636.0	632.0	629.0
Approximate Emergency Spillway Elevation ¹ , ft	Not Present	Not Present	624.0
Starting Water Surface Elevation ¹ (SWSE), ft	630.2	625.2	622.1
Peak Water Surface Elevation ¹ (PWSE), ft	631.4	626.7	623.9
Time to Peak, hr	No Discharge	10.6	No Discharge
Surface Area ² , ac	18.1	34.8	16.1
Storage ³ , ac-ft	19.5	52.9	29.0

Notes:

¹Elevations are based on the NAVD88 datum

²Surface area is defined as the water surface area at the PWSE

³Storage is defined as the volume between the SWSE and PWSE

AP2 no longer retains free water as the CCR surface impoundments was closed in 2020 [6]. Therefore, the spillways are no longer present and the information regarding these structures, as presented in the Initial HoC report, is no longer applicable to AP2.


CLOSING

This letter has been prepared to document Geosyntec's evaluation of changes that have occurred at AP1, AP2, the GMF GSP, and the GMF RP since the Initial HoC was developed, based on reasonably and readily available information provided by IPRG, observed by Geosyntec during the site visit, or generated by Geosyntec as part of subsequent calculations.

Sincerely,



Lucas P. Carr, P.E.
Senior Engineer



John Seymour, P.E.
Senior Principal

REFERENCES

- [1] AECOM, "History of Construction, USEPA Final CCR Rule, 40 CFR § 257.73(c), Coffeen Power Station, Coffeen, Illinois," October 2016.
- [2] United States Environmental Protection Agency, "40 CFR Parts 257 and 261, Hazardous and Solid Waste Management System, Disposal of Coal Combustion Residuals from Electric Utilities, Final Rule, 2015," 2015.
- [3] Illinois Environmental Protection Agency, "35 Ill. Adm. Code Part 845, Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments," Springfield, IL, 2021.
- [4] AECOM, "History of Construction, USEPA Final CCR Rule, 40 CFR § 257.73(c), Hennepin Power Station, Hennepin, Illinois," October 2016.
- [5] V. Modeer, "Closure Plan for Existing CCR Surface Impoundment, Coffeen Power Station, Illinois Power Generating Company, Ash Pond No. 2," October 17, 2016.
- [6] D. Tickner, "Coffeen Power Station; Ash Pond No. 2; Notification of Completion of Closure," December 17, 2020.

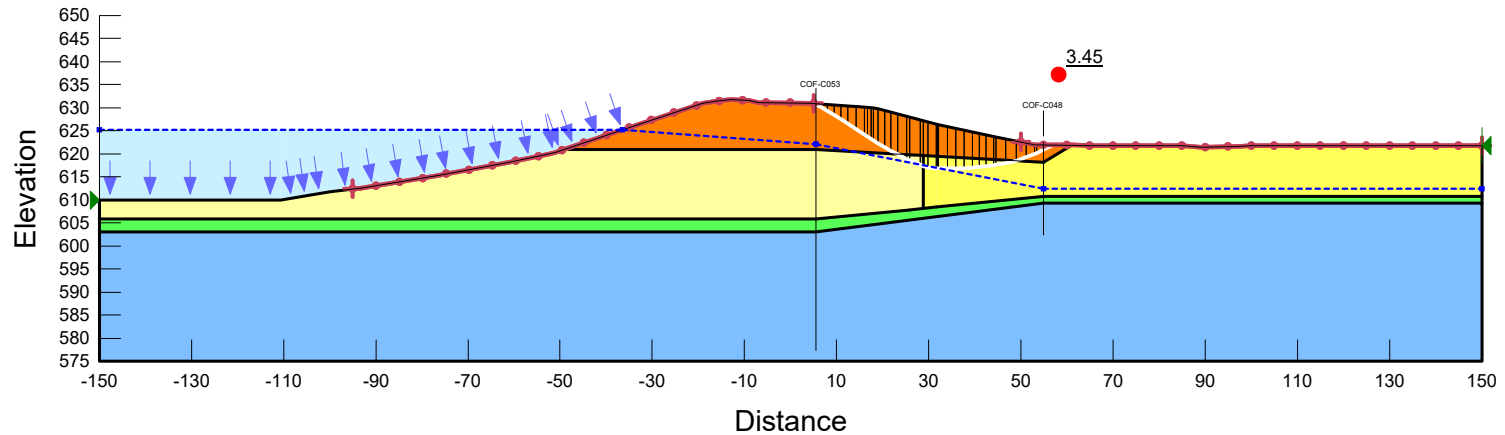
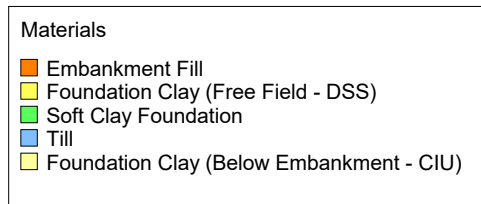
Attachment D

Periodic Structural Stability and Safety Factor Assessment Analyses

Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 13+50
 Slope Stability - Static Drained

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 32 ° Phi-B: 0 ° Piezometric Line: 1



\\STLOUISMO-01\Data\Company\Projects_post_2014\GLP8027_CCR_ReCert500_Technical\502_COF\502d_Periodic_Report\GMF\Revised SFA\13+50\
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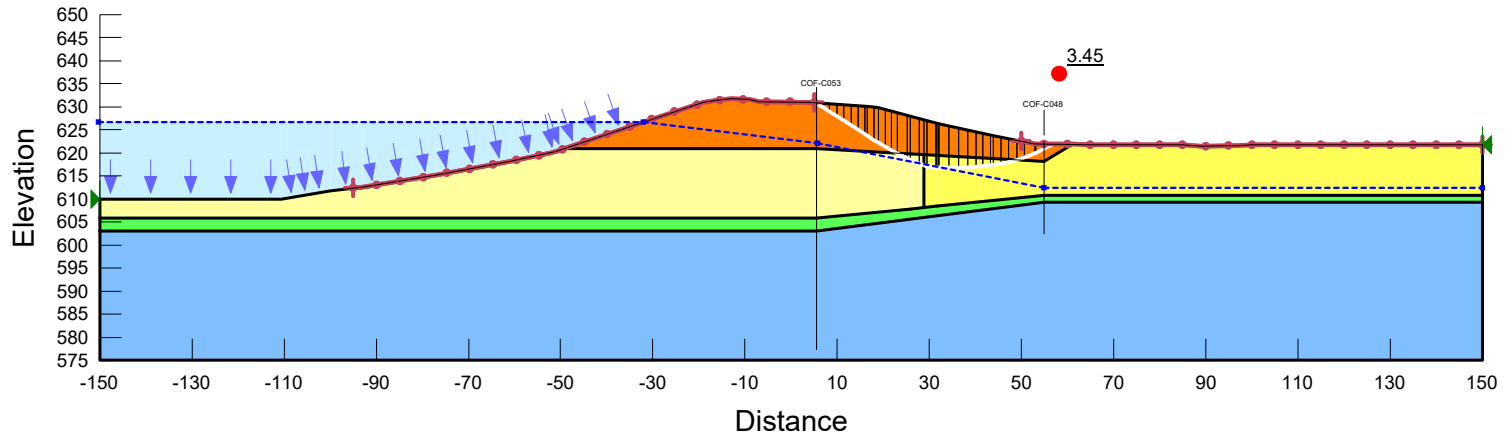
Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 13+50
 Slope Stability - Static Drained

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/13/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 32 ° Phi-B: 0 ° Piezometric Line: 1

Materials

- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)



Dynegey Coffeen
 GMF Gypsum Stack Pond
 Station 13+50
 Peak Undrained Soil Strengths
 Pseudostatic - Entry-Exit

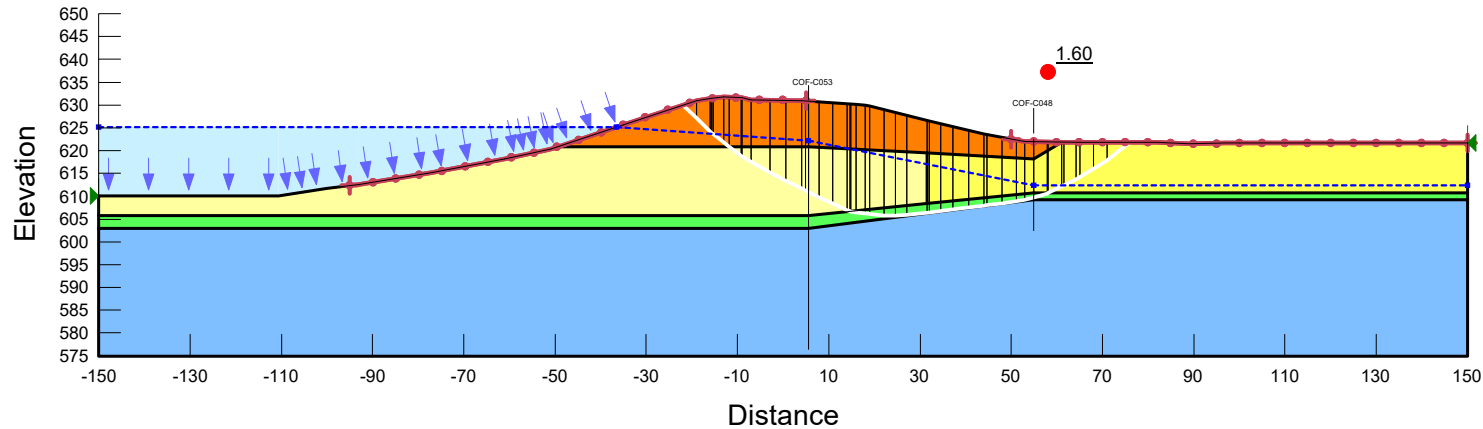
Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 275 psf Piezometric Line: 1
 Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Materials

- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)

Seismic Load:
 $kh = 0.13 \text{ g}$



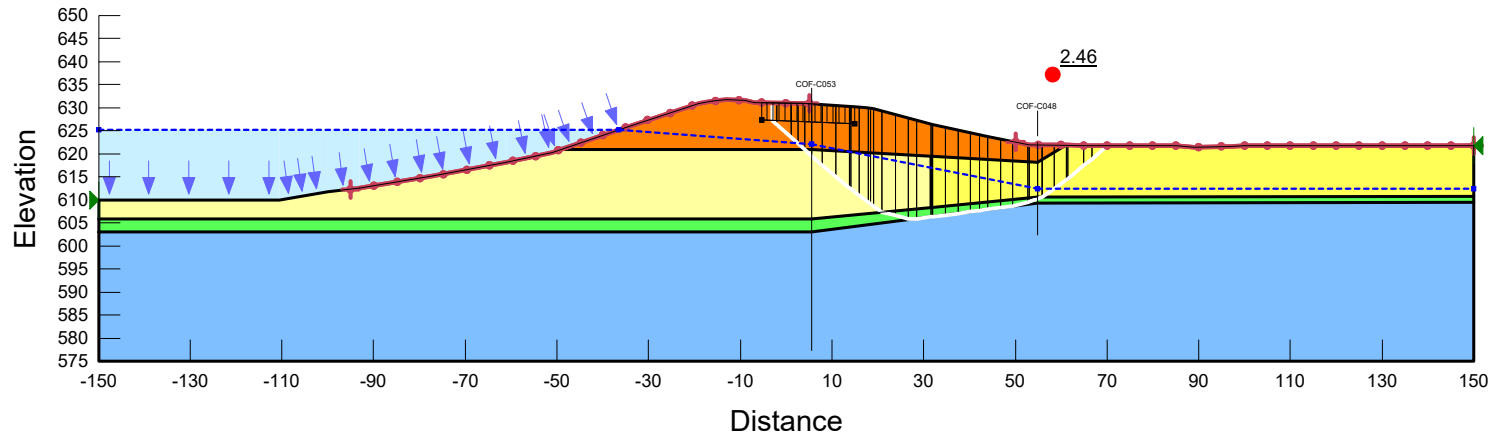
Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 13+50
 Slope Stability - Post Earthquake

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.16 Minimum Strength: 200 psf Piezometric Line: 1
 Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Materials

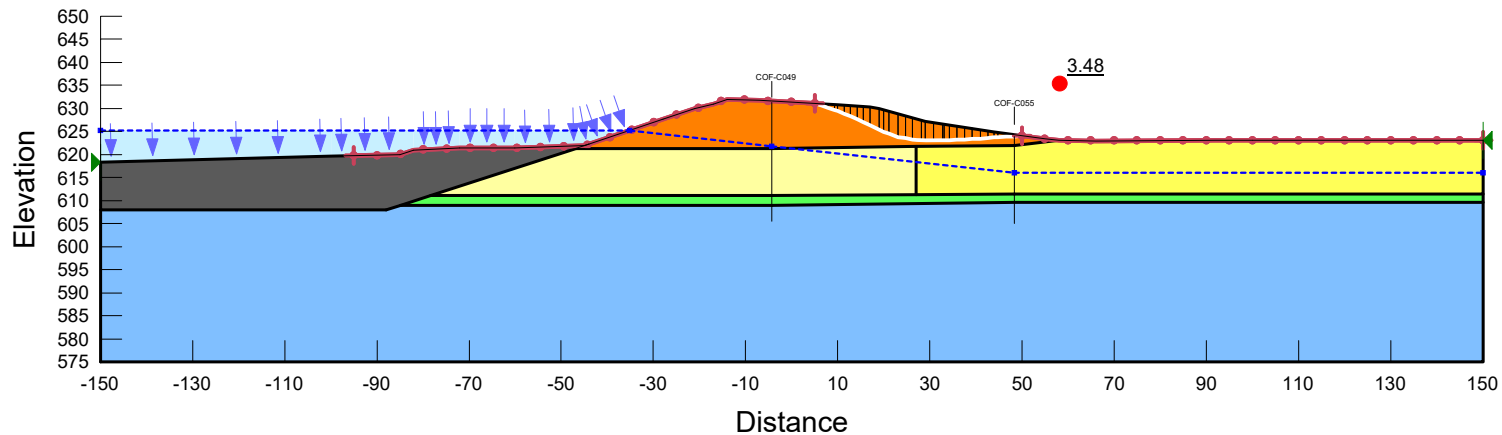
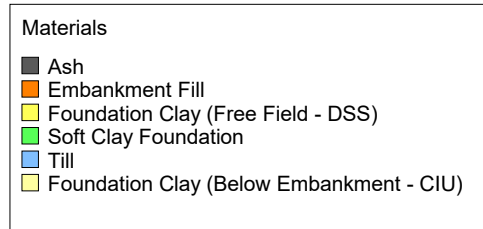
- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)



Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 22+50
 Slope Stability - Static Drained

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

Name: Ash Model: Mohr-Coulomb Unit Weight: 112 pcf Cohesion: 0 psf Phi: 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 32 ° Phi-B: 0 ° Piezometric Line: 1

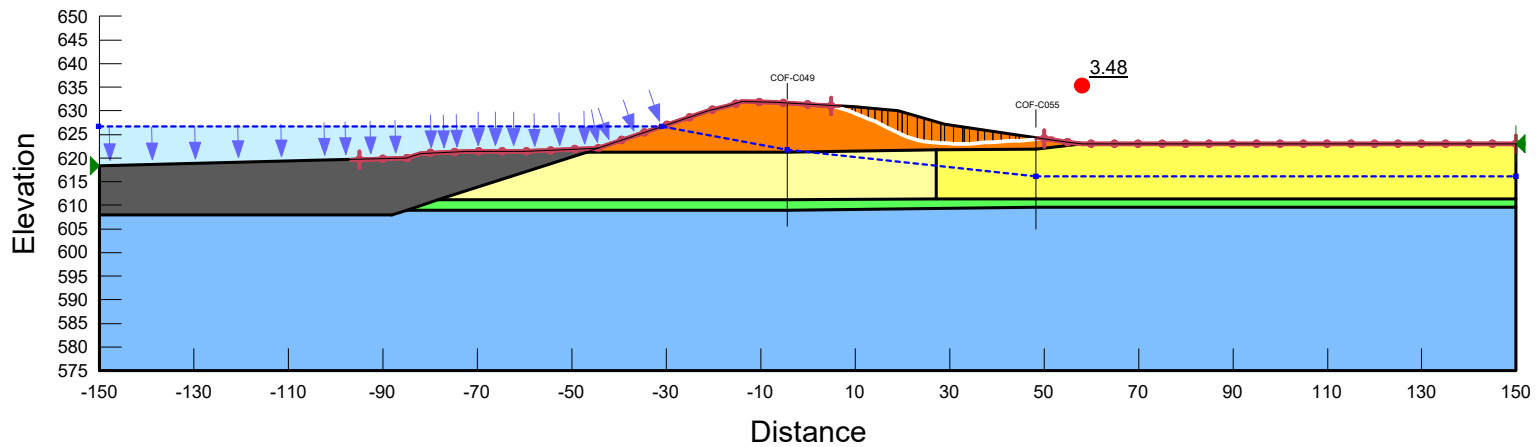
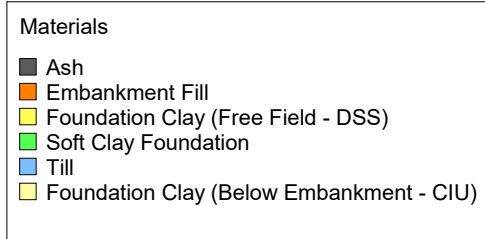


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Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 22+50
 Slope Stability - Static Drained

Design by: Lucas Carr
 Modified by: Betty Tesfu
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/13/2021

Name: Ash Model: Mohr-Coulomb Unit Weight: 112 pcf Cohesion: 0 psf Phi: 32 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 40 ° Phi-B: 0 ° Piezometric Line: 1
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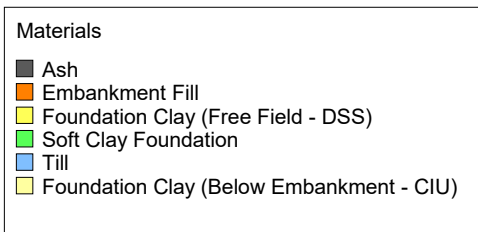


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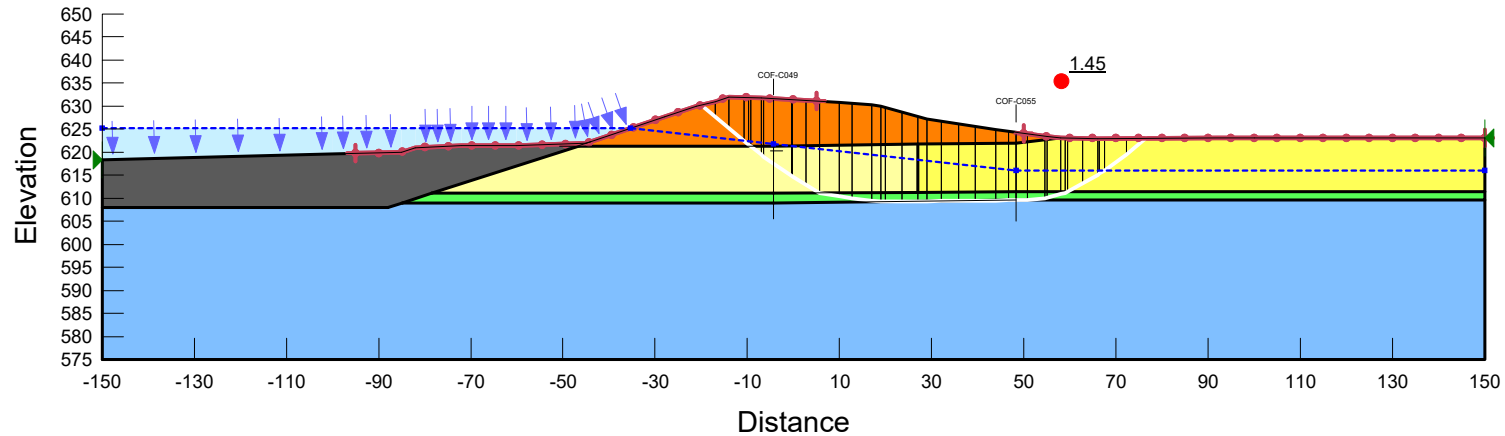
Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 22+50
 Peak Undrained Soil Strengths
 Pseudostatic - Entry & Exit

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

Name: Ash Model: S=f(overburden) Unit Weight: 112 pcf Tau/Sigma Ratio: 0.4 Minimum Strength: 0 psf Piezometric Line: 1
 Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.24 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.22 Minimum Strength: 275 psf Piezometric Line: 1
 Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.39 Minimum Strength: 700 psf Piezometric Line: 1



Seismic Load:
 $kh = 0.13 g$



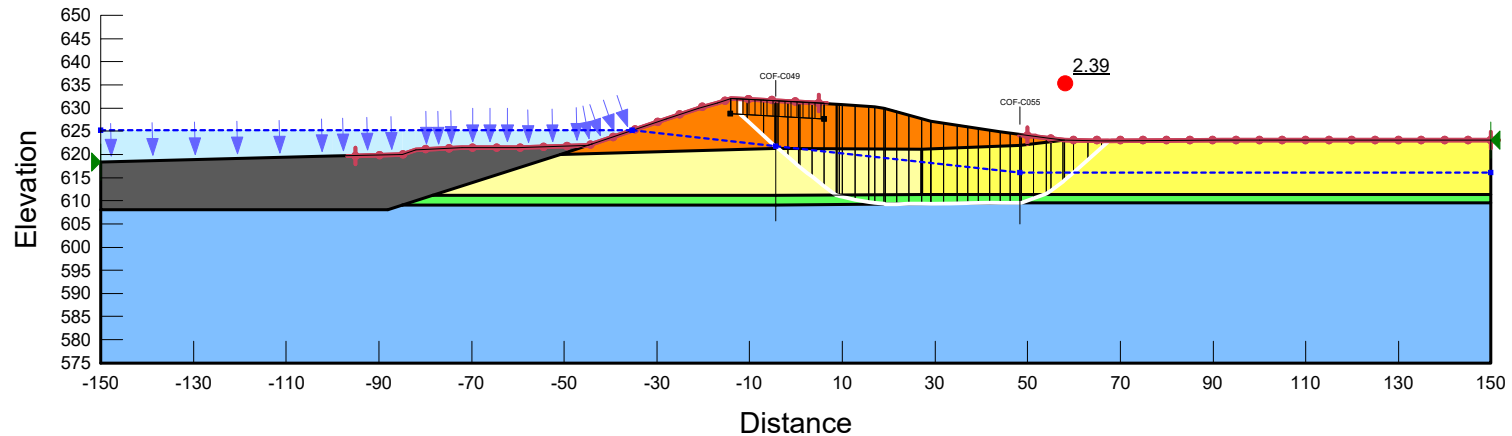
Coffeen Power PlantGMF Gypsum Stack Pond
 Station 22+50
 Slope Stability - Post Earthquake

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

Name: Ash Model: S=f(overburden) Unit Weight: 112 pcf Tau/Sigma Ratio: 0.05 Minimum Strength: 0 psf Piezometric Line: 1
 Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.24 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.13 Minimum Strength: 200 psf Piezometric Line: 1
 Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.39 Minimum Strength: 700 psf Piezometric Line: 1

Materials

- Ash
- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)



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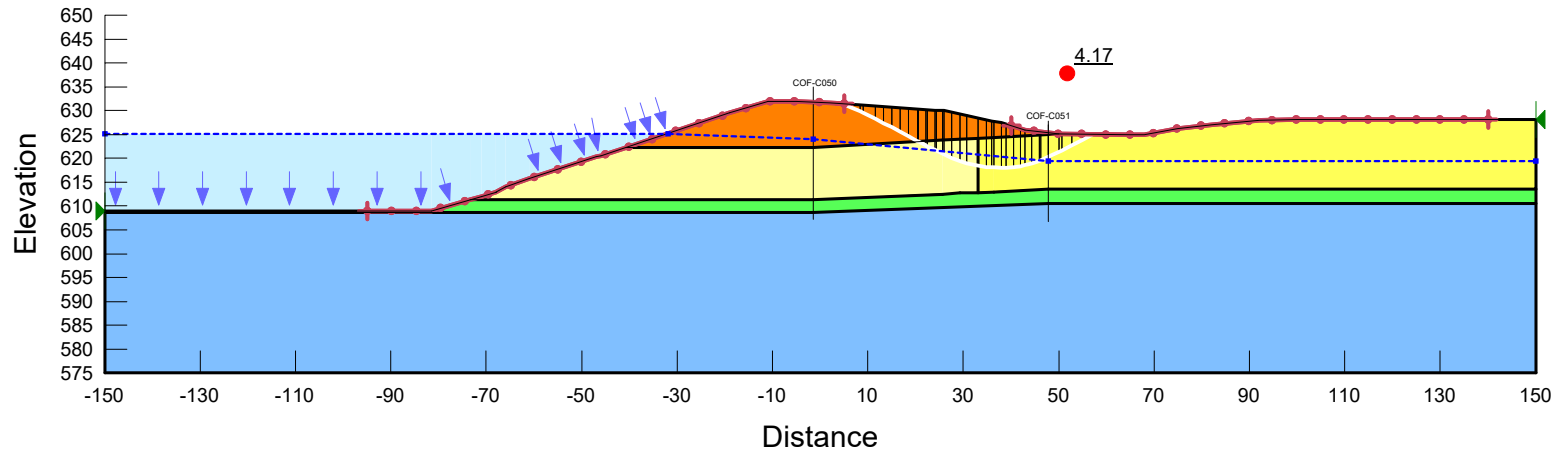
Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 46+50
 Static Drained - Entry & Exit

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/8/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1

Materials

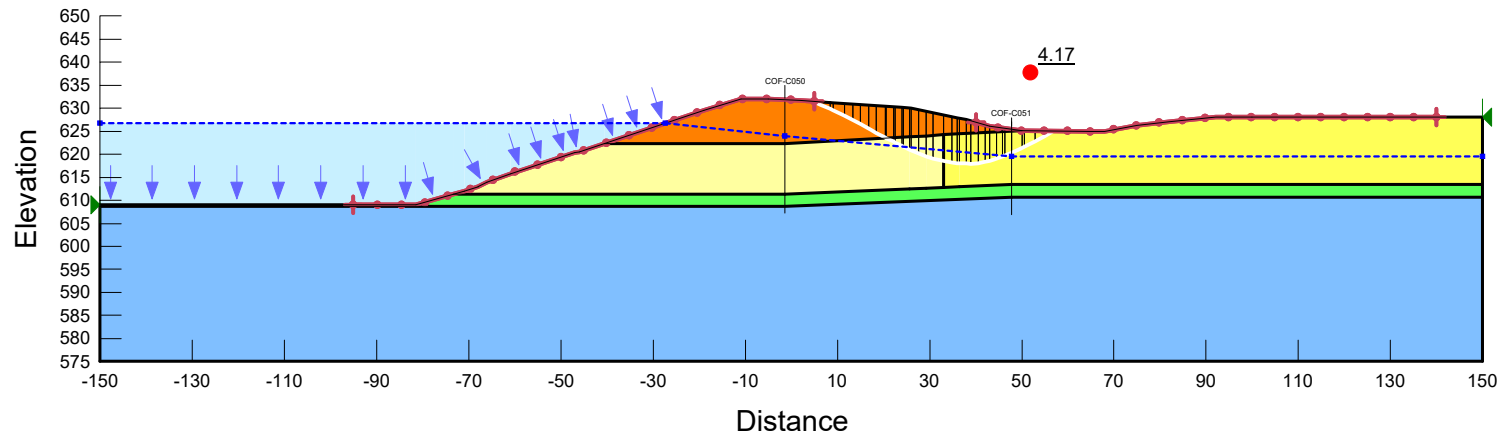
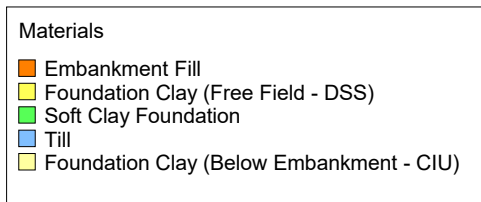
- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)



Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 46+50
 Surcharge Pool - Entry & Exit

Design by: Lucas Carr
 Modified by: Betty Tesfu
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/13/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1



Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 46+50
 Peak Undrained Soil Strengths
 Name: Pseudostatic - Entry & Exit

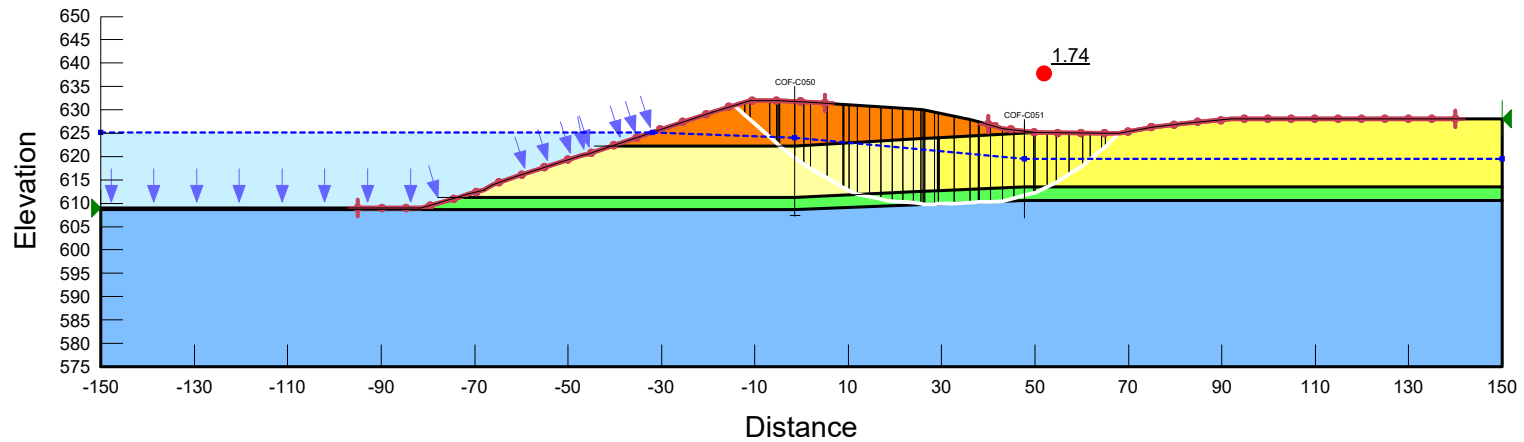
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 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

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 Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 275 psf Piezometric Line: 1
 Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Materials

- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)

Seismic Load
 $kh = 0.13 \text{ g}$



\\STLOUISMO-01\Data\Company\Projects_post_2014\GLP8027_CCR_ReCert500_Technical\502_COE\502d_Periodic_Report\GMF\Revised SFA\46+50\
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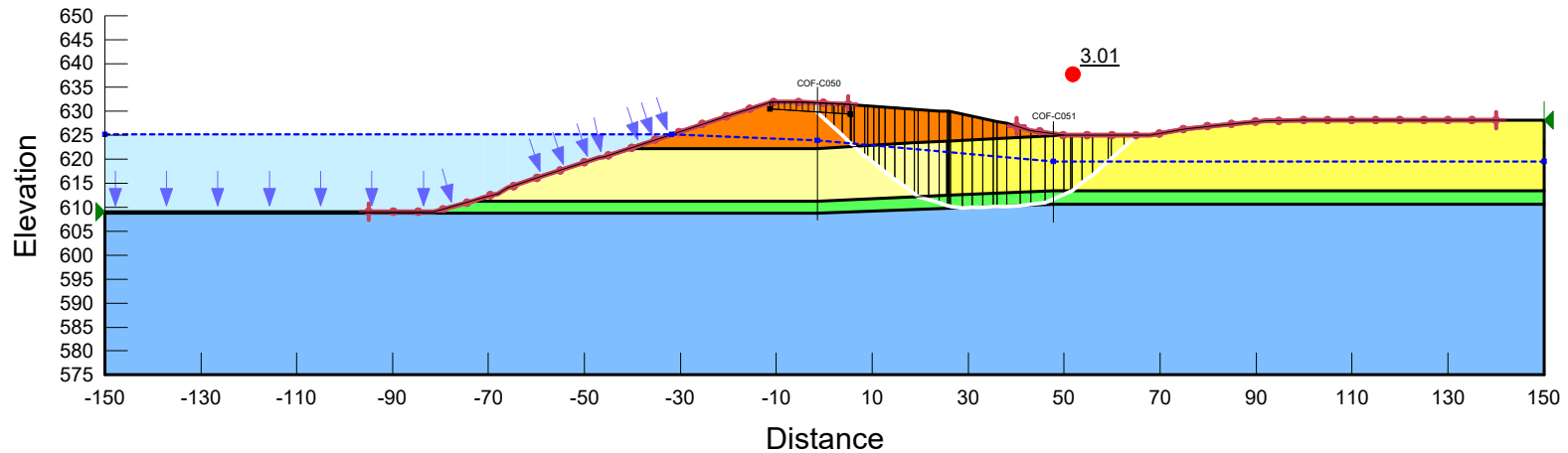
Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 46+50
 Slope Stability - Post Earthquake

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

Name: Embankment Fill Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.6 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.28 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.16 Minimum Strength: 200 psf Piezometric Line: 1
 Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1

Materials

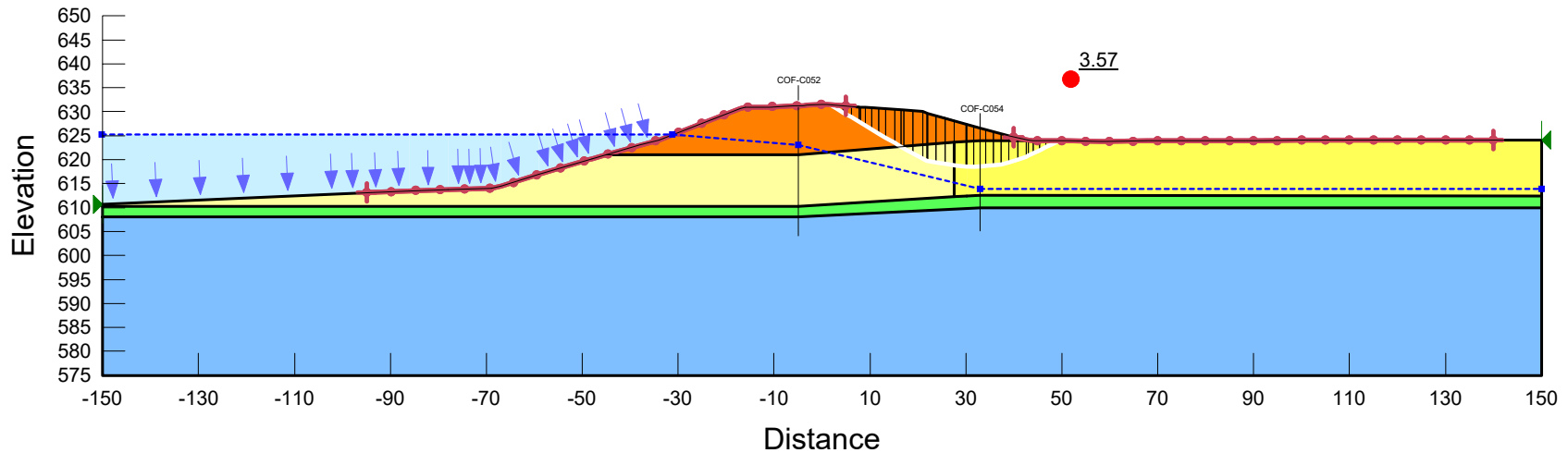
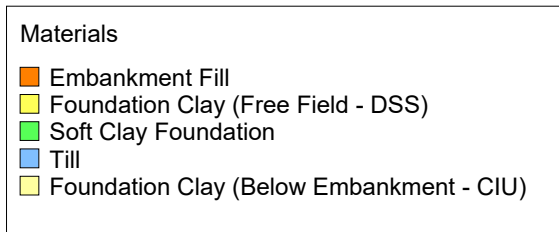
- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)



Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 58+00
 Slope Stability - Static Drained

Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/8/2021

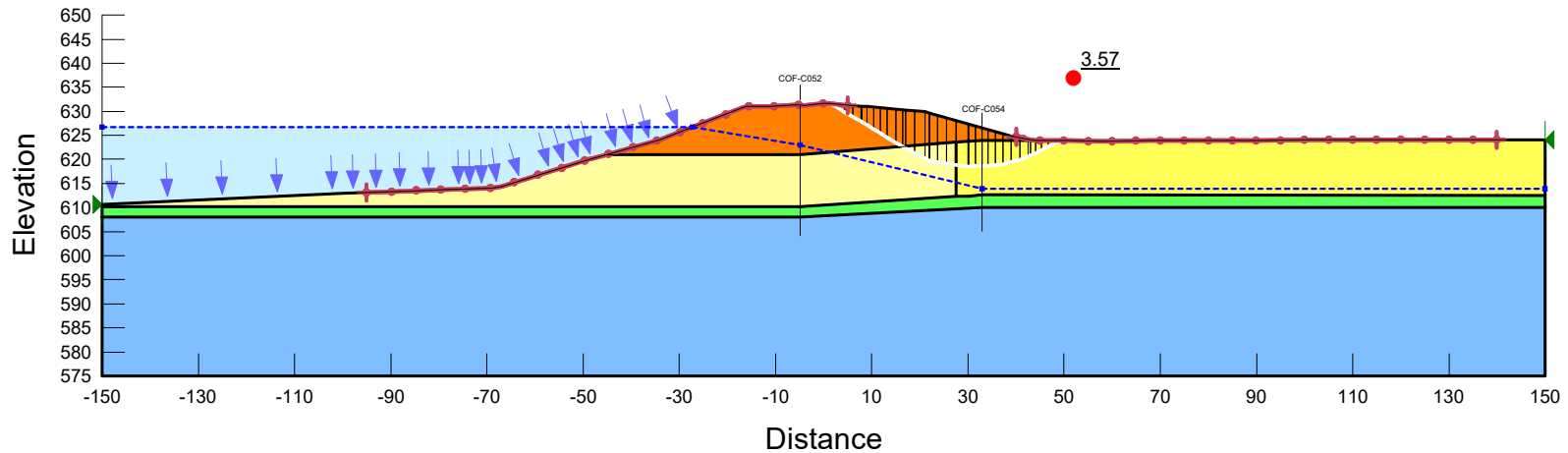
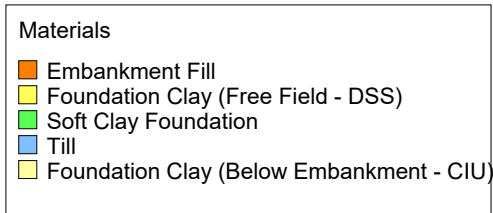
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 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1



Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 58+00
 Slope Stability - Static Drained

Design by: Lucas Carr
 Modified by: Betty Tesfu
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/13/2021

Name: Embankment Fill Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 31 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion': 0 psf Phi': 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion': 0 psf Phi': 32 ° Phi-B: 0 ° Piezometric Line: 1



Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 58+00
 Peak Undrained Soil Strengths
 Pseudostatic - Entry-Exit

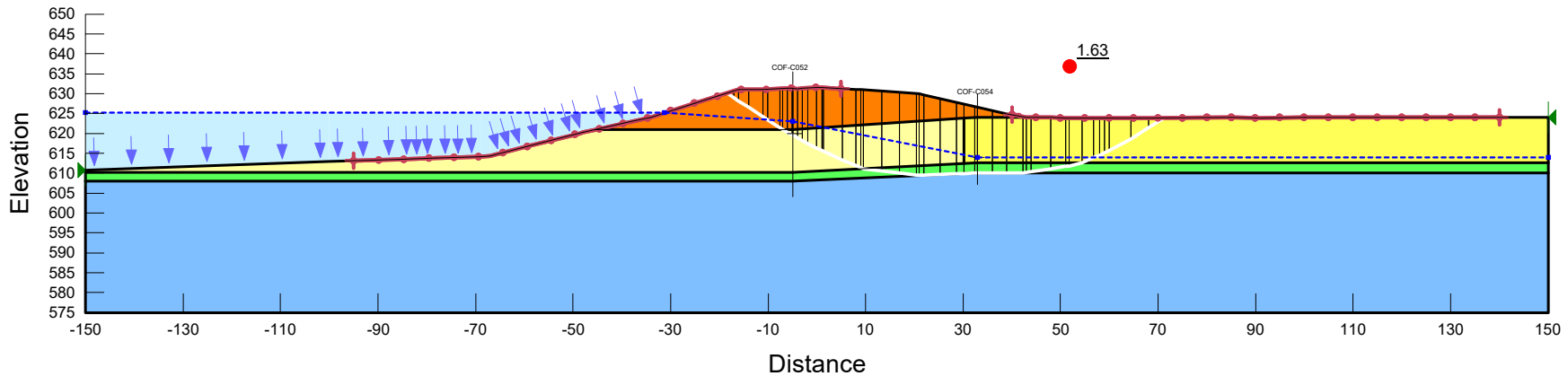
Design by: Lucas Carr
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/14/2021

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 Name: Foundation Clay (Free Field - DSS) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.24 Minimum Strength: 450 psf Piezometric Line: 1
 Name: Soft Clay Foundation Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.23 Minimum Strength: 275 psf Piezometric Line: 1
 Name: Till Model: S=f(overburden) Unit Weight: 135 pcf Tau/Sigma Ratio: 0.45 Minimum Strength: 700 psf Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: S=f(overburden) Unit Weight: 125 pcf Tau/Sigma Ratio: 0.39 Minimum Strength: 700 psf Piezometric Line: 1

Materials

- Embankment Fill
- Foundation Clay (Free Field - DSS)
- Soft Clay Foundation
- Till
- Foundation Clay (Below Embankment - CIU)

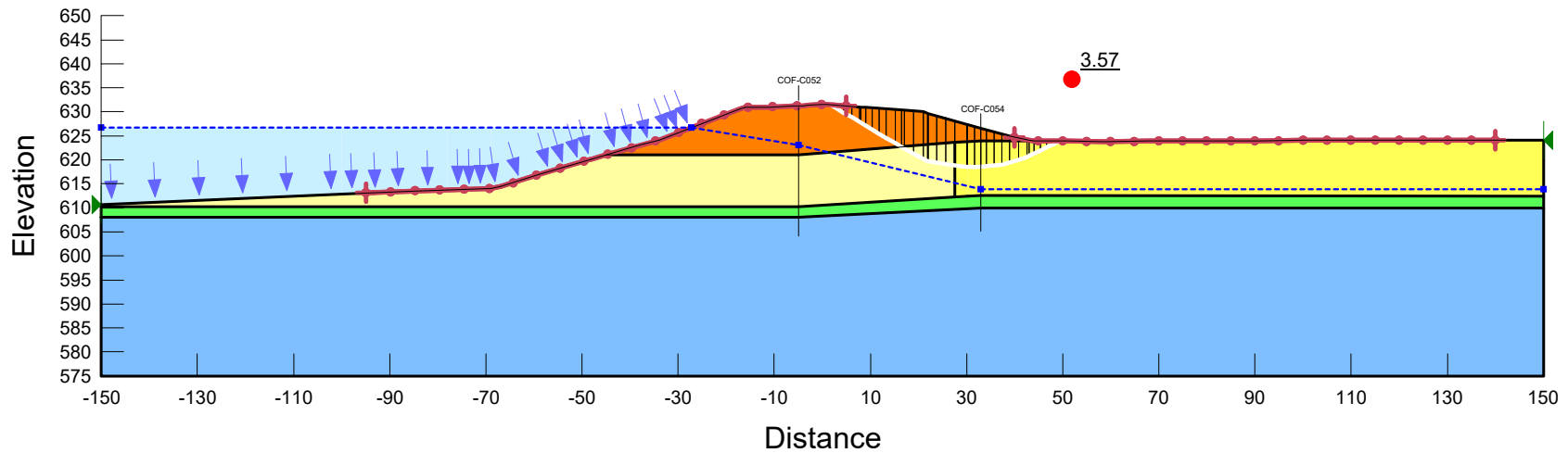
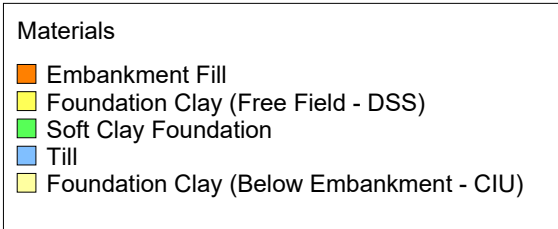
Seismic Load
 $kh = 0.13 \text{ g}$



Coffeen Power Plant
 GMF Gypsum Stack Pond
 Station 58+00
 Slope Stability - Static Drained

Design by: Lucas Carr
 Modified by: Betty Tesfu
 Checked by: Nick Sanna
 Modified by: Pourya Kargar
 Checked by: Zachary Fallert
 Date: 9/13/2021

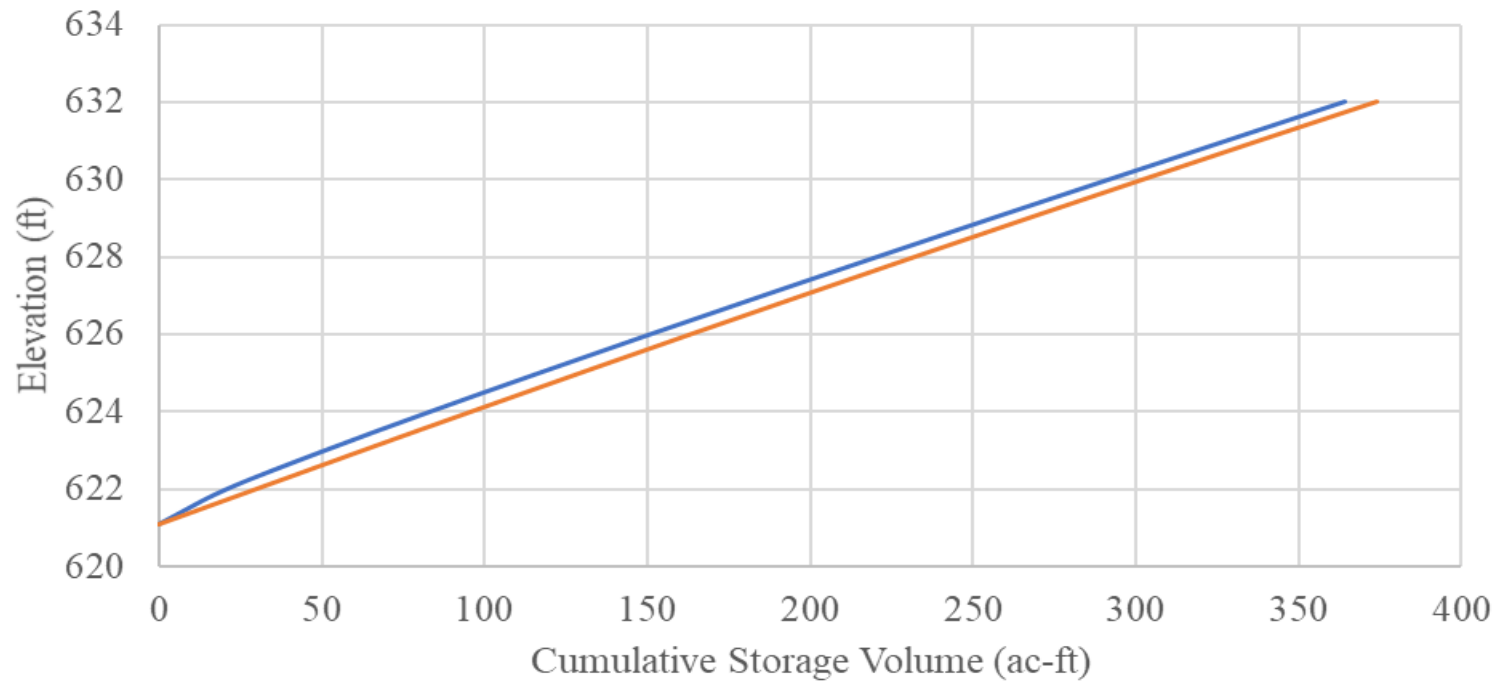
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 Name: Foundation Clay (Free Field - DSS) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Soft Clay Foundation Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 30 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Till Model: Mohr-Coulomb Unit Weight: 135 pcf Cohesion: 0 psf Phi: 40 ° Phi-B: 0 ° Piezometric Line: 1
 Name: Foundation Clay (Below Embankment - CIU) Model: Mohr-Coulomb Unit Weight: 125 pcf Cohesion: 0 psf Phi: 32 ° Phi-B: 0 ° Piezometric Line: 1



Attachment E

Periodic Inflow Design Flood Control System Plan Analyses

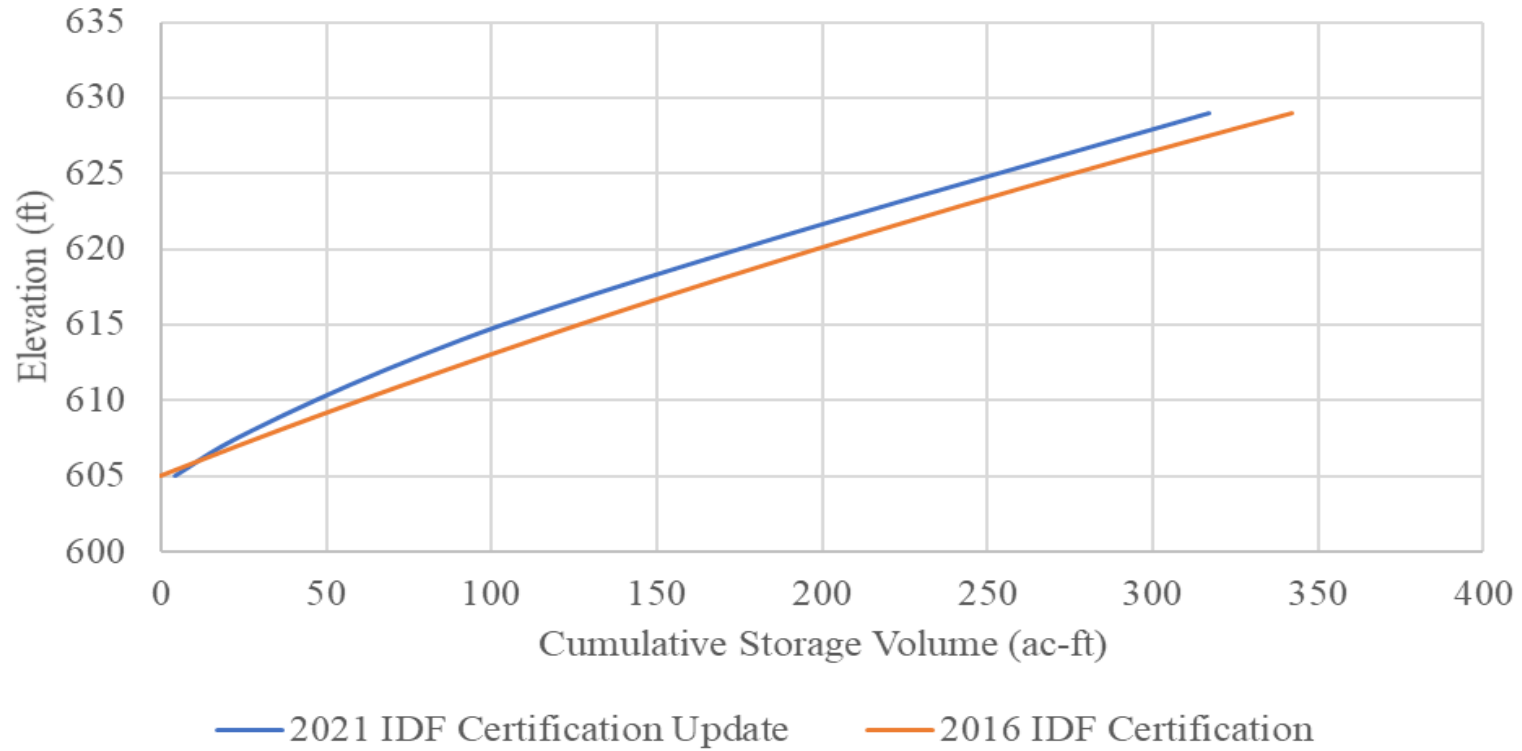
Coffeen GMF Pond - Cumulative Storage



— 2021 IDF Certification Update — 2016 IDF Certification

COFFEEN GMF GSP CUMULATIVE STORAGE PERIODIC CERTIFICATION COFFEEN POWER PLANT COFFEEN, ILLINOIS	
GLP8027	9/14/2021
Figure E-1	

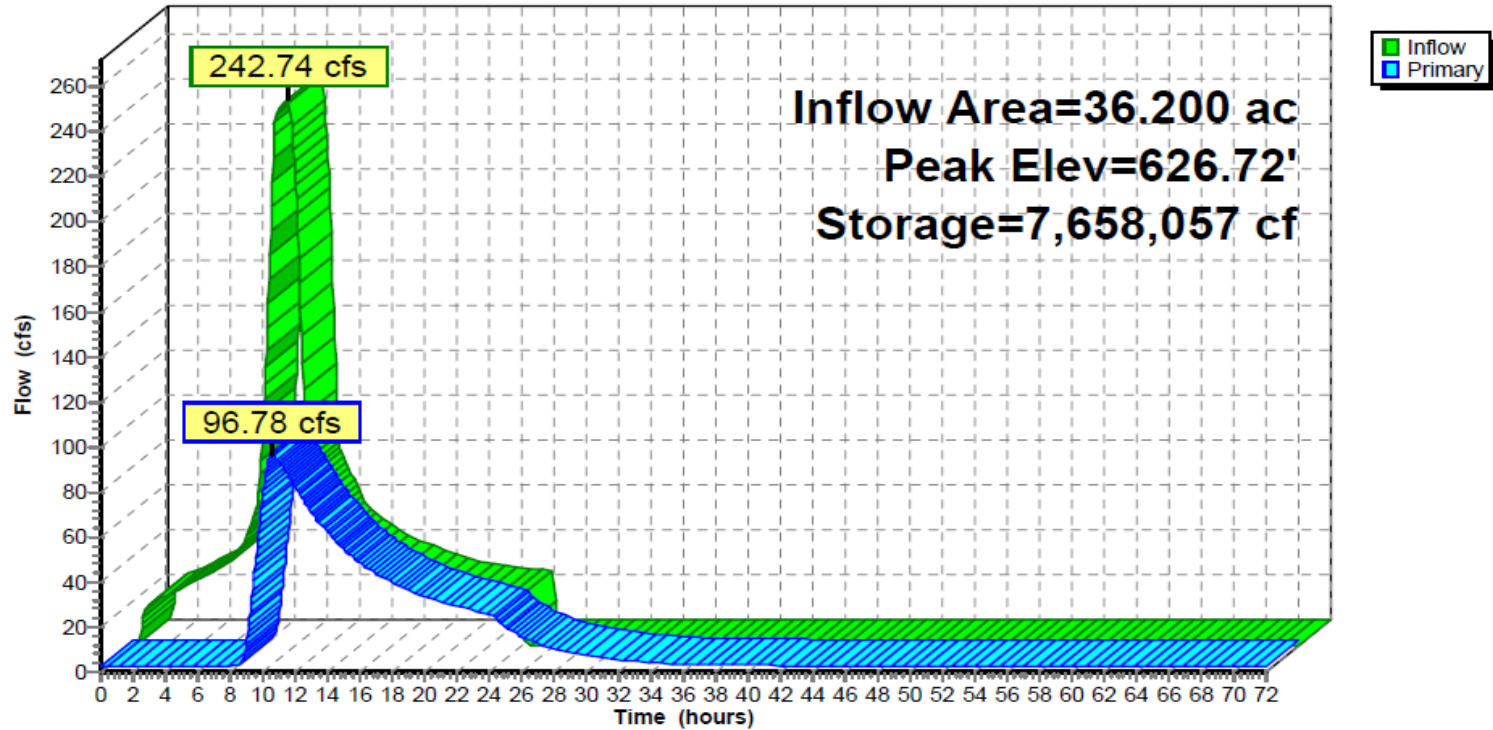
Coffeen GMFR Pond - Cumulative Storage



COFFEEN GMF RECYCLE POND CUMULATIVE STORAGE PERIODIC CERTIFICATION COFFEEN POWER PLANT COFFEEN, ILLINOIS	
GLP8027	9/14/2021
Figure E-2	

Pond 2P: Gypsum Stack Pond

Hydrograph



GMF GSP IDF HYDROGRAPH
PERIODIC CERTIFICATION
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

Geosyntec
consultants

Figure

E-3

GLP8027

9/14/2021

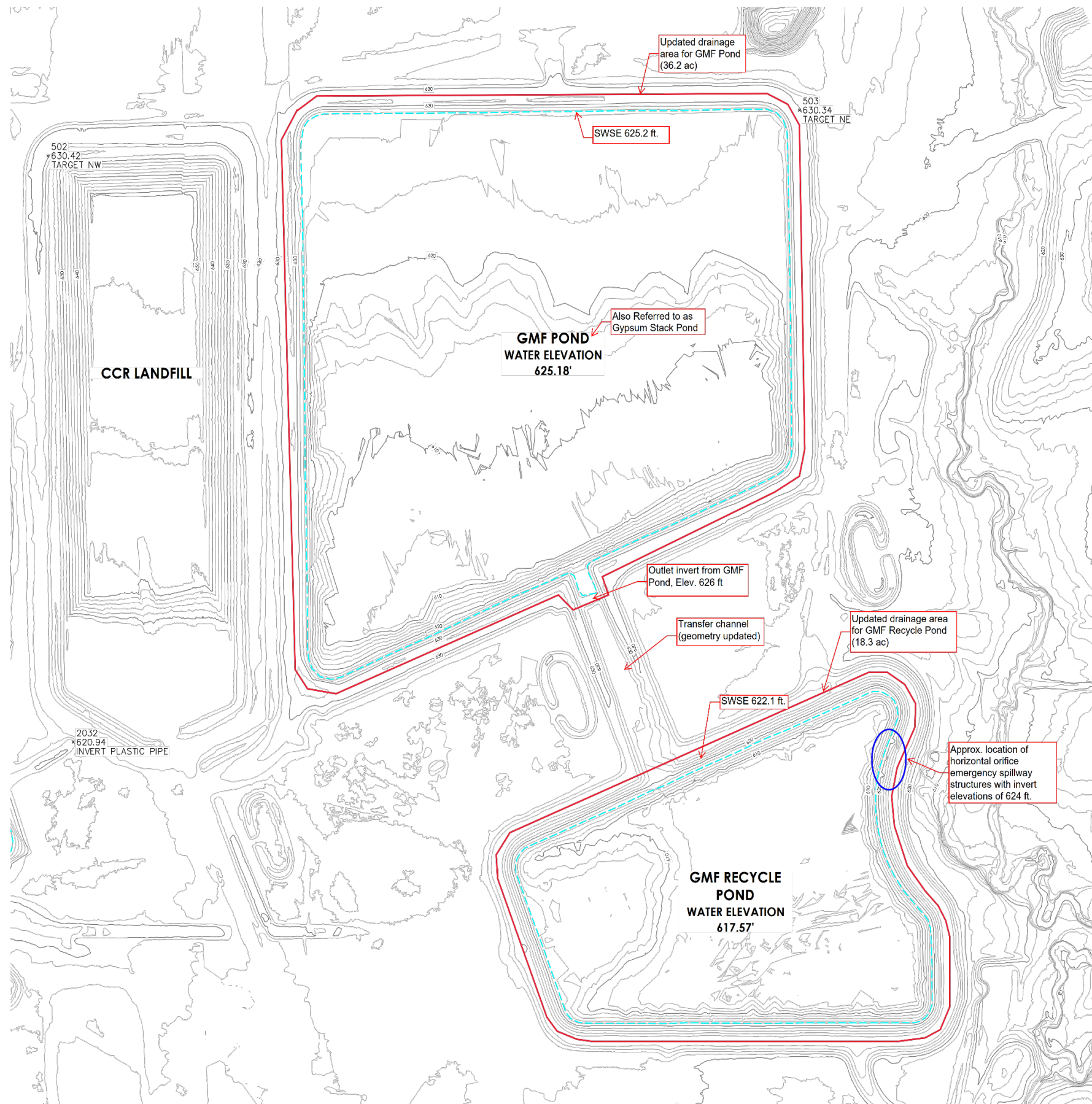

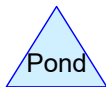
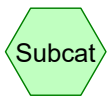
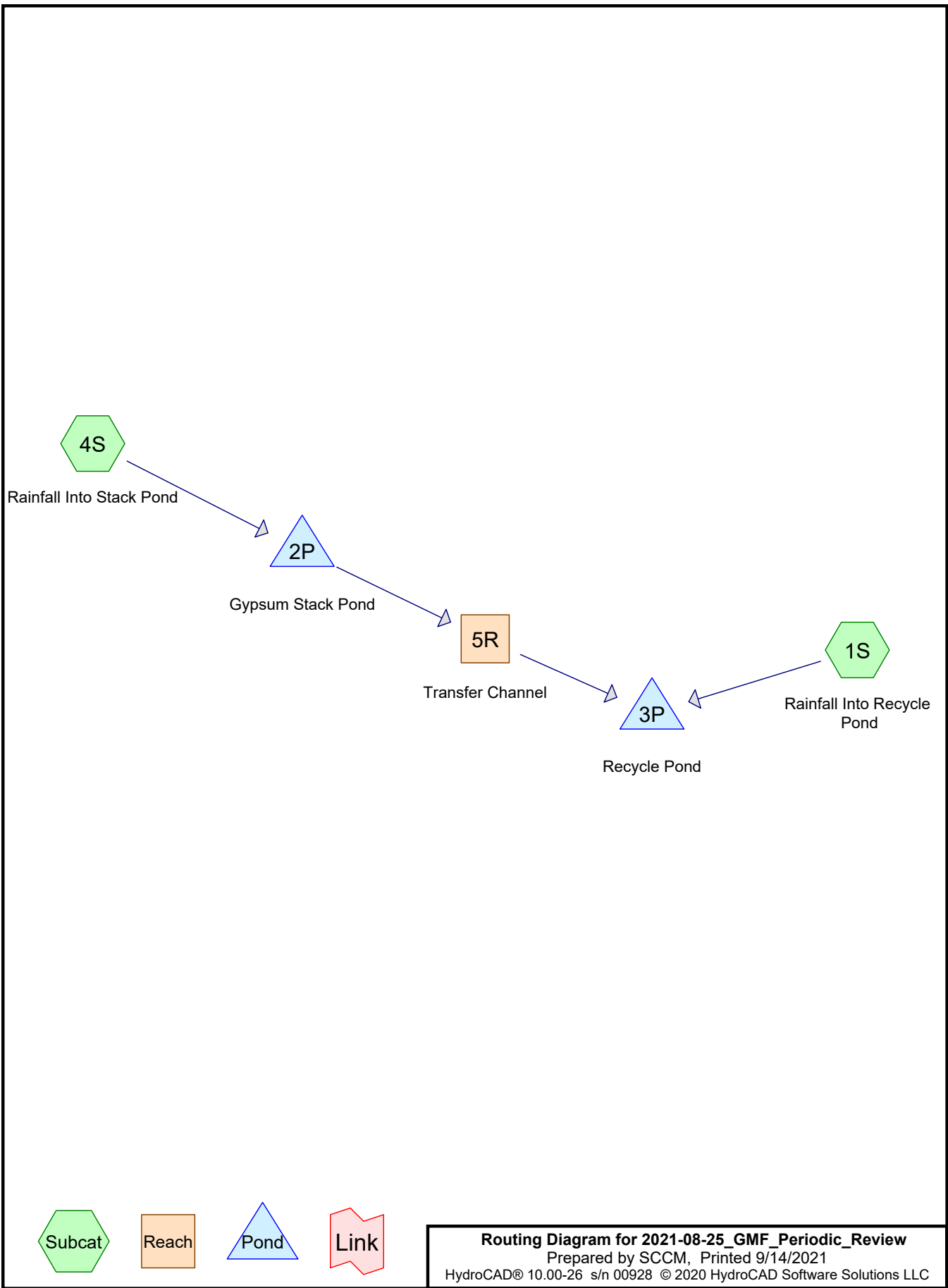


Figure based on IngenAE 2020 Site Topo

NOT TO SCALE

Coffeen Power Plant GMF and GMFR Pond Hydrologic Workmap	
	
GLP8027	September 2021
Figure E-4	



Routing Diagram for 2021-08-25_GMF_Periodic_Review
 Prepared by SCCM, Printed 9/14/2021
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2021-08-25_GMF_Periodic_Review

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Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
54.500	98	Water Surface, HSG C (1S, 4S)
54.500	98	TOTAL AREA

2021-08-25_GMF_Periodic_Review

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Page 3

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
54.500	HSG C	1S, 4S
0.000	HSG D	
0.000	Other	
54.500		TOTAL AREA

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Page 4

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	54.500	0.000	0.000	54.500	Water Surface	1S, 4S
0.000	0.000	54.500	0.000	0.000	54.500	TOTAL AREA	

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Page 5

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	2P	619.00	617.60	580.0	0.0024	0.013	14.0	0.0	0.0
2	3P	615.00	613.00	92.0	0.0217	0.013	45.0	0.0	0.0
3	3P	615.00	613.00	92.0	0.0217	0.013	45.0	0.0	0.0
4	3P	615.00	613.00	92.0	0.0217	0.013	45.0	0.0	0.0

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1S: Rainfall Into Recycle Runoff Area=18.300 ac 100.00% Impervious Runoff Depth=34.01"
Tc=6.0 min CN=98 Runoff=122.71 cfs 51.860 af

Subcatchment 4S: Rainfall Into Stack Runoff Area=36.200 ac 100.00% Impervious Runoff Depth=34.01"
Tc=6.0 min CN=98 Runoff=242.74 cfs 102.586 af

Reach 5R: Transfer Channel Avg. Flow Depth=0.48' Max Vel=5.94 fps Inflow=96.78 cfs 82.274 af
n=0.010 L=450.0' S=0.0044 '/' Capacity=7,454.18 cfs Outflow=96.77 cfs 82.255 af

Pond 2P: Gypsum Stack Pond Peak Elev=626.72' Storage=7,658,057 cf Inflow=242.74 cfs 102.586 af
Outflow=96.78 cfs 82.274 af

Pond 3P: Recycle Pond Peak Elev=624.70' Storage=10,813,783 cf Inflow=183.51 cfs 134.115 af
Primary=38.24 cfs 34.428 af Secondary=38.24 cfs 34.428 af Tertiary=38.24 cfs 34.428 af Outflow=114.71 cfs 103.284 af

Total Runoff Area = 54.500 ac Runoff Volume = 154.445 af Average Runoff Depth = 34.01"
0.00% Pervious = 0.000 ac 100.00% Impervious = 54.500 ac

Summary for Subcatchment 1S: Rainfall Into Recycle Pond

Runoff = 122.71 cfs @ 9.62 hrs, Volume= 51.860 af, Depth=34.01"

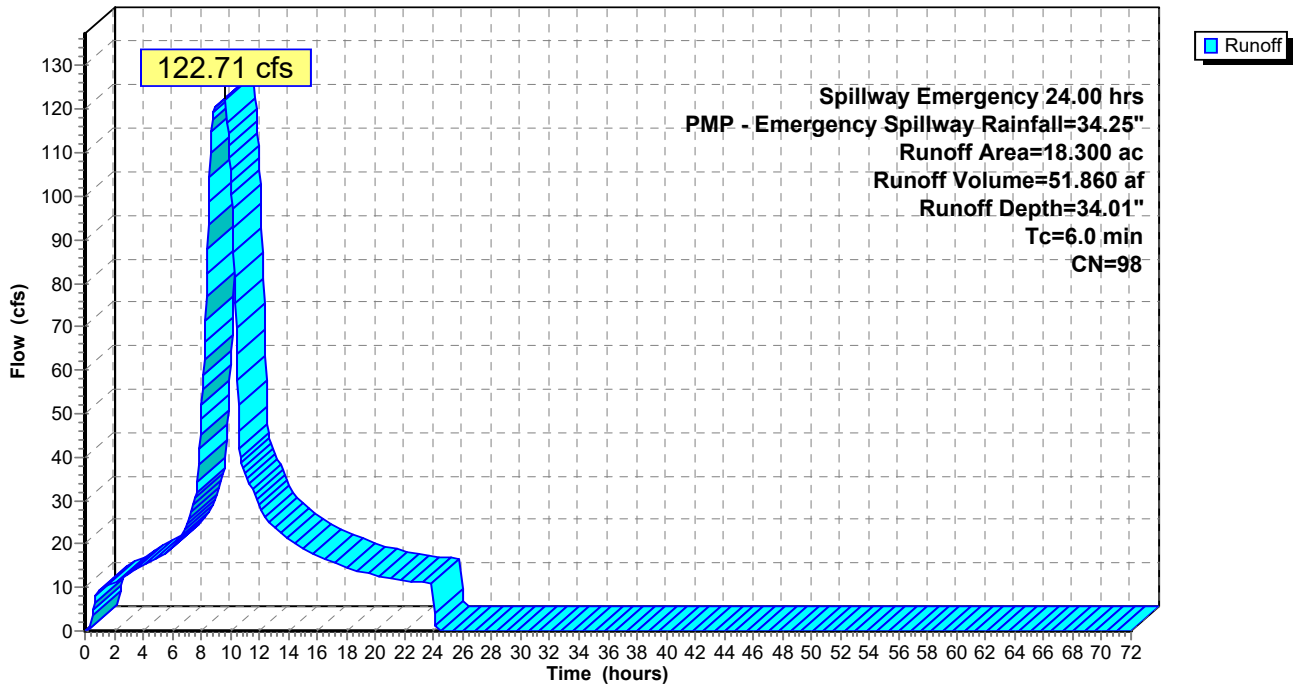
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Spillway Emergency 24.00 hrs PMP - Emergency Spillway Rainfall=34.25"

Area (ac)	CN	Description
* 18.300	98	Water Surface, HSG C
18.300		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Fall

Subcatchment 1S: Rainfall Into Recycle Pond

Hydrograph



Summary for Subcatchment 4S: Rainfall Into Stack Pond

Runoff = 242.74 cfs @ 9.62 hrs, Volume= 102.586 af, Depth=34.01"

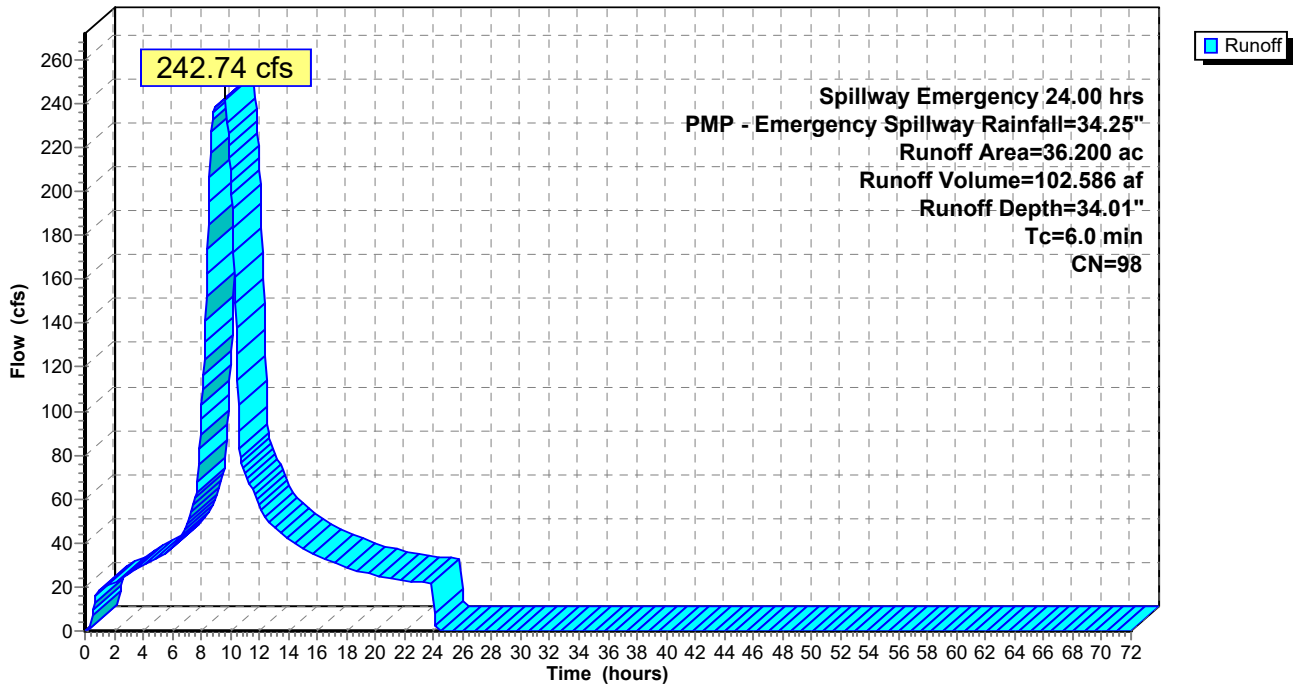
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Spillway Emergency 24.00 hrs PMP - Emergency Spillway Rainfall=34.25"

Area (ac)	CN	Description
36.200	98	Water Surface, HSG C
36.200		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Direct Fall

Subcatchment 4S: Rainfall Into Stack Pond

Hydrograph



Summary for Reach 5R: Transfer Channel

Inflow Area = 36.200 ac, 100.00% Impervious, Inflow Depth > 27.27" for PMP - Emergency Spillway event
 Inflow = 96.78 cfs @ 10.58 hrs, Volume= 82.274 af
 Outflow = 96.77 cfs @ 10.59 hrs, Volume= 82.255 af, Atten= 0%, Lag= 0.8 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Max. Velocity= 5.94 fps, Min. Travel Time= 1.3 min
 Avg. Velocity = 2.28 fps, Avg. Travel Time= 3.3 min

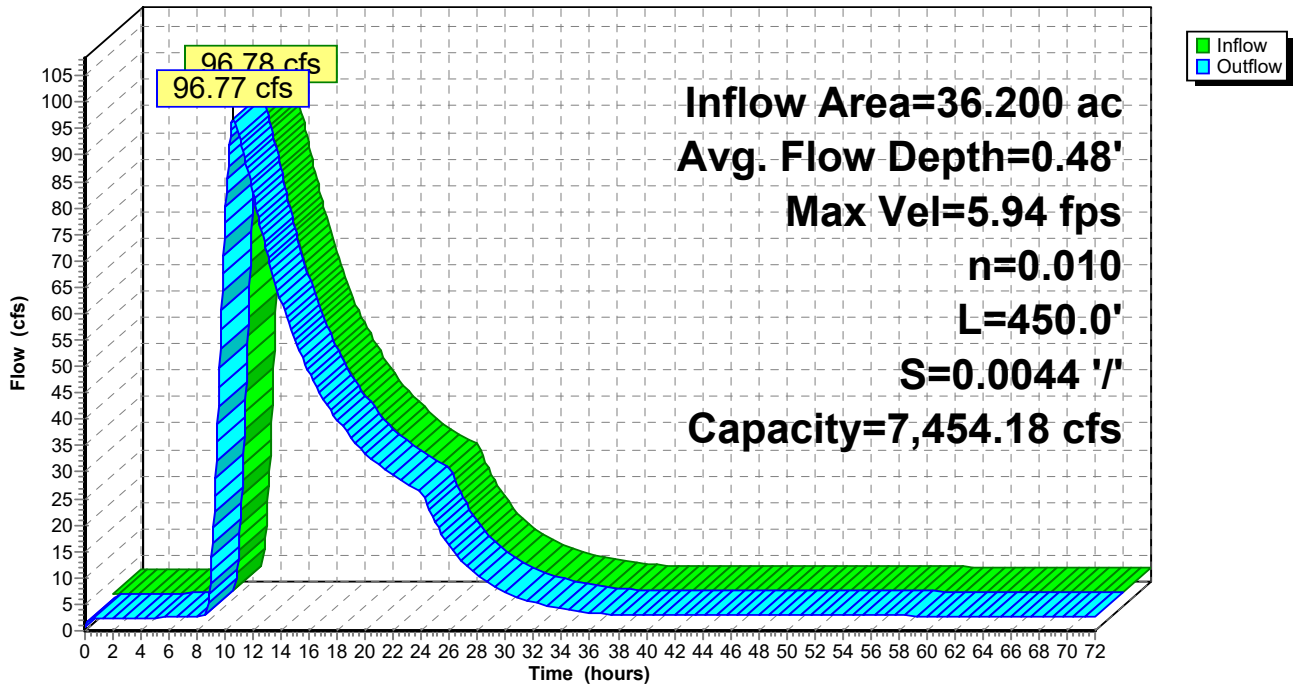
Peak Storage= 7,334 cf @ 10.59 hrs
 Average Depth at Peak Storage= 0.48'
 Bank-Full Depth= 6.00' Flow Area= 279.0 sf, Capacity= 7,454.18 cfs

32.70' x 6.00' deep channel, n= 0.010 PVC, smooth interior
 Side Slope Z-value= 3.0 1.6 '/' Top Width= 60.30'
 Length= 450.0' Slope= 0.0044 '/'
 Inlet Invert= 624.00', Outlet Invert= 622.00'



Reach 5R: Transfer Channel

Hydrograph



Summary for Pond 2P: Gypsum Stack Pond

[44] Hint: Outlet device #2 is below defined storage

Inflow Area = 36.200 ac, 100.00% Impervious, Inflow Depth = 34.01" for PMP - Emergency Spillway event
 Inflow = 242.74 cfs @ 9.62 hrs, Volume= 102.586 af
 Outflow = 96.78 cfs @ 10.58 hrs, Volume= 82.274 af, Atten= 60%, Lag= 57.2 min
 Primary = 96.78 cfs @ 10.58 hrs, Volume= 82.274 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Starting Elev= 625.18' Surf.Area= 0 sf Storage= 5,353,910 cf
 Peak Elev= 626.72' @ 10.58 hrs Surf.Area= 0 sf Storage= 7,658,057 cf (2,304,147 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 554.6 min (1,215.4 - 660.8)

Volume	Invert	Avail.Storage	Storage Description
#1	621.10'	15,871,813 cf	Custom Stage Data Listed below

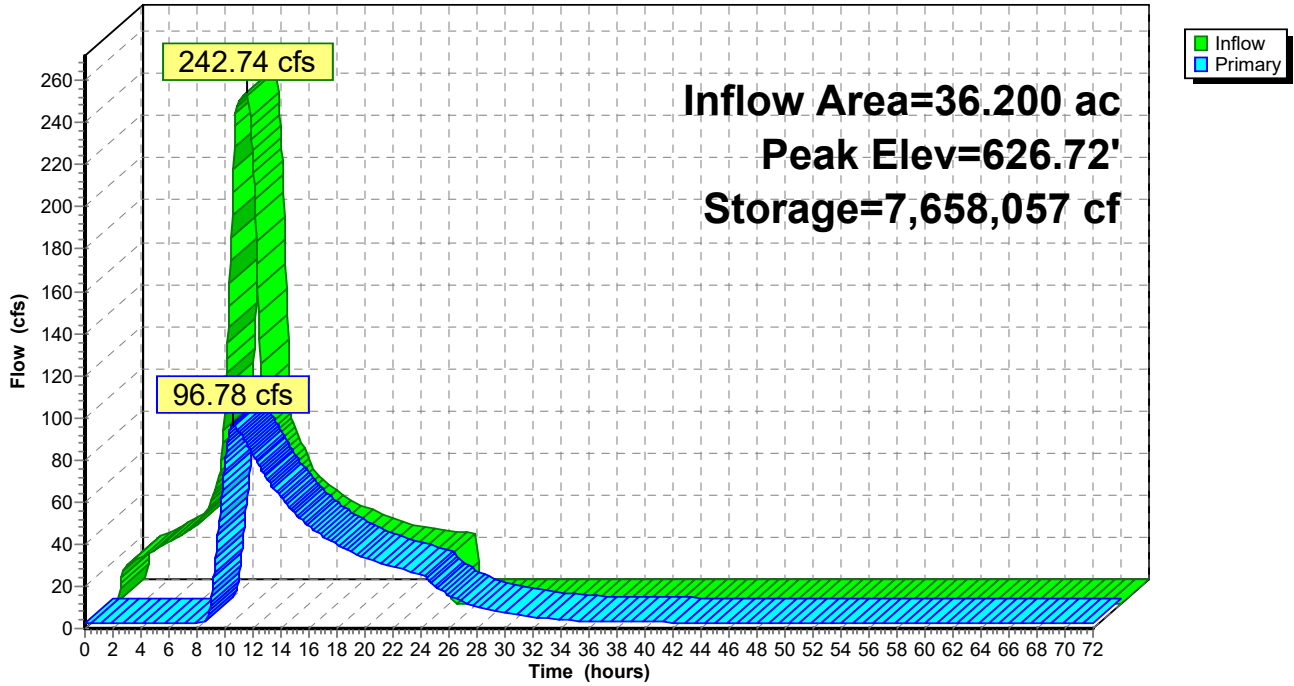
Elevation (feet)	Cum.Store (cubic-feet)
621.10	0
622.00	898,355
623.00	2,215,071
624.00	3,622,761
625.00	5,085,824
626.00	6,575,189
627.00	8,086,603
628.00	9,615,334
629.00	11,161,695
630.00	12,725,625
631.00	14,298,658
632.00	15,871,813

Device	Routing	Invert	Outlet Devices
#1	Primary	626.00'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 2.00 4.00 Width (feet) 45.00 60.00 75.00
#2	Primary	619.00'	14.0" Round Culvert L= 580.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 619.00' / 617.60' S= 0.0024 '/ Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.07 sf

Primary OutFlow Max=96.75 cfs @ 10.58 hrs HW=626.72' TW=624.48' (Dynamic Tailwater)
 1=Custom Weir/Orifice (Weir Controls 93.61 cfs @ 2.74 fps)
 2=Culvert (Outlet Controls 3.14 cfs @ 2.93 fps)

Pond 2P: Gypsum Stack Pond

Hydrograph



Summary for Pond 3P: Recycle Pond

[63] Warning: Exceeded Reach 5R INLET depth by 0.26' @ 12.35 hrs

Inflow Area = 54.500 ac, 100.00% Impervious, Inflow Depth > 29.53" for PMP - Emergency Spillway event
 Inflow = 183.51 cfs @ 10.06 hrs, Volume= 134.115 af
 Outflow = 114.71 cfs @ 11.82 hrs, Volume= 103.284 af, Atten= 37%, Lag= 105.4 min
 Primary = 38.24 cfs @ 11.82 hrs, Volume= 34.428 af
 Secondary = 38.24 cfs @ 11.82 hrs, Volume= 34.428 af
 Tertiary = 38.24 cfs @ 11.82 hrs, Volume= 34.428 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
 Starting Elev= 622.10' Surf.Area= 0 sf Storage= 9,024,347 cf
 Peak Elev= 624.70' @ 11.82 hrs Surf.Area= 0 sf Storage= 10,813,783 cf (1,789,436 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
 Center-of-Mass det. time= 234.9 min (1,236.7 - 1,001.8)

Volume	Invert	Avail.Storage	Storage Description
#1	604.90'	13,809,827 cf	Custom Stage Data Listed below

Elevation (feet)	Cum.Store (cubic-feet)
604.90	0
605.00	193,406
607.00	824,155
609.00	1,613,462
611.00	2,487,712
613.00	3,446,903
615.00	4,502,797
617.00	5,698,519
619.00	6,966,115
621.00	8,279,014
623.00	9,634,165
624.00	10,326,769
625.00	11,023,294
626.00	11,719,818
627.00	12,416,342
628.00	13,112,867
629.00	13,809,827

Device	Routing	Invert	Outlet Devices
#1	Primary	615.00'	45.0" Round Culvert L= 92.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 615.00' / 613.00' S= 0.0217 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 11.04 sf
#2	Secondary	615.00'	45.0" Round Culvert L= 92.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 615.00' / 613.00' S= 0.0217 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 11.04 sf
#3	Tertiary	615.00'	45.0" Round Culvert L= 92.0' CPP, square edge headwall, Ke= 0.500

			Inlet / Outlet Invert= 615.00' / 613.00' S= 0.0217 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 11.04 sf
#4	Device 1	624.00'	60.0" x 60.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	624.00'	60.0" x 60.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#6	Device 3	624.00'	60.0" x 60.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=38.23 cfs @ 11.82 hrs HW=624.70' (Free Discharge)

- ↑1=Culvert (Passes 38.23 cfs of 148.75 cfs potential flow)
- ↑4=Orifice/Grate (Weir Controls 38.23 cfs @ 2.73 fps)

Secondary OutFlow Max=38.23 cfs @ 11.82 hrs HW=624.70' (Free Discharge)

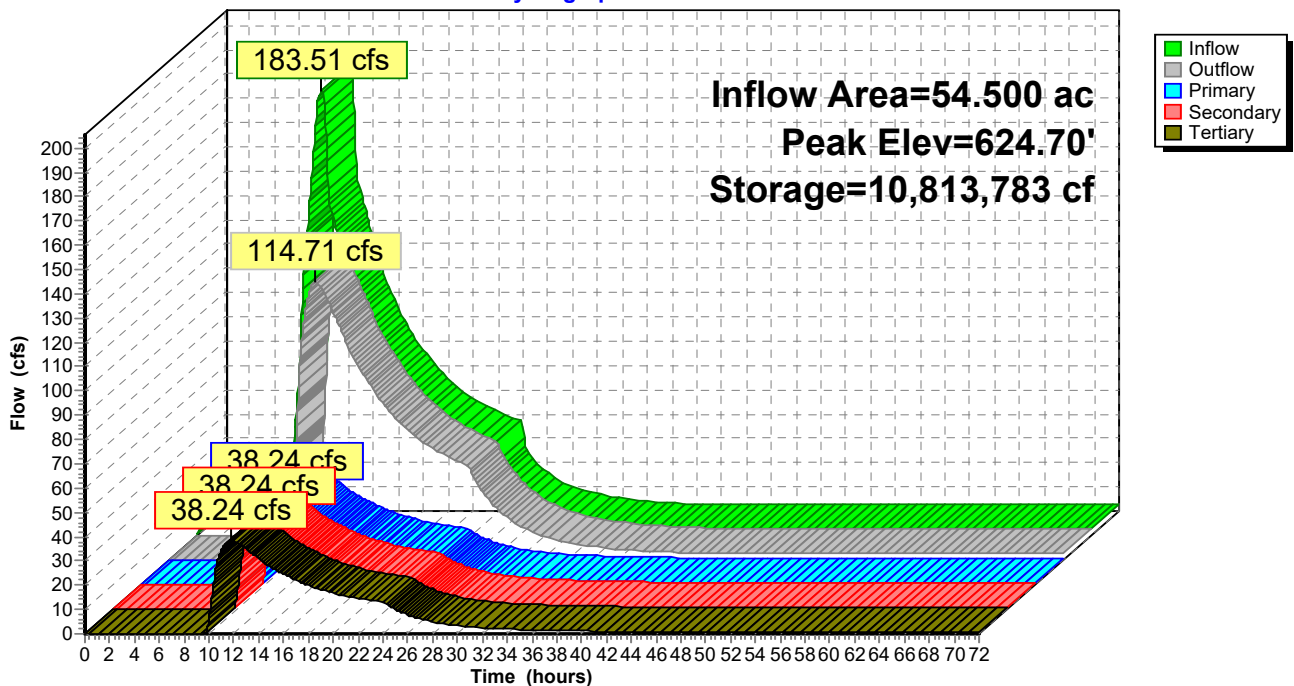
- ↑2=Culvert (Passes 38.23 cfs of 148.75 cfs potential flow)
- ↑5=Orifice/Grate (Weir Controls 38.23 cfs @ 2.73 fps)

Tertiary OutFlow Max=38.23 cfs @ 11.82 hrs HW=624.70' (Free Discharge)

- ↑3=Culvert (Passes 38.23 cfs of 148.75 cfs potential flow)
- ↑6=Orifice/Grate (Weir Controls 38.23 cfs @ 2.73 fps)

Pond 3P: Recycle Pond

Hydrograph



APPENDIX B

Narrative Description



APPENDIX B

**Narrative Description of Gypsum Management Facility
Gypsum Stack Pond**
Coffeen Power Plant

Submitted to:

Illinois Environmental Protection Agency

1021 North Grand Avenue East
P.O. Box 19276
Springfield, Illinois 62794-9276

Submitted by:

Illinois Power Resources Generating, LLC

1500 Eastport Plaza Drive
Collinsville, Illinois 62234

Prepared by:

Golder Associates USA Inc.

701 Emerson Road, Suite 250
Creve Coeur, Missouri, 63141

21465046

July 28, 2022

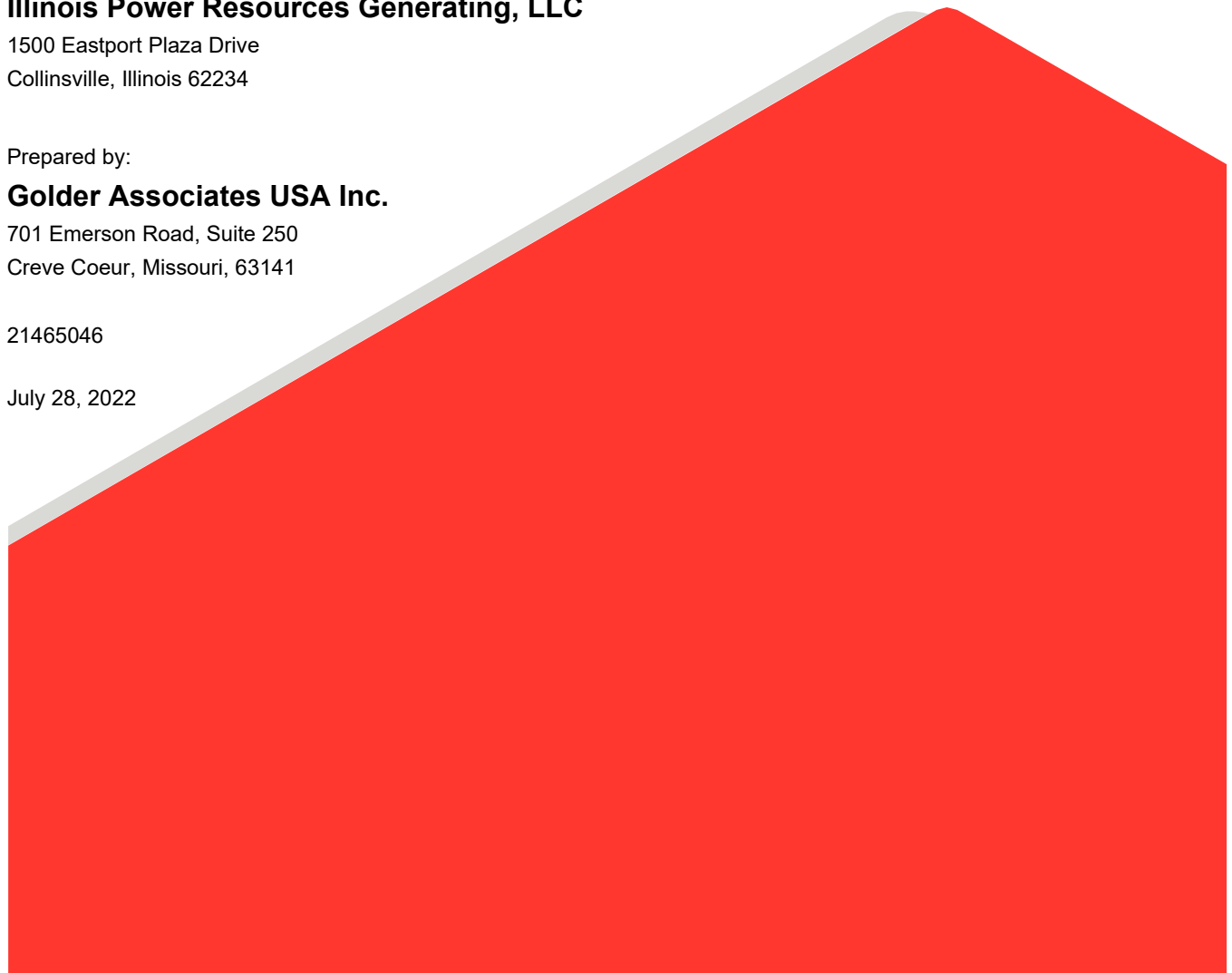


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1.1.1 Chemical Analysis	1
1.2 Facility Capacity	1
1.3 Facility Operation	1
1.4 Transportation Plan	1

ATTACHMENTS

Figure 1
Transportation Plan

Attachment 1
Chemical Analysis

1.0 FACILITY INFORMATION

1.1 Coal Combustion Residuals Material Received

The Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) is a wet impoundment basin used to store synthetic gypsum, a byproduct of the wet scrubber system used for flue gas desulfurization at the Coffeen Power Plant. The synthetic gypsum is generally of the same chemical structure as natural gypsum.

1.1.1 Chemical Analysis

Available information regarding chemical analysis of the gypsum that is stored in the GMF GSP is provided in Attachment 1.

1.2 Facility Capacity

The GMF GSP capacity was estimated as 650,000 cubic yards in the closure plan prepared in 2016 for compliance with 40 CFR 257.102. However, the Coffeen Power Plant was retired in December 2019, before the GMF GSP could be filled to its capacity. The amount of coal combustion residuals (CCR) currently contained in the GMF GSP was estimated by a volumetric analysis using Autodesk Civil 3D as approximately 295,500 cubic yards. No additional CCR will be placed in the GMF GSP.

1.3 Facility Operation

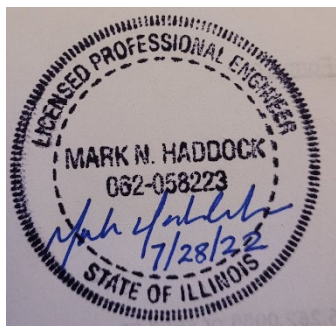
The GMF GSP is no longer receiving CCR or other waste streams. The GMF GSP received CCR from 2010 until the Coffeen Power Plant was retired in December 2019.

1.4 Transportation Plan

During operation, transport of CCR to the GMF GSP was by pipeline. Figure 1 shows the main route that is used for vehicle travel between the Coffeen Power Plant and the GMF GSP. Figure 1 also shows the route that will be used to transport borrow materials from a soil stockpile to the GMF GSP for closure and to transport soils excavated during closure to the soil stockpile. These routes are all on site.

Signature Page

Golder Associates USA Inc.



Mark Haddock, PE
Principal

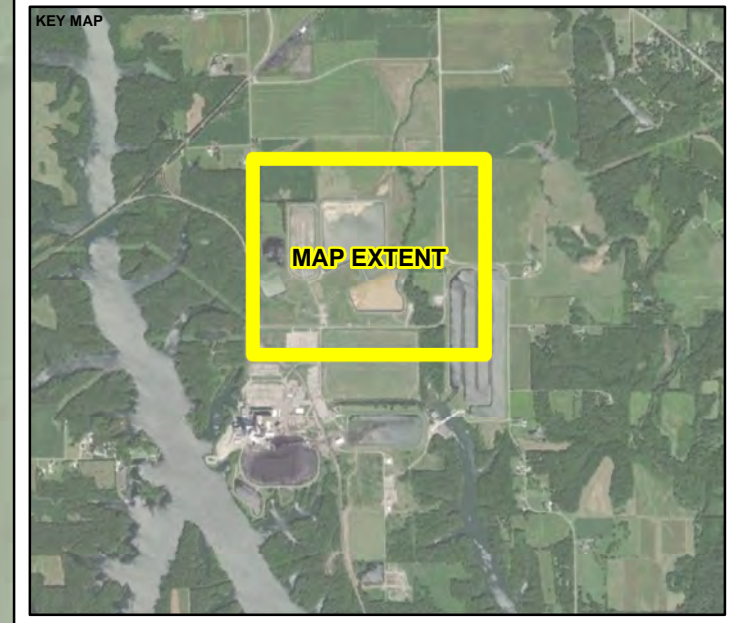
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FIGURE 1

Transportation Plan



- LEGEND**
- - - ROUTE BETWEEN GMF RP AND ON-SITE LANDFILL
 - TOPOGRAPHY/BATHYMETRY (2-FT CONTOUR INTERVAL)
 - GMF GYPSUM STACK POND
 - GMF RECYCLE POND



- REFERENCES**
1. AERIAL IMAGERY WITHIN PROJECT AREA: INGENAE, DECEMBER 2020.
 2. AERIAL IMAGERY IN SURROUNDING AREAS: ESRI, VIVID, MAXAR.
 3. WELLS, OUTFALLS, AND TOPOGRAPHY: INGENAE, DECEMBER 2020.
 4. NPDES OUTFALL LOCATIONS: ILLINOIS ENVIRONMENTAL PROTECTION AGENCY, 2021.

CLIENT
ILLINOIS POWER GENERATING COMPANY

PROJECT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND AND GMF RECYCLE POND

TITLE
SITE TRANSPORTATION PLAN

CONSULTANT	YYYY-MM-DD	2022-07-14
	DESIGNED	MWD
	PREPARED	KJC
	REVIEWED	MWD
	APPROVED	JJS

PATH: M:\VisualLuminaire\21465046\1147_Coffeen_Plan\Figure01_SiteTransportationPlan_DDP.mxd PRINTED ON: 2022-07-14 AT: 11:22:43 AM

1in IF THIS MEASUREMENT DOES NOT MATCH WHAT IS SHOWN, THE SHEET SIZE HAS BEEN MODIFIED FROM: ANSI B

ATTACHMENT 1

Chemical Analysis



Safety Data Sheet

Section 1 Identification of the Substance and of the Supplier

1.1 Product Identifier

Product Name/Identification:	FGD Gypsum
Synonyms:	Gypsum, calcium sulfate dihydrate, calcium sulphate dihydrate, gesso, alabaster, plaster of Paris.
Formula:	UVCB Substance

1.2 Relevant Identified Uses of the Substance or Mixture and Uses Advices Against

Relevant Identified Uses:	Component of wallboard, concrete, roofing material, bricks, cement kiln feed, agricultural amendment.
Uses Advised Against:	None known.

1.3 Details of the Supplier of the SDS

Manufacturer/Supplier:	Dynegy, Inc.
Street Address:	601 Travis Street, Suite 1400
City, State and Zip Code:	Houston, TX 77002
Customer Service Telephone:	800-633-4704


**Section 2
 Hazards Identification**

2.1 Classification of the Substance

GHS Classification(s) according to OSHA Hazard Communication Standard (29 CFR 1910.1200):

- STOT-SE Category 3 (Respiratory Irritation)
- STOT-RE Category 1 (Lungs)
- Carcinogen Category 1A

2.2 Label Elements

<i>Labelling according to 29 CFR 1910.1200 Appendices A, B and C*</i>	
Hazard Pictogram(s):	
Signal word:	DANGER
Hazard Statement(s):	<p><i>May cause respiratory irritation.</i></p> <p><i>Causes damage to lungs after repeated/prolonged exposure via inhalation.</i></p> <p><i>May cause cancer of the lungs.</i></p>
Precautionary Statement(s):	<p><i>Obtain special instructions before use.</i></p> <p><i>Do not handle until all safety precautions have been read and understood.</i></p> <p><i>Do not breathe dust.</i></p> <p><i>Wash hands thoroughly after handling.</i></p> <p><i>Do not eat, drink or smoke when using this product.</i></p> <p><i>Wear protective gloves/protective clothing/eye protection/face protection.</i></p> <p><i>Do not eat drink or smoke when using this product.</i></p> <p><i>Use outdoors or in a well-ventilated area.</i></p> <p><i>If inhaled: Remove to fresh air and keep comfortable for breathing.</i></p> <p><i>Get medical advice/attention if you feel unwell.</i></p> <p><i>Store in a secure area.</i></p> <p><i>Dispose of product in accordance with local/national regulations.</i></p>

The following elements may be present in trace amounts as oxides: aluminum, calcium, iron, magnesium, nickel, phosphorus, potassium, silicon, sulfur, titanium, and vanadium. The exact composition of the gypsum will be dependent on the fuel source and flue additives composed of many constituents.



2.3 Other Hazards

Listed Carcinogens:

-Respirable Crystalline Silica

IARC: [Yes] NTP: [Yes] OSHA: [Yes] Other: (ACGIH) [Yes]

**Section 3
 Composition/Information on Ingredients**

Substance	CAS No.	Percentage (%)	GHS Classification
Calcium sulfate, dihydrate	10104-14-1	90 - 99%	STOT – Single Exposure Category 3 (Respiratory Irritation)
Crystalline Silica	14808-60-7	≥0.1 - 3%	STOT – Repeated Exposure Category 1 (Lung) Carcinogen, Category 1A
Silica, crystalline respirable (RCS)	14808-60-7	See Footnote 1	STOT – Repeated Exposure Category 1 (Lungs) Carcinogen Category 1A
Fly Ash	68131-74-8	<2%	STOT – Single Exposure Category 3 (Respiratory Irritation)

Footnote 1: The percentage of respirable crystalline silica has not been determined. Therefore, a GHS classification of Carcinogen, Category 1A has been assigned.

**Section 4
 First Aid Measures**

4.1 Description of First Aid Measures

Inhalation:	If product is inhaled and irritation of the nose or coughing occurs, remove person to fresh air. Get medical advice/attention if respiratory symptoms persist.
Skin Contact:	If skin exposure occurs, wash with soap and water.
Eye Contact:	If product gets into the eye, rinse copiously with water for several minutes. Remove contact lenses, if present and easy to do. Seek medical attention/advice if irritation occurs or persists.
Ingestion:	No specific first aid measures are required.

4.2 Most Important Health Effects, Both Acute and Delayed

Acute Effects: Short-term airborne exposure to FGD gypsum dust may cause respiratory irritation. Direct exposure can dry and irritate the skin and cause dermatitis or eye irritation through mechanical abrasion.

Chronic Effects: Chronic (long-term) exposure to FGD gypsum may cause lung damage from repeated exposure. Prolonged inhalation of dusts containing respirable crystalline silica above certain concentrations may cause lung disease (silicosis) and lung cancer.

4.3 Indication of Any Immediate Medical Attention and Special Treatment Needed

Seek first aid or call a doctor or Poison Control Center if contact with eyes occurs and irritation remains after rinsing. Get medical advice if inhalation occurs and respiratory symptoms persist.

**Section 5
 Firefighting Measures**

5.1 Extinguishing Media

Suitable Extinguishing Media:	Product is not flammable. Use extinguishing media appropriate for surrounding fire.
Unsuitable Extinguishing Media:	Not applicable, the product is not flammable.

5.2 Special Hazards Arising from the Substance or Mixture

Hazardous Combustion Products:	Above 1450°C (~2600°F), gypsum decomposes to calcium oxide and sulfur dioxide.
---------------------------------------	--

5.3 Advice for Firefighters

Special Protective Equipment and Precautions for Firefighters:	As with any fire, wear self-contained breathing apparatus (NIOSH approved or equivalent) and full protective gear.
---	--

Section 6 Accidental Release Measures

6.1 Personal Precautions, Protective Equipment and Emergency Procedures

Personal precautions/Protective Equipment:	See Section 8.2.2 Individual Protective Measures. For concentrations exceeding Occupational Exposure Levels (OELs), use a self-contained breathing apparatus (SCBA).
Emergency procedures:	Use scooping, water spraying/flushing/misting or ventilated vacuum cleaning systems to clean up spills. Do not use pressurized air.

6.2 Environmental Precautions

Environmental precautions:	Prevent contamination of drains or waterways and dispose according to local and national regulations.
-----------------------------------	---

6.3 Methods and Material for Containment and Cleaning Up

Methods and materials for containment and cleaning up:	Do not use brooms or compressed air to clean surfaces. Use dust collection vacuum and extraction systems. Large spills of dry product should be removed by a vacuum system. Dampened material should be removed by mechanical means and recycled or disposed of according to local and national regulations.
---	---

See Sections 8 and 13 for additional information on exposure controls and disposal.

**Section 7
 Handling and Storage**

7.1 Precautions for Safe Handling

Practice good housekeeping. Use adequate exhaust ventilation, dust collection and/or water mist to maintain airborne dust concentrations below permissible exposure limits (note: respirable crystalline silica dust may be in the air without a visible dust cloud).

Do not permit dust to collect on walls, floors, sills, ledges, machinery, or equipment. Maintain and test ventilation and dust collection equipment. In cases of insufficient ventilation, wear a NIOSH approved respirator for silica dust when handling or disposing dust from this product. Avoid contact with skin and eyes. Wash or vacuum clothing that has become dusty. Avoid eating, smoking, or drinking while handling the material.

7.2 Conditions for Safe Storage, Including any Incompatibilities

Minimize dust produced during loading and unloading.

**Section 8
 Exposure Controls/Personal Protection**

8.1 Control Parameters

OCCUPATIONAL EXPOSURE LIMITS					
SUBSTANCE		OSHA PEL TWA (mg/m ³)	NIOSH REL TWA (mg/m ³)	ACGIH TLV TWA (mg/m ³)	CA - OSHA PEL (mg/m ³)
Particulates Not Otherwise Regulated	Total	15	15	10	10
	Respirable	5	5	3	5
Respirable Crystalline Silica	Total Respirable	0.05-	0.05	0.025	0.05
Calcium Sulfate, anhydrous (CAS# 7778-18-9)	Total Dust	*	10	10	*
	Respirable	*	5	-	*

Note: In the absence of a CA-PEL, the value for Particulates Not Otherwise Regulated (PNOR) is applied.

8.2 Exposure Controls

8.2.1 Engineering Controls



Provide ventilation to maintain the ambient workplace atmosphere below the occupational exposure limit(s). Use general and local exhaust ventilation and dust collection systems as necessary to minimize exposure.

8.2.2 Personal Protective Equipment (PPE)

Respiratory protection:	Wear a NIOSH approved particulate respirator if exposure to airborne particulates is unavoidable and where occupational exposure limits may be exceeded. If airborne exposures are anticipated to exceed applicable PELs or TLVs, a self-contained breathing apparatus or airline respirator is recommended.
Eye and face protection:	If eye contact is possible, wear protective glasses with side shields. Avoid contact lenses.
Hand and skin protection:	Wear gloves and protective clothing. Wash hands with soap and water after contact with material.

**Section 9
 Physical and Chemical Properties**

9.1 Information on Basic Physical and Chemical Properties

Property: Value	Property: Value
Appearance (physical state, color, etc.): White or gray cake-like material	Upper/lower flammability or explosive limits: Not applicable
Odor: Odorless	Vapor Pressure (Pa): Not applicable
Odor threshold: Not applicable	Vapor Density: Not applicable
pH (25 °C) (in water): 6 - 8	Specific gravity or relative density: 2.0 – 2.9
Melting point/freezing point (°C): 128	Water Solubility: 0.1 – 0.3%
Initial boiling point and boiling range (°C): >163	Partition coefficient: n-octane/water: Not determined
Flash point (°C): Not determined	Auto ignition temperature (°C): Not applicable
Evaporation rate: Not applicable	Decomposition temperature (°C): 1450
Flammability (solid, gas): Nonflammable/non-combustible	Viscosity: Not applicable

**Section 10
 Stability and Reactivity**

10.1 Reactivity:	Avoid contact with strong acids or oxidizers and diazomethane.
10.2 Chemical stability:	The material is stable under normal use conditions.
10.3 Possibility of hazardous reactions:	The material is a relatively stable, inert material; polymerization will not occur.
10.4 Conditions to avoid:	Product can become airborne in moderate winds. Dry material should be stored in silos. Materials stored out of doors should be covered or maintained in a damp condition.
10.5 Incompatible materials:	Acids, ammonium salts, diazomethane, phosphorus and aluminum metal.
10.6 Hazardous decomposition products:	None known.

Section 11
Toxicological Information

11.1 Information on Toxicological Effects

Endpoint	Data
Acute oral toxicity	Oral LD50 > 2000 mg/kg
Acute dermal toxicity	No data.
Acute inhalation toxicity	Inhalation LC50: > 3.26 mg/L
Skin corrosion/irritation	Not irritating or corrosive to skin based on 4-hour, semi-occlusive exposure to rabbits.
Eye damage/irritation	No positive responses in rabbits based upon 24-, 48-, and 72-hour mean scores for corneal opacity, iritis, conjunctival redness/edema.
Respiratory/skin sensitization	Not a respiratory or dermal sensitizer.
Germ cell mutagenicity	Several in vitro and in vivo mutagenicity assays determined that calcium sulfate, dihydrate was non-mutagenic, with and without metabolic activation.
Carcinogenicity	No data on calcium sulfate, dihydrate. Carcinogenic studies were not conducted based on the non-neoplastic effects noted in the oral and inhalation repeated dose studies as well as the negative mutagenicity assays. Respirable crystalline silica has been identified as a carcinogen by NTP, IARC, ACGIH and OSHA.
Reproductive toxicity	No significant developmental or reproductive toxicity were identified in rabbits after exposure to either calcium sulfate, dehydrate or calcium sulfate, dihydrate.
STOT-SE	Acute toxicity testing did not result in direct organ toxicity after a single exposure to calcium sulfate, dihydrate. However, as the form tested was not indicated, FGD gypsum dust may result in mechanical respiratory irritation.
STOT-RE	A repeat dose oral toxicity study (35-45 days) with calcium sulfate, dihydrate conducted using rats reported a NOAEL for males of 100 mg/kg/day on the basis of decreased total protein, albumin, blood urea nitrogen, and creatinine levels observed at the 300 and 1,000 mg/kg/day dose groups. No effects were observed in females. Repeated inhalation exposures to high levels of respirable crystalline silica may result in lung damage (silicosis) and lung cancer.
Aspiration Hazard	Not applicable based on product form.

**Section 12
 Ecological Information**

12.1 Toxicity

Calcium sulfate (CAS# 7778-18-9) ¹	
Toxicity to Fish	Fathead Minnow (<i>Pimephales promelas</i>): Acute, 7-day LC50 > 1,970 mg/L Acute, 96-hour NOEC = 1,470 mg/L
	Rainbow trout (<i>Oncorhynchus mykiss</i>): Chronic, 30-day NOEC (survival, growth, reproduction) = 732 mg/L
	Bluegill (<i>Lepomis macrochirus</i>): Acute, 96-hour LC50 = 2,890 mg/L
Toxicity to Aquatic Invertebrates	Water Flea (<i>Ceriodaphnia dubia</i>): Acute, 96-hour NOEC = 1447.4 mg/L
	Water Flea (<i>Daphnia magna</i>): Acute, 48-hour LC50 = 1,970 mg/L Chronic, 21-day NOEC (and 42-day post-hatch): 1,600 mg/L
Toxicity to Aquatic Algae and Plants	Algae (<i>Navicula seminulum</i> ; <i>Nitzschia linearis</i>): Chronic, 96-hour LC50 and EC50 (growth) = 3,200 mg/L

¹The aquatic toxicity of sulfate has been shown to be dependent on water hardness, generally decreasing as hardness increases.

12.2 Persistence and Degradability

Not relevant for inorganic materials.

12.3 Bioaccumulative Potential

This material does not contain any compounds that that would bioaccumulate up the food chain.

12.4 Mobility in Soil

No data available.

12.5 Results of PBT and vPvB Assessment

This material does not contain any compounds classified as “persistent, bioaccumulative or toxic” nor as “very persistent/very bioaccumulative”.

12.6 Other Adverse Effects

None known.

**Section 13
 Disposal Considerations**

See Sections 7 and 8 above for safe handling and use, including appropriate industrial hygiene practices.
 Dispose of all waste product and containers in accordance with federal, state and local regulations.

**Section 14
 Transport Information**

Regulatory entity: U.S. DOT	Shipping Name:	Not Regulated
	Hazard Class:	Not Regulated
	ID Number:	Not Regulated
	Packing Group:	Not Regulated

Section 15
Regulatory Information

15.1 Safety, Health and Environmental Regulations/Legislation Specific for the Mixture

- TSCA Inventory Status
 - FGD gypsum as well as listed impurities are on the TSCA Inventory.

- California Proposition 65

The following substances are known to the State of California to be carcinogens and/or reproductive toxicants:

- Respirable crystalline silica

- State Right-to-Know (RTK)

Component	CAS	MA ^{1,2}	NJ ^{3,4}	PA ⁵	RI ⁶
Gypsum; calcium sulfate; calcium sulfate dihydrate	7778-18-9 or 10101-41-4	Yes	Yes	Yes	No
Calcium carbonate	1317-65-3	Yes	Yes	Yes	No
Silica-crystalline (SiO ₂), quartz	14808-60-7	Yes	Yes	Yes	No

¹ Massachusetts Department of Public Health, no date

² 189th General Court of The Commonwealth of Massachusetts, no date

³ New Jersey Department of Health and Senior Services, 2010a

⁴ New Jersey Department of Health, 2010b

⁵ Pennsylvania Code, 1986

⁶ Rhode Island Department of Labor and Training, no date

Section 16
Other Information, Including Date of Preparation or Last Revision

16.1 Indication of Changes

Date of preparation or last revision: February 27, 2018

16.2 Abbreviations and Acronyms

- ACGIH: American Conference of Industrial Hygienists
- CA: California
- CAS: Chemical Abstract Services
- CCP: Coal Combustion Product
- CFR: Code of Federal Regulations
- EPA: Environmental Protection Agency
- FGD: Flue Gas Desulfurization
- GHS: Globally Harmonized System of Classification and Labelling
- IARC: International Agency for Research on Cancer
- LC50: Concentration estimated to result in the mortality of 50% of an animal population
- LD50: Dose estimated to result in the mortality of 50% of an animal population
- MA: Massachusetts
- NA: Not Applicable
- NJ: New Jersey

Preparation Date: 02/27/2018



- NOEC: No observed effect concentration
- NIOSH: National Institute of Occupational Safety and Health
- NOx: Nitrogen oxides
- NTP: US National Toxicology Program
- OEL: Occupational Exposure Limit
- OSHA: Occupational Safety and Health Administration
- PA: Pennsylvania
- PBT: Persistent, Toxic and Bioaccumulative
- PEL: Permissible exposure limit
- PPE: Personal Protective Equipment
- REL: Recommended exposure limit
- RI: Rhode Island
- RCS: Respirable Crystalline Silica
- RTK: Right-to-Know
- SCBA: Self-contained breathing apparatus
- SDS: Safety Data Sheet
- STEL: Short-term exposure limit
- STOT-RE: Specific target organ toxicity-repeated exposure
- STOT-SE: Specific target organ toxicity-single exposure
- TLV: Threshold limit value
- TSCA: Toxic Substances Control Act
- TWA: Time-weighted average
- UEL: Upper explosive limit
- UVCB: Unknown or Variable Composition/Biological
- U.S.: United States
- U.S. DOT: United States of Department of Transportation

16.3 Other Hazards

Hazardous Materials Identification System (HMIS)							
Degree of hazard (0= low, 4 = extreme)							
Health:	1*	Flammability:	0	Physical Hazards:	0	Personal protection:**	

* Chronic Health Effects

** Appropriate personal protection is defined by the activity to be performed.

See Section 8 for additional information.

DISCLAIMER:

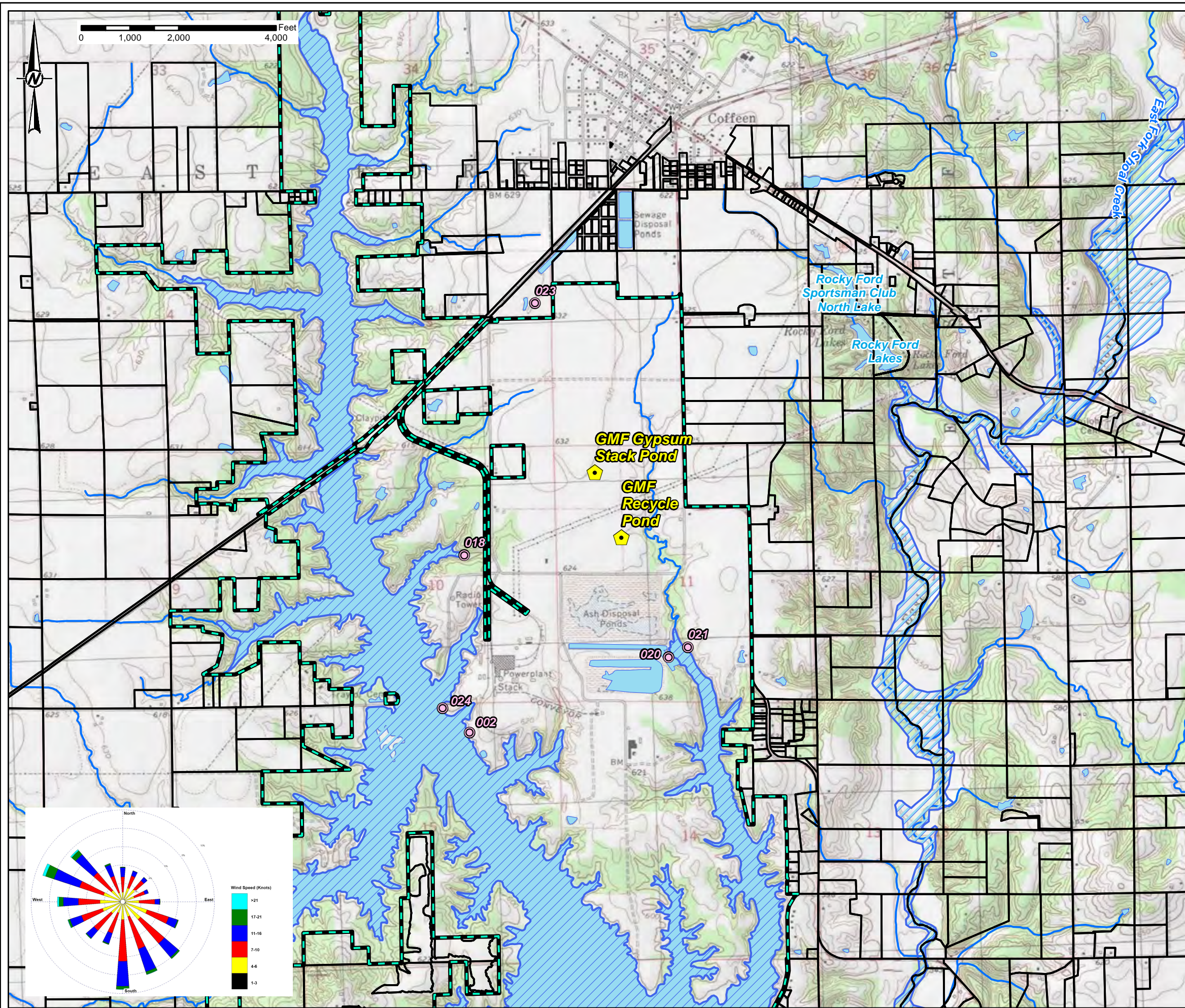
This SDS has been prepared in accordance with the Hazard Communication Rule 29 CFR 1910.1200. Information herein is based on data considered to be accurate as of date prepared. No warranty or representation, express or implied, is made as to the accuracy or completeness of this data and safety information. No responsibility can be assumed for any damage or injury resulting from abnormal use, failure to adhere to recommended practices, or from any hazards inherent in the nature of the product.

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APPENDIX C

Map Package



LEGEND

- SITE LOCATION
- NPDES OUTFALL (PERMIT ILL0000108)
- SURFACE WATER - LINEAR FEATURE
- SURFACE WATER BODY
- FEMA 100-YEAR FLOODPLAIN (ZONE A)
- IPRG BOUNDARY
- NEIGHBORING PROPERTY BOUNDARIES



NOTES

1. A SEARCH OF THE ILLINOIS NATURAL AREAS INVENTORY (INAI) AND ILLINOIS NATURE PRESERVES COMMISSION (INPC) PROTECTED LANDS RETURNED NO PROTECTED AREAS WITHIN THE EXTENT OF THIS MAP.
2. THE IDNR NATURAL HERITAGE DATABASE THREATENED AND ENDANGERED SPECIES BY COUNTY LISTS 4 STATE THREATENED AND 7 STATE ENDANGERED SPECIES IN MONTGOMERY COUNTY, BUT NO SPATIAL DATA IS AVAILABLE. THE CLOSEST CRITICAL HABITAT PROTECTED BY THE NATIONAL ENDANGERED SPECIES ACT IS THAT OF THE INDIANA BAT, OVER 75 MILES SOUTHWEST OF THE SITE.
3. ACCORDING TO THE NATIONAL REGISTER OF HISTORIC PLACES, THERE ARE NO HISTORIC FEATURES WITHIN THE EXTENT OF THIS MAP.

REFERENCES

1. TOPOGRAPHIC BASEMAP: UNITED STATES GEOLOGICAL SURVEY 7.5-MINUTE TOPOGRAPHIC QUADRANGLES SHOWN: "COFFEEN, IL" AND "FILLMORE, IL".
2. FLOODPLAIN DATA: FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA). DIGITIZED FROM PANEL 1709920009A, EFFECTIVE ON JANUARY 9, 1981.
3. SURFACE WATER FEATURES: NATIONAL HYDROGRAPHY DATASET (NHD), USGS.
4. PREVAILING WIND DIRECTION GRAPHIC: ILLINOIS STATE CLIMATOLOGIST OFFICE. 1961-1990 ANNUAL AVERAGE WIND SPEEDS AND DIRECTIONS, AS MEASURED AT STATION #13994 - ST LOUIS/LAMBERT INTL ARPT, MO.
5. NPDES PERMIT LOCATION: ILLINOIS ENVIRONMENTAL PROTECTION AGENCY.
6. PROPERTY BOUNDARIES: MONTGOMERY COUNTY GIS, JULY 2022.

CLIENT
ILLINOIS POWER GENERATING COMPANY

PROJECT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND AND GMF RECYCLE POND

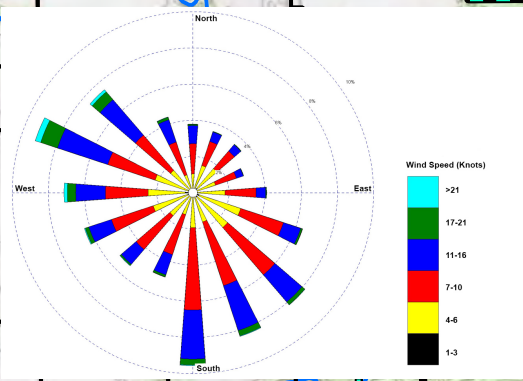
TITLE
SITE LOCATION MAP

CONSULTANT	YYYY-MM-DD	2022-07-28
	DESIGNED	MWD
	PREPARED	KJC
	REVIEWED	MWD
	APPROVED	JJS

PROJECT NO. 21465046 **FIGURE** 1

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LEGEND

- WELL
- ROUTE BETWEEN GMF RP AND ON-SITE LANDFILL
- TOPOGRAPHY/BATHYMETRY (2-FT CONTOUR INTERVAL)
- GMF GYPSUM STACK POND
- GMF RECYCLE POND



- REFERENCES**
1. AERIAL IMAGERY WITHIN PROJECT AREA: INGENAE, DECEMBER 2020.
 2. AERIAL IMAGERY IN SURROUNDING AREAS: ESRI, VIVID, MAXAR.
 3. WELLS, OUTFALLS, AND TOPOGRAPHY: INGENAE, DECEMBER 2020.
 4. NPDES OUTFALL LOCATIONS: ILLINOIS ENVIRONMENTAL PROTECTION AGENCY, 2021.

CLIENT
ILLINOIS POWER GENERATING COMPANY

PROJECT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND AND GMF RECYCLE POND

TITLE
SITE PLAN

CONSULTANT	DATE	BY
	YYYY-MM-DD	2022-07-28
	DESIGNED	MWD
	PREPARED	KJC
	REVIEWED	MWD
	APPROVED	JJS

PROJECT NO.
21465046

FIGURE
2

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APPENDIX D

Hydrogeologic Site Characterization

Intended for
Illinois Power Generating Company

Date
October 25, 2021

Project No.
1940100806-002

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

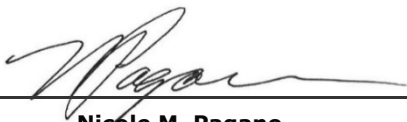
GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND

Project Name **Coffeen Power Plant GMF Gypsum Stack Pond**
Project No. **1940100806-002**
Recipient **Illinois Power Generating Company**
Document Type **Hydrogeologic Site Characterization Report**
Revision **FINAL**
Date **October 25, 2021**

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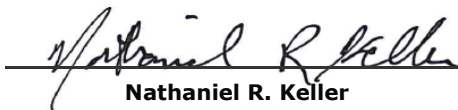
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APPENDICES

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ACRONYMS AND ABBREVIATIONS

°F	degrees Fahrenheit
§	Section
%	percent
35 I.A.C.	Title 35 of the Illinois Administrative Code
40 C.F.R.	Title 40 of the Code of Federal Regulations
AP1	Ash Pond No. 1
AP2	Ash Pond No. 2
bgs	below ground surface
CCR	coal combustion residuals
CCR Rule	40 C.F.R. § 257 Subpart D
cm/s	centimeters per second
CPP	Coffeen Power Plant
DA	deep aquifer
DCU	deep confining unit
ESRI	Environmental Systems Research Institute, Inc.
FEMA	Federal Emergency Management Agency
ft/day	feet per day
ft/ft	feet per foot
g	horizontal acceleration
GMF GSP	Gypsum Management Facility Gypsum Stack Pond
GMF RP	Gypsum Management Facility Recycle Pond
GMP	Groundwater Monitoring Plan
GMZ	Groundwater Management Zone
GWPS	Groundwater Protection Standard
Hanson	Hanson Professional Services, Inc.
HCR	Hydrogeologic Site Characterization Report
HDPE	high-density polyethylene
HELP	Hydrologic Evaluation of Landfill Performance
HUC	Hydrologic Unit Code
ID	identification
IDNR	Illinois Department of Nature Resources
IEPA	Illinois Environmental Protection Agency
ILWATER	Illinois Water and Related Wells
IPGC	Illinois Power Generating Company
ISAS	Illinois State Archaeological Survey
ISGS	Illinois State Geological Survey
LCU	lower confining unit
LF	Landfill
mg/L	milligrams per liter
mil	millimeter
msl	above mean sea level
NAVD88	North American Vertical Datum of 1988

NGVD29	National Geodetic Vertical Datum of 1929
NID	National Inventory of Dams
NPDES	National Pollutant Discharge Elimination System
No.	number
NRT	Natural Resource Technology, Inc.
NRT/OBG	NRT, an OBG Company
Part 845	Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments: 35 I.A.C. § 845
pcf	pounds per cubic foot
PMP	potential migration pathways
Ramboll	Ramboll Americas Engineering Solutions, Inc.
SI	Surface Impoundments
Site	combined area including GMF RP and GMF GSP
SSURGO	Soil Survey Geographic Database
SU	Standard Units
TDS	total dissolved solids
UCU	upper confining unit
USEPA	United States Environmental Protection Agency
USFWS	United States Fish and Wildlife Service
USGS	United States Geological Survey

EXECUTIVE SUMMARY

This Hydrogeologic Site Characterization Report (HCR) for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) expands upon the hydrogeology and groundwater quality data presented in previous hydrogeologic investigation reports prepared for the Coffeen Power Plant (CPP) (Natural Resource Technology [NRT], 2017; Hanson Professional Services, Inc. [Hanson], 2009; Hanson, 2016). This report has been assembled to satisfy the information and analysis requirements of Title 35 of the Illinois Administrative Code (35 I.A.C.) Section (§) 845.620 as summarized in **Table ES-1**. The conceptual site model includes hydrogeologic and groundwater quality data specific to the GMF GSP, which has been collected from 2015 to 2021. The GMF GSP is part of the CPP, which is two miles south of the city of Coffeen, Illinois and about eight miles southeast of the city of Hillsboro, Illinois.

The CPP operated as a coal-fired power plant from 1964 until November 2019 and has five coal combustion residuals (CCR) management units. The CCR unit that is the subject of this report is: GMF GSP, Vistra Identification [ID] Number [No.] 103, Illinois Environmental Protection Agency [IEPA] ID No. W1350150004-03, and National Inventory of Dams [NID] No. IL50579. The GMF GSP is a 77-acre, lined surface impoundment (SI) used to manage CCR waste streams at the CPP.

CPP is located between the two lobes of Coffeen Lake (**Figure 1-1**), which was formed in 1963 by damming the McDavid Branch of the East Fork of Shoal Creek. Coffeen Lake encompasses approximately 1,100 acres and was created to provide a source of cooling water for the CPP. Coffeen Lake borders the CPP to the west, east, and south, and agricultural land is located to the north.

Unlithified material present above the bedrock in the vicinity of the CPP was categorized into hydrostratigraphic units for this HCR. In addition to the CCR Unit, the hydrostratigraphic units occur in the following order (from surface downward) and include:

- **Upper Confining Unit (UCU):** Composed of the Roxana and Peoria Silts (Loess Unit) and the upper clayey portion of the Hagarstown Member which are classified as silts to clayey silts and gravelly clay below the surficial soil. Construction of the GMF GSP required the excavation and removal of this layer within the unit footprint and the UCU has been eroded east of the GMF GSP, near the Unnamed Tributary.
- **Uppermost Aquifer:** The uppermost aquifer is the Hagarstown Member which is classified as primarily sandy to gravelly silts and clays with thin beds of sands. Similar to the Loess Unit, the Hagarstown Member was excavated to facilitate construction of the GMF GSP and the Hagarstown Member is also absent in some locations near the Unnamed Tributary.
- **Lower Confining Unit (LCU):** Comprised of the Vandalia Member, Mulberry Grove Member, and Smithboro Member. These units include a sandy to silty till with thin, discontinuous sand lenses, a discontinuous and limited extent sandy silt which has infilled prior erosional features, and silty to clayey diamicton, respectively.
- **Deep Aquifer (DA):** Sand and sandy silt/clay units of the Yarmouth Soil, which include accretionary deposits of fine sediment and organic materials, typically less than five feet thick and discontinuous across the CPP.

- **Deep Confining Unit (DCU):** Comprised of the Banner Formation, generally consists of clays, silts, and sands. The Lierle Clay Member is the upper layer of the Banner Formation which was encountered at the CPP.

Bedrock of the Bond Formation which consists of limestone and calcareous clays and shale, was not encountered in the borings advanced at the CPP.

Flow of groundwater from central portions of the CPP to Coffeen Lake or the Unnamed Tributary through the uppermost aquifer are the primary pathways for contaminant migration. Groundwater elevations are primarily controlled by surface topography, geologic unit topography, and water levels within Coffeen Lake and the Unnamed Tributary. A groundwater divide trending north-south is observed running through the approximate center of the CPP. Phreatic surfaces or water elevations within the SIs are generally consistent and have not been observed to fluctuate with groundwater elevations, indicating limited hydraulic connection with the SIs.

35 I.A.C. § 845 parameters were monitored in the uppermost aquifer monitoring wells at the GMF GSP as part of the Title 40 of the Code of Federal Regulations (40 C.F.R.) § 257 and IEPA groundwater monitoring programs from 2015 to 2021. These data were supplemented with installation and sampling of additional wells installed in 2021. The results indicate the following parameters were detected at concentrations/measurements greater than (or less than for pH) the applicable 35 I.A.C. § 845.600 groundwater protection standards (GWPSs) and are considered potential exceedances:

- Arsenic in downgradient uppermost aquifer well G215.
- Beryllium in downgradient uppermost aquifer well G209.
- Cobalt in downgradient uppermost aquifer wells G209, G213, and G217.
- Lead in downgradient uppermost aquifer wells G209 and G213.
- pH (low) in downgradient uppermost aquifer well G206.
- Sulfate in downgradient uppermost aquifer well G215 and in downgradient DA well G206D.
- Thallium in downgradient uppermost aquifer well G209.
- Total dissolved solids (TDS) in downgradient DA well G206D.

Groundwater monitoring results were compared to the applicable 35 I.A.C. § 845.600 GWPSs to determine potential exceedances. Potential exceedances include results reported during the background groundwater monitoring or prior period that are greater than the GWPS. The results are considered potential exceedances because the results were compared directly to the standard and did not include an evaluation of background groundwater quality or utilize the statistical methodologies proposed in the groundwater monitoring plan (GMP) provided in the Operating Permit application. Exceedances will be determined following IEPA approval of the GMP.

TABLE ES-1. PART 845 REQUIREMENTS CHECKLIST

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Part 845 Reference	Individual Part 845 Components Reviewed for Completeness	Location of Information in HCR
845.620(b)	The hydrogeologic site characterization shall include but not be limited to the following:	
845.620(b)(1)	Geologic well logs/boring logs;	Table 3-1 Figure 3-1 Appendix C
845.620(b)(2)	Climatic aspects of the site, including seasonal and temporal fluctuations in groundwater flow;	Sections 3.2.4 & 3.3.1 Table 3-3 Figures 3-2 through 3-5
845.620(b)(3)	Identification of nearby surface water bodies and drinking water intakes;	Sections 3.3.2 & 5.2 Appendix B
845.620(b)(4)	Identification of nearby pumping wells and associated uses of the groundwater;	Section 5.1 Appendix B
845.620(b)(5)	Identification of nearby dedicated nature preserves;	Section 5.3 Appendix B
845.620(b)(6)	Geologic setting;	Sections 2.4 & 2.5 Figures 2-2 through 2-4
845.620(b)(7)	Structural characteristics;	Section 2.4.3 Figure 2-5
845.620(b)(8)	Geologic cross-sections;	Figure 2-7
845.620(b)(9)	Soil characteristics;	Section 2.3 Figure 2-2 Tables 2-1 & 2-4
845.620(b)(10)	Identification of confining layers;	Sections 3.2.1

TABLE ES-1. PART 845 REQUIREMENTS CHECKLIST

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Part 845 Reference	Individual Part 845 Components Reviewed for Completeness	Location of Information in HCR
845.620(b)(11)	Identification of potential migration pathways;	Section 3.2.1 & 3.2.3
845.620(b)(12)	Groundwater quality data;	Section 4.2 Table 4-1
845.620(b)(13)	Vertical and horizontal extent of the geologic layers to a minimum depth of 100 feet below land surface, including lithology and stratigraphy;	Section 2.8 Figures 2-7 & 2-8 Appendix C
845.620(b)(14)	A map displaying any known underground mines beneath a CCR surface impoundment;	Section 2.7 Appendix B
845.620(b)(15)	Chemical and physical properties of the geologic layers to a minimum depth of 100 feet below land surface;	Section 2.8 Tables 2-1, 2-2, & 2-4 Appendices D & F
845.620(b)(16)	Hydraulic characteristics of the geologic layers identified as migration pathways and geologic layers that limit migration, including:	Sections 3.2.4.1, 3.2.5, & 3.2.6 Tables 3-2 to 3-4 Appendices D & F
845.620(b)(16)(A)	water table depth;	Section 3.2.4 Figures 3-3 & 3-4 Appendix E
845.620(b)(16)(B)	hydraulic conductivities;	Sections 3.2.5 Tables 2-1 & 3-3 Appendices D & F
845.620(b)(16)(C)	effective and total porosities;	Section 2.5.1 Table 2-1

TABLE ES-1. PART 845 REQUIREMENTS CHECKLIST

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Part 845 Reference	Individual Part 845 Components Reviewed for Completeness	Location of Information in HCR
845.620(b)(16)(D)	direction and velocity of groundwater flow; and	Section 3.2.4 to 3.2.6 Tables 3-2 & 3-4 Figures 3-3 & 3-4
845.620(b)(16)(E)	map of the potentiometric surface;	Figures 3-3 & 3-4
845.620(b)(17)	Groundwater classification pursuant to 35 I.A.C. § 620; and	Section 3.2.7

[O: LDC 06/15/21, U: LDC 08/19/21; C: EJT 08/19/21; U:KLT 8/24/21, C: LDC 09/17/21]

Notes:

-- = reference to main regulation

35 I.A.C. § 620 = Title 35 of the Illinois Administrative Code, Part 620

HCR = Hydrogeologic Site Characterization Report

1. INTRODUCTION

1.1 Overview

In accordance with requirements of the Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments: 35 I.A.C. § 845 (Part 845) (IEPA, 2021), Ramboll Americas Engineering Solutions, Inc. (Ramboll) has prepared this HCR on behalf of CPP (**Figure 1-1**), operated by Illinois Power Generating Company (IPGC). This report will apply specifically to the CCR Unit referred to as the GMF GSP. However, information gathered to evaluate other CCR units on site regarding geology, hydrogeology, and groundwater quality is included, where appropriate. The GMF GSP is a lined impoundment with an underdrain system that covers an area of approximately 77 acres. This HCR includes Part 845 content requirements specific to 35 I.A.C. Part 845.620(b) (Hydrogeologic Site Characterization) for the GMF GSP at CPP.

1.2 Part 845 Description

Part 845 contains comprehensive rules for the design, construction, operation, corrective action, closure, and post closure care of SIs containing CCR. CCR is commonly referred to as coal ash, and CCR SIs are commonly referred to as coal ash ponds. This rule includes GWPSs applicable to each CCR SI at the waste boundary and requires each owner or operator to monitor groundwater. IEPA's rule includes a permitting program as well as all federal standards for CCR SIs promulgated by the United States Environmental Protection Agency (USEPA). In addition, the rule includes procedures for public participation, closure alternatives analyses, and closure prioritization. The rule also includes financial assurance requirements for CCR SIs.

1.3 Previous Investigations and Reports

Numerous hydrogeologic investigations have been performed concerning the CCR Units located at the CPP. The information presented in this HCR includes data collected in support of the monitoring well network established for development of the GMP and supplements comprehensive data collection and evaluations from prior hydrogeologic investigation reports (recent to oldest), including, but not limited to, the following:

- ***NRT, January 24, 2017. Hydrogeologic Site Characterization Report, Ash Pond 2, Coffeen Power Station, Coffeen, Illinois.***
Summarizes the results of numerous hydrogeologic investigations that have been performed at the Site, including recent data collected to comply with 40 C.F.R. § 257 Subpart D (CCR Rule) as well as comprehensive data collection and evaluations from prior hydrogeologic investigation reports.
- ***NRT, January 24, 2017. Groundwater Management Zone Application, Coffeen Ash Pond No. 2, Coffeen Power Station, Coffeen, Illinois.***
Establishes a three-dimensional Groundwater Management Zone (GMZ) containing groundwater being managed to mitigate a potential release of CCR constituents from Ash Pond No. 2 (AP2).
- ***NRT, January 24, 2017. Groundwater Monitoring Plan. Coffeen Power Station, Coffeen, Illinois.***
The plan describes the groundwater monitoring and reporting to be completed in support of the Closure Plan for AP2.

- ***NRT, January 24, 2017. Hydrostatic Modeling Report. Coffeen Power Station, Coffeen, Illinois.***
Utilized the Hydrologic Evaluation of Landfill Performance (HELP) model to predict percolation from AP2 and to evaluate AP2 hydrostatic conditions in response to the proposed cover system as described in the Revised 30% Closure Design Package.
- ***NRT, January 24, 2017. Groundwater Modeling Report. Coffeen Power Station, Coffeen, Illinois.***
Included simulations of the site hydrology, the extent of CCR leachate impacts on groundwater and the effect of pond closure on groundwater quality.
- ***Hanson, April 16, 2016. Corrective Action Plan.***
A plan to remediate groundwater exceedances around AP2 and other units. Proposed plan includes reduction in leachate within AP2, enhanced cover system on AP2, and a GMZ.
- ***AECOM, April 2016. Revised 30% Closure Design Package for Coffeen Power Station Ash Pond No. 2.***
A 30% design package for closure of AP2 including the design basis and summary in addition to preliminary construction costs and schedule.
- ***Hanson, April 2016. Uppermost Aquifer Considerations.***
A discussion of the construction of the gypsum pond and relation to the uppermost aquifer in the vicinity of the site.
- ***Hanson, 2015. G153 Assessment.***
Evaluation of manganese, sulfate, and TDS concentrations that were identified at concentrations greater than Class I Groundwater Standards at well G153, which concluded that elevated concentrations were also found upgradient of the Storm Water Runoff Pond and intrawell standards should be utilized.
- ***Hanson, July 2011. Hydrogeologic Report.***
Supports permit applications for the GMF GSP and GMF Recycle Pond (GMF RP).

In conjunction with this report, a GMP is being prepared for the GMF GSP.

1.4 Site Location and Background

The CPP is located in Montgomery County, in central Illinois, within Section 11 Township 7 North and Range 7 East. The CPP is approximately two miles south of the city of Coffeen and about eight miles southeast of the city of Hillsboro, IL (**Figure 1-1**). The GMF GSP is located between the two lobes of Coffeen Lake (identified as "Coffeen Lake" and "Unnamed Tributary" on **Figures 1-1 and 1-2**) to the west, east, and south, and is bordered by agricultural land to the north. The approximately 1,100-acre Coffeen Lake was built by damming the McDavid Branch of the East Fork of Shoal Creek in 1963 for use as an artificial cooling lake for the CPP. Historically, several coal mines were operated at depth in the vicinity of the CPP as well as a US Minerals processing facility located to the north. **Figure 1-2** is a site map showing the location of Ash Pond No. 1 (AP1), AP2, GMF GSP (Part 845 regulated CCR Unit and subject of this HCR), GMF RP, and Landfill (LF). The GMF RP is located south and immediately adjacent to the GMF GSP; therefore, the geology and hydrogeology are similar and results from the 845 investigations from both units are included and discussed in this report, and in the HCR prepared for the GMF RP. The combined area including the GMF RP and GMF GSP will hereinafter be referred to as the Site and data from both units will be utilized in portions of **Sections 2 and 3**. Groundwater quality

data in **Section 4** includes only data from monitoring wells specific to the subject unit (GMF GSP).

1.5 Site History and CCR Units

The CPP was a coal-fired electrical generating plant that began operation in 1964. The plant initially burned bituminous coal from Illinois and CCR from the coal fired units was disposed of in AP1. AP2 was also utilized in the early 1970's and AP1 was reconstructed in 1978. Both of these units were used until the mid-1980's. Beginning in 2010, CCR material was placed in the LF and GMF Units.

GMF Gypsum Stack Pond (GMF GSP): The 77-acre GMF GSP received blowdown from the air emission scrubbers and was put into operation in 2010. Construction of the GMF GSP was in accordance with Water Pollution Control Permit 2008-EA-4661 and features a composite 60-millimeter (mil) high-density polyethylene (HDPE) liner with 3 feet of recompacted soil with a hydraulic conductivity of 1×10^{-7} centimeters per second (cm/s), with internal piping and drains to collect contact water. Construction of the unit required excavation to approximately 603 feet and installation of a groundwater underdrain system to eliminate inward pressure on the liner prior to placement of CCR. The GMF GSP underdrain was actively pumped during construction, but is not actively pumped. IPGC ceased receipt of waste to the GMF GSP prior to April 11, 2021.

GMF Recycle Pond (GMF RP): The 17-acre GMF RP received blowdown from the air emission scrubbers and was put into operation in 2010. Construction of the GMF RP was in accordance with Water Pollution Control Permit 2008-EA-4661 and features a composite 60-mil HDPE liner with 3 feet of recompacted soil with a hydraulic conductivity of 1×10^{-7} cm/s, with internal piping and drains to collect contact water. Construction of the unit required excavation to approximately 601 feet and installation of a groundwater underdrain system to eliminate inward pressure on the liner prior to placement of CCR. The GMF RP underdrain is a passive, gravity drained system. IPGC ceased receipt of waste to the GMF RP prior to April 11, 2021.

Ash Pond No. 1 (AP1): This SI (also known as the Bottom Ash/ Recycle Pond) is a reclaimed ash pond that was reconstructed utilizing the existing earthen berms with reinforcement, as provided by Water Pollution Control Permit 1978-EA-389, issued by IEPA on May 26, 1978. AP1 (existing unlined SI) covers an area of approximately 23 acres, has berms up to 41 feet above the surrounding land surface, and a volume of 300 acre-feet. It primarily received bottom ash and low volume wastes from floor drains in the main power block building. Several years ago, air heater wash and boiler chemical cleaning wastes were directed to AP1, but this practice was discontinued. The bottom ash was periodically removed for beneficial uses by a third-party contractor. Sluicing of waste to AP1 ceased prior to November 4, 2019.

Ash Pond No. 2 (AP2): AP2 is a closed (IEPA approved) SI with a surface area of approximately 60 acres and berms 47 feet higher than the surrounding land surface. AP2 was originally removed from service and capped in the mid 1980's. A clay and soil cap was placed on the surface of the pond with contouring and drainage provided to direct storm water to four engineered revetment down drain structures. Prior to capping, this pond was identified as Outfall 004 in the facility National Pollutant Discharge Elimination System (NPDES) operating permit, IL0000108. Additional closure activities include the construction of a geomembrane cover system that began in July 2019 and was completed on November 17, 2020. The construction was completed in accordance with the Closure and Post Closure Care Plan approved by IEPA on January 30, 2018.

Landfill (LF): Fly ash was managed in a permitted composite lined landfill constructed in 2010. The LF has an active groundwater underdrain system that is currently being pumped. Additionally, the ash landfill leachate collection system is restricted by rule to no more than one foot of leachate on the composite liner. An IEPA groundwater monitoring program is in effect for the GMF (under Bureau of Water) and Ash Landfill (under Bureau of Land).

The approximate dates of construction of each successive stage of the CCR Units at the CPP are summarized in **Table A** below (AECOM, 2016).

Table A. History of Construction

Date	Event
1964	Construction of AP1 (formerly identified as the Bottom Ash Recycle Pond)
1971	Construction of AP2
1978-1979	Installation of internal embankment and new recycle intake structure in AP1 and abandonment of existing outfall structure
1984-1985	Closure of AP2 by installing a clay cover
2000	Installation of a sheet pile wall to facilitate construction of drainage flume along the northeast corner of AP1
2006	Bottom ash system modified in AP1
2008-2010	Construction of the GMF GSP and the GMF RP
2009	Installation of well dewatering system in AP2
2015	Notice of intent to close AP2
2015	Closure plans for AP1, AP2, GMF GSP, GMF RP, and LF submitted to IEPA
2018	IEPA approved Closure and Post-Closure Care Plan for AP2
2020	Completion of closure of AP2 with geomembrane cover system

2. REGIONAL AND LOCAL GEOLOGY

2.1 Topography

The CPP and embankments surrounding the GMF GSP are located at an elevation of approximately 632 feet North American Vertical Datum of 1988 (NAVD88) with the surrounding areas having low topographic relief, generally at an elevation of around 625 to 630 feet NAVD88 (**Figure 2-1**). East and south of the GMF GSP, towards the Unnamed Tributary and Coffeen Lake, the elevation decreases to less than 590 feet NAVD88.

Topographic maps drawn prior to construction (1947) indicate the areas of the CPP were generally from 600 to 640 feet above mean sea level (msl), with the elevations near the GMF GSP ranging from 620 to 640 feet msl (**Appendix A**). No former major drainage features run through the current extent of the GMF GSP.

2.2 Regional Geomorphology

The CPP is located in the central portion of the Springfield Plain of the Till Plains section, the largest physiographic division in Illinois, covering approximately four-fifths of the state. It is characterized by level to undulatory till plains with a few morainic ridges in a late youthful stage of erosion. The Springfield Plain includes the level to gently undulating portion of the Illinoian drift-sheet in central and south-central Illinois (Leighton et al., 1948; Zuehls et al., 1984). Distinguishing features include flatness and shallow drainage features. Moraines in this region are low and broad. Drainage systems are well developed, and the valleys tend to be shallow, broadly alluviated, and terraced (Leighton et al., 1948). Streams in the western portion of the Springfield Plain primarily flow westward, ultimately into the Mississippi River, while streams in the eastern portion flow eastward ultimately into the Wabash River.

2.3 Soils

Surficial soils at the CPP and vicinity are shown on **Figure 2-2** and based on Montgomery County soil survey data available in the Soil Survey Geographic Database (SSURGO) by the United States Department of Agriculture's Natural Resources Conservation Service provided by Environmental Systems Research Institute, Inc.'s (ESRI's) web hosted layer. Former soils underlying the CPP, not including the Fill and CCR within the limits of the GMF GSP are identified as: Herrick Biddle-Piasa silt loams (0 to 2 percent slopes) on the northwest boundary of the unit, surrounding background monitoring wells G270 and G280; Cowden-Piasa silt loams (0 to 2 percent slopes) southwest of the unit; Cowden silt loam (0 to 2 percent slopes) through the center of the unit; Marine silt loam (2 to 5 percent slopes) and Fishook silt loam (2 to 5 percent slopes, eroded) to the north and south of the unit, and through the eastern portion of the unit; and Atlas silt loam (5 to 10 percent slopes, eroded) east of the unit. Marine silt loam (2 to 5 percent slopes) is also found immediately east of the Unnamed Tributary.

2.4 Regional Geology

2.4.1 Unlithified Deposits

Pleistocene deposits of unlithified glacial diamictons, lacustrine/alluvial deposits, and windblown loess overlie Pennsylvanian-age bedrock throughout central Illinois. The most extensive glacial deposits are those from the Illinoian Stage which cover much of the state and are present at the CPP. Windblown (aeolian) deposits, the Peoria and Roxana Silts, cover the glacial deposits over a

majority of the state. These units are fine-grained deposits blown from river valleys by prevailing winds (Hansel and Johnson, 1996).

Surficial deposits, as reported and mapped on a regional scale by the Illinois State Geological Survey (ISGS), are the Vandalia Member (**Figure 2-3**), although the Hagarstown Member has been identified in the vicinity of the CPP. The general sequence of unlithified Quaternary deposits, depicted on **Figure 2-4**, from ground surface down is:

- **Loess Unit:** The loess unit is comprised of the Peoria and Roxana Silts. The Peoria Silt is generally classified and described as light yellow-tan to gray, fine sandy silt. The Roxana Silt is predominately silt-sized material, but can be sandier in localized areas and the base of this unit is often colluvium of silt, and sand (Hansel and Johnson, 1996).
- **Glasford Formation:** Till members present in the surrounding area include the Hagarstown Member, the Vandalia Member, the Mulberry Grove Member, and the Smithboro Member (youngest to oldest). The Hagarstown Member is bounded at the top by the Sangamon Soil. The Vandalia Member is described as a sandy till with thin lenticular bodies of silt, sand, and gravel. It is calcareous, except where weathered, generally gray, and moderately compact. The member consists of gravelly till, poorly sorted gravel, well sorted gravel, and sand. The Mulberry Grove Member is intermittent at the CPP, and is described as a calcareous gray silt and fine sand containing some fossil mollusks. The Smithboro Member is described as a gray, compact, silty till. The Smithboro is bounded below by the Yarmouth Soil (Willman and Frey, 1970).
- **Banner Formation:** Composed primarily of glacial tills and intercalated outwash of sand, gravel, and silt. Members differentiated in western Illinois include the Yarmouth Soil and the Lierle Clay (Hanson, 2009).

2.4.2 Bedrock

Unlithified deposits at the CPP and surrounding areas, described in **Section 2.4.1**, are underlain by rocks belonging to the Pennsylvanian Bond Formation (Kolata, 2005). Detailed descriptions of the Pennsylvanian strata of Illinois were published by Willman et al. (1975). The Bond Formation includes all strata from the base of the Shoal Creek Limestone Member or the LaSalle Limestone Member to the top of the Millersville Limestone Member or the Livingstone Limestone Member. It is overlain by the Mattoon Formation and underlain by the Modesto Formation. It varies from less than 150 feet thick in eastern Illinois to over 300 feet thick in southeastern Illinois, averaging about 250 feet. The Bond Formation is characterized by a high percentage of limestone and calcareous clays and shales. The Bond and Modesto Formations of the McLeansboro Group also contain multiple thin (typically less than 2 feet) intermittent coal beds. The upper formation of the Kewanee Group is the Carbondale Formation which contains multiple coal beds, including the Herrin (No. 6) Coal, of varying thicknesses (up to 7 feet) (ISGS, 2020). It is bound by thick limestone members (up to 50 feet), the thickest and purest limestones in the Pennsylvanian System of Illinois. Gray shales constitute the greatest part of the formation, although thick channel sandstones are developed locally.

The elevation of the bedrock surface in the area ranges from 450 to 500 feet msl. The bedrock surface slopes gently towards the west into a minor bedrock valley that runs north-south (Herzog et al., 1994). Well logs indicate that the lithology of the uppermost bedrock is predominantly shale (Zeizel, 1959).

2.4.3 Structure

The major geologic structural features of Illinois are shown on **Figure 2-5**. There are no major structural features in Montgomery County. The nearest major structural feature to CPP is the Loudon Anticline, which is north-south trending and located approximately 25 miles east of the CPP. Smaller-scale structural features within Montgomery County include the Hillsboro North and Hillsboro South Domes, which are located approximately 15 miles north of the CPP. The Crown Fault, which is a left-lateral fault, and the Girard Fault, which is a northeast dipping normal fault, are located approximately 31 miles northwest of the CPP.

Located south of the CPP in Bond County are the Ayers and Woburn Anticlines and the Greenville Dome. The Ayers Anticline is located approximately 10 miles south of the CPP and trends east-west. The Woburn Anticline is located approximately 10 miles southeast of the CPP and trends north-south. The Greenville Dome is located approximately 15 miles south of the CPP (Nelson, 1995). A review of the available data from the United States Geological Survey (USGS, 2010), ISGS, and other available structural information was completed by Haley & Aldrich, Inc., (2018) for the Location Restriction Demonstration to address the requirements of 40 C.F.R. § 257.62 (Fault Areas). The review found that the nearest known mapped fault is the Crown Fault referenced above, which is located approximately 31 miles northwest of the CPP, and the Centralia Fault zone, located approximately 35 miles southeast of the CPP. The timeframe of the most recent activity on these fault zones is unknown. There are no known active faults or fault damage zones that have had displacement in Holocene time reported or indicated within 200 feet of the GMF GSP (**Figure 2-5**).

2.4.4 Seismic Setting

The nearest areas of present-day fault related, seismic activity are the Northern Illinois Seismic Source Zone, the Wabash Valley Fault Zone near southwestern Indiana, and the New Madrid Fault Zone along the Ohio and Mississippi River Valleys in southeastern Illinois. No recent earthquake epicenters are located in Montgomery County. A magnitude 3.80 earthquake occurred approximately 15 miles south of CPP in Bond County in 1981 and a magnitude 3.60 earthquake occurred approximately 20 miles southeast of CPP in Fayette County in 1990.

35 I.A.C. § 845.330 requires that existing and new CCR SIs and lateral expansions of existing SIs must not be located in seismic impact areas, unless owners or operators demonstrate that the SI is designed to resist the maximum horizontal acceleration(g) in lithified earth material. This requirement is identical to that in 40 C.F.R. § 257.63. The definition of a seismic impact zone is "areas having a 2 percent or greater probability that the maximum expected horizontal acceleration, expressed as a percentage of the earth's gravitation pull, will exceed 0.10 g in 50 years." Although the GMF GSP is located within a seismic impact zone, it satisfies the demonstration requirements of 35 I.A.C. § 845.330. The AECOM report titled "CCR Certification Report: Initial Structural Stability Assessment, Initial Safety Factor Assessment, and Initial Inflow Design Flood Control System Plan for the GMF Pond at Coffeen Power Station", dated October 2016, includes engineering analysis, calculations, and findings that support the requirements of 40 C.F.R. § 257.63 (Haley & Aldrich, Inc., 2018), and, by extension, 35 I.A.C. § 845.330.

2.4.5 Mining Activities

Several coal mines, both strip and underground types, previously operated in Montgomery County, Illinois. A survey to identify historic mining activities was conducted for a 1,000-meter

radius around the Site. Based on records obtained from ISGS, three mines were identified within a 1,000-meter radius of the GMF GSP. A map showing the extent of historic mines is provided in **Appendix B**.

In the southeast portion of the CPP is the Hillsboro Mine (ISGS Mine No. 871), which was operated as a room and pillar panel mine. Operations began in 1964 under the Truax-Traer Coal Company. The mine was purchased by the Consolidation Coal Company in 1971 and production ceased in 1983. An approximately 5- to 7-foot-thick seam of Herrin Coal was mined at approximately 500 feet below ground surface (bgs) (ISGS, 2019). The mine showed indications of small-scale faulting, roof stability issues and floor heaving. The GMF GSP falls within the buffer zone of the Hillsboro Mine.

To the north/northwest is the Clover Leaf No. 4 Mine (ISGS Mine No. 442), which was operated as a room and pillar panel mine. Operations began in 1906 under the Clover Leaf Mining Company. Production discontinued in 1924 under Clover Leaf Coal Company ownership. An approximately 6- to 8-foot-thick seam of Herrin Coal was mined at approximately 510 feet bgs (ISGS, 2019). The GMF GSP directly overlies the southernmost portion of the Clover Leaf No. 4 Mine, and is entirely within the buffer zone of the mine.

To the northeast is the Clover Leaf No. 1 Mine (ISGS Mine No. 3001), which was operated as a room and pillar mine. Operations began in 1889 under the Coffeen Coal & Coke Company. The mine was purchased by the Clover Leaf Coal Company in 1901, and production ceased in 1908. An approximately 7- to 8-foot-thick seam of Herrin Coal was mined at approximately 534 feet bgs (ISGS, 2019). The GMF GSP does not overlie the Clover Leaf No. 1 Mine nor its corresponding buffer zone.

2.5 Site Geology

The Quaternary deposits in the vicinity of the CPP consist mainly of diamictons and interbedded outwash deposits that were deposited during Illinoian and Pre-Illinoian glaciations. The CPP geology summarized below is from a combination of the Hydrogeologic Monitoring Plan (NRT, an OBG Company [NRT/OBG], 2017) and a field investigation performed in 2021 to collect additional data for the discussion of vertical and horizontal lithology, stratigraphy, chemical properties, and physical properties of geologic layers to a minimum of 100 feet bgs as specified in 35 I.A.C. § 845.620(b). Field investigation locations are shown on **Figure 2-6** and cross-sections are included in **Figure 2-7**. Soil boring logs and well construction logs are provided in **Appendix C**. Samples for geotechnical analysis were collected from interpreted geologic units and composited to obtain a representative sample of the entire geologic unit prior to submittal (**Table 2-1**). The unconsolidated deposits and bedrock which occur at the CPP include the following units (beginning at the ground surface):

- **CCR:** CCR consisting of gypsum is present within the GMF GSP and GMF RP and non-CCR fill material consisting of silt, clay, and sand comprises the berms surrounding the GMF GSP and GMF RP.
- **Loess Unit:** Clays and silts, including undifferentiated Roxana Silt and Peoria Silt with thicknesses ranging from 1 to 16 feet, where present at the CPP.
- **Hagarstown Member:** The Hagarstown Member (consisting of gravelly clay till and sandy materials in contact with the Vandalia Member (also referred to as Hagarstown Beds) has been separated into two units for this discussion: the first unit, consisting of the gravelly clay

till and the second unit consisting of sandy material overlying the Vandalia Member. The Upper Hagarstown Member is up to 6 feet thick, while the sandy portions, where present, are generally less than 3 feet thick, although thicknesses up to 7 feet have been observed north of the LF.

- **Vandalia Member:** Sandy, silt, or clay till that is generally greater than 15 feet thick.
- **Mulberry Grove Member:** Gray silt and sandy silt/clay unit found between the Vandalia Till and the Smithboro Till. Generally thin and not laterally continuous across the CPP.
- **Smithboro Member:** Thick, gray compacted silty clay diamicton.
- **Yarmouth Soil:** Sand and sandy silt/clay, which include accretionary deposits of fine sediment and organic materials, typically less than 5 feet thick and not laterally continuous.
- **Lierle Clay Member:** Clay and silt with some sand which is the upper portion of the Banner Formation. No borings advanced on site penetrated the full thickness of the Lierle Clay.

2.5.1 Fill and CCR

Gypsum scrubber waste and other non-CCR wastes are present within the GMP GSP. The gypsum grab sample locations are shown on **Figure 2-6**. The elevation at the top of the fill layer estimated from the topographic surface (**Figure 2-1**) within the limits of the GMF GSP is from approximately 610 to 624 feet NAVD88.

Borings were not advanced during the 2021 investigation in the GMF GSP due to safety concerns, and two grab samples of gypsum were collected near the northwest corner of the GMF GSP. Based on a topographic survey conducted in 2021 and the base of the GMF GSP (**Figure 2-8**), gypsum thickness is estimated from approximately 4 feet at the southern extent of the pond up to a maximum depth of 17 feet in the northern extent of the pond. This is consistent with the gypsum thickness estimated at a maximum of 16 feet in 2016 (AECOM, 2016).

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the CCR material in the grab sample indicated the following:

- Moisture content is 25.3 percent.
- Dry density is 78.0 pounds per cubic feet (pcf).
- Particle size distribution is 0 percent gravel, 22 percent sand, 78 percent fines.
- The sample was not analyzed for specific gravity, so porosity was not calculated. Based on grain size of the sample the estimated porosity of the sample is 3 to 19 percent (Fetter, 2001).

Solids samples collected from the gypsum grab sample were also collected for chemical analysis. The results of solids samples collected from within the GMF GSP are summarized in **Table 2-2**. Additionally, the NE Riser (**Figure 3-1**), which collects contact water from process piping located in the northeast corner of the GMF GSP, was sampled as a representative source water location in 2021. The results of source water samples collected from the GMF GSP are summarized in **Table 2-3**.

2.5.2 Loess Unit

The Loess Unit is the uppermost unlithified unit identified at the CPP. This unit is comprised of the combined Roxana and Peoria Silt and extends from beneath the topsoil, derived from the loess, to the top of the Hagarstown Member. The loess has been classified as silt or clayey silt, with minor amounts of sand. The Loess Unit is generally considered unsaturated, and the uppermost aquifer is recharged by precipitation that percolates through this unit.

The top of the Loess Unit was typically encountered from approximately 606 to 628 feet NAVD88. Loess Unit thickness ranges from 0 feet (absent) to 16 feet. Construction of the LF, GMF GSP, and GMF RP required the excavation and removal of this layer within the unit footprints. The Loess Unit is typically thickest to the north, and is absent near historic drainage features to the south.

During the 2021 investigation of the GMF GSP, the Loess Unit was typically encountered from 2 to 5 feet bgs, at elevations of approximately 626 to 629 feet NAVD88, and was generally 8 to 14 feet thick, where present near the GMF GSP and GMF RP. The Loess Unit was absent in borings G283 and G285, located near the Unnamed Tributary.

The geotechnical testing results are summarized in **Table 2-1** and geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Loess Unit indicated the following:

- Average moisture content is 20.6 percent and ranges from 20.2 to 21.2 percent.
- Average calculated porosity is 35.2 percent and ranges from 35.1 to 35.3 percent.
- Average dry density is 104.9 pcf and ranges from 103.4 to 105.9 pcf.
- Average specific gravity is 2.58 and ranges from 2.56 to 2.60.
- Particle size distribution is 0 percent gravel, 20 percent sand, and 80 percent fines (46 to 50 percent silt, and 30 to 34 percent clay).

Soil samples collected from the Loess Unit were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.3 Hagarstown Member

The Hagarstown Member (also referred to as Hagarstown Beds) exhibits two units; the first unit, consisting of the gravelly clay till and the second consisting of sandy material overlying the Vandalia Member. The clay till portion had varying thicknesses ranging from approximately 2 to 6 feet as observed adjacent to, south, and west of the Pond. Based on the historic topographic map (**Appendix A**), the Hagarstown Member is not present in former drainage features.

During construction of the LF, GMF GSP, and GMF RP, the Loess Unit and the Hagarstown Member were excavated to facilitate construction and eliminate groundwater flow into excavations. The excavations were backfilled with structural fill and an underdrain system was installed to mitigate inward hydraulic pressure and potential liner uplift damage before the CCR units were filled. The LF underdrain system remains but is no longer actively pumped. The GMF GSP underdrain system has not been actively pumped since construction was completed. The GMF RP gravity underdrain remains in place.

Where present, the sandy portion of the Hagarstown Member is generally 2 to 4 feet thick. The composition of the sandy portion of the Hagarstown unit varies across the CPP and has been

classified as gravelly till, poorly sorted gravel, well sorted gravel, sand, and silty sand. The elevation of the top of the Hagarstown generally declines as the unit approaches Coffeen Lake or other topographic drainage features.

During the 2021 investigation, the sandy portion of the Hagarstown Member near the GMF GSP was generally encountered at 18 feet bgs, at an elevation of approximately 612 feet NAVD88, and was generally 2 to 4 feet thick, where present (**Figure 2-7**).

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Hagarstown Member indicated the following:

- Average moisture content of samples collected near the GMF GSP is 13.4 percent and ranges from 10.1 to 16.6 percent.
- Average calculated porosity of samples collected near the GMF GSP is 28.6 percent and ranges from 26.5 to 30.6 percent.
- Average dry density of samples collected near the GMF GSP is 115.0 pcf and ranges from 112.6 to 117.4 pcf.
- Average specific gravity of samples collected near the GMF GSP is 2.58 and ranges from 2.56 to 2.60.
- Particle size distribution of samples collected near the GMF GSP is 0 percent gravel, 46 to 57 percent sand, and 43 to 54 percent fines (28 to 34 percent silt and 15 to 20 percent clay).

Soil samples collected from the Hagarstown Member were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.4 Vandalia Member

The Vandalia (Till) Member is a sandy/silty till with thin, discontinuous lenses of silt, sand, and gravel. The Vandalia Member was encountered in all borings advanced at the CPP. The Vandalia Member typically ranged in thickness from 11.7 feet in the northern portion of the CPP, to 31.0 feet between the GMF GSP and the GMF RP. Similar to the observed top elevation of the Hagarstown Member, the top of the Vandalia Member declines in elevation near Coffeen Lake and topographic drainage features. This unit is relatively thick throughout the CPP, with an average thickness of over 15 feet (Hanson, 2009).

During the 2021 investigation, the Vandalia Member was encountered from 1.5 to 34 feet bgs, at elevations of approximately 598 to 608 feet NAVD88, where present. The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Vandalia Member indicated the following:

- Average moisture content is 8.6 percent and ranges from 8.3 to 8.9 percent.
- Average calculated porosity is 18.6 percent and ranges from 17.7 to 19.4 percent.
- Average dry density is 132.1 pcf and ranges from 131.4 to 132.7 pcf.
- Average specific gravity is 2.60 and ranges from 2.56 to 2.64.

- Particle size distribution is 0 percent gravel, 46 to 47 percent sand, and 53 to 54 percent fines.

Soil samples collected from the Vandalia Member were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.5 Mulberry Grove Member

The Mulberry Grove (Silt) Member typically consists of a thin, lenticular unit of gray sandy silt (Willman et al., 1975). It represents the interval between the retreat of the glacier that deposited the Smithboro Member and the advance of the glacier that deposited the Vandalia Member. At the CPP, the Mulberry Grove Member is represented by pockets (generally less than 2 feet thick) of gray sandy silt. This unit was absent in many borings through the central portion of the Site from south to north. Where sampled, the Mulberry Grove Member ranged in thickness from 0.5 to 4.9 feet near the GMF GSP (Hanson, 2009). During the 2021 investigation, the Mulberry Grove Silt was not encountered in the borings near the GMF GSP. These silts appear to be deposited in depressions found in the surface of the underlying Smithboro Member.

2.5.6 Smithboro Member

The Smithboro (Till) Member is described as a gray, compact, silty, clayey diamicton. The Smithboro Member ranges in thickness from 6.7 to 21.2 feet northwest of the landfill.

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Smithboro Member indicated the following:

- The moisture content is 14.2 percent and ranges from 13.4 to 14.9 percent.
- Porosity is 27.3 percent and ranges from 26.2 to 28.4 percent.
- Dry density is 118.6 pcf and ranges from 116.6 to 120.6 pcf.
- Specific gravity is 2.62 and ranges from 2.61 to 2.62.
- Particle size distribution is 0 percent gravel, 26 to 29 percent sand, and 72 to 74 percent fines.

Soil samples collected from the Smithboro Member were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.7 Yarmouth Soil

The Yarmouth Soil is described as the weathered zone on the Kansan drift, but in some places, it consists of accretionary deposits of fine sediment and organic material that accumulated in poorly drained areas on the surface of the Kansan deposits. Historical borings in the northern portion of the CPP which encountered the Yarmouth were summarized previously by Hanson (2009) as ranging in thickness from 0 feet (absent) to 5.1 feet.

During the 2021 investigation, the Yarmouth Soil was encountered from 46 to 55 feet bgs, at an elevation from approximately 565 to 577 feet NAVD88, and was 1 to 3 feet thick, where present. The measured thickness was consistent with previous investigations.

The geotechnical testing results are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Yarmouth Soil indicated the following:

- Moisture content of the G206D sample is 14.9 percent.
- No samples were analyzed for dry density; therefore, average porosity was not calculated for the Yarmouth Soil. Based on material type encountered in the borings, the effective porosity is expected to range from 10 to 28 percent (Fetter, 2001).
- Specific gravity of the G206D sample is 2.75.
- Particle size distribution of the G206D sample is 0 percent gravel, 78 percent sand, 13 percent silt, and 9 percent clay.

Soil samples collected from the Yarmouth Soil were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.8 Lierle Clay Member/ Banner Formation

The Lierle Clay Member is the uppermost member of the Kansan Stage Banner Formation. It is described as an accretion gley with clay, silt, and some sand. It was encountered by Hanson (2009) in all but a few borings on site. During the 2021 investigation, borings G206D and SB289 encountered the Lierle Clay at 54 and 57 feet bgs, at approximately 571 feet NAVD88 in both borings. No borings penetrated the full thickness of the Banner Formation near the GMF GSP.

The geotechnical testing results from the sample collected at SB289 are summarized in **Table 2-1** and the geotechnical laboratory report is included in **Appendix D**. Geotechnical testing results from the Lierle Clay Member indicated the following:

- Moisture content of the sample collected from SB289 is 25 percent.
- Calculated porosity of the sample collected from SB289 is 40.0 percent.
- Dry density of the sample collected from SB289 is 98.4 pcf.
- Specific gravity of the sample collected from SB289 is 2.63.
- Particle size distribution of the sample collected from SB289 is 0 percent gravel, 21 percent sand, 36 percent silt, and 43 percent clay.

Soil samples collected from the Lierle Clay were also submitted to an analytical laboratory for chemical analysis. The results of this chemical analysis are summarized in **Table 2-4**.

2.5.9 Bedrock

Pennsylvanian-age Bond Formation bedrock was not encountered in any borings advanced at the CPP, so site-specific information is not available.

3. REGIONAL AND LOCAL HYDROGEOLOGY

3.1 Regional Hydrogeology

The water table conforms more or less to the topographic features of the land surface. Recharge occurs in the uplands and flows towards drainage features. Moderate thicknesses of unconsolidated materials fill shallow valleys or are present on the uplands bordering the main valleys. These materials contain thin and discontinuous deposits of sand and gravel. Potable water in Montgomery County is primarily serviced by the Hillsboro and Litchfield Water Departments. Surface water of Lake Glenn Shoals and Old Hillsboro Lake serves Hillsboro, Illinois and the surrounding communities (e.g., Coffeen) (Hillsboro, 2021). Groundwater for domestic and farm supplies is obtained locally in this area from wells drilled in sand and gravel, but in some places good water-yielding deposits are absent and water from the unconsolidated material is obtainable only with large-diameter dug wells (Selkregg et al., 1957).

3.2 Site Hydrogeology

Over 100 monitoring wells have been installed since 2006 to monitor groundwater conditions around the five CCR units at the CPP for both State and Federal programs. Three monitoring wells were installed in 2008 and fourteen monitoring wells (one replacement) were installed in 2010 around the GMF GSP. One replacement well was installed near the GMF GSP in 2017 to meet requirements of the CCR Rule. In 2021, one additional well and one soil boring were installed to provide information to meet requirements of Part 845. A summary of the current monitoring well networks, and construction details, is included in **Table 3-1** and locations shown on **Figure 3-1**. This section discusses the recently (2021) collected information, focusing on the existing well network and monitoring wells installed after 2015 around the GMF GSP, as well as appropriate historical data from wells installed prior to 2015.

3.2.1 Hydrostratigraphic Units

Six hydrostratigraphic units have been identified at the CPP based on stratigraphic relationships and common hydrogeologic characteristics, and are summarized as follows:

- **CCR:** This unit is composed of CCR, consisting primarily of gypsum scrubber waste. This also includes earthen fill deposits of predominantly silt and clay materials from on-site excavations that were used to construct berms and roads surrounding the various impoundments across the CPP.
- **UCU:** Consists of the Loess Unit and the upper clayey portion of the Hagarstown Member which has generally lower vertical permeability and generally greater than 60 percent fines (**Table 2-1**). This Unit was encountered across most of the CPP, with the exception of the eastern edges of the SIs near the Unnamed Tributary where the unit was eroded following deposition or locations where it has been excavated for construction.
- **Uppermost Aquifer:** This unit consists primarily of sand and sandy silts and clays at the base of the Hagarstown Member and, in some locations, the uppermost weathered sandy clay portion of the Vandalia Member. This unit is absent in several locations due to weathering and in others due to excavation during construction of the CCR Unit. The hydraulic characteristics of the Hagarstown Member indicate the unit has a moderate hydraulic conductivity.

- **LCU:** This unit is composed of the sandy clay till of the Vandalia Member, the silt of the Mulberry Grove Formation, and the compacted clay till of the Smithboro Member. The unit underlies the uppermost aquifer and was encountered in all boring locations on the CPP. Results from laboratory tests completed for vertical hydraulic conductivity indicate the Vandalia Member has a very low vertical hydraulic conductivity.
- **DA:** This unit consists primarily of sandy silt and sands of the Yarmouth Soil, which are thin (less than 5 feet) and discontinuous across the CPP.
- **DCU:** This unit underlies the DA and is composed of the Banner Formation, of which the thick Lierle Clay is the first encountered unit. No boring penetrated the full thickness of this formation.

3.2.2 Uppermost Aquifer

The base of the Hagarstown Member is identified as the uppermost aquifer on Site. The sandy clay and sand of the uppermost aquifer is confined except where site excavations and ravines extend through the Loess Unit into the Hagarstown Member. The top of the uppermost aquifer was evaluated with respect to the location restrictions in 2018 (Haley & Aldrich, Inc., 2018) and provided in **Figure 3-2**. The top of the uppermost aquifer occurs at an elevation of 609 to 611 feet and was removed below the footprint of the GMF GSP (**Figure 2-7**). The base of the uppermost aquifer and the material on which the GMF GSP liner was placed is the top of the LCU which is comprised of the low permeability Vandalia Member, Mulberry Grove Member, and Smithboro Till.

3.2.3 Potential Migration Pathways

Potential migration pathways (PMPs) were interpreted using the lithologic composition and hydrogeologic properties (hydraulic conductivity and hydraulic position with respect to the unit) of the screened materials. In addition to the physical properties, the analytical results from the baseline groundwater monitoring performed in wells screened in the confining units and DA were used to identify PMPs. The uppermost aquifer is the first occurrence of groundwater and therefore the PMPs identified are in geologic units located below the uppermost aquifer. Monitoring well G206D is considered a DA PMP monitoring location and monitors the potential migration of impacts through the LCU.

3.2.4 Water Table Elevation and Groundwater Flow Direction

The NE Riser was utilized during the 2021 investigation to collect contact water samples from the GMF GSP. A transducer was installed near the NE Riser during the 2021 investigation to monitor pond water levels in the GMF GSP. The water elevations in the GMF GSP showed minimal variation, with elevations from approximately 625 to 627 feet NAVD88 (**Appendix E**).

No monitoring wells were installed in the UCU during 2021 investigation activities and no wells have historically been installed across solely the UCU because it is not present or is unsaturated in portions of the CPP.

During the 2021 Part 845 investigation, groundwater elevations in the uppermost aquifer ranged from approximately 591 to 625 feet NAVD88 across the CPP (**Appendix E**). Groundwater elevations were typically highest towards the northern extent of the CPP, near the GMF GSP and GMF RP, except monitoring well G307 south of AP1, which consistently had the highest groundwater elevation. Groundwater elevations were lowest near the Unnamed Tributary and

east of AP1 towards Coffeen Lake. Groundwater elevations in the vicinity of the GMF GSP were typically from 617 to 622 ft NAVD88 (**Figures 3-3 and 3-4**). No seasonal variation has been observed in the uppermost aquifer monitoring wells, and any seasonal responses may be muted by the proximity and hydraulic connection to Coffeen Lake.

Overall groundwater flow within the uppermost aquifer is divided towards the two lobes of Coffeen Lake. The groundwater divide runs approximately through the center of the CPP, with groundwater east of the divide flowing east to southeast towards the Unnamed Tributary or the eastern lobe and groundwater west of the divide flowing west to southwest towards the western lobe. Groundwater flows east to southeast across the GMF GSP (**Figures 3-3 and 3-4** and **Appendix E**) toward the Unnamed Tributary. Based on the elevations of the Tributary (**Figure 2-7**) and groundwater elevations measured east of the tributary (**Figures 3-3 and 3-4**) the tributary is a hydraulic barrier and prevents groundwater migration east of the Unnamed Tributary. Although elevations vary seasonally, the groundwater flow direction in the uppermost aquifer is consistent and likely controlled by the proximity and hydraulic connection to Coffeen Lake.

Monitoring wells G206D, G275D, and G314D are screened across the DA. Groundwater elevation within the DA typically ranges from approximately 567 to 590 feet NAVD88. G206D is nearest the GMF GSP and typically has groundwater elevations ranging from about 584 to 591 feet NAVD88. Groundwater contour maps are not generated for the DA; however, groundwater flow within the DA is expected to generally follow subsurface topography for the unit.

3.2.4.1 Vertical Hydraulic Gradient

Vertical hydraulic gradients were calculated using available groundwater elevation data from March through July 2021 and from historic readings from 2017 to 2019 at nested well locations within the uppermost aquifer, LCU, and DA. Vertical hydraulic gradients for the GMF GSP are presented in **Table 3-2**. Vertical hydraulic gradients for other nested well locations at the CPP, discussed below, are presented in **Appendix E**. The results of the vertical hydraulic gradient calculations between hydrostratigraphic units are summarized below:

- Uppermost aquifer to upper LCU (Vandalia Member)
 - In 2021, vertical gradients in well nest G405/T408, located north of AP2, were consistently downward, with an average vertical gradient of 0.03 feet per foot (ft/ft). From 2017 to 2020, vertical gradients in well nest G405/T408 varied between upward and downward, with an average (downward) vertical gradient of 0.04 ft/ft.
 - In 2021, vertical gradients in well nest G406/T409, located near the southwest corner of AP2, were consistently upward, with an average vertical gradient of -0.18 ft/ft. From 2017 to 2020, vertical gradients in well nest G406/T409 varied between upward and downward, with an average (downward) vertical gradient of 0.02 ft/ft.
 - Vertical hydraulic gradients indicate there is intermittent migration of groundwater from the uppermost aquifer downward into the LCU in the vicinity of well nests G405/T408 and G406/T409.

- Uppermost aquifer to DA
 - During 2021, vertical gradients at well nest G206/G206D, located near the southwest corner of the GMF GSP, were consistently strongly downward, with an average vertical gradient of 1.01 ft/ft.
- Upper LCU (Vandalia Member) to Lower LCU (Smithboro Member)
 - In 2021, vertical gradients at well nest T408/G45D, located north of AP2, were consistently downward with an average vertical gradient of 0.20 ft/ft. The direction is consistent with measurements from 2017 to 2020 although less than the average downward gradient measured (2.02 ft/ft).
 - In 2021, vertical gradients at well nest T409/G46D, located near the southwest corner of AP2, were downward with an average vertical gradient of 0.25 ft/ft. This direction is consistent with measurements from 2017 to 2020, although less than the average downward vertical gradient measured (1.28 ft/ft).
- LCU to DA
 - During 2021, vertical gradients in well nest G314/G314D were consistently strongly downward, with an average vertical gradient of 1.32 ft/ft.

Vertical hydraulic gradients indicate there is consistently downward migration of groundwater in most areas of the CPP, with the exception being northwest of AP1, where consistent upward gradients were measured between the upper LCU and UA in 2021.

3.2.4.2 Impact of Existing Ponds and Ash Saturation

During construction of the GMF GSP, the Loess Unit and the Hagarstown Member were excavated within the footprint of the GMF GSP. Groundwater surface does not appear to be affected by water levels in the GMF GSP, which is hydraulically isolated by a composite HDPE liner with 3 feet of recompacted soil with a hydraulic conductivity of 1×10^{-7} cm/s. Changes in pond elevations in 2021 are minimal, and do not result in, or vary with, corresponding changes in groundwater elevations. Saturated gypsum has been observed within the GMF GSP. As discussed above, the water within the GMF GSP is hydraulically isolated from surrounding groundwater; therefore, the thickness of saturated gypsum within the pond will vary with the level of water maintained in the pond.

3.2.4.3 Impact of Coffeen Lake on Groundwater Flow

Groundwater contour maps prepared from elevation data measured in monitoring wells indicate groundwater elevations can be variable but flow directions are generally consistent. Groundwater generally flows from the center of the CPP west towards Coffeen Lake, and east towards the Unnamed Tributary, the eastern lobe of Coffeen Lake, and the discharge flume, resulting in a groundwater divide (high) running through the middle of the CPP.

Construction of the LF, GMF GSP, and GMF RP required removal of the Hagarstown Member, in effect removing the aquifer beneath the footprint of these units (Hanson, 2016). It is uncertain whether these constructed units significantly limit lateral groundwater flow, either by creating no flow zones or by capturing groundwater via their dewatering (NRT, 2017).

3.2.5 Hydraulic Conductivities

3.2.5.1 Field Hydraulic Conductivities

Field hydraulic conductivity tests were performed by Hanson in 2021 as part of characterization efforts to complete Part 845 requirements. Individual field hydraulic conductivity test results conducted at the GMF GSP are summarized in **Table 3-3** and the field hydraulic conductivity data is included in **Appendix F**. The results of the tests are summarized as follows:

- Uppermost aquifer: Hydraulic conductivities near the GMF GSP ranged from 2.5×10^{-4} to 4.0×10^{-3} cm/s. Tests had a geometric mean of 1.4×10^{-3} cm/s. This is generally consistent, although higher than tests conducted prior to 2017 as part of CCR Rule characterization efforts that indicated hydraulic conductivities varied from 1.7×10^{-5} to 2.1×10^{-3} cm/s with a geometric mean of 2.9×10^{-4} cm/s (NRT, 2017).
- LCU: Hydraulic conductivities across the CPP ranged from 1.2×10^{-4} and 4.5×10^{-3} cm/s. Tests had a geometric mean of 7.2×10^{-4} cm/s (NRT, 2017). No monitoring wells near the GMF GSP are screened within the LCU. Monitoring wells with the highest hydraulic conductivities were located near the GMF RP and wells with the lowest hydraulic conductivities were located near AP1. Prior to 2017, field hydraulic conductivity tests completed in the LCU for monitoring well and temporary piezometers (G45D, G46D, T408, and T409) indicate horizontal conductivities from 4.0×10^{-8} and 3.4×10^{-5} . The elevated hydraulic conductivity values (10^{-4} to 10^{-3} cm/s) in wells near the GMF GSP relative to other areas of the CPP are likely not representative of the primary LCU lithology, but instead reflect the isolated and discontinuous sandy lenses in which the wells are screened (NRT, 2017).
- DA: Geometric mean hydraulic conductivity at DA well G314D, near AP1, was 8.7×10^{-5} cm/s and was slightly lower than tests completed in the northern portion of the CPP in 2009 that resulted in hydraulic conductivity values ranging from 1.3×10^{-4} to 1.7×10^{-3} cm/s, with a geometric mean of 4.4×10^{-4} cm/s (NRT, 2017). Field hydraulic conductivity testing was not performed on DA monitoring well G206D, located near the GMF GSP.
- No monitoring wells are screened only within the DCU, and no field hydraulic conductivity tests were conducted for the DCU.

3.2.5.2 Laboratory Hydraulic Conductivities

Falling head permeability tests (ASTM D5084 Method F) were performed in the laboratory on nine samples collected primarily from CCR and confining units at the CPP during the 2021 investigations. Samples collected from locations near the GMF GSP are shown on **Figure 2-6**. The geotechnical laboratory report is provided in **Appendix D**. The results samples are summarized in **Table 2-1** and discussed below.

- CCR: One geotechnical sample of CCR (gypsum) was collected as a grab sample near the NE Riser and the vertical hydraulic conductivity is 8.9×10^{-4} cm/s.
- UCU:
 - The 2021 sitewide geometric mean of vertical hydraulic conductivities of three samples collected from the UCU is 2.5×10^{-8} cm/s, which is consistent with historically reported values. Vertical hydraulic conductivity of a sample collected from SB289 near the GMF GSP is 1.1×10^{-8} cm/s.

- Geotechnical tests conducted prior to 2017 indicated UCU vertical hydraulic conductivity values ranging from 1.3×10^{-8} to 5.0×10^{-7} cm/s, with a geometric mean of 1.0×10^{-7} cm/s (NRT, 2017).
- Uppermost Aquifer: One geotechnical sample of uppermost aquifer material was collected from G275D, near the GMF RP, with a vertical hydraulic conductivity of 1.6×10^{-4} cm/s. No uppermost aquifer samples collected near the GMF GSP were analyzed for vertical hydraulic conductivity.
- LCU:
 - The 2021 sitewide geometric mean of vertical hydraulic conductivities of three samples collected from the LCU is 1.8×10^{-7} cm/s. Vertical hydraulic conductivities from 2021 are consistent with those observed historically. No LCU samples collected near the GMF GSP were analyzed for vertical hydraulic conductivity.
 - Intermittently present within the LCU is the Mulberry Grove Member. Historic vertical hydraulic conductivities of the Mulberry Grove Member were measured as 1.6×10^{-6} and 1.9×10^{-6} cm/s.
 - Historic laboratory tests reported LCU hydraulic conductivity values ranging from 6.8×10^{-9} to 4.5×10^{-6} cm/s, with a geometric mean of 3.0×10^{-8} cm/s (NRT, 2017).
- **DA:** No laboratory vertical hydraulic conductivity tests were completed during 2021 on DA materials.
- **DCU:** No laboratory vertical hydraulic conductivity tests were completed during 2021 on DCU materials. Historic vertical hydraulic conductivity tests were performed on samples collected north and west of the GMF GSP. Vertical hydraulic conductivities of 6.8×10^{-9} and 4.5×10^{-6} cm/s were reported (NRT, 2017).

3.2.6 Horizontal Groundwater Gradients and Flow Velocity

Horizontal gradient and flow velocities are calculated using the flow path from R104 to G215 (**Table 3-4**). Horizontal gradients are from 0.003 to 0.005 ft/ft, equating to a minimum flow velocity of 0.05 feet/day (ft/day) and a maximum flow velocity of 0.11 ft/day. Average calculated flow velocity across the GMF GSP is 0.08 ft/day; however, the flow velocity is not representative of actual groundwater flow conditions since the uppermost aquifer is not present beneath the entire footprint of the GMF GSP.

3.2.7 Groundwater Classification

Per 35 I.A.C. § 620.210, groundwater within the uppermost aquifer at the GMF GSP meets the definition of a Class I – Potable Resource Groundwater based on the following criteria:

- Groundwater in the uppermost aquifer is located 10 feet or more below the land surface and
- Within a geologic material which is capable of a hydraulic conductivity of 1×10^{-4} cm/s or greater using a slug test (**Table 3-3**).

3.3 Surface Water Hydrology

3.3.1 Climate

Average climatic data was obtained from the National Oceanic and Atmospheric Administration National Centers for Environmental Information Climate Data Online. The data was recorded from 2001 to May 2021 from Hillsboro, Illinois, which is located approximately eight miles northwest of CPP. The data includes monthly maximum and monthly minimum daily temperatures (degrees Fahrenheit [°F]) and average rainfall for each month calculated from daily values collected over the 20-year period. The data is summarized in **Table B** below.

Table B: Average Monthly Temperature Extremes and Precipitation for Hillsboro, Illinois

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec	Annual
Maximum Temperature (°F)	38.4	42.8	54.8	67.2	77.2	86.1	88.4	86.0	82.3	69.5	54.2	44.9	66.3
Minimum Temperature (°F)	22.3	24.5	34.9	44.3	55.4	64.2	67.2	64.7	58.6	46.6	34.5	28.3	45.8
Precipitation (inches)	1.94	2.14	2.78	5.72	4.18	4.64	3.71	3.37	2.77	3.29	2.88	2.95	40.4

<https://www.ncei.noaa.gov/orders/cdo/2651630.csv>

3.3.2 Surface Waters

The primary surface water body in the area, Coffeen Lake, is comprised of two lobes (identified as “Coffeen Lake” and “Unnamed Tributary” on **Figure 1-2**). The main body of Coffeen Lake is immediately adjacent to CPP on the west and south and the Unnamed Tributary borders CPP to the east.

In 1963, a 75 foot-high earthen dam was built across the McDavid Branch of East Fork Shoal Creek, creating Coffeen Lake for use as an artificial cooling lake for CPP. Coffeen Lake covers approximately 1,100 acres. The lake is part of the Shoal Creek Watershed (Hydrologic Unit Code [HUC] 07140203), which encompasses approximately 916 square miles. The average depth of Coffeen Lake is approximately 19 feet and the maximum depth is approximately 59 feet (Illinois Department of Nature Resources [IDNR], 2014). The average elevation of Coffeen Lake is approximately 591 feet NAVD88.

A USGS stream gage (USGS 05593900) for East Fork Shoal Creek near Coffeen, Illinois (latitude 39.1347 degrees north, longitude 89.3525 degrees west) is located approximately 6.5 miles northeast (upstream) of CPP. The gage datum elevation is 574.76 feet National Geodetic Vertical Datum of 1929 (NGVD29) (574.39 feet NAVD88). Daily gage heights for the period of January 1, 2018 through March 30, 2021 are shown below in **Figure A** (USGS, 2021a). The gage height of approximately 2 feet, representing approximate baseflow, occurs at an elevation of about 576.39 feet NAVD88.

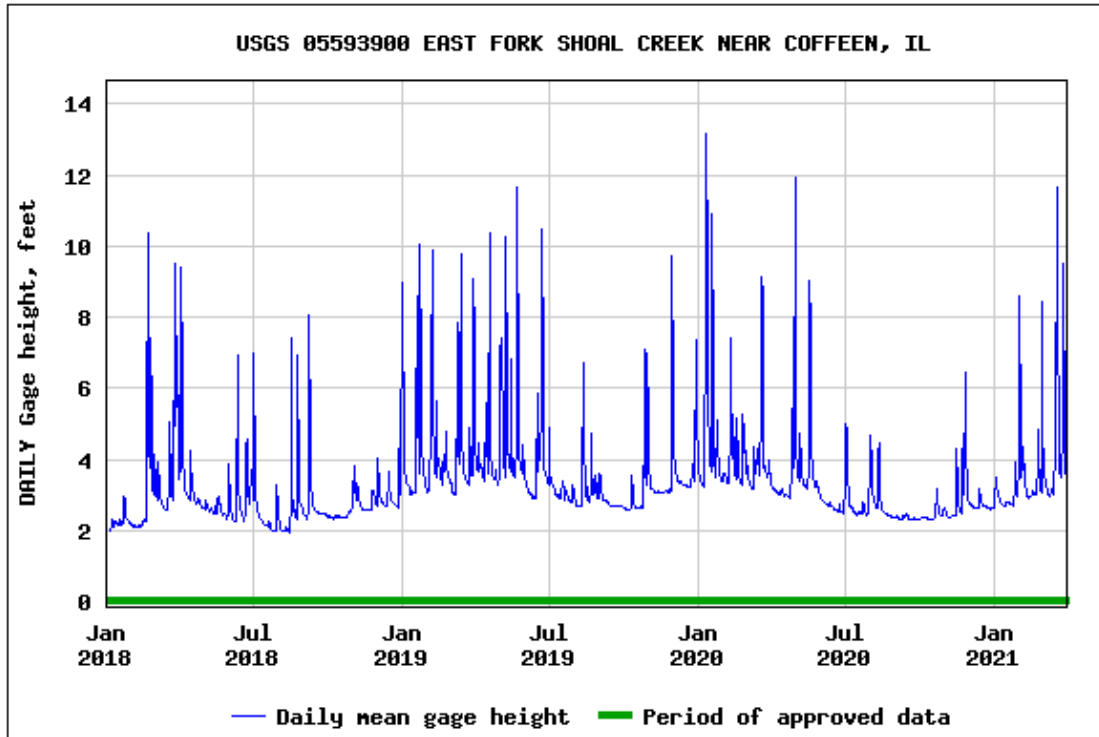


Figure A. Daily Gage Height of East Fork Shoal Creek Near Coffeen, Illinois (USGS 05593900)

Historically, Coffeen Lake received water discharge from both CPP and the Hillsboro Mine in addition to natural precipitation and drainage from East Fork Shoal Creek. At present, Coffeen Lake receives discharge from CPP under NPDES Permit No. IL 0000108. Additionally, an emergency spillway, located at the northeast corner of the GMF RP, discharges to the Unnamed Tributary, east of the CPP.

4. GROUNDWATER QUALITY

4.1 Summary of Groundwater Monitoring Activities

In accordance with the IEPA Water Pollution Control Permit No. 2020-EO-65043 the GMF GSP has been sampled since 2008. The monitoring program includes quarterly sampling and analysis of dissolved indicator parameters, and annual sampling for total concentrations. In 2015, additional well installation and groundwater sampling was initiated to meet the requirements of 40 C.F.R. § 257. Groundwater samples were collected and totals analyses were completed for Appendix III and Appendix IV parameters. In 2021, additional borings and wells were installed to comply with Part 845; wells were sampled for the parameters listed in 35 I.A.C. § 845.600. A review and summary of data from the annual IEPA groundwater sampling, 40 C.F.R. § 257, and Part 845 monitoring programs is included in the evaluation of groundwater quality at the Site.

4.1.1 40 C.F.R. § 257 Program Monitoring and Well Network

The 40 C.F.R. § 257 monitoring well network consists of seven monitoring wells screened in the uppermost aquifer, including two background monitoring wells (G200 and R201) and five compliance wells (G206, G209, G212, G215, and G218). The boring logs, well construction forms, and other related monitoring well forms for the 40 C.F.R. § 257 monitoring well network are available in the Operating Records as required by 40 C.F.R. § 257.91 for the CCR Unit, and included in **Appendix C** of this HCR. The well locations are shown on **Figure 3-1**.

40 C.F.R. § 257 monitoring well network groundwater samples are collected and analyzed for the laboratory parameters from Appendix III and Appendix IV of 40 C.F.R. § 257 as summarized in **Table C** below.

Table C. 40 C.F.R. § 257 Groundwater Monitoring Program Parameters

Field Parameters ¹			
Groundwater Elevation	pH		
Appendix III Parameters (Total, except TDS)			
Boron	Chloride	Sulfate	
Calcium	TDS	Fluoride	
Appendix IV Parameters (Total)			
Antimony	Beryllium	Cobalt	Molybdenum
Arsenic	Cadmium	Lead	Selenium
Barium	Chromium	Lithium	Thallium
Radium 226 and 228 combined			

¹Dissolved oxygen, temperature, specific conductance, oxidation/reduction potential, and turbidity were recorded during sample collection.

4.1.2 IEPA Groundwater Monitoring

Routine quarterly groundwater monitoring is completed for a monitoring well network that includes wells for both the GMF GSP and GMF RP. The IEPA monitoring well network consists of thirty-one monitoring wells screened in the uppermost aquifer (G102, G103, R104, G105, G106, G200, G205, G206, G207, G208, G209, G210, G211, G212, G213, G214, G215, G216, G217, G218, G270, G271, G272, G273, G274, G275, G276, G277, G279, G280 and R201) in

accordance with IEPA Water Pollution Control Permit No. 2020-EO-65043. The boring logs, well construction forms for the IEPA monitoring well network are included in **Appendix C** of this HCR. The IEPA monitoring well network well locations are shown on **Figure 3-1**. Quarterly and annual samples are analyzed for the following field and laboratory parameters listed in **Table D** below.

Table D. IEPA Groundwater Monitoring Program Parameters

Field Parameters¹			
pH		Elevation of Groundwater Surface	Specific Conductance
Depth to Water (below measuring point, below ground surface)		Elevation of Measuring Point	Temperature
Metals (Dissolved)			
Antimony	Cadmium	Manganese	Thallium
Arsenic	Chromium	Mercury	Vanadium
Aluminum	Cobalt	Molybdenum	Zinc
Barium	Copper	Nickel	
Beryllium	Iron	Selenium	
Boron	Lead	Silver	
Inorganics (Dissolved)			
Chloride	Fluoride	TDS	
Cyanide	Sulfate		
Other (Total)			
Phenols			

Note: Parameters are monitored as dissolved quarterly, and as dissolved and total annually.

¹Dissolved oxygen, oxidation/reduction potential, and turbidity were recorded during sample collection.

4.1.3 Part 845 Well Installation and Groundwater Monitoring

In 2021, one additional monitoring well (G206D), one source sample collection point (NE Riser), and one soil boring (SB289) were installed around the GMF GSP to assess the vertical and horizontal lithology, stratigraphy, chemical properties, and physical properties of geologic layers to a minimum of 100 feet bgs as specified in 35 I.A.C. § 845.620(b). The boring logs, well construction forms, and other related monitoring well forms for the monitoring well network are included in **Appendix C** of this HCR.

Prospective Part 845 monitoring wells were sampled for eight rounds from February to August 2021 and the results were assessed for selection of the GMF GSP Part 845 monitoring well network presented in the GMP. Samples were collected from the new monitoring points and analyzed for 35 I.A.C. § 845.600 parameters summarized in **Table E** below. Part 845 groundwater monitoring results are discussed below in **Section 4.2**.

Table E. Part 845 Groundwater Monitoring Program Parameters

Field Parameters ¹			
pH	Groundwater Elevation	Turbidity	
Metals (Total)			
Antimony	Boron	Cobalt	Molybdenum
Arsenic	Cadmium	Lead	Selenium
Barium	Calcium	Lithium	Thallium
Beryllium	Chromium	Mercury	
Inorganics (Total)			
Fluoride	Sulfate	Chloride	TDS
Other (Total)			
Radium 226 and 228 combined			

¹ Dissolved oxygen, temperature, specific conductance, and oxidation/reduction potential were recorded during sample collection.

4.2 Groundwater Monitoring Results and Analysis

Groundwater data collected from the GMF GSP 40 C.F.R. § 257 network monitoring wells from 2015 to 2021 were supplemented with sampling of additional locations in 2021 and evaluated with respect to standards included in 35 I.A.C. § 845.600(a)(1). This data set was selected because it includes parameters (total metals) consistent with the parameter list in 35 I.A.C. § 845.600(a)(1). Based on this data set there were no concentrations of antimony, barium, boron, cadmium, calcium, chloride, fluoride, lithium, molybdenum, radium 226 and 228 combined, or selenium greater than the GWPSs. Results indicate that the parameters discussed in the following sections were detected at concentrations greater than the applicable 35 I.A.C. § 845.600(a)(1) standards and are considered potential exceedances^[1]. A summary of groundwater analytical data is provided in **Table 4-1**. Field parameters are included in **Table 4-2** and groundwater elevations are provided in **Appendix E**.

4.2.1 Arsenic

Arsenic has been detected intermittently at concentrations greater than the GWPS (0.010 milligrams per liter [mg/L]) at one downgradient uppermost aquifer well (G215). Arsenic was also detected above the GWPS in background uppermost aquifer well G200 during one event in October 2015. Arsenic concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.11 mg/L. Arsenic concentrations in the background uppermost aquifer well noted above ranged from non-detect to 0.038 mg/L.

^[1] Potential exceedances include results reported during the eight rounds of baseline groundwater monitoring that are greater than the applicable 35 I.A.C. § 845.600(a)(1) standards. The results are considered potential exceedances because they were compared directly to the standard and did not include an evaluation of background groundwater quality or apply the statistical methodologies proposed in the Groundwater Monitoring Plan (GMP). For simplicity, "GWPS" will be used hereafter in discussing potential exceedances. Exceedances will be determined following IEPA approval of the GMP.

The DA monitoring well did not have concentrations greater than the arsenic GWPS.

4.2.2 Beryllium

Beryllium was detected at concentrations greater than the GWPS (0.004 mg/L) in downgradient uppermost aquifer well G209 during one event in October 2015. Beryllium was also detected above the GWPS in background uppermost aquifer well R201 during one event in February 2016. Beryllium concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.0042 mg/L. Beryllium concentrations in the background uppermost aquifer well noted above ranged from non-detect to 0.0067 mg/L.

The DA monitoring well did not have concentrations greater than the beryllium GWPS.

4.2.3 Cobalt

Cobalt was detected infrequently at concentrations greater than the GWPS (0.006 mg/L) in three downgradient uppermost aquifer wells (G209, G213, and G217). Cobalt has also been detected intermittently above the GWPS in background uppermost aquifer well G200. Cobalt concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.002) to 0.016 mg/L. Cobalt concentrations in the background uppermost aquifer well noted above ranged from non-detect to 0.053 mg/L.

The DA monitoring well did not have concentrations greater than the cobalt GWPS.

4.2.4 Lead

Lead was detected infrequently at concentrations greater than the GWPS (0.0075 mg/L) in two downgradient uppermost aquifer wells (G209 and G213). Lead has been detected in upgradient uppermost aquifer well G102. Lead has also been detected intermittently above the GWPS in background uppermost aquifer well G200. Lead concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.029 mg/L. Lead concentrations in the upgradient uppermost aquifer well noted above ranged from non-detect to 0.0097 mg/L. Lead concentrations in the background uppermost aquifer well noted above ranged from non-detect (at a reporting limit of 0.0005) to 0.082 mg/L.

The DA monitoring well did not have concentrations greater than the lead GWPS.

4.2.5 pH

During one event in June 2016, pH was detected at measurements less than the lower GWPS (6.5 Standard Units [SU]) at downgradient uppermost aquifer well G206. Measurements of pH have ranged from 6.2 to 7.5 SU at this well.

None of the uppermost aquifer wells had measurements greater than the upper pH GWPS.

The DA monitoring well did not have measurements less than the lower pH GWPS or higher than the upper pH GWPS.

4.2.6 Sulfate

Sulfate is a primary indicator parameter for CCR leachate impacts on groundwater quality. Sulfate has been detected infrequently at concentrations greater than the GWPS (400 mg/L) at

downgradient uppermost aquifer well G215. Sulfate concentrations at G215 ranged from 56 to 490 mg/L, with a median concentration of 120 mg/L.

Sulfate has been detected occasionally at concentrations greater than the GWPS at downgradient DA well G206D. Sulfate concentrations at G206D ranged from 250 to 600 mg/L, with a median concentration of 510 mg/L.

4.2.7 Thallium

Thallium was detected above the GWPS (0.002 mg/L) in downgradient uppermost aquifer well G209 during one event in October 2015. Thallium concentrations in the downgradient uppermost aquifer wells listed above ranged from non-detect (at a reporting limit of 0.001) to 0.0035 mg/L.

The DA monitoring well did not have concentrations greater than the thallium GWPS.

4.2.8 Total Dissolved Solids

During 2021 groundwater monitoring events, TDS was detected above the GWPS (1,200 mg/L) in downgradient DA well G206D during two events in March and April. Concentrations at G206D range from 1,200 to 1,300 mg/L, with a median concentration of 1,250 mg/L.

None of the uppermost aquifer wells had concentrations greater than the TDS GWPS.

5. EVALUATION OF POTENTIAL RECEPTORS

5.1 Water Well Survey

A water well survey was conducted for a 1,000-meter radius around the GMF GSP. Based on State of Illinois records obtained from the ISGS Illinois Water and Related Wells (ILWATER) Map¹ there are sixteen Illinois water wells located within 1,000 meters of the GMF GSP. These included eleven monitoring wells, four farm/domestic wells, and one industrial use well. A map of wells in the vicinity of the GMF GSP is presented in **Appendix B**.

5.2 Surface Water

A search was performed utilizing the United States Fish and Wildlife Service (USFWS) Wetlands Mapper² and the USGS National Map³ for surface water bodies within 1,000 meters of the GMF GSP. The predominant surface water body in the region is Coffeen Lake and associated tributaries. Coffeen Lake consists of two lobes which are located approximately 1,300 feet west, 3,700 feet south, and 150 feet east and downgradient from the GMF GSP. A USGS stream gage (USGS 05593900) for the East Fork Shoal River near Coffeen, Illinois is located 6.5 miles north and east (upstream) of CPP.

Additional surface waters indicated in the USFWS Wetland Mapper and USGS National Map include several man-made freshwater ponds ranging from a roughly 0.6 acre freshwater forested/shrub wetland located northwest of the GMF GSP, several man-made freshwater ponds ranging from 0.1 to 0.8 acres, and one emergent wetland approximately 1.6 acres in size to the southeast of the GMF GSP. A map of wetlands and surface waters in the vicinity of the GMF GSP is presented in **Appendix B**.

The USGS National Map places CPP within the Shoal Creek watershed (HUC 07140203), which is part of the Middle Kaskaskia River Watershed (USGS, 2021b).

A Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map for Montgomery County, Illinois (Map No. 170992 0009 A, effective: January 9, 1981) is available in **Appendix G**. The GMF GSP does not occur within the special flood hazard zones identified on the 1981 FEMA map. The flood hazard areas shown on the map are defined as those areas subject to inundation by the 1 percent annual chance flood (*i.e.*, 100-year flood), also known as the base flood, that has a 1 percent chance of being equaled or exceeded in any given year.

5.3 Nature Preserves, Historic Sites, Endangered/Threatened Species

A comprehensive search of the IDNR Natural Heritage Database⁴ for natural areas and protected areas within 1,000 meters of the GMF GSP was performed. No natural or protected areas within the IDNR database were identified within 1,000 meters of the GMF GSP. A list of sites identified at the county level is found in **Appendix B**.

¹ ISGS ILWATER Map:

<https://prairieresearch.maps.arcgis.com/apps/webappviewer/index.html?id=e06b64ae0c814ef3a4e43a191cb57f87>

² USFWS Wetlands Mapper: <https://www.fws.gov/wetlands/data/mapper.html>

³ USGS National Map: <https://apps.nationalmap.gov/viewer/>

⁴ IDNR Natural Heritage Database:

<https://www2.illinois.gov/dnr/conservation/NaturalHeritage/Pages/NaturalHeritageDatabase.aspx>

The IDNR Natural Heritage Database Threatened and Endangered Species by County⁵ lists eleven threatened and endangered species as located within Montgomery County, including six endangered and five threatened species. Habitats for endangered or threatened species are identified at the county level only.

Additionally, a search of the IDNR Historic Preservation Division⁶ database for historic sites in the vicinity of the Site yielded six results at the county level located within Montgomery County. Four of these sites were identified from the Illinois Natural Areas Inventory and two were identified from the Illinois Nature Preserves list. None of these sites fall within 1,000 meters of the GMF GSP. The Illinois State Archaeological Survey (ISAS)⁷ databases that do not require credentials to access were also searched and yielded no results within 1,000 meters of the GMF GSP.

⁵ Illinois Threatened and Endangered Species by County:
https://www2.illinois.gov/dnr/ESPB/Documents/ET_by_County.pdf

⁶ IDNR Historic Preservation Division: <https://www2.illinois.gov/dnrhistoric/Pages/default.aspx>

⁷ ISAS: <https://www.isas.illinois.edu/>

6. CONCLUSIONS

Based on extensive site investigation and monitoring, the GMF GSP has been characterized and a detailed site conceptual model has been developed. Results of these hydrogeologic studies were summarized and updated to include geologic, hydrogeologic, and groundwater quality data collected with a focus on the GMF GSP (Part 845 regulated CCR Unit and subject of this HCR).

Results of these hydrogeologic studies were reintroduced in this HCR and updated to include geologic, hydrogeologic, and groundwater quality data collected with a focus on the GMF (Part 845 regulated CCR Unit and subject of this HCR). The data were summarized and evaluated for changes in groundwater conditions since the previous investigations; available groundwater quality data for the GMF GSP collected from 2015 to present was compared to the Part 845 GWPS.

The results of the hydrogeologic and groundwater quality evaluation are:

- There are eight principal unlithified units above the bedrock in the vicinity of the GMF GSP, these include the following in descending order:
 - **CCR:** CCR consisting primarily of gypsum scrubber waste is present within the GMF GSP and non-CCR fill material consisting of silt, clay, and sand comprises the berms surrounding the GMF GSP.
 - **Loess Unit:** Clays and silts, including undifferentiated Roxana Silt and Peoria Silt with thicknesses ranging from 1 to 16 feet, where present. Construction of the GMF GSP required the excavation and removal of this layer within the unit footprint.
 - **Hagarstown Member:** The Hagarstown Member (consisting of gravelly clay till and sandy materials in contact with the Vandalia Member (also referred to as Hagarstown Beds) which has been subdivided into two units: the first unit consists of the gravelly clay till and the second unit consists of sandy material overlying the Vandalia Member. The Upper Hagarstown Member is up to 6 feet thick, while the sandy portions, where present, are generally less than 3 feet thick. During construction of the GMF GSP, the Hagarstown Member was excavated to facilitate construction and eliminate groundwater flow into excavations.
 - **Vandalia Member:** Sandy, silt till, or clay till that is generally greater than 15 feet thick.
 - **Mulberry Grove Member:** Gray silt and sandy silt/clay unit found between the Vandalia Member and the Smithboro Member. Generally thin and not laterally continuous across the Site.
 - **Smithboro Member:** Thick, gray compacted silty clay diamicton.
 - **Yarmouth Soil:** Sand and sandy silt/clay, which include accretionary deposits of fine sediment and organic materials, typically less than 5 feet thick and not laterally continuous.
 - **Lierle Clay Member:** Clay and silt with some sand which is the upper portion of the Banner Formation. No borings advanced on site penetrated the full thickness of the Lierle Clay.
 - **Bedrock:** Was not encountered in any deep borings advanced at the GMF GSP.

- Unlithified materials, described above, in the vicinity of the CPP were categorized into hydrostratigraphic units for this HCR. In addition to the CCR Unit, the hydrostratigraphic units occur in the following order (from surface downward) and include:
 - **UCU:** Composed of the Loess Unit and clayey portions of the Hagarstown Formation which are classified as silts to clayey silts and gravelly clay below the surficial soil. The UCU has been eroded east of the GMF GSP, near the Unnamed Tributary.
 - **Uppermost Aquifer:** The uppermost aquifer is the Hagarstown Member which is classified as primarily sandy to gravelly silts and clays with thin beds of sands. Similar to the Loess Unit, the Hagarstown is also absent in some locations near the Unnamed Tributary.
 - **LCU:** Comprised of the Vandalia Member, Mulberry Grove Member, and Smithboro Member. These units include a sandy to silty till with thin, discontinuous sand lenses, a discontinuous and limited extent sandy silt which has infilled prior erosional features, and silty to clayey diamicton, respectively.
 - **DA:** Sand and sandy silt/clay units of the Yarmouth Soil, which include accretionary deposits of fine sediment and organic materials, typically less than 5 feet thick and discontinuous across the Site.
 - **DCU:** Comprised of the Banner Formation, generally consists of clays, silts, and sands. The Lierle Clay Member is the upper layer of the Banner Formation which was encountered at the Site.
- The elevations of water within the GMF GSP are greater than the surrounding areas; however, the GMF GSP is lined and water elevation within the GMF GSP does not vary coincidentally with surrounding groundwater elevations.
- Groundwater flow within the uppermost aquifer is divided towards the two lobes of Coffeen Lake. The groundwater divide runs approximately through the center of the CPP property, with groundwater east of the divide flowing east to southeast towards the Unnamed Tributary or the eastern lobe and groundwater west of the divide flowing west to southwest towards the western lobe. Groundwater flows east to southeast across the GMF GSP.
- Vertical gradients measured near the site indicate downward flow from the uppermost aquifer to the LCU and DA. The DA has been identified as a PMP due to the presence of downward gradients and the relatively higher hydraulic conductivities measured in the DA.
- As determined by the detailed geologic information provided for the GMF GSP, and the hydrogeologic and groundwater quality data, groundwater within the uppermost aquifer at the GMF GSP is classified as Class I – Potable Resource Groundwater.
- Potential exceedances of 35 I.A.C. § 845.600 GWPSs were detected in monitoring wells downgradient of the GMF GSP in the various hydrostratigraphic units as follows:
 - Arsenic in downgradient uppermost aquifer well G215. Arsenic was also detected in background uppermost aquifer well G200.
 - Beryllium in downgradient uppermost aquifer well G209. Beryllium was also detected in background uppermost aquifer well R201.
 - Cobalt in downgradient uppermost aquifer wells G209, G213, and G217. Cobalt was also detected in background uppermost aquifer well G200.

- Lead in downgradient uppermost aquifer wells G209 and G213. Lead was also detected in upgradient uppermost aquifer well G102 and in background uppermost aquifer well G200.
- pH (lower limit) in downgradient uppermost aquifer well G206.
- Sulfate in downgradient uppermost aquifer well G215 and in downgradient DA well G206D.
- Thallium in downgradient uppermost aquifer well G209.
- TDS in downgradient DA well G206D.

Groundwater results are considered potential exceedances because they were compared directly to the standard and did not include an evaluation of background groundwater quality or apply the statistical methodologies proposed in the GMP.

This HCR satisfies Part 845 content requirements specific to 35 I.A.C. § 845.620(b) (Hydrogeologic Site Characterization) for the GMF GSP at the CPP.

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TABLES

TABLE 2-1. GEOTECHNICAL DATA SUMMARY
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample ID	Field Location ID	Top of Sample (ft bgs)	Bottom of Sample (ft bgs)	Moisture Content (%)	Dry Density (pcf)	Specific Gravity	Calculated Porosity ¹ (%)	Vertical Hydraulic Conductivity (cm/s)	LL	PL	PI	USCS	Gravel (%)	Sand (%)	Fines (%)
Loess Unit															
G206D/Comp 1	G206D	4	16	20.5	105.3	2.60	35.1	--	42	18	24	CL	0	20	80
SB289/Comp 1	SB289	4	16	21.2	103.4	2.56	35.3	--	40	18	22	CL	0	20	80
SB289, ST5	SB289	8	10	20.2	105.9	--	--	1.1E-08	--	--	--	CL/CH	--	--	--
Hagarstown Member															
G206D/Comp 2	G206D	18.8	20	10.1	117.4	2.56	26.5	--	22	15	7	SC	0	57	43
SB289/Comp 2	SB289	18	22	16.6	112.6	2.60	30.6	--	23	14	9	CL	0	46	54
Vandalia Member															
G206D/Comp 3	G206D	20	31.8	8.3	131.4	2.56	17.7	--	21	13	8	CL	0	46	54
SB289/Comp 3	SB289	22	32.9	8.9	132.7	2.64	19.4	--	19	12	7	ML	0	47	53
Smithboro Member															
G206D/Comp 4	G206D	34	52	13.4	120.6	2.62	26.2	--	29	14	15	CL	0	29	72
SB289/Comp 4	SB289	34	52	14.9	116.6	2.61	28.4	--	31	15	16	CL	0	26	74
Yarmouth Soil															
G206D/Comp 5	G206D	54	58	14.9	--	2.75	--	--	18	12	6	SM	0	78	22
Lierle Clay															
SB289/Comp 5	SB289	54	60	25	98.4	2.63	40.0	--	48	18	30	CL	0	21	79
CCR															
Gypsum	NE Riser	grab	grab	25.3	78.0	--	--	8.9E-04	NP	NP	NP	Gypsum	0	22	78

[O:KLT, QC: FPO; U: FPO, QC:KLT 8/9/21; U:KLT 8/13/21, C:EDP 8/30/21]

Notes:

¹ Porosity calculated as relationship of bulk density (ρ_b) to particle density (ρ_d) ($n = 100[1 - (\rho_b/\rho_d)]$)

-- = not analyzed

% = Percent

bgs = below ground surface

cm/s = centimeters per second

ft = foot/feet

GMF = Gypsum Management Facility

LL = Liquid limit

NP = Non Plastic

pcf = pounds per cubic foot

PI = Plasticity Index

PL = Plastic Limit

USCS = Unified Soil Classification System

CH = Fat Clay

CL = Lean Clay

ML = Silt

SC = Clayey Sand

SM = Silty Sand

TABLE 2-2. CCR ANALYTICAL RESULTS
 HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND
 COFFEEN, ILLINOIS

Sample Location	Sample Depth (ft BGS)	Sample Date	Antimony (mg/kg)	Arsenic (mg/kg)	Barium (mg/kg)	Beryllium (mg/kg)	Boron (mg/kg)	Cadmium (mg/kg)	Calcium (mg/kg)	Chloride (mg/kg)	Chromium (mg/kg)	Cobalt (mg/kg)	Fluoride (mg/kg)	Lead (mg/kg)	Lithium (mg/kg)	Mercury (mg/kg)	Molybdenum (mg/kg)	Selenium (mg/kg)	Sulfate (mg/kg)	Thallium (mg/kg)
GSP Gypsum 1	0-0	01/29/2021	<1.5	<0.51	6.6	<0.51	13	<0.51	--	25	<2	<1	13	0.67	<2.6	<0.1	1.2	<0.51	19000	<0.51
GSP Gypsum 2	0-0	03/09/2021	<3	<1	13	<1	<10	<1	130000	260	<4	<2	7.6	<1	<5	--	<1	<1	15000	<1

Notes:
 < = concentration is less than the concentration shown, which corresponds to the reporting limit for the method.
 -- = data not available
 BGS = below ground surface
 CCR = coal combustion residuals
 ft = feet
 mg/kg = milligrams per kilogram

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TABLE 2-3. LEACHATE ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)
NE Riser	03/31/2021	<0.003	0.0021	0.046	<0.001	36	0.002	490	1300	<0.004	0.0048	31.9	<0.001	0.16	<0.0002	0.036	7.2	1.41	0.63	10000	0.0016
NE Riser	04/21/2021	<0.003	0.0023	0.041	<0.001	44	0.0022	530	1500	<0.004	0.0064	31.4	<0.001	0.16	<0.0002	0.04	7.3	0.0394	0.61	10000	0.002
NE Riser	05/05/2021	<0.003	0.0024	0.041	<0.001	45	0.0021	530	1600	<0.004	0.0062	31.6	<0.001	0.17	<0.0002	0.039	7.2	0.137	0.55	12000	0.0025
NE Riser	05/18/2021	<0.003	0.0027	0.041	<0.001	45	0.0022	550	1300	<0.004	0.0062	32.5	<0.001	0.18	<0.0002	0.04	7.2	0.609	0.56	12000	0.0019
NE Riser	06/14/2021	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	0.412	--	--	--
NE Riser	07/27/2021	<0.003	0.0022	0.042	<0.001	41	0.0022	540	1400	<0.004	0.0064	1.66	<0.001	0.18	0.00024	0.041	7.1	0.363	0.52	11000	0.0023

Notes:

Field readings are reported with as many significant figures as provided by analytical laboratory.

-- = data not available

< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method.

mg/L = milligrams per liter

pCi/L = picocuries per liter

SU = standard units

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TABLE 2-4. SOIL ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Geologic Unit	Sample Depth (ft BGS)	Sample Date	Antimony (mg/kg)	Arsenic (mg/kg)	Barium (mg/kg)	Beryllium (mg/kg)	Boron (mg/kg)	Cadmium (mg/kg)	Chloride (mg/kg)	Chromium (mg/kg)	Cobalt (mg/kg)	Fluoride (mg/kg)	Lead (mg/kg)	Lithium (mg/kg)	Mercury (mg/kg)	Molybdenum (mg/kg)	Selenium (mg/kg)	Sulfate (mg/kg)	Thallium (mg/kg)
G206D	Fill/Loess Unit	4-16	01/25/2021	<3.6	9.1	65	<1.2	<12	<1.2	46	10	7.7	4.7	4.9	6.8	<0.24	<1.2	<1.2	300	<1.2
G206D	Hagarstown Member	18.8-20	01/25/2021	<3.6	3.1	110	<1.2	<12	<1.2	14	6.2	3	<3	4.8	<6.1	<0.24	<1.2	<1.2	26	<1.2
G206D	Vandalia Till Member	20-31.8	01/25/2021	<3.3	1.3	33	<1.1	<11	<1.1	<11	9.4	4	2.8	5.9	13	<0.22	<1.1	<1.1	<11	<1.1
G206D	Smithboro Till Member	34-52	01/25/2021	<3.6	3.8	89	<1.2	<12	<1.2	<12	11	5.3	3.6	8	11	<0.24	1.4	<1.2	<12	<1.2
G206D	Yarmouth Soil/Lierle Clay	54-58	01/25/2021	<3.6	1.9	160	<1.2	<12	<1.2	<12	11	6.4	7.6	12	8.4	<0.24	<1.2	<1.2	<12	<1.2
SB289	Loess Unit	4-16	01/26/2021	<2.8	<0.95	30	<0.95	<9.5	<0.95	14	4.9	<1.9	3.9	3.8	<4.7	<0.19	<0.95	<0.95	23	<0.95
SB289	Hagarstown Member	18-22	01/26/2021	<2.8	1.3	29	<0.95	<9.5	<0.95	17	6.7	6.8	0.47	5.4	6.8	<0.19	<0.95	<0.95	61	<0.95
SB289	Vandalia Member	22-32.9	01/27/2021	<3	1.8	12	<1	<10	<1	<10	4.8	2.5	<2.5	2.8	5.9	<0.2	<1	<1	16	<1
SB289	Smithboro Member	34-52	01/27/2021	<3	2.6	34	<1	<10	<1	<10	7.6	3.5	3.4	6.3	8.9	<0.2	<1	<1	<10	<1
SB289	Lierle Clay	54-60	01/27/2021	<3	2.8	110	<1	<10	<1	<10	11	4.7	5.7	13	7.1	<0.2	<1	<1	<10	<1

Notes:
< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method.
BGS = below ground surface
ft = foot or feet
mg/kg = milligrams per kilogram

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TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G045D	LCU	08/17/2016	623.81	623.81	Top of PVC	620.94	31.88	41.52	589.06	579.42	41.92	578.90	9.6	2	39.064349	-89.396281
G046D	LCU	08/19/2017	625.24	625.24	Top of PVC	621.91	41.61	51.26	580.30	570.65	51.65	569.90	9.7	2	39.060305	-89.398524
G101	UA	02/02/2010	--	627.60	Top of Disk	625.27	15.68	20.32	609.59	604.95	20.89	603.40	4.6	2	39.071386	-89.400107
G102	UA	04/28/2006	--	629.04	Top of Disk	626.18	12.02	16.78	614.16	609.40	17.15	609.00	4.8	2	39.071387	-89.398991
G103	UA	02/15/2010	--	633.80	Top of Disk	627.94	15.88	20.67	612.06	607.27	21.09	606.90	4.8	2	39.070412	-89.399107
G104	UA	02/15/2010	--	632.94	Top of Disk	627.96	14.91	19.61	613.05	608.35	20.08	605.80	4.7	2	39.069451	-89.399104
G105	UA	02/16/2010	--	632.08	Top of Disk	626.86	16.11	20.90	610.75	605.96	21.37	604.40	4.8	2	39.068491	-89.3991
G106	UA	02/16/2010	--	631.15	Top of Disk	625.96	14.37	18.96	611.59	607.00	19.44	605.50	4.6	2	39.06753	-89.399097
G107	UA	02/17/2010	630.22	630.22	Top of Disk	628.20	13.87	18.50	614.33	609.70	19.00	607.50	4.6	2	39.067106	-89.399646
G108	UA	02/12/2010	--	630.22	Top of Disk	625.58	16.82	21.50	608.76	604.08	22.00	603.60	4.7	2	39.066984	-89.400035
G109	UA	02/11/2010	--	629.76	Top of Disk	624.79	15.39	19.93	609.40	604.86	20.50	604.30	4.5	2	39.067045	-89.400423
G110	UA	02/11/2010	--	629.65	Top of Disk	624.81	15.05	19.59	609.76	605.22	20.16	604.70	4.5	2	39.067172	-89.400704
G111	UA	02/11/2010	--	629.90	Top of Disk	625.28	14.61	19.15	610.67	606.13	19.72	605.60	4.5	2	39.067292	-89.40097
G119	UA	02/09/2010	--	631.55	Top of Disk	626.57	17.29	21.83	609.28	604.74	22.38	604.20	4.5	2	39.068986	-89.401213
G120	UA	02/08/2010	--	631.87	Top of Disk	627.21	15.10	19.62	612.11	607.59	20.21	605.10	4.5	2	39.069479	-89.401214
G121	UA	02/04/2010	--	632.83	Top of Disk	627.94	16.79	21.47	611.15	606.47	21.95	603.80	4.7	2	39.069781	-89.401216
G122	UA	02/04/2010	--	632.69	Top of Disk	628.05	16.51	21.05	611.54	607.00	21.66	606.20	4.5	2	39.070098	-89.401218
G123	UA	02/04/2010	--	632.96	Top of Disk	628.12	20.94	25.46	607.18	602.66	26.07	602.10	4.5	2	39.070399	-89.401219
G124	UA	02/03/2010	--	633.39	Top of Disk	628.70	15.98	20.51	612.72	608.19	21.06	606.70	4.5	2	39.070715	-89.40122
G125	UA	02/03/2010	--	633.51	Top of Disk	628.85	17.03	21.56	611.82	607.29	22.04	606.80	4.5	2	39.071003	-89.401221
G126	UA	02/10/2010	--	625.39	Top of Disk	622.96	12.89	17.43	610.07	605.53	18.00	605.00	4.5	2	39.067304	-89.401274
G151	UA	12/19/2011	--	625.93	Top of Disk	622.82	15.34	19.84	607.48	602.98	20.46	602.40	4.5	2	39.0672	-89.40159
G152	UA	12/20/2011	--	626.52	Top of Disk	623.06	13.59	18.09	609.47	604.97	18.57	604.50	4.5	2	39.066275	-89.401289

TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G153	UA	12/15/2011	626.35	626.40	Top of Disk	623.23	15.90	20.34	607.33	602.89	20.80	602.50	4.4	2	39.065857	-89.402567
G154	UA	12/16/2011	--	626.35	Top of Disk	623.52	14.26	18.76	609.26	604.76	19.10	603.50	4.5	2	39.067089	-89.403574
G155	UA	12/19/2011	--	625.86	Top of Disk	622.89	15.09	19.58	607.80	603.31	23.23	599.70	4.5	2	39.067493	-89.402659
G200	UA	02/25/2008	--	625.94	Top of Disk	623.27	12.19	16.98	611.08	606.29	17.36	605.30	4.8	2	39.075139	-89.395009
G201	UA	02/25/2008	627.15	627.15	Top of Riser	624.19	13.01	17.80	611.18	606.39	18.15	606.00	4.8	2	39.075141	-89.397829
G205	UA	02/21/2008	--	624.34	Top of Disk	622.10	10.04	14.53	612.06	607.57	15.07	606.10	4.5	2	39.068596	-89.394147
G206	UA	10/14/2010	--	632.82	Top of Disk	630.53	17.51	21.92	613.02	608.61	22.42	606.50	4.4	2	39.067399	-89.398548
G206D	DA	01/25/2021	634.14	634.14	Top of PVC	631.41	49.20	59.00	582.21	572.41	59.39	571.41	9.8	2	39.067428	-89.398493
G207	UA	10/08/2010	--	633.21	Top of Disk	630.61	18.24	22.77	612.37	607.84	23.30	606.60	4.5	2	39.067568	-89.397952
G208	UA	10/07/2010	--	633.16	Top of Disk	630.57	17.53	22.06	613.04	608.51	22.60	606.60	4.5	2	39.067743	-89.397402
G209	UA	10/07/2010	--	632.91	Top of Disk	630.57	17.74	22.28	612.83	608.29	22.81	606.60	4.5	2	39.067923	-89.39685
G210	UA	10/06/2010	--	632.99	Top of Disk	630.48	19.39	23.93	611.09	606.55	24.46	605.50	4.5	2	39.068088	-89.396322
G211	UA	10/11/2010	--	632.64	Top of Disk	630.31	17.34	21.88	612.97	608.43	22.41	606.30	4.5	2	39.068263	-89.395792
G212	UA	10/11/2010	--	632.89	Top of Disk	630.59	16.74	21.29	613.85	609.30	21.81	606.60	4.6	2	39.06843	-89.395318
G213	UA	10/12/2010	--	632.81	Top of Disk	630.34	16.75	21.29	613.59	609.05	21.82	606.30	4.5	2	39.068585	-89.394822
G214	UA	10/14/2010	--	632.85	Top of Disk	630.39	17.75	22.14	612.64	608.25	22.65	606.40	4.4	2	39.068919	-89.393982
G215	UA	10/13/2010	--	633.06	Top of Disk	630.48	19.41	23.80	611.07	606.68	24.31	606.20	4.4	2	39.069309	-89.39394
G216	UA	10/13/2010	--	632.76	Top of Disk	630.28	20.04	24.42	610.24	605.86	24.93	604.30	4.4	2	39.069765	-89.393946
G217	UA	10/12/2010	--	633.10	Top of Disk	630.67	20.49	24.88	610.18	605.79	25.38	604.70	4.4	2	39.07034	-89.393959
G218	UA	10/12/2010	--	633.11	Top of Disk	630.64	20.33	24.77	610.31	605.87	25.27	604.60	4.4	2	39.070876	-89.393956
G270	UA	02/26/2008	--	625.86	Top of Disk	623.73	13.13	17.92	610.60	605.81	18.27	605.50	4.8	2	39.066564	-89.397403
G271	UA	09/10/2009	--	625.57	Top of Disk	622.89	9.96	14.31	612.93	608.58	14.79	606.90	4.4	2	39.065007	-89.395587
G272	UA	09/10/2009	--	623.81	Top of Disk	620.72	9.11	13.98	611.61	606.74	14.32	606.40	4.9	2	39.064989	-89.394785

TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G273	UA	09/10/2009	--	623.02	Top of Disk	620.17	9.08	14.56	611.09	605.61	15.10	604.20	5.5	2	39.064985	-89.393973
G274	UA	09/16/2009	--	624.04	Top of Disk	621.67	12.90	17.67	608.77	604.00	18.06	603.60	4.8	2	39.064991	-89.393198
G275	UA	09/16/2009	--	618.26	Top of Disk	616.14	8.22	12.62	607.92	603.52	13.19	603.00	4.4	2	39.065151	-89.392561
G275D	DA	01/14/2021	620.31	620.31	Top of PVC	617.52	49.76	59.55	567.76	557.97	59.89	517.80	9.8	2	39.065121	-89.392595
G276	UA	09/16/2009	--	632.00	Top of Disk	629.14	22.41	27.22	606.73	601.92	27.65	601.10	4.8	2	39.065534	-89.392617
G277	UA	09/14/2009	--	623.08	Top of Disk	620.79	14.29	18.77	606.50	602.02	19.24	600.80	4.5	2	39.065927	-89.392572
G278	UA	09/11/2009	631.19	631.17	Top of Disk	628.85	18.93	23.70	609.92	605.15	24.06	604.80	4.8	2	39.066737	-89.393161
G279	UA	09/10/2009	--	632.04	Top of Disk	629.19	22.40	26.79	606.79	602.40	27.30	601.20	4.4	2	39.067156	-89.392998
G280	UA	02/26/2008	625.35	625.35	Top of Riser	623.11	12.79	17.63	610.32	605.48	17.98	605.10	4.8	2	39.067216	-89.394992
G281	UA	09/08/2015	--	626.36	Top of Disk	623.82	15.51	20.16	608.31	603.66	20.30	603.50	4.7	2	39.065405	-89.399322
G283	LCU	01/14/2021	610.75	610.75	Top of PVC	608.30	8.39	18.17	599.91	590.13	18.36	589.90	9.8	2	39.064645	-89.392119
G284	UA	02/03/2021	618.42	618.42	Top of PVC	615.33	8.08	12.85	607.25	602.48	13.23	601.30	4.8	2	39.065487	-89.390631
G285	LCU	01/25/2021	613.52	613.52	Top of PVC	610.54	13.68	23.45	596.86	587.09	23.83	584.50	9.8	2	39.066513	-89.391474
G286	UA	01/18/2021	613.13	613.13	Top of PVC	609.97	3.37	8.16	606.60	601.81	8.50	600.00	4.8	2	39.067277	-89.391883
G287	UA	01/20/2021	617.45	617.45	Top of PVC	614.34	5.43	10.25	608.91	604.09	10.59	602.50	4.8	2	39.068297	-89.392388
G288	UA	01/19/2021	620.07	620.07	Top of PVC	617.08	7.59	12.26	609.49	604.82	12.75	603.10	4.7	2	39.067834	-89.390082
G301	UA	09/04/2015	--	622.65	Top of Disk	620.88	11.31	15.96	608.96	604.31	16.21	604.10	4.7	2	39.05951	-89.395415
G302	UA	09/04/2015	--	620.04	Top of Disk	618.52	13.21	17.86	604.74	600.09	18.39	599.60	4.7	2	39.059544	-89.393192
G303	UA	08/26/2010	--	622.02	Top of Disk	619.33	10.00	20.00	609.07	599.07	20.40	598.70	10	2	39.057144	-89.391721
G304	UA	08/26/2010	--	626.72	Top of Disk	623.32	10.00	20.00	613.32	603.32	20.40	602.90	10	2	39.057205	-89.395663
G305	UA	05/03/2016	625.67	625.67	Top of PVC	623.23	13.44	18.27	609.10	604.27	18.50	604.10	4.8	2	39.056558	-89.396798
G306	UA	05/03/2016	625.91	625.91	Top of PVC	623.57	13.07	17.68	609.77	605.16	17.90	604.80	4.6	2	39.056494	-89.393556
G307	UA	07/27/2016	624.60	624.60	Top of PVC	624.73	12.96	17.80	609.12	604.28	18.22	603.90	4.8	2	39.057214	-89.395545

TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G307D	LCU	01/19/2021	624.88	624.88	Top of PVC	622.51	48.98	58.75	573.53	563.76	59.60	562.50	9.8	2	39.05721	-89.39552
G308	UA	01/18/2021	624.59	624.59	Top of PVC	621.59	10.10	14.89	611.49	606.70	15.24	605.80	4.8	2	39.057379	-89.397134
G309	UA	01/21/2021	625.88	625.88	Top of PVC	622.77	12.97	17.75	609.80	605.02	18.10	604.70	4.8	2	39.058508	-89.397243
G310	UA	02/09/2021	622.87	622.87	Top of PVC	619.89	10.24	15.03	609.65	604.86	15.38	604.00	4.8	2	39.059532	-89.396907
G311	UA	01/13/2021	621.04	621.04	Top of PVC	618.32	9.27	14.04	609.05	604.28	14.40	603.90	4.8	2	39.059513	-89.394363
G311D	LCU	01/12/2021	621.24	621.24	Top of PVC	618.39	50.16	60.10	568.23	558.29	60.58	557.80	9.9	2	39.059513	-89.394312
G312	UA	01/15/2021	619.78	619.78	Top of PVC	616.92	9.79	14.58	607.13	602.34	14.93	601.70	4.8	2	39.059558	-89.391983
G313	UA	02/05/2021	614.30	614.30	Top of PVC	611.51	6.30	11.11	605.21	600.40	11.46	599.50	4.8	2	39.058773	-89.391124
G314	LCU	02/05/2021	613.88	613.88	Top of PVC	611.11	14.56	19.58	596.55	591.53	20.02	591.10	5	2	39.05782	-89.390964
G314D	DA	02/04/2021	613.70	613.70	Top of PVC	610.87	39.34	49.11	571.53	561.76	49.47	510.60	9.8	2	39.057852	-89.390958
G315	UA	01/14/2021	623.52	623.52	Top of PVC	620.94	9.69	14.48	611.25	606.46	14.85	605.00	4.8	2	39.057165	-89.393667
G316	LCU	02/26/2021	602.59	602.59	Top of PVC	599.64	10.02	14.82	589.62	584.82	15.16	583.90	4.8	2	39.057847	-89.389698
G317	UA	02/12/2021	641.93	641.93	Top of PVC	638.85	30.14	34.93	608.71	603.92	35.28	602.90	4.8	2	39.056727	-89.390148
G401	UA	09/14/2015	--	625.57	Top of Disk	623.03	14.36	18.79	608.67	604.24	19.29	603.70	4.4	2	39.060259	-89.395295
G402	UA	08/27/2010	--	613.37	Top of Disk	610.36	10.00	20.00	600.36	590.36	20.40	590.00	10	2	39.060207	-89.391712
G403	UA	09/11/2015	--	626.47	Top of Disk	623.81	13.11	17.78	610.70	606.03	18.15	605.70	4.7	2	39.063167	-89.398779
G404	UA	05/01/2007	--	615.67	Top of Disk	613.57	6.42	11.17	607.15	602.40	11.62	601.60	4.8	2	39.064329	-89.392493
G405	UA	05/01/2007	--	623.63	Top of Disk	621.40	9.01	13.76	612.39	607.64	14.21	607.20	4.8	2	39.064345	-89.396234
G406	UA	08/19/2016	625.36	625.36	Top of PVC	621.86	13.56	18.37	608.30	603.49	18.75	603.10	4.8	2	39.060309	-89.398508
G407	UA	08/16/2016	621.32	621.32	Top of PVC	618.35	13.78	18.61	604.57	599.74	19.04	598.40	4.8	2	39.061574	-89.402004
G410	UA	02/23/2018	--	619.79	Top of Disk	617.21	8.89	13.68	608.32	603.53	14.09	603.10	4.8	2	39.061572	-89.403763
G411	UA	02/22/2018	--	623.25	Top of Disk	620.49	11.21	16.07	609.28	604.42	16.47	604.00	4.9	2	39.063979	-89.404033
MW01D	DA	05/03/2006	609.02	609.02	Top of PVC	607.08	33.29	38.05	573.79	569.03	38.41	567.10	4.8	2	39.067068	-89.402747

TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
MW02S	UA	05/05/2006	627.12	627.12	Top of PVC	624.16	10.34	15.12	613.82	609.04	15.51	608.70	4.8	2	39.071017	-89.403648
MW02D	LCU	05/05/2006	626.99	626.99	Top of PVC	624.14	22.03	26.83	602.11	597.31	27.22	596.90	4.8	2	39.071031	-89.403649
MW03D	DA	04/27/2006	629.01	629.01	Top of PVC	625.86	52.29	57.06	573.57	568.80	57.40	567.90	4.8	2	39.071386	-89.398976
MW04S	UA	05/11/2006	625.89	625.89	Top of PVC	622.63	9.83	14.26	612.80	608.37	14.77	607.90	4.4	2	39.075356	-89.399232
MW05S	UA	05/17/2006	625.95	625.95	Top of PVC	622.65	12.66	17.41	609.99	605.24	17.71	604.90	4.8	2	39.075866	-89.403333
MW05D	DA	05/17/2006	625.91	625.91	Top of PVC	622.65	45.57	50.33	577.08	572.32	50.72	568.70	4.8	2	39.075863	-89.403313
MW06S	UA	05/04/2006	626.15	626.15	Top of PVC	623.37	11.04	15.62	612.33	607.75	16.08	607.30	4.6	2	39.078189	-89.403644
MW07S	UA	05/09/2006	627.60	627.60	Top of PVC	624.90	9.91	13.79	614.99	611.11	14.39	610.50	3.9	2	39.0786	-89.399383
MW08S	UA	05/10/2006	628.01	628.01	Top of PVC	625.09	11.51	16.00	613.58	609.09	16.60	608.00	4.5	2	39.080234	-89.399079
MW09S	UA	05/03/2006	627.62	627.62	Top of PVC	624.70	11.21	15.62	613.49	609.08	16.20	608.50	4.4	2	39.079954	-89.394899
MW09D	LCU	05/03/2006	627.61	627.61	Top of PVC	624.68	45.81	50.57	578.87	574.11	51.00	570.70	4.8	2	39.07994	-89.394899
MW10S	UA	05/02/2006	624.45	624.45	Top of PVC	621.43	11.28	15.76	610.15	605.67	16.30	605.10	4.5	2	39.07601	-89.394068
MW10D	LCU	05/01/2006	624.47	624.47	Top of PVC	621.33	41.74	46.57	579.59	574.76	47.02	572.60	4.8	2	39.075995	-89.39407
MW11S	UA	04/28/2006	625.27	625.27	Top of PVC	622.04	8.89	13.63	613.15	608.41	14.08	608.00	4.7	2	39.071888	-89.393913
MW11D	LCU	04/28/2006	625.52	625.52	Top of PVC	622.19	28.31	33.04	593.88	589.15	33.50	585.90	4.7	2	39.071888	-89.393894
MW12S	UA	05/10/2006	625.31	625.31	Top of PVC	622.24	10.61	15.18	611.63	607.06	15.61	606.60	4.6	2	39.068514	-89.394199
MW12D	DA	05/10/2006	625.21	625.21	Top of PVC	622.24	42.46	46.99	579.78	575.25	47.47	572.20	4.5	2	39.068501	-89.394199
MW13S	UA	05/09/2006	625.96	625.96	Top of PVC	622.80	11.43	16.23	611.37	606.57	16.62	606.20	4.8	2	39.066297	-89.40118
MW13D	DA	05/09/2006	625.86	625.86	Top of PVC	622.85	49.81	54.60	573.04	568.25	55.00	567.90	4.8	2	39.066293	-89.401163
MW14S	UA	05/02/2006	626.88	626.88	Top of PVC	624.62	12.26	17.02	612.36	607.60	17.38	607.20	4.8	2	39.069153	-89.400442
MW15S	UA	04/25/2006	626.66	626.66	Top of PVC	623.83	14.41	19.16	609.42	604.67	19.62	604.20	4.8	2	39.069772	-89.397088
MW15D	LCU	04/25/2006	626.44	626.44	Top of PVC	623.83	33.68	38.45	590.15	585.38	38.80	585.00	4.8	2	39.06977	-89.397073
MW16S	UA	04/25/2006	629.47	629.47	Top of PVC	626.32	14.59	19.41	611.73	606.91	19.76	606.40	4.8	2	39.073571	-89.397006

TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
MW16D	DA	04/25/2006	629.38	629.38	Top of PVC	626.37	45.90	50.34	580.47	576.03	50.78	575.40	4.4	2	39.073571	-89.397036
MW17S	UA	05/04/2006	630.56	630.56	Top of PVC	627.28	14.02	23.56	613.26	603.72	24.11	603.20	9.5	2	39.07715	-89.396978
MW17D	DA	05/04/2006	630.29	630.29	Top of PVC	627.47	48.82	53.32	578.65	574.15	53.87	573.60	4.5	2	39.077151	-89.396958
MW18S	UA	05/11/2006	628.66	628.66	Top of PVC	625.69	11.31	15.79	614.38	609.90	16.40	609.30	4.5	2	39.077033	-89.401698
MW20S	UA	05/01/2007	622.90	622.90	Top of PVC	620.26	8.41	13.22	611.85	607.04	13.67	604.30	4.8	2	39.064968	-89.394322
R104	UA	10/08/2010	--	632.84	Top of Disk	629.03	14.59	19.32	614.44	609.71	19.85	609.20	4.7	2	39.069474	-89.399109
R201	UA	10/08/2010	--	626.34	Top of Disk	624.02	14.59	19.32	609.43	604.70	19.85	604.20	4.7	2	39.075142	-89.397855
R205	UA	03/20/2017	--	624.52	Top of Disk	621.91	11.32	16.01	610.59	605.90	16.42	605.50	4.7	2	39.068593	-89.394164
T127	UA	02/10/2010	--	630.96	Top of Disk	625.53	17.53	22.07	608.00	603.46	22.64	602.90	4.5	2	39.068119	-89.40121
T128	UA	02/09/2010	631.03	630.93	Top of Disk	626.27	16.53	21.04	609.74	605.23	21.64	602.20	4.5	2	39.068532	-89.401211
T202	UA	10/15/2010	--	628.63	Top of Disk	626.22	12.27	16.65	613.95	609.57	17.21	608.20	4.4	2	39.071776	-89.397705
T408	LCU	08/17/2016	624.08	624.08	Top of PVC	621.09	20.66	25.49	600.43	595.60	25.92	595.20	4.8	2	39.064353	-89.396307
T409	LCU	08/19/2016	625.01	625.01	Top of PVC	621.85	21.79	26.59	600.06	595.26	26.99	594.90	4.8	2	39.0603	-89.398538
TA31	UA	10/28/2014	626.55	626.55	Top of PVC	623.89	15.09	19.57	608.80	604.32	20.19	603.70	4.5	2	39.071368	-89.401366
TA32	UA	10/27/2014	621.42	621.42	Top of PVC	618.93	11.31	15.68	607.62	603.25	16.47	602.50	4.4	2	39.074093	-89.402223
TA33	UA	06/02/2015	625.27	625.27	Top of PVC	622.51	12.23	16.89	610.28	605.62	17.44	605.10	4.7	2	39.071556	-89.403506
TA34	UA	06/03/2015	626.52	626.52	Top of PVC	624.10	10.92	15.41	613.18	608.69	16.10	608.00	4.5	2	39.069631	-89.402759
TR32	UA	07/02/2021	621.68	621.68	Top of PVC	619.28	11.00	15.68	608.28	603.60	16.17	603.11	4.68	2	39.074064	-89.397758
NE Riser	S	--	--	626.13	--	--	--	--	--	--	--	--	--	--	39.071111	-89.393889
SG-02	SW	--	--	605.87	Top of Prot Casing	605.87	--	--	--	--	--	--	--	--	39.059695	-89.391429
SG-03	SW	--	--	594.94	Top of Prot Casing	594.94	--	--	--	--	--	--	--	--	39.059092	-89.390342
SG-04	SW	--	--	599.52	Top of Prot Casing	599.52	--	--	--	--	--	--	--	--	39.064146	-89.390504

TABLE 3-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND
 COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
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Notes:

All elevation data are presented relative to the North American Vertical Datum 1988 (NAVD88), GEOID 12A

-- = data not available

BGS = below ground surface

DA = deep aquifer

ft = foot or feet

HSU = Hydrostratigraphic Unit

LCU = lower confining unit

PVC = polyvinyl chloride

S = source water

SW = surface water

UA = uppermost aquifer

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TABLE 3-2. VERTICAL HYDRAULIC GRADIENTS
 HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
 COFFEEN POWER PLANT
 GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND
 COFFEEN, ILLINOIS

Date	G206 Groundwater Elevation (ft NAVD88)	G206D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
	UA	DA (PMP)				
3/29/2021	--	583.94	--	--	--	--
4/20/2021	622.07	585.96	36.11	33.51	1.08	down
5/3/2021	622.60	587.42	35.18	33.51	1.05	down
5/17/2021	622.31	587.81	34.50	33.51	1.03	down
6/9/2021	621.71	584.19	37.52	33.51	1.12	down
6/23/2021	620.54	589.66	30.88	33.51	0.92	down
7/12/2021	622.39	590.72	31.67	33.51	0.95	down
7/26/2021	622.00	591.14	30.86	33.51	0.92	down
Middle of screen elevation G206					610.8	
Middle of screen elevation G206D					577.3	

[O: KLT 6/4/21, C:YMD 6/7/21][U:KLT 8/25/21, C:EDP 8/31/21]

Notes:

¹ Distance change was calculated using the midpoint of the piezometer screen and water table surface. If the water table surface was above the top of the monitoring well screen, then distance change was calculated using the midpoint of both screens.

² Vertical gradients between ± 0.0015 are considered flat, and typically have less than 0.02 foot difference in groundwater elevation between wells.

-- = no data collected on date / no vertical gradient calculated

DA = deep aquifer

dh = head change

dl = distance change

ft = foot/feet

NAVD88 = North American Vertical Datum of 1988

PMP = potential migration pathway

UA = uppermost aquifer

TABLE 3-3. FIELD HYDRAULIC CONDUCTIVITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Well ID	Gradient Position	Bottom of Screen Elevation (ft NAVD88)	Screen Length ¹ (ft)	Field Identified Screened Material	Slug Type	Analysis Method	Falling Head (Slug In) Hydraulic Conductivity (cm/s)	Rising Head (Slug Out) Hydraulic Conductivity (cm/s)	Minimum Hydraulic Conductivity (cm/s)	Maximum Hydraulic Conductivity (cm/s)	Hydraulic Conductivity Geometric Mean (cm/s)
Uppermost Aquifer											
G206	D	608.61	4.41	SM, s(CL), CL	solid	Kansas Geological Survey	5.0E-04	4.9E-04	2.5E-04	4.0E-03	1.4E-03
G209	D	608.29	4.54	CL	solid	Kansas Geological Survey	- -	2.5E-04			
G212	D	609.30	4.55	SM, s(CL), CL	solid	Kansas Geological Survey	2.1E-03	1.8E-03			
G215	D	606.68	4.39	SM, s(CL), ML	solid	Kansas Geological Survey	4.0E-03	3.5E-03			
G218	D	605.87	4.44	SM, SC, CL	solid	Kansas Geological Survey	2.6E-03	2.4E-03			

[O: KLT, C:EDP 8/31/21]

Notes:

- 1. All wells are constructed from 2 inch PVC with 0.01 inch slotted screens.
- - = Test not analyzed/performed
- cm/s = centimeters per second
- D = downgradient
- ft = foot/feet
- NAVD88 = North American Vertical Datum of 1988
- PVC = polyvinyl chloride

USCS = Unified Soil Classification System

- CL = Lean Clay
- s(CL) = Sandy Lean Clay
- ML = Silt
- SC = Clayey Sand
- SM = Silty Sand

TABLE 3-4. HORIZONTAL HYDRAULIC GRADIENTS AND GROUNDWATER FLOW VELOCITIES
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND
COFFEEN, ILLINOIS

Notes:

- ¹ Hydraulic conductivity values used above are average of the individual wells used in each velocity calculation as derived from slug tests completed in February and March 2021 by Ramboll.
- ² Effective porosity used in these calculations was derived from an average between estimated values of 0.20 for silt materials, 0.267 for gravel, 0.07 for clay, and 0.28 for sand from *Morris, D.A. and A.I. Johnson, 1967. Summary of hydrologic and physical properties of rock and soil materials as analyzed by the Hydrologic Laboratory of the U.S. Geological Survey, U.S. Geological Survey Water-Supply Paper 1839-D, 42p. and Heath, R.C., 1983. Basic ground-water hydrology, U.S. Geological Survey Water-Supply Paper 2220, 86p.* Effective porosity may be as high as maximum total porosity (50%) calculated in Table 2-1.
- ³ The uppermost aquifer was excavated within the footprint of the GMF GSP during construction. Flow calculated between R104 and G215 may not be representative of actual groundwater flow conditions around the GMF GSP.
- = not calculated
% = percent
ft = foot/feet
ft/day = feet per day
ft/ft = feet per foot
NAVD88 = North American Vertical Datum of 1988
NM = Not Measured

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G102	01/20/2015	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.6	--	--	--	--	590
G102	04/08/2015	<0.003	<0.001	0.047	<0.001	<0.01	<0.001	62	28	0.016	<0.002	0.4	<0.001	--	0.00028	--	7.1	--	0.0028	86	<0.001	390
G102	07/23/2015	<0.003	0.0014	0.059	<0.001	<0.01	<0.001	--	30	0.0076	<0.002	0.429	0.0017	--	<0.0002	0.0013	7.1	--	0.0027	95	<0.001	420
G102	10/06/2015	<0.003	<0.001	0.058	<0.001	<0.01	<0.001	--	73	0.015	<0.002	0.272	<0.001	--	<0.0002	0.0016	7.1	--	0.0038	110	<0.001	590
G102	11/16/2015	<0.003	<0.001	0.069	<0.001	<0.01	<0.001	78	88	0.015	<0.002	0.291	<0.001	<0.01	<0.0002	<0.001	6.9	1.11	0.0025	140	<0.001	600
G102	02/17/2016	<0.003	<0.001	0.038	<0.001	0.014	<0.001	59	19	<0.004	<0.002	0.328	<0.001	<0.01	<0.0002	<0.001	7.5	1.76	0.0012	64	<0.001	400
G102	05/16/2016	<0.003	<0.001	0.029	<0.001	0.01	<0.001	42	3.8	<0.004	<0.002	0.525	<0.001	<0.01	<0.0002	0.0012	7.2	0.0616	0.0012	53	<0.001	310
G102	08/02/2016	<0.003	<0.001	0.044	<0.001	0.014	<0.001	69	42	<0.004	<0.002	0.383	<0.001	<0.01	<0.0002	<0.001	7.2	2.03	0.0032	86	<0.001	400
G102	11/19/2016	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	77	40	<0.004	<0.002	0.389	<0.001	<0.01	<0.0002	<0.001	7.1	0.86	0.0028	95	<0.001	490
G102	02/09/2017	<0.003	0.0063	0.094	<0.001	0.019	<0.001	62	6.4	0.032	0.0057	0.419	0.0079	0.016	<0.0002	0.0017	7.1	1.16	0.0013	55	<0.001	370
G102	05/22/2017	<0.003	<0.001	0.036	<0.001	<0.01	<0.001	40	4	<0.004	<0.002	0.426	<0.001	<0.01	<0.0002	<0.001	7.2	0.773	<0.001	51	<0.001	300
G102	07/09/2017	<0.003	0.0068	0.11	<0.001	0.017	<0.001	83	45	0.029	0.0058	0.27	0.0097	0.016	<0.0002	0.0013	7.2	0.78	0.003	98	<0.001	440
G102	10/25/2017	--	--	--	--	0.014	--	56	54	--	--	0.366	--	--	--	--	7.1	--	--	130	--	540
G102	01/26/2018	--	--	--	--	--	--	--	46	--	--	<0.25	--	--	--	--	7.1	--	--	--	--	--
G102	05/10/2018	--	--	--	--	<0.01	--	56	14	--	--	0.295	--	--	--	--	7.2	--	--	74	--	330
G102	10/24/2018	--	--	--	--	0.018	--	85	40	--	--	0.322	--	--	--	--	7.0	--	--	130	--	510
G102	01/17/2019	--	--	--	--	0.015	--	78	47	--	--	0.323	--	--	--	--	7.2	--	--	130	--	560
G102	08/12/2019	--	--	--	--	0.022	--	93	22	--	--	0.402	--	--	--	--	7.2	--	--	86	--	420
G102	01/21/2020	--	--	--	--	0.015	--	56	3.4	--	--	0.388	--	--	--	--	7.6	--	--	49	--	330
G102	08/11/2020	--	--	--	--	<0.01	--	75	30	--	--	0.265	--	--	--	--	7.3	--	--	120	--	470
G102	01/26/2021	--	--	--	--	0.017	--	55	12	--	--	0.365	--	--	--	--	7.1	--	--	70	--	410
G103	01/20/2015	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.6	--	--	--	--	420

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G103	04/08/2015	<0.003	0.0011	0.079	<0.001	<0.01	<0.001	80	56	0.0075	<0.002	0.4	<0.001	--	<0.0002	--	7.2	--	<0.001	54	<0.001	450
G103	07/23/2015	<0.003	0.0017	0.12	<0.001	<0.01	<0.001	--	55	0.0054	<0.002	0.262	0.0011	--	<0.0002	0.0029	7.2	--	<0.001	76	<0.001	390
G103	10/06/2015	<0.003	0.0012	0.1	<0.001	<0.01	<0.001	--	66	0.019	<0.002	<0.25	<0.001	--	<0.0002	0.0034	7.3	--	<0.001	69	<0.001	470
G105	01/20/2015	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.3	--	--	--	--	620
G105	04/08/2015	<0.003	0.0015	0.081	<0.001	0.098	<0.001	87	35	<0.004	<0.002	0.4	<0.001	--	<0.0002	--	7.0	--	0.0015	130	<0.001	550
G105	07/23/2015	<0.003	<0.001	0.068	<0.001	0.069	<0.001	--	34	<0.004	<0.002	0.336	<0.001	--	<0.0002	0.0023	7.1	--	<0.001	110	<0.001	540
G105	10/06/2015	<0.003	0.0013	0.078	<0.001	0.13	<0.001	--	37	0.0046	<0.002	0.304	0.0011	--	<0.0002	0.0028	7.0	--	0.0011	110	<0.001	560
G106	01/20/2015	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.5	--	--	--	--	440
G106	04/08/2015	<0.003	<0.001	0.057	<0.001	<0.01	<0.001	84	28	0.012	<0.002	0.47	<0.001	--	<0.0002	--	7.1	--	0.0025	56	<0.001	480
G106	07/23/2015	<0.003	<0.001	0.059	<0.001	<0.01	<0.001	--	27	<0.004	<0.002	0.378	<0.001	--	<0.0002	0.0019	7.2	--	0.0017	55	<0.001	460
G106	10/06/2015	<0.003	<0.001	0.06	<0.001	0.016	<0.001	--	25	0.013	<0.002	0.343	<0.001	--	<0.0002	0.0031	7.3	--	<0.001	55	<0.001	420
G106	11/17/2015	<0.003	<0.001	0.06	<0.001	<0.01	<0.001	80	26	<0.004	<0.002	0.394	<0.001	<0.01	<0.0002	0.0018	7.1	0.815	0.002	56	<0.001	380
G106	02/17/2016	<0.003	<0.001	0.054	<0.001	<0.01	<0.001	82	30	<0.004	<0.002	0.371	<0.001	<0.01	<0.0002	0.0021	7.2	0.701	0.0019	56	<0.001	440
G106	05/16/2016	<0.003	<0.001	0.055	<0.001	<0.01	<0.001	84	25	<0.004	<0.002	0.528	<0.001	<0.01	<0.0002	0.0019	7.1	0.244	0.0019	58	<0.001	380
G106	08/04/2016	<0.003	<0.001	0.056	<0.001	<0.01	<0.001	80	27	<0.004	<0.002	0.504	<0.001	<0.01	<0.0002	0.0025	7.0	0.4	0.0019	58	<0.001	420
G106	11/19/2016	<0.003	0.0013	0.067	<0.001	<0.01	<0.001	86	29	<0.004	<0.002	0.465	<0.001	<0.01	<0.0002	<0.001	7.0	0.923	<0.001	43	<0.001	440
G106	02/09/2017	<0.003	0.0011	0.057	<0.001	0.012	<0.001	73	28	<0.004	<0.002	0.462	<0.001	<0.01	<0.0002	0.0024	7.0	1.2	<0.001	63	<0.001	420
G106	05/22/2017	<0.003	0.0015	0.078	<0.001	<0.01	<0.001	68	30	<0.004	0.0038	0.44	<0.001	<0.01	<0.0002	0.0028	7.1	0.533	<0.001	67	<0.001	380
G106	07/09/2017	<0.003	0.0012	0.052	<0.001	<0.01	<0.001	84	29	<0.004	<0.002	0.366	<0.001	<0.01	<0.0002	0.0018	7.0	1.93	<0.001	60	<0.001	380
G106	10/25/2017	--	--	--	--	<0.01	--	72	48	--	--	0.47	--	--	--	--	7.3	--	--	45	--	410
G106	05/10/2018	--	--	--	--	<0.01	--	73	38	--	--	0.516	--	--	--	--	7.0	--	--	88	--	420
G106	08/09/2018	--	--	--	--	--	--	--	--	--	--	0.46	--	--	--	--	7.0	--	--	--	--	--

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G106	10/24/2018	--	--	--	--	0.014	--	85	35	--	--	0.441	--	--	--	--	6.8	--	--	75	--	480
G106	01/17/2019	--	--	--	--	0.058	--	110	35	--	--	0.453	--	--	--	--	6.9	--	--	78	--	480
G106	08/13/2019	--	--	--	--	0.029	--	94	44	--	--	0.468	--	--	--	--	6.9	--	--	36	--	520
G106	01/21/2020	--	--	--	--	0.04	--	100	48	--	--	0.398	--	--	--	--	7.3	--	--	98	--	560
G106	08/11/2020	--	--	--	--	0.016	--	87	39	--	--	0.36	--	--	--	--	7.0	--	--	69	--	490
G106	01/26/2021	--	--	--	--	0.045	--	100	59	--	--	0.399	--	--	--	--	7.5	--	--	140	--	600
G106	06/29/2021	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.2	--	--	--	--	--
G200	01/20/2015	--	0.0022	--	--	<0.01	<0.001	100	80	--	--	--	0.004	--	--	--	7.4	--	--	100	--	570
G200	04/10/2015	<0.003	0.0017	0.1	<0.001	<0.01	<0.001	110	87	<0.004	<0.002	0.303	0.0033	--	<0.0002	--	7.0	--	0.0079	87	<0.001	580
G200	07/22/2015	<0.003	0.0087	0.18	<0.001	<0.01	<0.001	--	96	0.017	0.0088	0.301	0.017	--	<0.0002	<0.001	7.0	--	0.0084	88	<0.001	630
G200	10/05/2015	<0.003	0.038	0.78	0.0038	0.048	0.0012	--	85	0.086	0.053	0.346	0.082	--	<0.0002	0.0039	7.1	--	0.013	90	<0.001	660
G200	11/23/2015	<0.003	0.007	0.17	<0.001	0.39	<0.001	100	75	0.012	0.0068	0.337	0.01	0.019	<0.0002	0.0017	7.2	1.65	0.0041	94	<0.001	520
G200	02/12/2016	<0.003	0.0082	0.24	0.0013	0.014	<0.001	150	93	0.013	0.0074	0.415	0.018	0.021	<0.0002	<0.001	7.2	3.84	0.0097	97	<0.001	540
G200	05/10/2016	<0.003	0.0025	0.13	<0.001	<0.01	<0.001	100	96	0.0041	<0.002	0.389	0.0058	<0.01	<0.0002	<0.001	7.1	0.849	0.0071	100	<0.001	480
G200	07/30/2016	<0.003	<0.001	0.059	<0.001	<0.01	<0.001	88	82	0.0049	<0.002	0.384	0.0012	<0.01	<0.0002	<0.001	7.1	0.662	0.0032	100	<0.001	520
G200	11/18/2016	<0.003	<0.001	0.053	<0.001	0.01	<0.001	88	75	<0.004	<0.002	0.431	<0.001	<0.01	<0.0002	<0.001	7.2	0.29	0.0032	110	<0.001	520
G200	02/10/2017	<0.003	<0.001	0.074	<0.001	<0.01	<0.001	85	82	0.0052	<0.002	0.305	0.0013	<0.01	<0.0002	<0.001	7.1	0.534	0.0067	100	<0.001	700
G200	05/18/2017	<0.003	<0.001	0.063	<0.001	0.01	<0.001	84	96	<0.004	<0.002	0.3	<0.001	<0.01	<0.0002	<0.001	7.0	1.01	0.0062	90	<0.001	620
G200	07/13/2017	<0.003	<0.001	0.057	<0.001	<0.01	<0.001	87	88	<0.004	<0.002	0.299	<0.001	<0.01	<0.0002	<0.001	7.1	0.906	0.0034	110	<0.001	540
G200	10/28/2017	--	--	--	--	0.34	--	81	65	--	--	0.328	--	--	--	--	7.2	--	--	100	--	520
G200	01/25/2018	--	--	--	--	--	--	--	71	--	--	0.303	--	--	--	--	7.2	--	--	--	--	--
G200	05/11/2018	--	--	--	--	<0.01	--	90	85	--	--	<0.25	--	--	--	--	7.0	--	--	100	--	460

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G200	11/02/2018	--	--	--	--	0.011	--	95	61	--	--	0.391	--	--	--	--	7.0	--	--	100	--	480
G200	01/16/2019	--	--	--	--	0.048	--	350	54	--	--	0.386	--	--	--	--	7.1	--	--	110	--	700
G200	08/12/2019	--	--	--	--	<0.01	--	92	58	--	--	0.405	--	--	--	--	7.0	--	--	110	--	540
G200	01/21/2020	--	--	--	--	<0.01	--	110	100	--	--	0.302	--	--	--	--	7.2	--	--	120	--	520
G200	08/11/2020	--	--	--	--	<0.01	--	85	63	--	--	0.427	--	--	--	--	7.2	--	--	110	--	530
G200	01/29/2021	--	--	--	--	0.014	--	81	53	--	--	0.36	--	--	--	--	7.3	--	--	110	--	580
G200	03/29/2021	<0.003	<0.001	0.044	<0.001	0.04	<0.001	82	50	<0.004	<0.002	0.32	<0.001	<0.02	<0.0002	<0.001	7.3	0.345	0.0019	100	<0.001	460
G200	04/21/2021	<0.003	<0.001	0.047	<0.001	0.035	<0.001	88	63	<0.004	<0.002	0.435	<0.001	<0.02	<0.0002	<0.001	7.2	0.0349	0.0022	95	<0.001	570
G200	05/06/2021	<0.003	0.0035	0.08	<0.001	0.012	<0.001	140	73	0.0096	0.0029	<0.25	0.0033	<0.02	<0.0002	0.002	7.1	0.0892	0.0035	100	<0.001	570
G200	05/17/2021	<0.003	<0.001	0.055	<0.001	<0.01	<0.001	91	78	<0.004	<0.002	0.342	<0.001	<0.02	<0.0002	<0.001	7.2	0.182	0.0023	110	<0.001	560
G200	06/14/2021	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	0	--	--	--	--
G200	07/28/2021	<0.003	<0.001	0.044	<0.001	0.11	<0.001	83	51	<0.004	<0.002	0.255	<0.001	<0.02	<0.0002	<0.001	7.0	0.239	0.0028	100	<0.001	500
G206	01/21/2015	--	<0.001	--	--	<0.01	<0.001	84	28	--	--	--	<0.001	--	--	--	7.3	--	--	130	--	480
G206	04/09/2015	<0.003	<0.001	0.052	<0.001	0.026	<0.001	92	29	<0.004	<0.002	0.411	<0.001	--	<0.0002	--	7.0	--	<0.001	130	<0.001	480
G206	07/22/2015	<0.003	<0.001	0.094	<0.001	<0.01	<0.001	--	24	<0.004	<0.002	0.39	<0.001	--	<0.0002	0.0025	7.0	--	0.0068	32	<0.001	380
G206	10/07/2015	<0.003	0.0028	0.065	<0.001	0.042	<0.001	--	34	<0.004	<0.002	0.284	<0.001	--	<0.0002	0.0016	6.8	--	<0.001	110	<0.001	480
G206	11/18/2015	<0.003	0.0039	0.062	<0.001	<0.01	<0.001	79	32	0.0041	<0.002	0.433	<0.001	<0.01	<0.0002	<0.001	7.1	0.317	<0.001	95	<0.001	460
G206	02/24/2016	<0.003	<0.001	0.056	<0.001	0.033	<0.001	78	26	<0.004	<0.002	0.507	<0.001	<0.01	<0.0002	0.0014	6.7	0.292	<0.001	150	<0.001	500
G206	06/27/2016	<0.003	0.0012	0.062	<0.001	<0.01	<0.001	94	25	<0.004	<0.002	0.469	<0.001	<0.01	<0.0002	0.0025	6.2	0.647	<0.001	130	<0.001	420
G206	08/06/2016	<0.003	0.002	0.064	<0.001	<0.01	<0.001	90	27	0.0042	<0.002	0.449	0.0022	<0.01	<0.0002	0.0024	7.1	0.857	<0.001	130	<0.001	420
G206	11/22/2016	<0.003	<0.001	0.048	<0.001	0.11	<0.001	63	30	<0.004	<0.002	0.463	<0.001	<0.01	<0.0002	0.0015	7.1	--	<0.001	130	<0.001	480
G206	12/07/2016	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	1.62	--	--	--	--

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G206	02/11/2017	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	70	29	<0.004	<0.002	0.547	<0.001	<0.01	<0.0002	0.0013	7.2	1.2	<0.001	150	<0.001	680
G206	05/18/2017	<0.003	<0.001	0.043	<0.001	<0.01	<0.001	66	29	<0.004	<0.002	<0.25	<0.001	<0.01	<0.0002	0.0011	7.0	0.555	<0.001	120	<0.001	460
G206	07/15/2017	<0.003	0.0019	0.055	<0.001	<0.01	<0.001	61	31	<0.004	<0.002	0.453	<0.001	<0.01	<0.0002	<0.001	7.1	1.33	<0.001	100	<0.001	480
G206	10/30/2017	--	--	--	--	<0.01	--	90	30	--	--	0.472	--	--	--	--	7.2	--	--	120	--	460
G206	05/15/2018	--	--	--	--	0.032	--	73	26	--	--	0.48	--	--	--	--	7.0	--	--	130	--	450
G206	11/02/2018	--	--	--	--	<0.01	--	85	25	--	--	0.36	--	--	--	--	7.0	--	--	120	--	440
G206	01/17/2019	--	--	--	--	<0.01	--	81	27	--	--	0.458	--	--	--	--	7.1	--	--	110	--	480
G206	08/14/2019	--	--	--	--	0.013	--	120	22	--	--	0.506	--	--	--	--	7.1	--	--	120	--	470
G206	01/21/2020	--	--	--	--	<0.01	--	84	24	--	--	0.389	--	--	--	--	7.5	--	--	120	--	470
G206	05/05/2020	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.5	--	--	--	--	--
G206	08/13/2020	--	--	--	--	0.015	--	81	23	--	--	0.391	--	--	--	--	7.4	--	--	130	--	500
G206	01/27/2021	--	--	--	--	<0.01	--	65	22	--	--	0.426	--	--	--	--	7.1	--	--	130	--	480
G206D	03/30/2021	<0.003	0.0039	0.14	<0.001	0.13	<0.001	140	70	0.0095	0.003	0.666	0.0031	<0.02	<0.0002	0.033	7.1	0.451	<0.001	590	<0.001	1300
G206D	04/22/2021	<0.003	0.0032	0.089	<0.001	0.12	<0.001	120	67	<0.004	<0.002	0.774	0.0025	<0.02	<0.0002	0.029	7.2	0.14	<0.001	600	<0.001	1300
G206D	05/05/2021	<0.003	0.0039	0.12	<0.001	0.11	<0.001	130	56	<0.004	<0.002	0.766	<0.001	<0.02	<0.0002	0.03	7.1	0.147	<0.001	510	<0.001	1200
G206D	05/18/2021	<0.003	0.0052	0.12	<0.001	0.13	<0.001	130	52	<0.004	<0.002	0.499	<0.001	<0.02	<0.0002	0.029	7.2	0.247	<0.001	250	<0.001	1200
G206D	06/14/2021	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	0.142	--	--	--	--
G206D	07/27/2021	<0.003	0.0024	0.079	<0.001	0.12	<0.001	110	42	<0.004	<0.002	0.99	<0.001	<0.02	<0.0002	0.027	7.5	0.952	<0.001	400	<0.001	1100
G207	01/21/2015	--	<0.001	--	--	<0.01	<0.001	61	56	--	--	--	<0.001	--	--	--	7.7	--	--	72	--	360
G207	04/09/2015	<0.003	0.0017	0.11	<0.001	0.03	<0.001	65	55	0.0072	<0.002	0.466	0.0019	--	<0.0002	--	7.1	--	0.0012	69	<0.001	370
G207	07/22/2015	<0.003	0.0026	0.11	<0.001	<0.01	<0.001	--	62	<0.004	<0.002	0.512	0.0032	--	<0.0002	0.0016	7.2	--	<0.001	16	<0.001	440
G207	10/07/2015	<0.003	0.004	0.12	<0.001	0.021	<0.001	--	60	<0.004	<0.002	0.444	0.0011	--	<0.0002	0.0016	7.2	--	<0.001	21	<0.001	--

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G208	01/21/2015	--	<0.001	--	--	<0.01	<0.001	58	25	--	--	--	<0.001	--	--	--	7.6	--	--	36	--	340
G208	04/09/2015	<0.003	<0.001	0.1	<0.001	<0.01	<0.001	62	24	<0.004	<0.002	0.47	<0.001	--	<0.0002	--	7.2	--	0.0044	35	<0.001	300
G208	07/22/2015	<0.003	<0.001	0.062	<0.001	<0.01	<0.001	--	27	<0.004	<0.002	0.396	<0.001	--	<0.0002	<0.001	7.1	--	<0.001	110	<0.001	530
G208	10/07/2015	<0.003	<0.001	0.091	<0.001	0.022	<0.001	--	23	<0.004	<0.002	0.387	<0.001	--	<0.0002	0.0017	7.2	--	0.0033	33	<0.001	370
G209	01/21/2015	--	<0.001	--	--	0.011	<0.001	150	73	--	--	--	<0.001	--	--	--	7.4	--	--	310	--	940
G209	04/09/2015	<0.003	0.0019	0.1	<0.001	<0.01	<0.001	180	69	0.0063	0.0025	0.409	0.0029	--	<0.0002	--	6.9	--	0.0022	280	<0.001	910
G209	07/22/2015	<0.003	0.0075	0.13	<0.001	<0.01	<0.001	--	69	0.017	0.0095	0.422	0.0099	--	<0.0002	0.0035	7.0	--	0.002	270	<0.001	970
G209	10/07/2015	0.004	0.0085	0.11	0.0042	0.025	0.0041	--	59	0.035	0.0072	0.328	0.007	--	0.00027	0.0077	7.1	--	0.0047	270	0.0035	940
G209	11/18/2015	<0.003	0.0021	0.072	<0.001	<0.01	<0.001	160	67	<0.004	<0.002	0.398	<0.001	<0.01	<0.0002	0.0023	7.0	0.469	0.0036	280	<0.001	810
G209	02/23/2016	<0.003	<0.001	0.066	<0.001	<0.01	<0.001	150	70	<0.004	<0.002	0.475	<0.001	<0.01	<0.0002	0.0015	7.0	0.903	<0.001	280	<0.001	760
G209	05/11/2016	<0.003	<0.001	0.061	<0.001	<0.01	<0.001	160	59	<0.004	<0.002	0.461	<0.001	<0.01	<0.0002	0.0015	7.1	1.48	<0.001	280	<0.001	800
G209	08/06/2016	<0.003	0.0017	0.065	<0.001	0.014	<0.001	160	67	<0.004	<0.002	0.468	<0.001	<0.01	<0.0002	0.0018	7.2	0.673	<0.001	270	<0.001	760
G209	11/22/2016	<0.003	0.0022	0.045	<0.001	0.015	<0.001	100	70	<0.004	<0.002	0.42	<0.001	<0.01	<0.0002	<0.001	7.1	0.832	<0.001	270	<0.001	750
G209	02/11/2017	<0.003	<0.001	0.07	<0.001	<0.01	<0.001	120	60	<0.004	<0.002	0.358	<0.001	<0.01	<0.0002	0.0027	7.0	0.103	<0.001	260	<0.001	960
G209	05/18/2017	<0.003	0.0029	0.077	<0.001	<0.01	<0.001	130	63	<0.004	<0.002	0.263	0.0012	<0.01	<0.0002	0.0019	7.2	1.31	<0.001	240	0.001	820
G209	07/15/2017	<0.003	0.0057	0.063	<0.001	0.012	<0.001	120	72	<0.004	<0.002	0.437	<0.001	<0.01	<0.0002	<0.001	7.3	0.602	<0.001	120	<0.001	780
G209	10/31/2017	--	--	--	--	0.012	--	150	63	--	--	0.519	--	--	--	--	7.1	--	--	95	--	730
G209	01/25/2018	--	--	--	--	--	--	120	--	--	--	0.456	--	--	--	--	7.0	--	--	--	--	--
G209	05/15/2018	--	--	--	--	0.019	--	140	65	--	--	0.428	--	--	--	--	7.2	--	--	250	--	760
G209	11/02/2018	--	--	--	--	0.013	--	160	59	--	--	0.41	--	--	--	--	7.2	--	--	240	--	740
G209	01/17/2019	--	--	--	--	0.011	--	150	68	--	--	0.426	--	--	--	--	7.1	--	--	250	--	860
G209	05/03/2019	--	--	--	--	--	--	150	--	--	--	--	--	--	--	--	7.7	--	--	--	--	--

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G209	08/14/2019	--	--	--	--	0.011	--	160	61	--	--	0.586	--	--	--	--	7.2	--	--	240	--	830
G209	01/22/2020	--	--	--	--	0.017	--	150	59	--	--	0.406	--	--	--	--	6.9	--	--	250	--	730
G209	05/05/2020	--	--	--	--	--	--	140	--	--	--	--	--	--	--	--	7.2	--	--	--	--	--
G209	08/13/2020	--	--	--	--	0.018	--	150	65	--	--	0.474	--	--	--	--	7.2	--	--	270	--	800
G209	01/27/2021	--	--	--	--	<0.01	--	120	77	--	--	0.401	--	--	--	--	7.1	--	--	250	--	810
G210	01/21/2015	--	<0.001	--	--	<0.01	<0.001	73	55	--	--	--	<0.001	--	--	--	7.6	--	--	99	--	500
G210	04/09/2015	<0.003	<0.001	0.04	<0.001	0.055	<0.001	78	52	<0.004	<0.002	0.422	<0.001	--	<0.0002	--	7.0	--	0.0024	91	<0.001	490
G210	07/22/2015	<0.003	<0.001	0.037	<0.001	<0.01	<0.001	--	55	<0.004	<0.002	0.46	<0.001	--	<0.0002	<0.001	7.1	--	0.0014	84	<0.001	540
G210	10/07/2015	<0.003	<0.001	0.033	<0.001	0.013	<0.001	--	49	0.011	<0.002	0.368	<0.001	--	<0.0002	0.0018	7.2	--	<0.001	87	<0.001	500
G211	01/21/2015	--	<0.001	--	--	<0.01	<0.001	77	48	--	--	--	<0.001	--	--	--	7.6	--	--	80	--	490
G211	04/09/2015	<0.003	<0.001	0.081	<0.001	<0.01	<0.001	84	54	<0.004	<0.002	0.366	<0.001	--	<0.0002	--	7.1	--	0.0015	87	<0.001	480
G211	07/22/2015	<0.003	<0.001	0.087	<0.001	<0.01	<0.001	--	44	<0.004	<0.002	0.409	<0.001	--	<0.0002	<0.001	7.1	--	0.0025	74	<0.001	540
G211	10/07/2015	<0.003	<0.001	0.092	<0.001	0.014	<0.001	--	41	<0.004	<0.002	0.31	<0.001	--	<0.0002	<0.001	7.3	--	0.0013	78	<0.001	500
G212	01/21/2015	--	0.0021	--	--	0.019	<0.001	62	39	--	--	--	0.0035	--	--	--	7.5	--	--	59	--	400
G212	04/09/2015	<0.003	<0.001	0.056	<0.001	<0.01	<0.001	60	44	<0.004	<0.002	0.346	<0.001	--	<0.0002	--	7.0	--	0.0046	66	<0.001	410
G212	07/22/2015	<0.003	<0.001	0.057	<0.001	<0.01	<0.001	--	38	<0.004	<0.002	0.396	<0.001	--	<0.0002	0.0036	7.0	--	0.0042	56	<0.001	460
G212	10/07/2015	<0.003	<0.001	0.057	<0.001	0.026	<0.001	--	43	0.0043	<0.002	<0.25	<0.001	--	<0.0002	0.0035	7.0	--	0.0044	65	<0.001	410
G212	11/18/2015	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	55	38	<0.004	<0.002	0.34	<0.001	<0.01	<0.0002	0.0015	7.2	0.132	0.0037	54	<0.001	380
G212	02/19/2016	<0.003	<0.001	0.05	<0.001	<0.01	<0.001	58	41	<0.004	<0.002	0.339	<0.001	<0.01	<0.0002	<0.001	7.3	0.582	0.0048	59	<0.001	380
G212	05/11/2016	<0.003	<0.001	0.05	<0.001	<0.01	<0.001	58	37	<0.004	<0.002	0.421	<0.001	<0.01	<0.0002	<0.001	7.3	0.759	0.0041	59	<0.001	400
G212	08/06/2016	<0.003	<0.001	0.059	<0.001	0.016	<0.001	59	37	0.004	<0.002	0.369	0.0016	<0.01	<0.0002	0.0012	7.3	0.992	0.004	55	<0.001	330
G212	11/23/2016	<0.003	<0.001	0.049	<0.001	<0.01	<0.001	51	42	<0.004	<0.002	0.399	<0.001	<0.01	<0.0002	<0.001	7.1	--	0.0043	54	<0.001	340

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G212	12/07/2016	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	1.64	--	--	--	--
G212	02/15/2017	<0.003	0.001	0.058	<0.001	<0.01	<0.001	53	37	<0.004	<0.002	0.369	0.001	<0.01	<0.0002	<0.001	7.1	0.488	0.0041	55	<0.001	420
G212	05/22/2017	<0.003	<0.001	0.061	<0.001	<0.01	<0.001	46	39	<0.004	<0.002	0.372	<0.001	<0.01	<0.0002	0.0011	7.0	0.729	0.0039	57	<0.001	360
G212	07/15/2017	<0.003	<0.001	0.052	<0.001	<0.01	<0.001	46	44	<0.004	<0.002	0.377	<0.001	<0.01	<0.0002	<0.001	7.6	0.654	0.0046	53	<0.001	430
G212	10/31/2017	--	--	--	--	<0.01	--	50	42	--	--	0.326	--	--	--	--	7.3	--	--	55	--	340
G212	05/14/2018	--	--	--	--	0.014	--	51	40	--	--	0.407	--	--	--	--	7.2	--	--	52	--	350
G212	11/02/2018	--	--	--	--	<0.01	--	53	43	--	--	0.289	--	--	--	--	7.3	--	--	49	--	600
G212	01/16/2019	--	--	--	--	<0.01	--	56	43	--	--	0.394	--	--	--	--	7.3	--	--	53	--	440
G212	08/14/2019	--	--	--	--	<0.01	--	53	43	--	--	0.437	--	--	--	--	7.3	--	--	51	--	380
G212	01/22/2020	--	--	--	--	0.012	--	61	42	--	--	0.283	--	--	--	--	7.2	--	--	58	--	340
G212	08/13/2020	--	--	--	--	<0.01	--	54	42	--	--	0.323	--	--	--	--	7.3	--	--	53	--	430
G212	01/26/2021	--	--	--	--	0.032	--	56	41	--	--	<0.25	--	--	--	--	6.8	--	--	55	--	400
G212	06/29/2021	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.4	--	--	--	--	--
G213	01/21/2015	--	<0.001	--	--	<0.01	<0.001	66	44	--	--	--	<0.001	--	--	--	7.5	--	--	55	--	390
G213	04/09/2015	<0.003	<0.001	0.064	<0.001	<0.01	<0.001	70	47	<0.004	<0.002	0.35	0.0017	--	<0.0002	--	7.0	--	0.0024	57	<0.001	420
G213	07/22/2015	<0.003	0.0072	0.16	<0.001	<0.01	<0.001	--	42	0.026	0.01	0.41	0.016	--	<0.0002	0.0012	7.0	--	0.0035	50	<0.001	440
G213	10/07/2015	<0.003	0.0045	0.1	0.002	0.014	0.0018	--	40	0.026	0.0057	0.314	0.008	--	<0.0002	0.004	7.0	--	0.0038	51	0.0014	400
G214	01/21/2015	--	<0.001	--	--	<0.01	<0.001	81	100	--	--	--	<0.001	--	--	--	7.5	--	--	68	--	480
G214	04/09/2015	<0.003	<0.001	0.1	<0.001	<0.01	<0.001	88	110	<0.004	<0.002	0.417	<0.001	--	<0.0002	--	7.0	--	0.0011	73	<0.001	500
G214	07/22/2015	<0.003	<0.001	0.092	<0.001	<0.01	<0.001	--	82	<0.004	<0.002	0.446	<0.001	--	<0.0002	0.0011	7.1	--	<0.001	68	<0.001	530
G214	10/07/2015	<0.003	<0.001	0.089	<0.001	0.011	<0.001	--	67	0.0067	<0.002	0.324	<0.001	--	<0.0002	0.0019	7.2	--	<0.001	76	<0.001	540
G215	01/21/2015	--	0.0064	--	--	0.033	<0.001	110	60	--	--	--	<0.001	--	--	--	7.3	--	--	170	--	640

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G215	04/09/2015	<0.003	0.046	0.17	<0.001	0.041	<0.001	120	53	<0.004	<0.002	0.418	<0.001	--	<0.0002	--	6.8	--	<0.001	140	<0.001	620
G215	07/22/2015	<0.003	0.053	0.17	<0.001	0.023	<0.001	--	45	<0.004	<0.002	0.345	0.0017	--	<0.0002	<0.001	6.9	--	<0.001	110	<0.001	660
G215	10/07/2015	<0.003	0.04	0.15	<0.001	0.035	<0.001	--	46	<0.004	<0.002	0.321	0.0015	--	<0.0002	<0.001	6.8	--	<0.001	110	<0.001	550
G215	11/24/2015	<0.003	0.11	0.23	<0.001	0.037	<0.001	110	47	<0.004	0.0028	0.34	0.0039	<0.01	<0.0002	0.0011	7.2	2.42	<0.001	110	<0.001	500
G215	02/18/2016	<0.003	0.0034	0.095	<0.001	0.027	<0.001	100	52	<0.004	<0.002	0.359	<0.001	<0.01	<0.0002	<0.001	7.2	0.852	<0.001	130	<0.001	520
G215	05/11/2016	0.0045	0.0068	0.088	<0.001	0.026	<0.001	89	43	<0.004	<0.002	0.463	<0.001	<0.01	<0.0002	<0.001	6.9	0.468	0.0024	110	<0.001	460
G215	07/30/2016	<0.003	0.013	0.096	<0.001	0.015	<0.001	89	47	<0.004	<0.002	0.432	<0.001	<0.01	<0.0002	<0.001	6.9	0.0216	<0.001	110	<0.001	480
G215	11/23/2016	<0.003	0.0086	0.082	<0.001	0.023	<0.001	68	48	<0.004	<0.002	0.429	<0.001	<0.01	<0.0002	<0.001	6.9	--	<0.001	100	<0.001	500
G215	12/07/2016	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	1.58	--	--	--	--
G215	02/18/2017	<0.003	0.012	0.095	<0.001	0.021	<0.001	86	46	<0.004	<0.002	0.369	<0.001	<0.01	<0.0002	<0.001	7.3	0.344	<0.001	110	<0.001	510
G215	05/22/2017	<0.003	0.036	0.15	<0.001	0.024	<0.001	82	42	<0.004	<0.002	<0.25	<0.001	<0.01	<0.0002	<0.001	7.4	1.24	<0.001	100	<0.001	470
G215	07/15/2017	<0.003	0.044	0.13	<0.001	0.027	<0.001	79	55	<0.004	<0.002	0.423	<0.001	<0.01	<0.0002	<0.001	7.0	1.01	<0.001	110	<0.001	550
G215	10/31/2017	--	--	--	--	0.025	--	90	48	--	--	0.42	--	--	--	--	7.2	--	--	110	--	470
G215	05/15/2018	--	--	--	--	0.063	--	130	70	--	--	0.329	--	--	--	--	6.9	--	--	220	--	660
G215	11/02/2018	--	--	--	--	0.088	--	120	55	--	--	0.314	--	--	--	--	6.8	--	--	170	--	480
G215	01/16/2019	--	--	--	--	0.097	--	120	61	--	--	0.379	--	--	--	--	6.9	--	--	180	--	800
G215	08/14/2019	--	--	--	--	0.085	--	100	49	--	--	0.458	--	--	--	--	7.0	--	--	120	--	520
G215	01/22/2020	--	--	--	--	0.064	--	99	48	--	--	0.35	--	--	--	--	7.1	--	--	130	--	460
G215	08/13/2020	--	--	--	--	0.051	--	110	70	--	--	0.366	--	--	--	--	7.2	--	--	210	--	710
G215	01/26/2021	--	--	--	--	0.36	--	180	120	--	--	<0.25	--	--	--	--	6.8	--	--	490	--	1100
G215	06/29/2021	--	--	--	--	--	--	180	110	--	--	--	--	--	--	--	7.1	--	--	470	--	950
G216	01/21/2015	--	0.0029	--	--	0.014	<0.001	100	78	--	--	--	<0.001	--	--	--	7.3	--	--	230	--	740

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G216	04/10/2015	<0.003	0.0064	0.18	<0.001	0.018	<0.001	140	66	<0.004	<0.002	0.385	0.0013	--	<0.0002	--	6.9	--	<0.001	210	<0.001	720
G216	07/22/2015	<0.003	0.0037	0.17	<0.001	0.021	<0.001	--	67	<0.004	<0.002	0.313	<0.001	--	<0.0002	0.0026	7.0	--	<0.001	220	<0.001	800
G216	10/07/2015	<0.003	0.002	0.16	<0.001	0.042	<0.001	--	85	<0.004	<0.002	0.295	<0.001	--	<0.0002	0.013	7.0	--	<0.001	210	<0.001	740
G217	01/21/2015	--	<0.001	--	--	<0.01	<0.001	110	82	--	--	--	<0.001	--	--	--	7.2	--	--	140	--	670
G217	04/10/2015	<0.003	<0.001	0.088	<0.001	<0.01	<0.001	130	77	<0.004	<0.002	0.401	<0.001	--	<0.0002	--	6.8	--	<0.001	130	<0.001	700
G217	07/22/2015	<0.003	0.0053	0.16	<0.001	<0.01	<0.001	--	78	0.011	0.012	0.31	0.0067	--	<0.0002	0.0014	7.0	--	<0.001	130	<0.001	750
G217	10/07/2015	<0.003	0.0019	0.11	<0.001	0.013	<0.001	--	76	0.0086	0.0032	0.295	0.0019	--	<0.0002	0.0013	7.1	--	<0.001	130	<0.001	660
G218	01/21/2015	--	0.0013	--	--	<0.01	<0.001	94	100	--	--	--	<0.001	--	--	--	7.2	--	--	110	--	660
G218	04/10/2015	<0.003	0.0011	0.14	<0.001	<0.01	<0.001	120	93	<0.004	<0.002	0.381	<0.001	--	<0.0002	--	6.8	--	<0.001	99	<0.001	640
G218	07/22/2015	<0.003	<0.001	0.15	<0.001	<0.01	<0.001	--	98	<0.004	<0.002	0.308	<0.001	--	<0.0002	<0.001	6.8	--	<0.001	97	<0.001	690
G218	10/07/2015	<0.003	<0.001	0.14	<0.001	<0.01	<0.001	--	96	0.03	<0.002	0.27	<0.001	--	<0.0002	0.0022	7.0	--	<0.001	95	<0.001	620
G218	11/24/2015	<0.003	0.0084	0.17	<0.001	<0.01	<0.001	120	99	0.01	<0.002	0.3	<0.001	<0.01	<0.0002	0.0015	7.1	1.23	<0.001	94	<0.001	620
G218	02/19/2016	<0.003	0.0018	0.15	<0.001	<0.01	<0.001	120	100	<0.004	<0.002	0.311	<0.001	<0.01	<0.0002	<0.001	7.0	1.28	<0.001	110	<0.001	560
G218	05/10/2016	<0.003	0.0015	0.14	<0.001	0.011	<0.001	110	97	<0.004	<0.002	0.439	<0.001	<0.01	<0.0002	<0.001	7.0	0.601	<0.001	140	<0.001	600
G218	07/30/2016	<0.003	0.0011	0.15	<0.001	<0.01	<0.001	130	100	<0.004	<0.002	0.382	<0.001	<0.01	<0.0002	<0.001	7.0	0.543	<0.001	120	<0.001	620
G218	11/23/2016	<0.003	0.0014	0.13	<0.001	<0.01	<0.001	92	97	<0.004	<0.002	0.373	<0.001	<0.01	<0.0002	<0.001	7.1	--	<0.001	130	<0.001	620
G218	12/07/2016	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	0.85	--	--	--	--
G218	02/18/2017	<0.003	0.0011	0.13	<0.001	<0.01	<0.001	110	88	<0.004	<0.002	0.308	<0.001	<0.01	<0.0002	<0.001	7.2	0.779	<0.001	130	<0.001	630
G218	05/22/2017	<0.003	<0.001	0.15	<0.001	<0.01	<0.001	100	84	<0.004	<0.002	<0.25	<0.001	<0.01	<0.0002	<0.001	7.1	0.975	<0.001	140	<0.001	600
G218	07/17/2017	<0.003	<0.001	0.14	<0.001	<0.01	<0.001	120	81	<0.004	<0.002	0.357	<0.001	<0.01	<0.0002	<0.001	7.1	0.704	<0.001	140	<0.001	720
G218	10/31/2017	--	--	--	--	<0.01	--	110	91	--	--	0.437	--	--	--	--	6.9	--	--	140	--	660
G218	01/26/2018	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	6.9	--	--	--	--	--

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
G218	05/15/2018	--	--	--	--	0.014	--	110	91	--	--	0.413	--	--	--	--	7.0	--	--	140	--	640
G218	11/02/2018	--	--	--	--	<0.01	--	130	84	--	--	0.375	--	--	--	--	6.9	--	--	140	--	280
G218	01/17/2019	--	--	--	--	<0.01	--	120	82	--	--	0.361	--	--	--	--	7.0	--	--	140	--	600
G218	08/14/2019	--	--	--	--	<0.01	--	130	81	--	--	0.449	--	--	--	--	7.0	--	--	150	--	660
G218	01/22/2020	--	--	--	--	0.011	--	130	83	--	--	0.379	--	--	--	--	7.1	--	--	170	--	560
G218	08/13/2020	--	--	--	--	<0.01	--	120	84	--	--	0.34	--	--	--	--	7.1	--	--	220	--	720
G218	01/26/2021	--	--	--	--	0.017	--	120	81	--	--	0.276	--	--	--	--	7.0	--	--	210	--	710
MW16S	04/09/2015	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.2	--	--	--	--	410
R104	01/20/2015	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	7.8	--	--	--	--	460
R104	04/08/2015	<0.003	0.0015	0.065	<0.001	<0.01	<0.001	66	47	0.0043	<0.002	0.4	<0.001	--	<0.0002	--	7.4	--	0.0074	72	<0.001	500
R104	07/23/2015	<0.003	<0.001	0.067	<0.001	<0.01	<0.001	--	45	<0.004	<0.002	0.349	0.0023	--	<0.0002	0.0044	7.4	--	0.0072	73	<0.001	450
R104	10/06/2015	<0.003	<0.001	0.063	<0.001	<0.01	<0.001	--	56	0.0044	<0.002	0.304	<0.001	--	<0.0002	0.0045	7.6	--	0.0062	72	<0.001	480
R104	11/17/2015	<0.003	<0.001	0.067	<0.001	<0.01	<0.001	69	51	0.0052	<0.002	0.358	0.0012	<0.01	<0.0002	0.0041	7.4	0.565	0.0063	77	<0.001	420
R104	02/17/2016	<0.003	<0.001	0.06	<0.001	0.012	<0.001	67	58	<0.004	<0.002	0.314	<0.001	<0.01	<0.0002	0.0042	8.1	0.363	0.0064	77	<0.001	440
R104	05/16/2016	<0.003	<0.001	0.062	<0.001	<0.01	<0.001	65	49	<0.004	<0.002	0.473	<0.001	<0.01	<0.0002	0.0038	7.4	0.145	0.006	74	<0.001	420
R104	08/03/2016	<0.003	<0.001	0.064	<0.001	<0.01	<0.001	68	50	<0.004	<0.002	0.372	<0.001	<0.01	<0.0002	0.0042	7.3	0.798	0.006	76	<0.001	460
R201	01/20/2015	--	<0.001	--	--	<0.01	<0.001	110	53	--	--	--	<0.001	--	--	--	7.2	--	--	210	--	750
R201	04/10/2015	<0.003	<0.001	0.084	<0.001	<0.01	<0.001	130	66	<0.004	<0.002	0.353	<0.001	--	<0.0002	--	6.8	--	<0.001	220	<0.001	760
R201	07/22/2015	<0.003	0.0011	0.099	<0.001	<0.01	<0.001	--	78	<0.004	<0.002	0.28	<0.001	--	<0.0002	<0.001	6.9	--	<0.001	250	<0.001	930
R201	10/05/2015	<0.003	<0.001	0.081	<0.001	<0.01	<0.001	--	74	<0.004	<0.002	0.353	<0.001	--	<0.0002	<0.001	7.0	--	<0.001	210	<0.001	840
R201	11/23/2015	<0.003	<0.001	0.078	<0.001	<0.01	0.0012	85	37	<0.004	<0.002	0.377	<0.001	<0.01	<0.0002	0.0069	7.3	0.202	<0.001	150	<0.001	560
R201	02/12/2016	<0.003	0.01	0.084	0.0067	0.014	<0.001	120	75	<0.004	<0.002	0.398	<0.001	<0.01	<0.0002	0.001	7.0	0.543	0.0091	240	<0.001	740

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
R201	05/10/2016	<0.003	<0.001	0.084	<0.001	<0.01	<0.001	120	85	<0.004	<0.002	0.447	<0.001	<0.01	<0.0002	<0.001	7.0	1.12	<0.001	260	<0.001	840
R201	07/30/2016	<0.003	0.0031	0.092	<0.001	<0.01	<0.001	120	85	<0.004	<0.002	0.368	<0.001	<0.01	<0.0002	<0.001	7.1	0.697	<0.001	260	<0.001	750
R201	11/18/2016	<0.003	0.0013	0.058	<0.001	<0.01	<0.001	81	39	<0.004	<0.002	0.494	<0.001	<0.01	<0.0002	<0.001	7.2	0.055	<0.001	160	<0.001	580
R201	02/11/2017	<0.003	0.0028	0.086	<0.001	<0.01	<0.001	100	79	<0.004	<0.002	0.285	<0.001	<0.01	<0.0002	<0.001	7.1	1.02	<0.001	230	<0.001	900
R201	05/18/2017	<0.003	0.0023	0.087	<0.001	0.011	<0.001	120	74	<0.004	<0.002	0.354	<0.001	<0.01	<0.0002	<0.001	7.2	1.51	<0.001	300	<0.001	820
R201	07/13/2017	<0.003	0.0037	0.17	<0.001	0.01	<0.001	120	81	<0.004	<0.002	0.284	<0.001	<0.01	<0.0002	<0.001	7.0	2.75	<0.001	250	<0.001	780
R201	10/28/2017	--	--	--	--	0.017	--	93	30	--	--	0.38	--	--	--	--	7.1	--	--	89	--	660
R201	01/25/2018	--	--	--	--	--	--	--	31	--	--	0.338	--	--	--	--	7.0	--	--	--	--	--
R201	05/11/2018	--	--	--	--	<0.01	--	87	54	--	--	0.306	--	--	--	--	7.1	--	--	190	--	640
R201	11/02/2018	--	--	--	--	<0.01	--	82	24	--	--	0.419	--	--	--	--	7.1	--	--	110	--	470
R201	01/16/2019	--	--	--	--	<0.01	--	100	48	--	--	0.341	--	--	--	--	7.1	--	--	150	--	790
R201	08/12/2019	--	--	--	--	<0.01	--	120	71	--	--	0.466	--	--	--	--	7.1	--	--	220	--	760
R201	01/21/2020	--	--	--	--	0.01	--	130	66	--	--	0.309	--	--	--	--	7.2	--	--	210	--	770
R201	08/11/2020	--	--	--	--	<0.01	--	120	87	--	--	0.364	--	--	--	--	6.9	--	--	240	--	790
R201	01/29/2021	--	--	--	--	0.01	--	94	46	--	--	<0.25	--	--	--	--	7.0	--	--	160	--	710
R201	03/29/2021	<0.003	0.0017	0.073	<0.001	0.019	<0.001	97	55	<0.004	<0.002	0.45	<0.001	<0.02	<0.0002	<0.001	7.0	0.272	<0.001	190	<0.001	660
R201	04/21/2021	<0.003	0.0013	0.071	<0.001	0.019	<0.001	100	63	<0.004	<0.002	0.359	<0.001	<0.02	<0.0002	<0.001	6.9	0.561	<0.001	200	<0.001	810
R201	05/06/2021	<0.003	0.0027	0.078	<0.001	<0.01	<0.001	110	67	0.0043	<0.002	<0.25	<0.001	<0.02	<0.0002	0.0011	7.2	0.742	<0.001	210	<0.001	730
R201	05/17/2021	<0.003	0.0025	0.078	<0.001	<0.01	<0.001	110	76	<0.004	<0.002	0.265	<0.001	<0.02	<0.0002	<0.001	7.0	1.19	<0.001	210	<0.001	770
R201	06/14/2021	<0.003	0.0028	0.082	<0.001	0.016	<0.001	120	68	<0.004	<0.002	0.332	<0.001	<0.02	<0.0002	<0.001	7.0	0.123	<0.001	230	<0.001	820
R201	06/29/2021	<0.003	0.01	0.13	<0.001	0.014	<0.001	140	81	0.0067	0.0037	0.356	0.0059	<0.02	<0.0002	0.002	7.1	1.17	<0.001	220	<0.001	560
R201	07/13/2021	<0.003	0.0022	0.069	<0.001	<0.01	<0.001	97	58	<0.004	<0.002	0.261	<0.001	<0.02	<0.0002	<0.001	7.0	0.581	<0.001	170	<0.001	710

TABLE 4-1. GROUNDWATER ANALYTICAL RESULTS
 HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND
 COFFEEN, ILLINOIS

Location	Sample Date	Antimony, total (mg/L)	Arsenic, total (mg/L)	Barium, total (mg/L)	Beryllium, total (mg/L)	Boron, total (mg/L)	Cadmium, total (mg/L)	Calcium, total (mg/L)	Chloride, total (mg/L)	Chromium, total (mg/L)	Cobalt, total (mg/L)	Fluoride, total (mg/L)	Lead, total (mg/L)	Lithium, total (mg/L)	Mercury, total (mg/L)	Molybdenum, total (mg/L)	pH (field) (SU)	Radium 226 and 228 combined (pCi/L)	Selenium, total (mg/L)	Sulfate, total (mg/L)	Thallium, total (mg/L)	Total Dissolved Solids (mg/L)
35 I.A.C. 845.600	Lower	0	0	0	0	0	0	--	0	0	0	0	0	0	0	0	6.5	0	0	0	0	0
	Upper	0.006	0.010	2.0	0.004	2	0.005	--	200	0.1	0.006	4.0	0.0075	0.04	0.002	0.1	9.0	5	0.05	400	0.002	1200
R201	07/28/2021	<0.003	0.0019	0.081	<0.001	0.011	<0.001	110	79	<0.004	<0.002	0.504	<0.001	<0.02	0.0011	0.044	7.2	0.744	<0.001	370	<0.001	750

Notes:

Detected at concentration greater than the GWPS

-- = data not available

GWPS = Groundwater protection standard

mg/L = milligrams per liter

pCi/L = picocuries per liter

SU = standard units

< = concentration is less than the concentration shown, which corresponds to the reporting limit for the method. Estimated concentrations below the reporting limit and associated qualifiers are not provided since they are not utilized in statistics to determine exceedances above Part 845 standards.

35 I.A.C. 845.600 = Residuals in Surface Impoundments: Title 35 of the Illinois Administrative Code § 845

TABLE 4-2. GROUNDWATER FIELD PARAMETERS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G102	01/20/2015	--	--	7.6	691	--	--
G102	04/08/2015	--	--	7.1	746	--	--
G102	07/23/2015	--	--	7.1	723	--	--
G102	10/06/2015	--	--	7.1	857	--	--
G102	11/16/2015	0.42	160	6.9	1040	12.6	3.9
G102	02/17/2016	1.10	152	7.5	682	11.3	--
G102	05/16/2016	0	136	7.2	575	16.1	30
G102	08/02/2016	0	96	7.2	648	18.0	30.6
G102	11/19/2016	0	74	7.1	782	13.8	20.5
G102	02/09/2017	0	72	7.1	820	14.5	45.5
G102	05/22/2017	0	61	7.2	1040	15.3	40.5
G102	07/09/2017	0	96	7.2	640	17.0	29.6
G102	10/25/2017	0	94	7.1	665	13.1	28.7
G102	01/26/2018	0	-96	7.1	690	11.1	19.7
G102	05/10/2018	0	94	7.2	661	14.4	16.7
G102	10/24/2018	0	87	7.0	693	14.5	14.8
G102	01/17/2019	0	99	7.2	652	12.3	28.6
G102	08/12/2019	0	98	7.2	700	16.8	27.7
G102	01/21/2020	6.10	148	7.6	599.2	9.9	101
G102	08/11/2020	1.10	129	7.3	949.2	18.3	0
G102	01/26/2021	2.10	141	7.1	685	11.3	3.42
G103	01/20/2015	--	--	7.6	555	--	--
G103	04/08/2015	--	--	7.2	761	--	--
G103	07/23/2015	--	--	7.2	815	--	--
G103	10/06/2015	--	--	7.3	896	--	--
G105	01/20/2015	--	--	7.3	700	--	--
G105	04/08/2015	--	--	7.0	897	--	--
G105	07/23/2015	--	--	7.1	912	--	--
G105	10/06/2015	--	--	7.0	1037	--	--
G106	01/20/2015	--	--	7.5	330	--	--
G106	04/08/2015	--	--	7.1	771	--	--
G106	07/23/2015	--	--	7.2	790	--	--
G106	10/06/2015	--	--	7.3	825	--	--
G106	11/17/2015	0	116	7.1	804	16.6	132
G106	02/17/2016	5.40	167	7.2	800	11.6	--
G106	05/16/2016	7.22	162	7.1	758	16.1	46
G106	08/04/2016	0	126	7.0	697	17.3	33.9
G106	11/19/2016	0	105	7.0	812	13.6	24.4
G106	02/09/2017	0	99	7.0	800	14.9	29.5
G106	05/22/2017	0	94	7.1	849	15.2	22.8
G106	07/09/2017	0	121	7.0	705	17.0	45
G106	10/25/2017	0	107	7.3	738	13.3	14.5
G106	05/10/2018	0	106	7.0	728	13.0	24.8
G106	08/09/2018	0	120	7.0	695	14.9	30.4
G106	10/24/2018	0	127	6.8	662	13.9	36.1
G106	01/17/2019	0	123	6.9	695	12.2	20.8

TABLE 4-2. GROUNDWATER FIELD PARAMETERS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G106	08/13/2019	0	121	6.9	712	17.4	29.8
G106	01/21/2020	1.00	81.4	7.3	951.1	10.8	25.4
G106	08/11/2020	0.41	-145	7.0	915.6	18.1	0
G106	01/26/2021	1.40	49.3	7.5	1223	11.4	4.95
G106	06/29/2021	1.70	-41.3	7.2	869.6	19.8	0
G200	01/20/2015	--	--	7.4	382	--	--
G200	04/10/2015	--	--	7.0	939	--	--
G200	07/22/2015	--	--	7.0	1024	--	--
G200	10/05/2015	--	--	7.1	984	--	--
G200	11/23/2015	7.18	29	7.2	948	15.7	844
G200	02/12/2016	10.00	134	7.2	1060	9.3	1000
G200	05/10/2016	0	-33	7.1	1010	18.3	1000
G200	07/30/2016	0	-62	7.1	990	19.2	1000
G200	11/18/2016	0	-76	7.2	874	16.3	1000
G200	02/10/2017	0	-72	7.1	852	15.7	1000
G200	05/18/2017	0	-66	7.0	904	15.4	1000
G200	07/13/2017	0	-63	7.1	940	18.1	1000
G200	10/28/2017	0	-65	7.2	1004	13.0	1000
G200	01/25/2018	0	-77	7.2	890	12.1	99
G200	05/11/2018	0	-55	7.0	1010	13.2	1000
G200	11/02/2018	0	-58	7.0	1020	13.8	1000
G200	01/16/2019	0	-61	7.1	980	12.3	1000
G200	08/12/2019	0	-63	7.0	967	17.0	1000
G200	01/21/2020	4.90	183	7.2	1039	10.4	42.6
G200	08/11/2020	1.80	104	7.2	929	19.1	21.8
G200	01/29/2021	2.70	43.6	7.3	914	9.1	98.3
G200	03/29/2021	1.80	28.1	7.3	886	12.8	115
G200	04/21/2021	0.76	92.7	7.2	898	10.6	13.2
G200	05/06/2021	1.40	10.8	7.1	917	11.9	2.71
G200	05/17/2021	0.22	34	7.2	933	14.2	189
G200	07/28/2021	0.77	-23.5	7.0	829.5	19.3	9.98
G206	01/21/2015	--	--	7.3	578	--	--
G206	04/09/2015	--	--	7.0	773	--	--
G206	07/22/2015	--	--	7.0	758	--	--
G206	10/07/2015	--	--	6.8	747	--	--
G206	11/18/2015	0	-165	7.1	830	16.6	65.4
G206	02/24/2016	1.50	22	6.7	769	12.9	19
G206	06/27/2016	0	-31	6.2	838	23.5	--
G206	08/06/2016	0	103	7.1	920	19.3	40.8
G206	11/22/2016	0	91	7.1	964	14.9	28.4
G206	02/11/2017	0	123	7.2	905	15.0	47
G206	05/18/2017	0	93	7.0	987	15.7	54.9
G206	07/15/2017	0	85	7.1	931	18.5	58.9
G206	10/30/2017	0	105	7.2	1010	12.8	31.8
G206	05/15/2018	0	91	7.0	938	15.5	43.3
G206	11/02/2018	0	98	7.0	872	15.1	40.6

TABLE 4-2. GROUNDWATER FIELD PARAMETERS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G206	01/17/2019	0	101	7.1	922	12.4	47
G206	08/14/2019	0	99	7.1	904	17.0	22.6
G206	01/21/2020	2.90	16.8	7.5	782.6	13.0	2.57
G206	05/05/2020	4.60	176	7.5	781	13.0	3.98
G206	08/13/2020	1.40	202	7.4	819	17.8	0.3
G206	01/27/2021	2.10	39.1	7.1	799	12.0	0.71
G206D	03/30/2021	0.69	-9.6	7.1	2027	15.4	226
G206D	04/22/2021	2.00	44.2	7.2	1892	15.4	54.5
G206D	05/05/2021	4.80	97.6	7.1	1764	18.8	33.5
G206D	05/18/2021	0.23	-48.4	7.2	1843	14.5	2.97
G206D	07/27/2021	2.00	78	7.5	924.3	20.6	20
G207	01/21/2015	--	--	7.7	444	--	--
G207	04/09/2015	--	--	7.1	607	--	--
G207	07/22/2015	--	--	7.2	693	--	--
G207	10/07/2015	--	--	7.2	672	--	--
G208	01/21/2015	--	--	7.6	396	--	--
G208	04/09/2015	--	--	7.2	531	--	--
G208	07/22/2015	--	--	7.1	638	--	--
G208	10/07/2015	--	--	7.2	743	--	--
G209	01/21/2015	--	--	7.4	1004	--	--
G209	04/09/2015	--	--	6.9	1280	--	--
G209	07/22/2015	--	--	7.0	1030	--	--
G209	10/07/2015	--	--	7.1	1086	--	--
G209	11/18/2015	0	-98	7.0	1024	16.5	149
G209	02/23/2016	0.07	76	7.0	1350	12.2	2.6
G209	05/11/2016	0	144	7.1	1400	19.0	30
G209	08/06/2016	0	149	7.2	1478	19.3	31.2
G209	11/22/2016	0	138	7.1	1610	15.1	36.4
G209	02/11/2017	0	-109	7.0	1490	14.5	69.4
G209	05/18/2017	0	-99	7.2	1420	15.5	62.2
G209	07/15/2017	0	161	7.3	1220	17.9	22
G209	10/31/2017	0	140	7.1	1450	12.9	26.4
G209	01/25/2018	0	101	7.0	1410	11.1	30.2
G209	05/15/2018	0	117	7.2	1480	13.5	28.5
G209	11/02/2018	0	148	7.2	1580	13.8	24.1
G209	01/17/2019	0	141	7.1	1490	12.9	30.6
G209	05/03/2019	0	159	7.7	1240	14.5	37
G209	08/14/2019	0	151	7.2	1490	17.1	29.3
G209	01/22/2020	1.10	-69	6.9	1310	11.6	22.6
G209	05/05/2020	0.88	-34.5	7.2	1182	13.0	0.7
G209	08/13/2020	1.50	-60.3	7.2	1285	18.6	5.4
G209	01/27/2021	3.20	47.8	7.1	1315	9.1	4.36
G210	01/21/2015	--	--	7.5	611	--	--
G210	04/09/2015	--	--	7.0	836	--	--
G210	07/22/2015	--	--	7.1	918	--	--
G210	10/07/2015	--	--	7.2	813	--	--

TABLE 4-2. GROUNDWATER FIELD PARAMETERS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G211	01/21/2015	--	--	7.6	604	--	--
G211	04/09/2015	--	--	7.1	819	--	--
G211	07/22/2015	--	--	7.1	855	--	--
G211	10/07/2015	--	--	7.3	926	--	--
G212	01/21/2015	--	--	7.5	525	--	--
G212	04/09/2015	--	--	7.0	712	--	--
G212	07/22/2015	--	--	7.0	744	--	--
G212	10/07/2015	--	--	7.0	825	--	--
G212	11/18/2015	0	-63	7.2	706	16.4	38.4
G212	02/19/2016	2.20	128	7.3	721	13.9	--
G212	05/11/2016	2.11	141	7.3	732	16.7	25
G212	08/06/2016	0	130	7.3	795	18.0	41.2
G212	11/23/2016	0	126	7.1	924	13.7	50.1
G212	02/15/2017	0	114	7.1	890	15.0	51.2
G212	05/22/2017	0	102	7.0	915	15.6	40.2
G212	07/15/2017	0	130	7.6	838	17.0	33.7
G212	10/31/2017	0	139	7.3	821	12.6	40.3
G212	05/14/2018	0	122	7.2	832	13.8	43.2
G212	11/02/2018	0	121	7.3	890	14.7	30.8
G212	01/16/2019	0	131	7.3	832	12.3	29.8
G212	08/14/2019	0	128	7.3	815	17.0	35.6
G212	01/22/2020	2.80	154	7.2	728.6	11.7	7.72
G212	08/13/2020	3.20	150	7.3	655	18.7	1.7
G212	01/26/2021	2.30	152	6.8	728	11.4	3.14
G212	06/29/2021	1.80	29.9	7.4	703.2	17.9	0
G213	01/21/2015	--	--	7.5	504	--	--
G213	04/09/2015	--	--	7.0	654	--	--
G213	07/22/2015	--	--	7.0	642	--	--
G213	10/07/2015	--	--	7.0	679	--	--
G214	01/21/2015	--	--	7.5	666	--	--
G214	04/09/2015	--	--	7.0	841	--	--
G214	07/22/2015	--	--	7.1	938	--	--
G214	10/07/2015	--	--	7.2	923	--	--
G215	01/21/2015	--	--	7.3	741	--	--
G215	04/09/2015	--	--	6.8	948	--	--
G215	07/22/2015	--	--	6.9	782	--	--
G215	10/07/2015	--	--	6.8	838	--	--
G215	11/24/2015	8.90	-59	7.2	907	16.0	909
G215	02/18/2016	0	-2	7.2	963	12.9	--
G215	05/11/2016	0	5	6.9	945	16.9	285
G215	07/30/2016	0	56	6.9	1020	18.3	161
G215	11/23/2016	0	61	6.9	1060	15.9	135
G215	02/18/2017	0	71	7.3	970	14.5	102
G215	05/22/2017	0	84	7.4	1030	15.2	91
G215	07/15/2017	0	61	7.0	992	17.5	1000
G215	10/31/2017	0	129	7.2	868	13.3	36.5

TABLE 4-2. GROUNDWATER FIELD PARAMETERS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
G215	05/15/2018	0	42	6.9	1020	15.2	132
G215	11/02/2018	0	52	6.8	1050	14.2	179
G215	01/16/2019	0	61	6.9	1022	12.6	123
G215	08/14/2019	0	56	7.0	998	17.2	1000
G215	01/22/2020	0.46	0.5	7.1	938.6	11.1	31.8
G215	08/13/2020	0.20	-35.2	7.2	1086	20.3	13.8
G215	01/26/2021	0.17	-2.9	6.8	1493	12.0	45.9
G215	06/29/2021	0.20	-34.2	7.1	1689	19.1	70.4
G216	01/21/2015	--	--	7.3	817	--	--
G216	04/10/2015	--	--	6.9	1070	--	--
G216	07/22/2015	--	--	7.0	1093	--	--
G216	10/07/2015	--	--	7.0	981	--	--
G217	01/21/2015	--	--	7.2	802	--	--
G217	04/10/2015	--	--	6.8	1049	--	--
G217	07/22/2015	--	--	7.0	1135	--	--
G217	10/07/2015	--	--	7.1	1219	--	--
G218	01/21/2015	--	--	7.2	817	--	--
G218	04/10/2015	--	--	6.8	1038	--	--
G218	07/22/2015	--	--	6.8	1002	--	--
G218	10/07/2015	--	--	7.0	980	--	--
G218	11/24/2015	8.30	-22	7.1	1080	15.8	623
G218	02/19/2016	0	28	7.0	1130	13.6	--
G218	05/10/2016	0	-7	7.0	1130	19.8	18
G218	07/30/2016	0	72	7.0	1040	18.5	62.4
G218	11/23/2016	0	76	7.1	945	14.6	40.9
G218	02/18/2017	0	57	7.2	1100	14.7	37.2
G218	05/22/2017	0	62	7.1	1150	15.8	32.4
G218	07/17/2017	0	59	7.1	990	20.0	52.7
G218	10/31/2017	0	69	6.9	1080	13.5	51.9
G218	01/26/2018	0	71	6.9	990	11.9	40.2
G218	05/15/2018	0	81	7.0	1030	15.4	52.1
G218	11/02/2018	0	61	6.9	1080	14.5	51.8
G218	01/17/2019	0	73	7.0	1061	12.5	51.8
G218	08/14/2019	0	69	7.0	1028	16.9	70.6
G218	01/22/2020	1.20	33.9	7.1	1125	13.2	23.2
G218	08/13/2020	0.70	9.6	7.1	1132	20.2	1.3
G218	01/26/2021	1.40	10.5	7.0	1162	12.2	108
MW16S	04/09/2015	--	--	7.2	659	--	--
R104	01/20/2015	--	--	7.8	591	--	--
R104	04/08/2015	--	--	7.4	793	--	--
R104	07/23/2015	--	--	7.4	724	--	--
R104	10/06/2015	--	--	7.6	659	--	--
R104	11/17/2015	8.20	149	7.4	820	17.1	7.6
R104	02/17/2016	7.30	166	8.1	839	10.9	--
R104	05/16/2016	2.67	169	7.4	776	17.6	5.8
R104	08/03/2016	0	119	7.3	695	19.1	15.2

TABLE 4-2. GROUNDWATER FIELD PARAMETERS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Dissolved Oxygen (mg/L)	Oxidation Reduction Potential (mV)	pH (field) (SU)	Specific Conductance (micromhos/cm)	Temperature (deg. C)	Turbidity (NTU)
R201	01/20/2015	--	--	7.2	445	--	--
R201	04/10/2015	--	--	6.8	1212	--	--
R201	07/22/2015	--	--	6.9	1102	--	--
R201	10/05/2015	--	--	7.0	1165	--	--
R201	11/23/2015	0	-69	7.3	985	15.6	80
R201	02/12/2016	11.00	101	7.0	1390	10.3	4.8
R201	05/10/2016	0	-106	7.0	1420	15.5	55
R201	07/30/2016	0	-96	7.1	1520	16.9	57.7
R201	11/18/2016	0	-78	7.2	1350	15.3	45
R201	02/11/2017	0	-57	7.1	1292	15.0	50.2
R201	05/18/2017	0	-59	7.2	1150	14.8	57.5
R201	07/13/2017	0	-98	7.0	1495	17.0	59
R201	10/28/2017	0	-99	7.1	1300	12.6	60.5
R201	01/25/2018	0	56	7.0	1500	11.5	55.9
R201	05/11/2018	0	-88	7.1	1490	13.9	53.9
R201	11/02/2018	0	-91	7.1	1590	14.0	68.7
R201	01/16/2019	0	-94	7.1	1500	12.7	65.8
R201	08/12/2019	0	-93	7.1	1483	17.4	49.6
R201	01/21/2020	0.82	-115	7.2	1198	9.0	5.76
R201	08/11/2020	1.80	-80.1	6.9	1390	18.4	120
R201	01/29/2021	0.27	-32.1	7.0	1106	8.5	65.9
R201	03/29/2021	0.39	-56.5	7.0	1164	13.3	9.32
R201	04/21/2021	0.69	-44.5	6.9	1043	11.0	8.93
R201	05/06/2021	4.80	-42.4	7.2	1236	11.9	119
R201	05/17/2021	0.06	-109	7.0	1278	13.7	4.34
R201	06/14/2021	0.04	-120	7.0	1353	19.9	1.43
R201	06/29/2021	0.06	-116	7.1	1339	23.3	130
R201	07/13/2021	0.38	-27.1	7.0	1145	15.8	14.4
R201	07/28/2021	0.23	-122	7.2	1226	20.0	8.99

Notes:

Field readings are reported with as many significant figures as provided by analytical laboratory.

-- = data not available

cm = centimeter

deg. C = degrees Celsius

mg/L = milligrams per liter

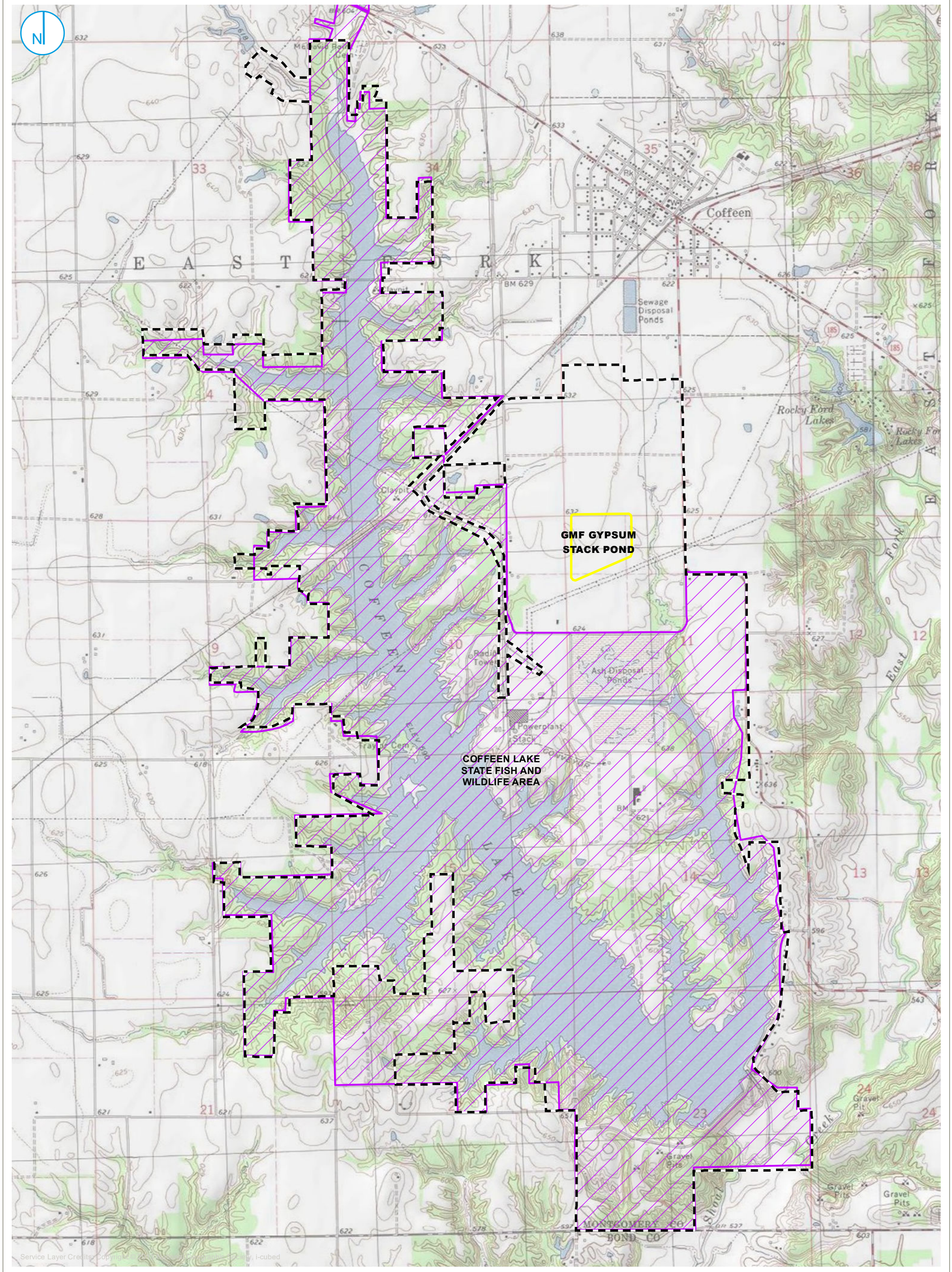
mV = millivolts

NTU = nephelometric turbidity units

SU = standard units

generated 10/22/2021, 11:50:42 AM CDT

FIGURES



- PART 845 REGULATED UNIT (SUBJECT UNIT)
- PROPERTY BOUNDARY
- COFFEEN LAKE STATE FISH AND WILDLIFE AREA

SITE LOCATION MAP

FIGURE 1-1

0 1,000 2,000
Feet

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
 ENGINEERING SOLUTIONS, INC.





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- COAL MINE SHAFT
- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- ▨ LIMITS OF FINAL COVER
- PROPERTY BOUNDARY

0 275 550
Feet

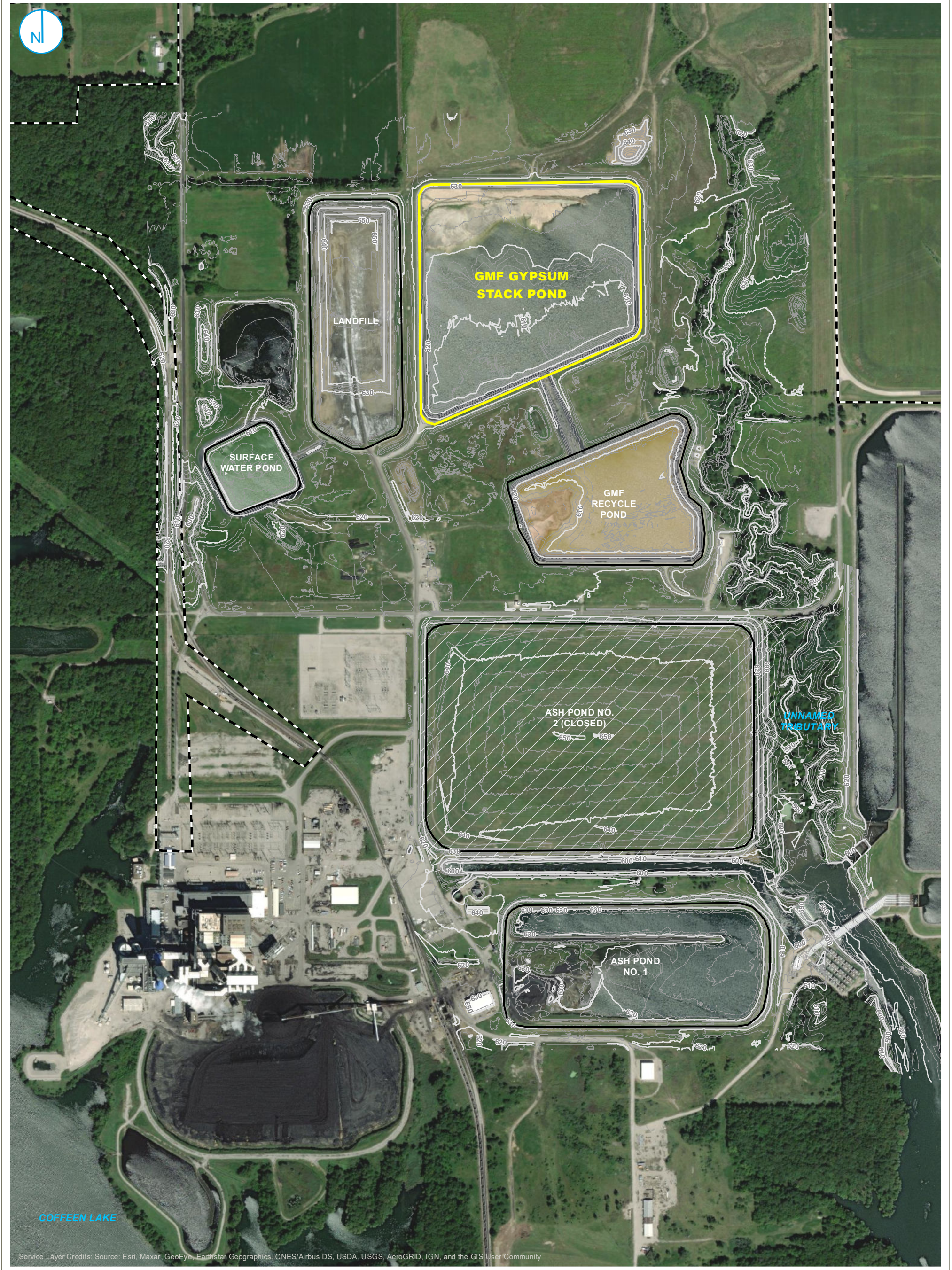
SITE MAP

FIGURE 1-2

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





- 10-FT ELEVATION CONTOUR
- 2-FT ELEVATION CONTOUR
- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- LIMITS OF FINAL COVER
- PROPERTY BOUNDARY

NOTE
 ELEVATION CONTOURS SHOWN IN FEET,
 NORTH AMERICAN VERTICAL DATUM OF
 1988

SITE TOPOGRAPHIC MAP

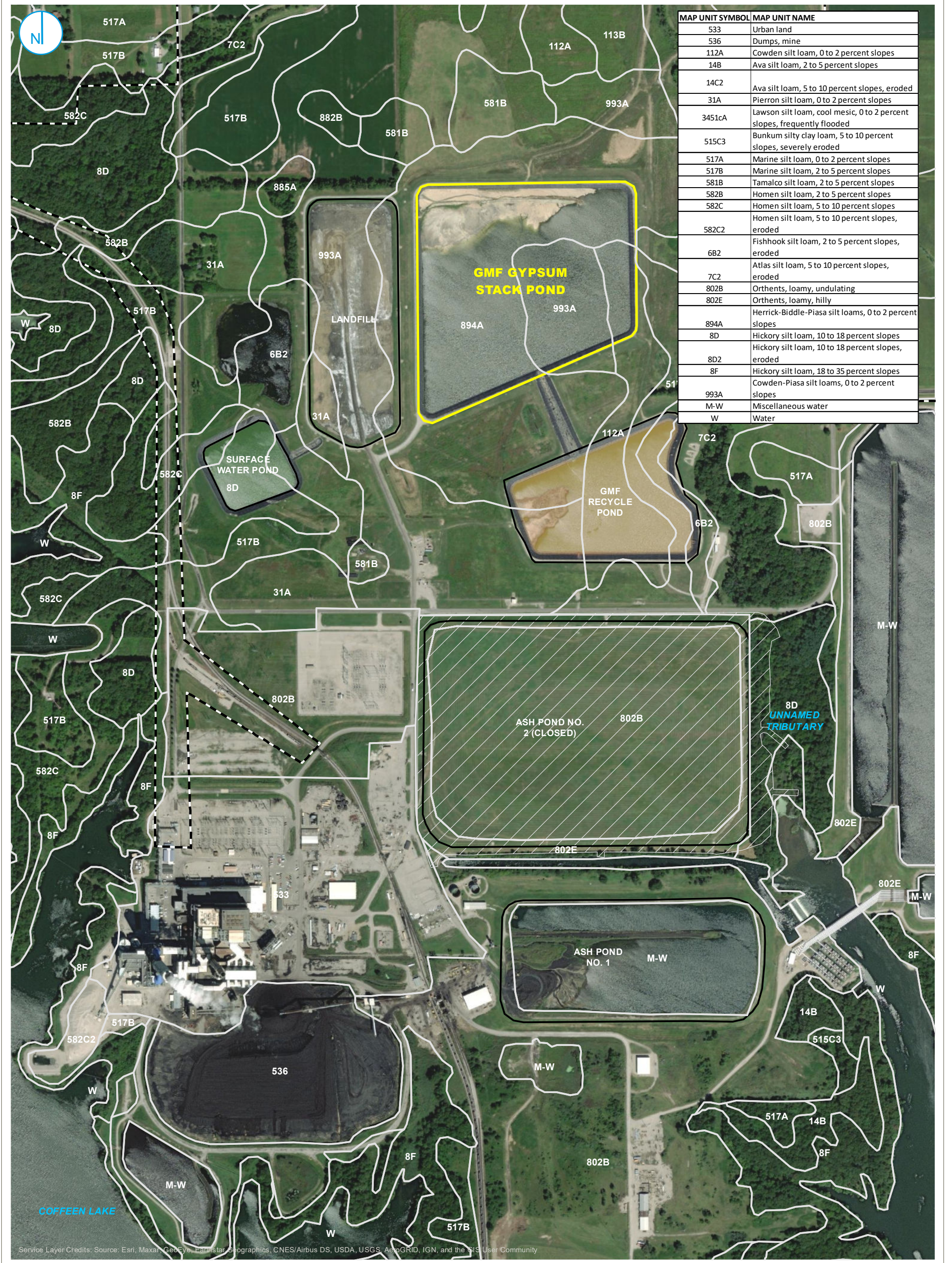
FIGURE 2-1

0 275 550 Feet
 SOURCE
 INGENAE SURVEY, 2021

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
 ENGINEERING SOLUTIONS, INC.





- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- LIMITS OF FINAL COVER
- PROPERTY BOUNDARY
- NRCS SOIL SURVEY MAP UNIT BOUNDARY

SOIL SURVEY MAP

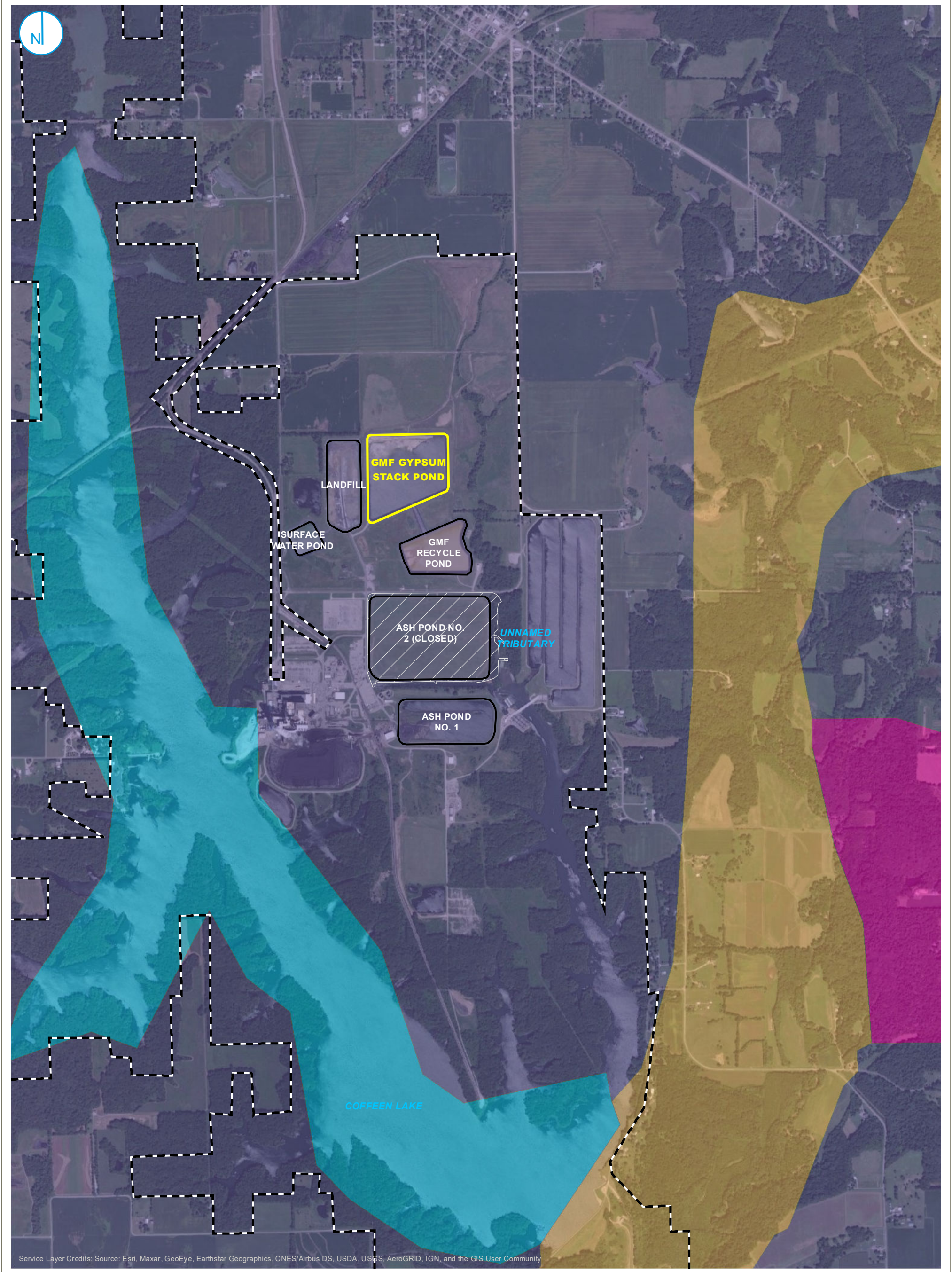
FIGURE 2-2

0 275 550 Feet

**HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS**

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- LIMITS OF FINAL COVER
- PROPERTY BOUNDARY
- CAHOKIA ALLUVIUM (INCLUDES ALLUVIAL FAN FACIES)
- HAGARSTOWN MEMBER
- VANDALIA TILL MEMBER
- WATER

0 750 1,500 Feet
SOURCE: ILLINOIS STATE GEOLOGICAL SURVEY (ISGS)

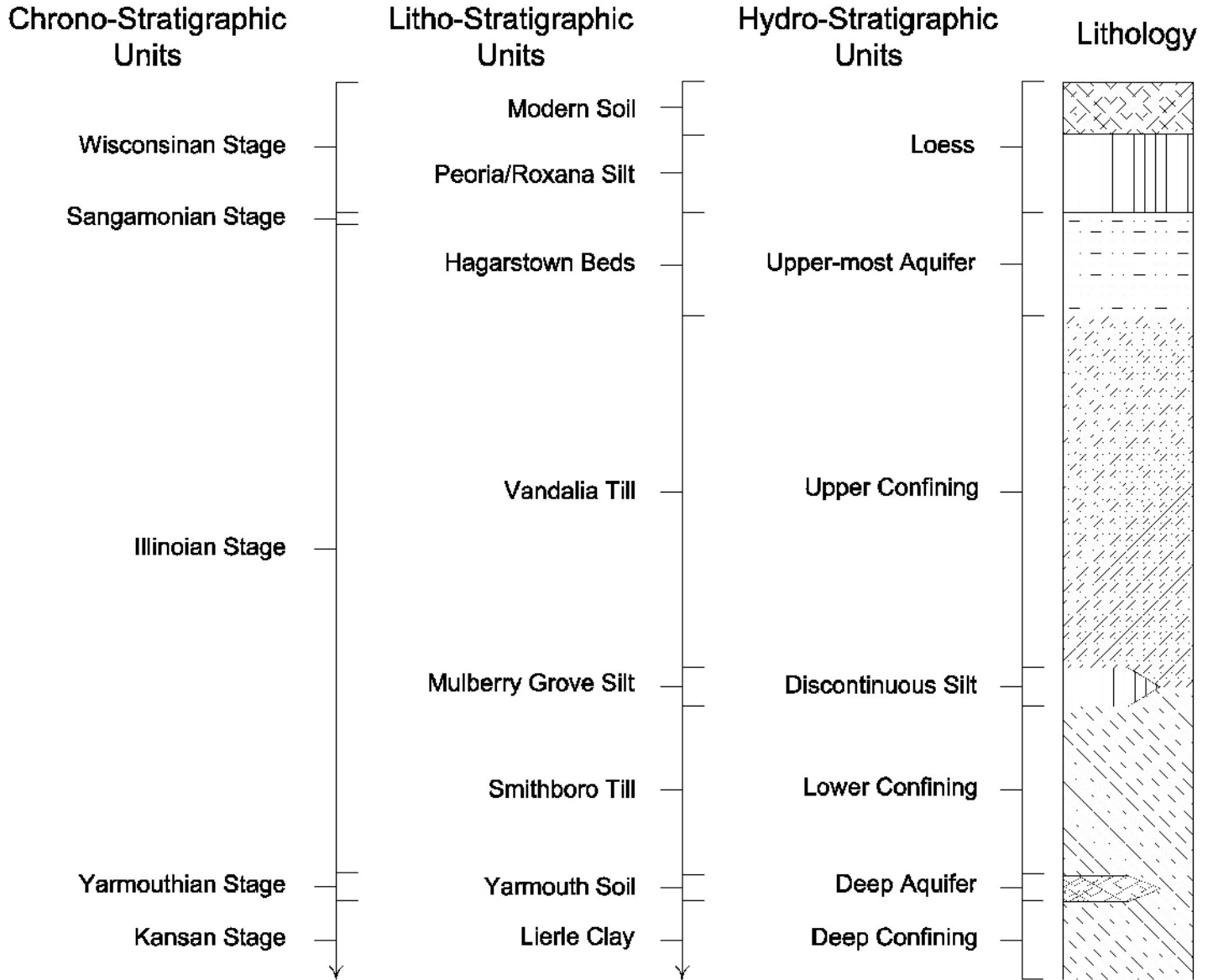
SURFICIAL GEOLOGIC DEPOSITS

FIGURE 2-3

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
 ENGINEERING SOLUTIONS, INC.



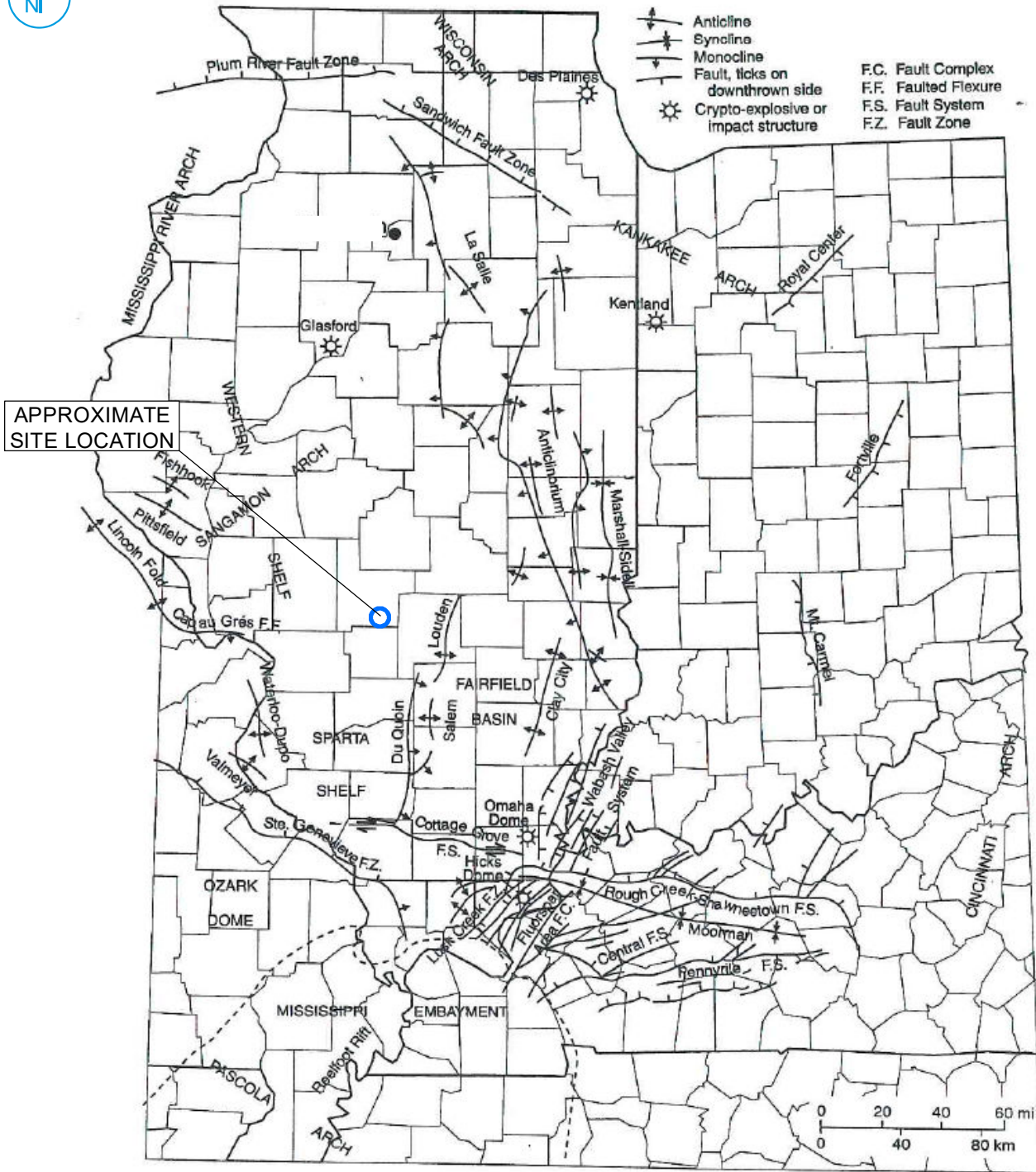


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Service Layer Credits:

GENERALIZED STRATIGRAPHIC COLUMN

FIGURE 2-4



SOURCE NOTE: MODIFIED FROM "NELSON, W.J. 1995, STRUCTURAL FEATURES IN ILLINOIS, ILLINOIS STATE GEOLOGICAL SURVEY, BULLETIN 100, CHAMPAIGN, ILLINOIS."








Service Layer Credits:

MAJOR STRUCTURAL FEATURES OF ILLINOIS

FIGURE 2-5



Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

-  MONITORING WELL
-  CCR SAMPLE
-  SOIL BORING
-  PART 845 REGULATED UNIT (SUBJECT UNIT)
-  SITE FEATURE
-  LIMITS OF FINAL COVER
-  PROPERTY BOUNDARY

0 275 550
 Feet

FIELD INVESTIGATION LOCATION MAP

FIGURE 2-6

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
 ENGINEERING SOLUTIONS, INC.





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- 10 FOOT HISTORIC ELEVATION CONTOUR
- 2 FOOT HISTORIC ELEVATION CONTOUR
- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- LIMITS OF FINAL COVER
- PROPERTY BOUNDARY

0 275 550
Feet

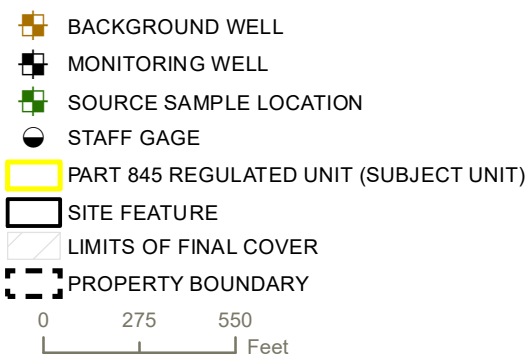
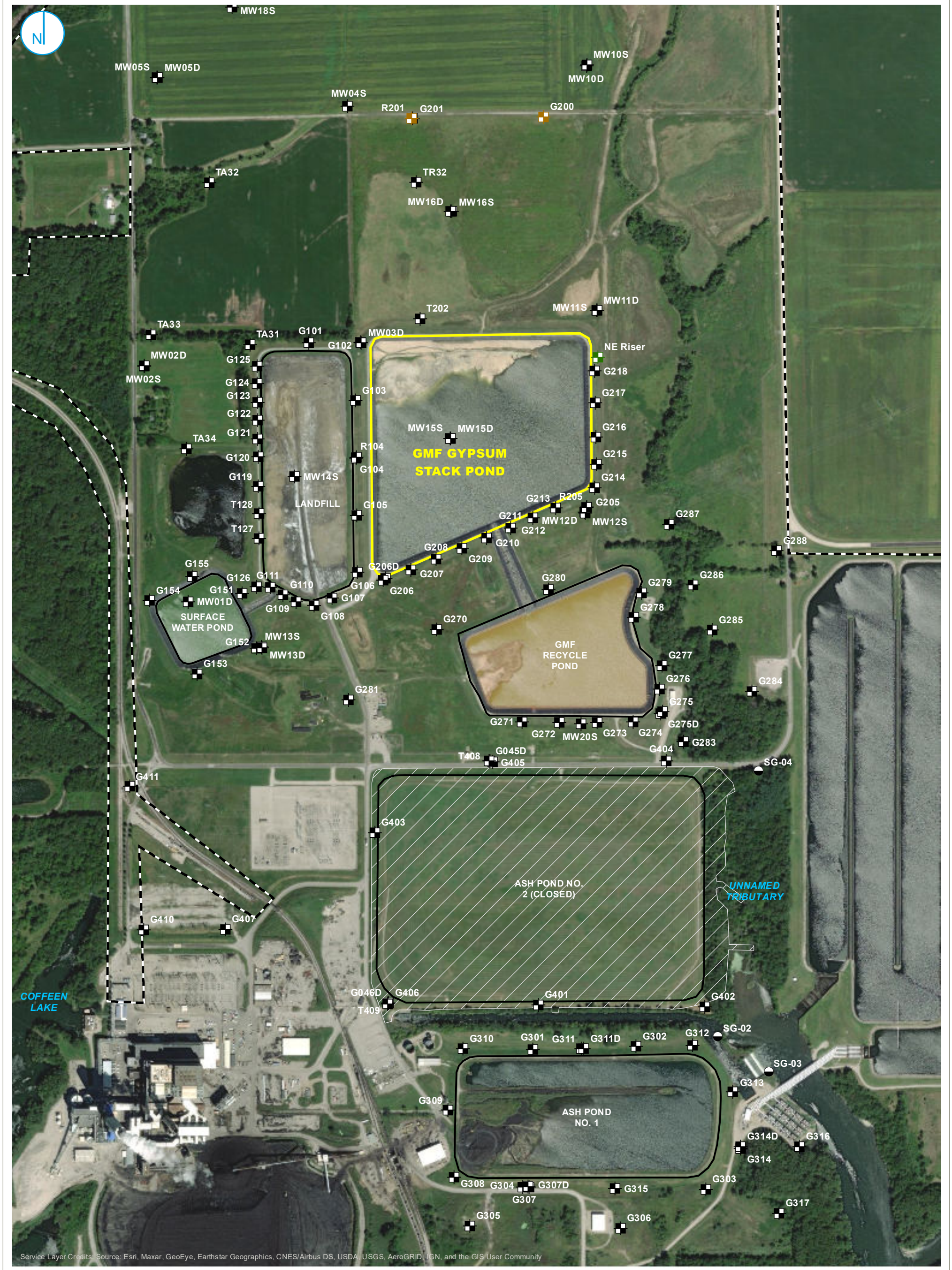
BOTTOM OF CCR MAP

FIGURE 2-8

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





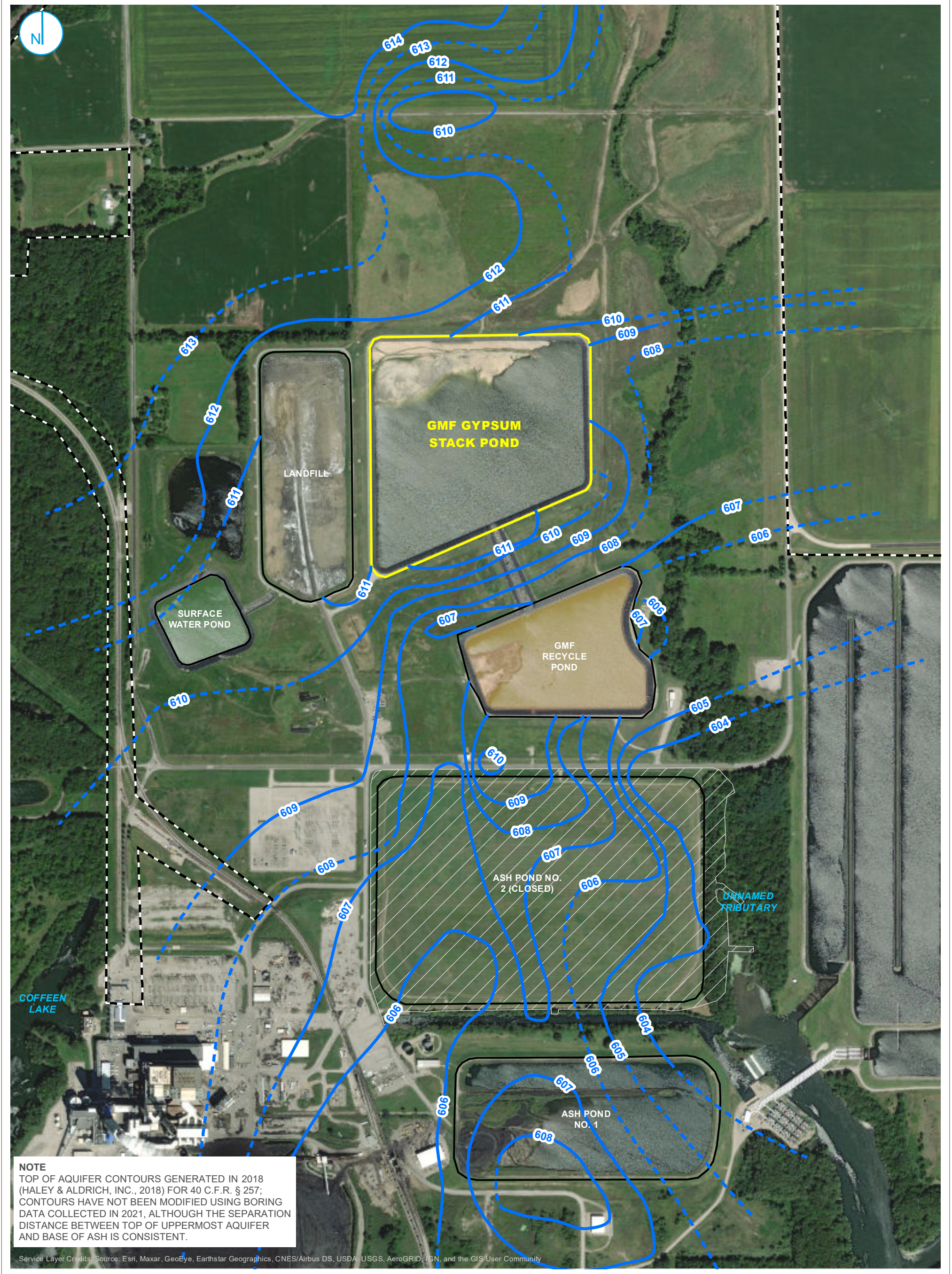
MONITORING WELL LOCATION MAP

FIGURE 3-1

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
 ENGINEERING SOLUTIONS, INC.





- HAGARSTOWN MEMBER ELEVATION CONTOUR (1-FT INTERVAL, NAVD88)
- - - INFERRED HAGARSTOWN MEMBER ELEVATION CONTOUR
- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- LIMITS OF FINAL COVER
- PROPERTY BOUNDARY

0 275 550
 Feet

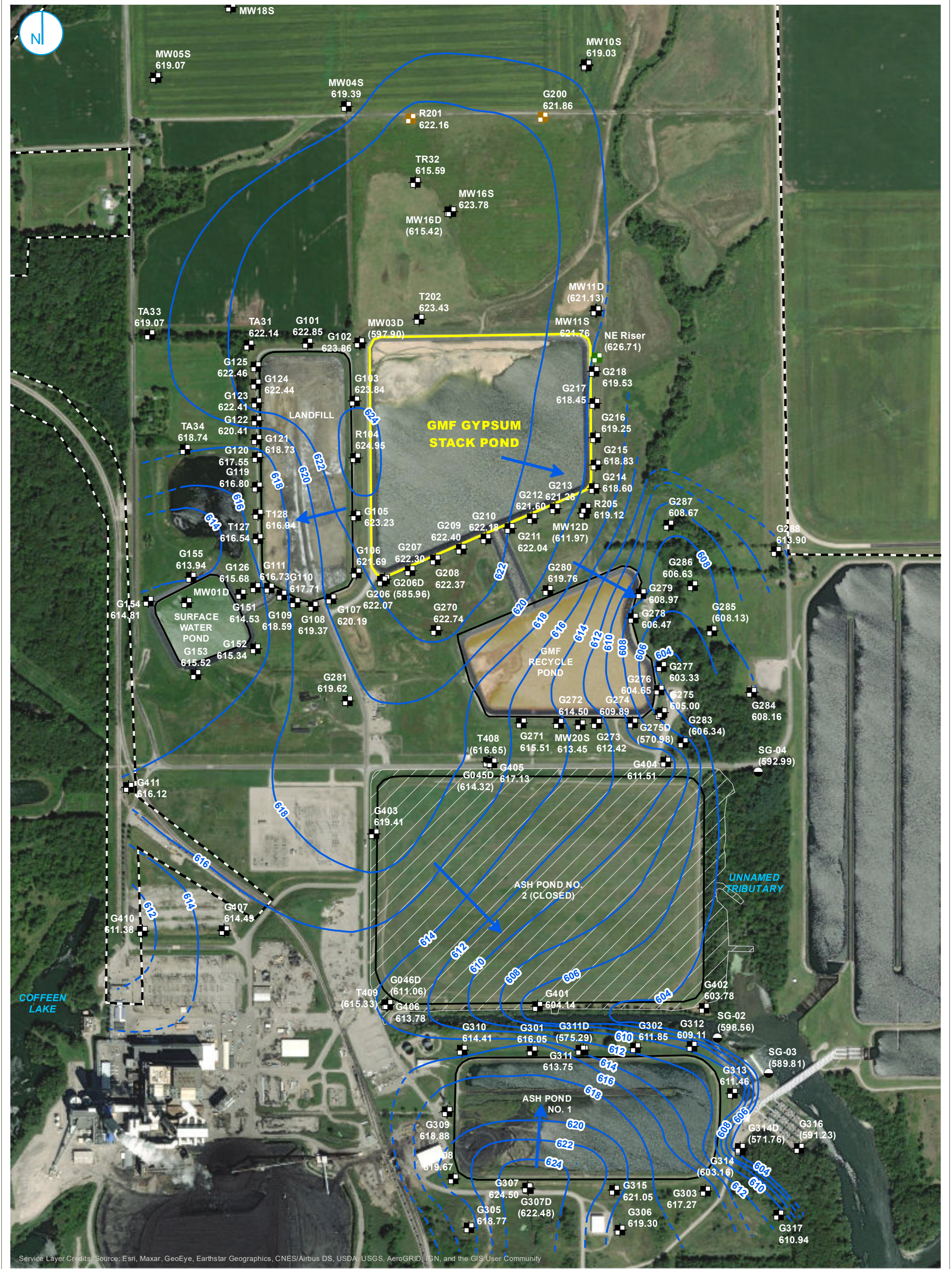
TOP OF UPPERMOST AQUIFER

FIGURE 3-2

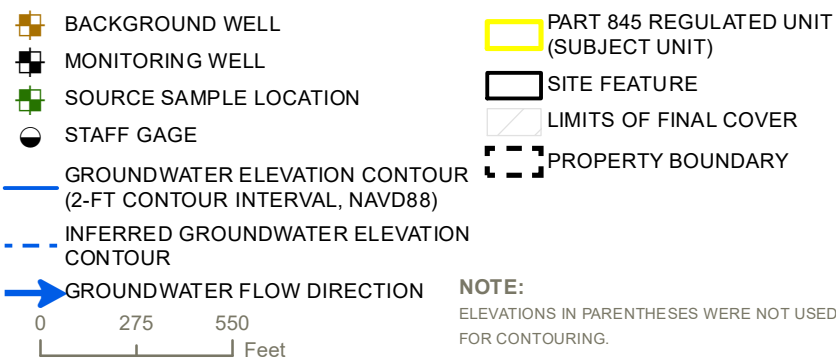
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
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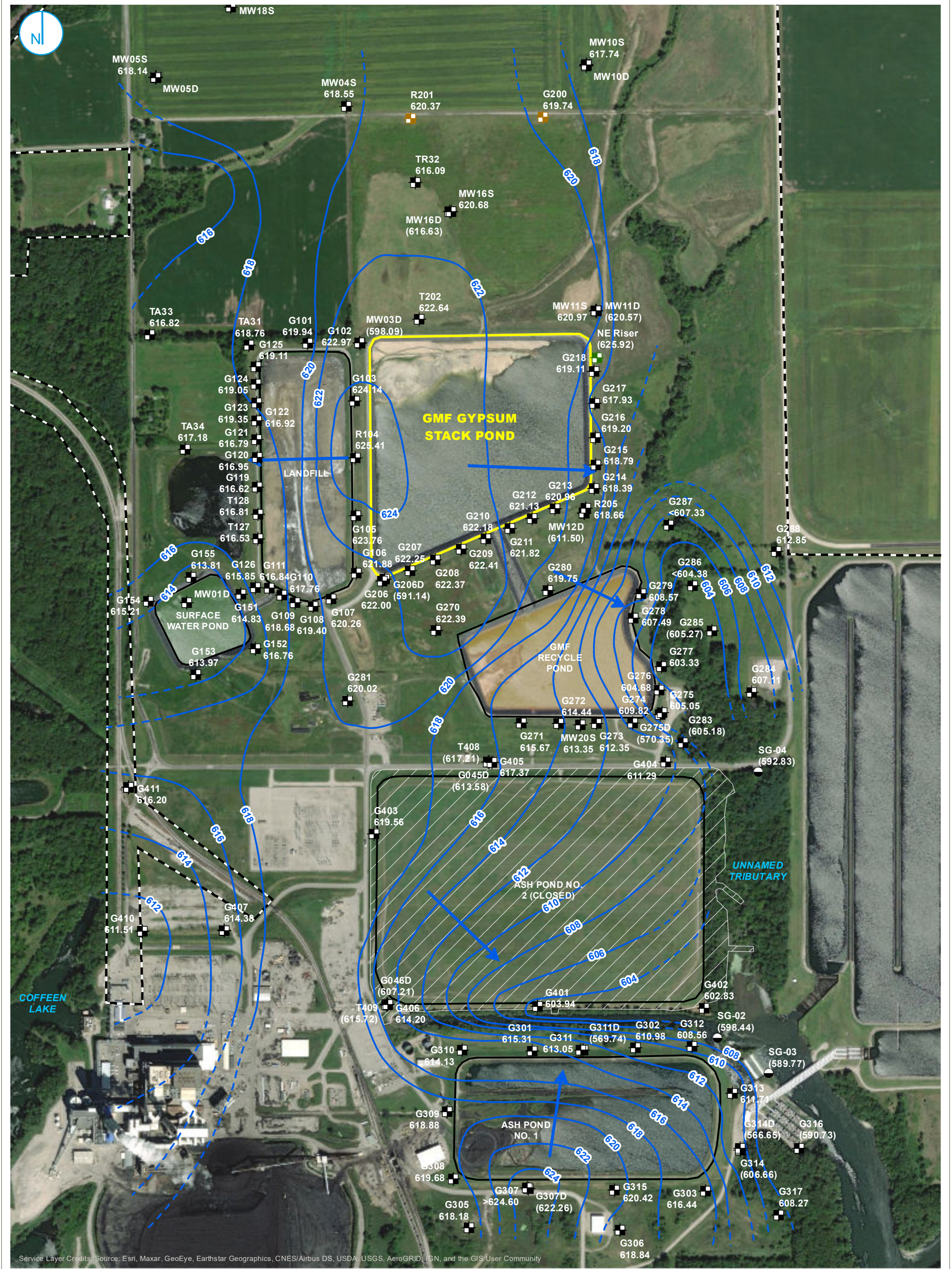
UPPERMOST AQUIFER GROUNDWATER ELEVATION CONTOURS
APRIL 20, 2021

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

FIGURE 3-3

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

	BACKGROUND WELL		PART 845 REGULATED UNIT (SUBJECT UNIT)
	MONITORING WELL		SITE FEATURE
	SOURCE SAMPLE LOCATION		LIMITS OF FINAL COVER
	STAFF GAGE		PROPERTY BOUNDARY
	GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)		
	INFERRED GROUNDWATER ELEVATION CONTOUR		
	GROUNDWATER FLOW DIRECTION		

NOTE: ELEVATIONS IN PARENTHESES WERE NOT USED FOR CONTOURING.

0 275 550 Feet

UPPERMOST AQUIFER GROUNDWATER ELEVATION CONTOURS JULY 26, 2021

FIGURE 3-4

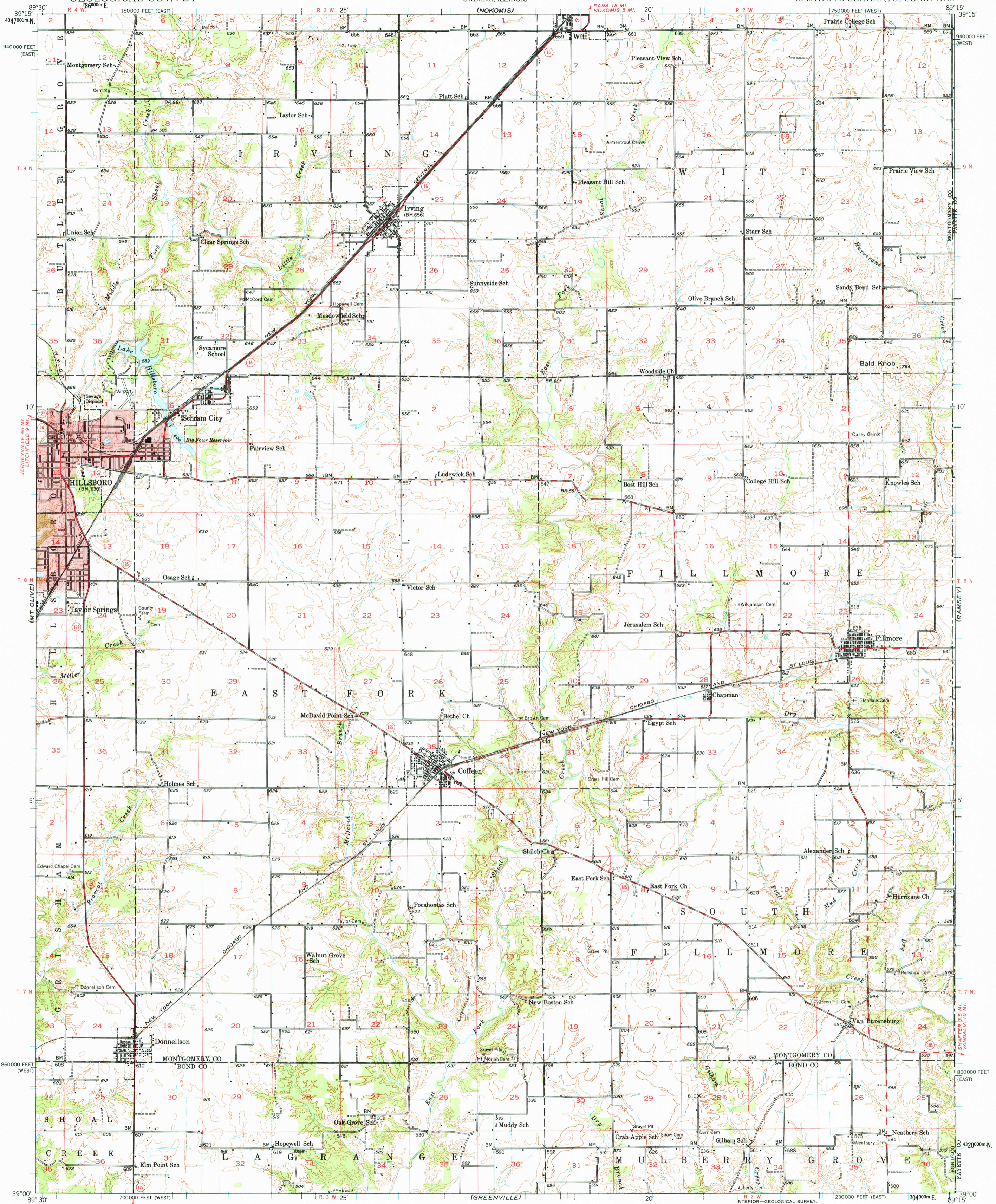
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
GMF GYPSUM STACK POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.



APPENDICES

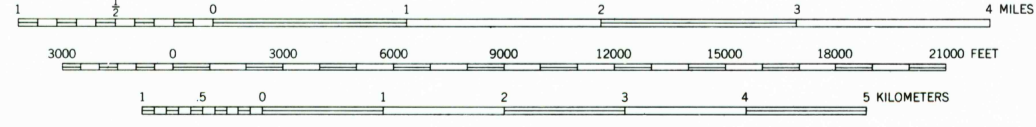
APPENDIX A
HISTORIC TOPOGRAPHIC MAP



Mapped, edited, and published by the Geological Survey
Control by USGS and USC&GS
Culture and drainage in part compiled from
aerial photographs taken 1938
Topography by plane-table methods 1947
Polyconic projection. 1927 North American datum
10,000-foot grids based on Illinois coordinate system,
east and west zones
Red tint indicates area in which only
landmark buildings are shown
1000-meter Universal Transverse Mercator grid ticks,
zone 16, shown in blue

TRUE NORTH
MAGNETIC NORTH
APPROXIMATE MEAN
DECLINATION, 1947

SCALE 1:62500



CONTOUR INTERVAL 20 FEET
DASHED LINES REPRESENT HALF-INTERVAL CONTOURS
DATUM IS MEAN SEA LEVEL

ROAD CLASSIFICATION
1963
Heavy-duty ——— Light-duty ———
Medium-duty ——— Unimproved dirt - - - - -
○ State Route



HILLSBORO, ILL.
N3900—W8915/15
1947

THIS MAP COMPLIES WITH NATIONAL MAP ACCURACY STANDARDS
FOR SALE BY U. S. GEOLOGICAL SURVEY, WASHINGTON 25, D. C.
AND BY THE STATE GEOLOGICAL SURVEY, URBANA, ILLINOIS
A FOLDER DESCRIBING TOPOGRAPHIC MAPS AND SYMBOLS IS AVAILABLE ON REQUEST

APPENDIX B
INFORMATION PERTINENT TO 35 I.A.C. § 845.220(a)(3)

Stack Unit Map

- (P,Q,R,p*,q* or r*) Glassford Formation
- 98
- Bedrock_Topography

IDOT_Roadways

- Interstate
- Freeway and Expressway
- Other Principal Arterial
- Minor Arterial
- Major Collector
- Minor Collector
- Local Road or Street

Ponds

- Gypsum Stack Pond
- Recycle Pond
- 1000m Pond Buffer

Sand Units

- Alluvium/Coluvium [surficial sand]
- Formation Sands [subsurface sand]

National Hydrography Dataset Flowlines

- Perennial
- Intermittent
- Ephemeral

National Wetlands Inventory

- Lake
- Riverine

Managed Lands

- State Trust Land, Protected Biodiversity Area
- Coffeen Lake State Fish and Wildlife Area

Underground Coal Mines

- Underground Coal Mines
- Abandoned Coal Mine Shafts

Completed Quadrangle Mine Outlines

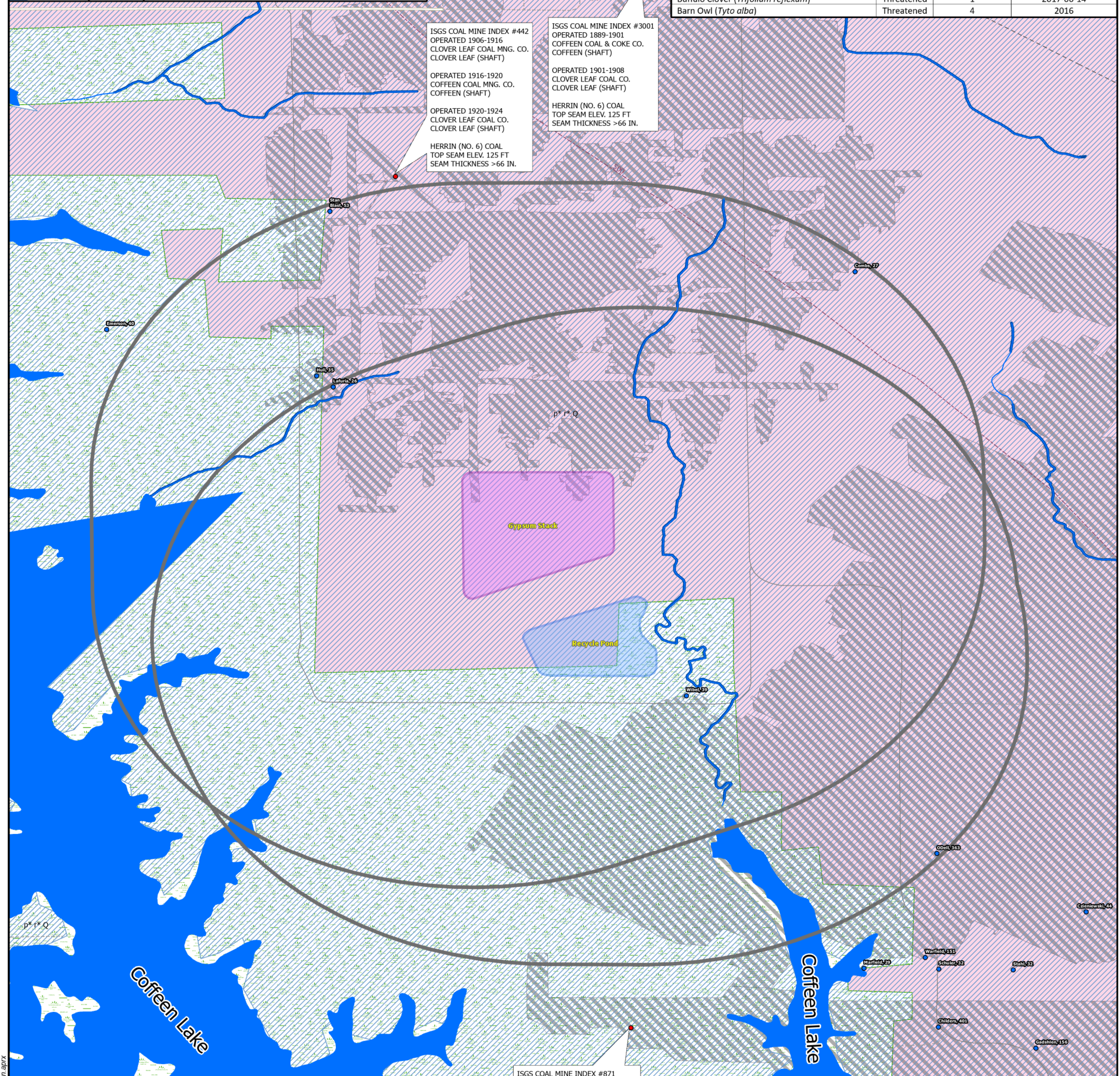
- Final
- Not Final

Water Wells*

- Water Wells*

Watershed:
HUC 8 - 07140203 (Shoal)
916 square miles

Montgomery County Threatened and Endangered Species			
Federally Listed Species			
Species	Status	Range	Habitat
Indiana Bat (<i>Myotis sodalis</i>)	Endangered	Montgomery	Caves, mines (hibernacula); small stream corridors with well-developed riparian woods, upland forests (foraging)
Northern Long-Eared Bat (<i>Myotis septentrionalis</i>)	Threatened	Montgomery	Caves, mines (hibernacula); swarming in surrounding woods in autumn, upland forests and woods (roosting/foraging)
State Listed Species			
Species	Status	# of Occurrences	Last Observed
Swamp Metalmark (<i>Calephelis muticum</i>)	Endangered	1	2003-08-09
Northern Harrier (<i>Circus cyaneus</i>)	Endangered	1	2015-02-15
Bunchflower (<i>Melanthium virginicum</i>)	Threatened	1	2017-06-14
Prairie Rose Gentian (<i>Sabatia campestris</i>)	Endangered	1	PRE-2012
Royal Catchfly (<i>Silene regia</i>)	Endangered	1	2002-07-18
Eastern Blue-eyed Grass (<i>Sisyrinchium atlanticum</i>)	Endangered	2	2002-06-12
Ornate Box Turtle (<i>Terrapene ornata</i>)	Threatened	1	2013-06-03
Buffalo Clover (<i>Trifolium reflexum</i>)	Threatened	1	2017-06-14
Barn Owl (<i>Tyto alba</i>)	Threatened	4	2016



ISGS COAL MINE INDEX #442
 OPERATED 1906-1916
 CLOVER LEAF COAL MNG. CO.
 CLOVER LEAF (SHAFT)

OPERATED 1916-1920
 COFFEEN COAL MNG. CO.
 COFFEEN (SHAFT)

OPERATED 1920-1924
 CLOVER LEAF COAL CO.
 CLOVER LEAF (SHAFT)

HERRIN (NO. 6) COAL
 TOP SEAM ELEV. 125 FT
 SEAM THICKNESS >66 IN.

ISGS COAL MINE INDEX #3001
 OPERATED 1889-1901
 COFFEEN COAL & COKE CO.
 COFFEEN (SHAFT)

OPERATED 1901-1908
 CLOVER LEAF COAL CO.
 CLOVER LEAF (SHAFT)

HERRIN (NO. 6) COAL
 TOP SEAM ELEV. 125 FT
 SEAM THICKNESS >66 IN.

ISGS COAL MINE INDEX #871
 OPERATED 1964-1970
 TRUAX TRAEER COAL CO.
 HILLSBORO (SHAFT)

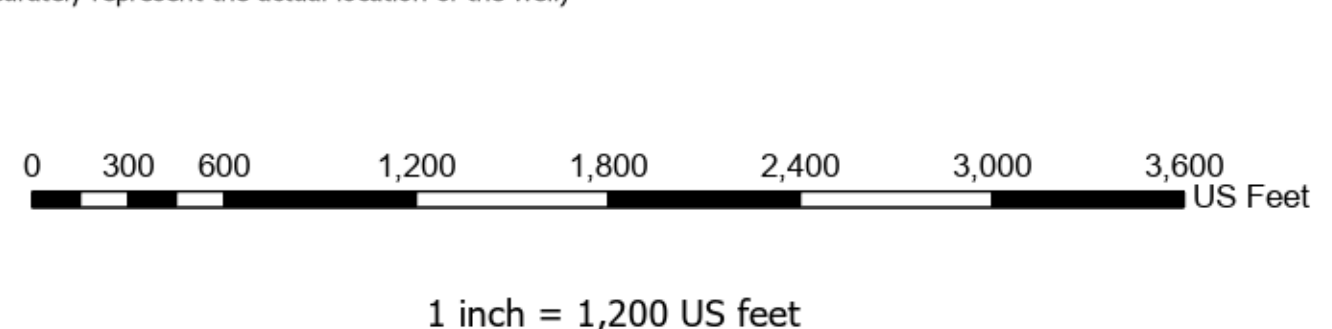
OPERATED 1971-1983
 CONSOLIDATION CC, MIDWEST DIV.
 HILLSBORO (SHAFT)

HERRIN (NO. 6) COAL
 TOP SEAM ELEV. 125 FT
 SEAM THICKNESS >66 IN.

Illinois Natural Heritage Database		
Illinois Natural Areas Inventory (INAI) sites		
INAI Name	INAI Number	Categories
Shoal Creek Barrens	0752	I, II
Gillespie Railroad Prairie	1337	II, III
Irving Railroad Prairie	0751	I
Roberts Cemetery Savanna	0750	I, II, III
INAI Category Descriptions:		
Cat. I = High quality natural community and natural community restorations		
Cat. II = Specific suitable habitat for state-listed species or state-listed species relocations		
Cat. III = State dedicated Nature Preserves, Land and Water Reserves, & Natural Heritage Landmarks		
Illinois Nature Preserves		
INPC Protected Areas	INPC Number	Distance from Site
Roberts Cemetery Savanna Nature Preserve	NP152	17 miles NW
Gillespie Prairie Land and Water Reserve	LWR012	22 miles W
Land Protection Programs:		
LWR = Land and Water Reserve		
NHL = Natural Heritage Landmark		
NP = Nature Preserve		

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*ISGS Water Well locations are provided by the original well driller to the nearest quarter, quarter section, and may not accurately represent the actual location of the well)



Gypsum Management Facility

Work Plan Desktop Evaluation
Coffeen Power Station
Coffeen, Montgomery Co., Illinois

Job Number: 20E0111A Sheet 1

WATER WELL SURVEY

Water Well	Top	Bottom
clay	0	26
Total Depth		26
Casing: 30" CONCRETE from 1' to 26'		
Water from clay at 0' to 0'.		
Driller's Log filed		
Owner Address: ,		
Location source: Location from permit		

Permit Date:

Permit #:

COMPANY Bekemeyer, Gust

FARM Lafuria, Mick

DATE DRILLED February 4, 1971

NO.

ELEVATION 0

COUNTY NO. 01725

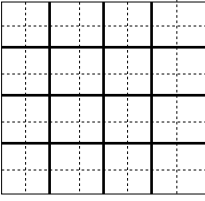
LOCATION NW SE SE

LATITUDE 39.07411

LONGITUDE -89.402784

COUNTY Montgomery

API 121350172500



3 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Private Water Well	Top	Bottom
brown top soil	0	1
yellow clay	1	7
yellow clay & sand	7	12
yellow gravel & sand	12	14
gray clay & sand	14	35
Total Depth		35
Casing: 36" CONCRETE from -1' to 35'		
Grout: CONCRETE from 0 to 10.		
Grout: GRAVEL from 10 to 35.		
Size hole below casing: 0"		
Water from gravel & sand at 13' to 14'.		
Owner Address: R.R. #1 Box #121 Coffeen, IL		
Location source: Location from permit		

Permit Date: September 22, 1983

Permit #: 109475

COMPANY Kohnen, Clarence

FARM Hull, Steve

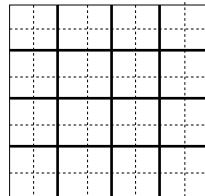
DATE DRILLED October 17, 1983 NO.

ELEVATION 0 COUNTY NO. 22828

LOCATION 210'N line, 190'W line of SE SE

LATITUDE 39.074433 LONGITUDE -89.403283

COUNTY Montgomery API 121352282800



3 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Private Water Well	Top	Bottom
brown top soil	0	1
yellow clay	1	8
yellow clay & sand	8	18
yellow gravel & sand	18	30
yellow sand & gravel	30	39
Total Depth		39
Casing: 36" CONCRETE from -1' to 39'		
" from 0' to 0'		
Grout: CONCRETE from 0 to 10.		
Grout: GRAVEL from 10 to 39.		
Size hole below casing: 0"		
Water from at 22' to 39'.		
Permanent pump installed at 38'		
on December 1, 1981, with a capacity of 10 gpm		
Owner Address: 109 Laredo Ave. St. Louis, MO		
Location source: Location from permit		

Permit Date: November 13, 1981

Permit #: 102105

COMPANY Kohnen, Clarence

FARM Wibel, William

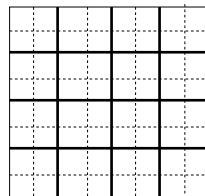
DATE DRILLED November 30, 1981 NO.

ELEVATION 0 COUNTY NO. 22832

LOCATION 115'S line, 102'W line of SE SE NW

LATITUDE 39.064493 LONGITUDE -89.391801

COUNTY Montgomery API 121352283200



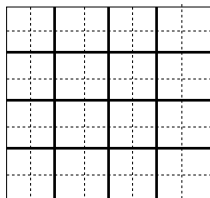
11 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
clay	0	3
silt	3	33
clay	33	35
Total Depth		35
Casing: 2" SCH 40 PVC from 0' to 23'		
Screen: 10' of 2" diameter .01 slot		
Grout: CEMENT/BENT from 0 to 19.		
Size hole below casing: 7.87"		
Water from at 0' to 10'.		
Static level 12' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL		
Location source: Location from the driller		

Permit Date: Permit #: none

COMPANY Fox Drilling
FARM White & Brewer
DATE DRILLED November 18, 1993 **NO.** PZ-2
ELEVATION 621GL **COUNTY NO.** 23615
LOCATION 1470'S line, 300'W line of NE
LATITUDE 39.068212 **LONGITUDE** -89.388821
COUNTY Montgomery **API** 121352361500



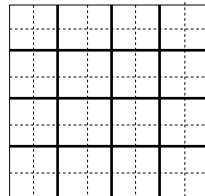
11 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
sandy clay	0	3
silty lclay	3	22
sandy, silty clay	22	23
Total Depth		23
Casing: 2" SCH 40 PVC from 0' to 12'		
Screen: 10' of 2" diameter .01 slot		
Grout: CEMENT/BENT 5% from 0 to 12.		
Size hole below casing: 7.87"		
Water from at 0' to 8'.		
Static level 10' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL		
Location source: Location from the driller		

Permit Date: Permit #: none

COMPANY Fox Drilling
FARM White & Brewer
DATE DRILLED NO. PZ-12
ELEVATION 624GL **COUNTY NO.** 23618
LOCATION 910'N line, 530'E line of section
LATITUDE 39.068862 **LONGITUDE** -89.382456
COUNTY Montgomery **API** 121352361800



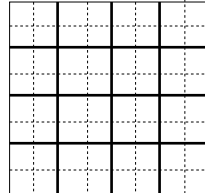
11 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
silty clay	0	10
silty sand	10	14
sandy silt	14	29
silty clay	29	33
clayey silt	33	40
Total Depth		40
Casing: 2" SCH 5 STAINLESS STL from 0' to 30'		
Screen: 10' of 2" diameter .01 slot		
Grout: CEMENT/BENT 5% from 0 to 27.		
Size hole below casing: 7.87"		
Water from at 0' to 5'.		
Static level 7' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL		
Location source: Location from the driller		

Permit Date: Permit #: none

COMPANY Fox Drilling
 FARM White & Brewer
 DATE DRILLED NO. G-119A
 ELEVATION 623GL COUNTY NO. 23619
 LOCATION 1060'N line, 730'E line of section
 LATITUDE 39.068452 LONGITUDE -89.383158
 COUNTY Montgomery API 121352361900



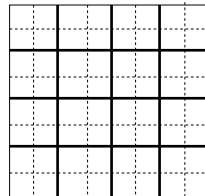
11 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
clayey silt	0	1
silty clay	1	10
silty sand	10	14
sandy silt	14	20
Total Depth		20
Casing: 2" SCH 40 PVC from 0' to 9'		
Screen: 10' of 2" diameter .01 slot		
Grout: CEMENT/BENT 5% from 0 to 4.		
Size hole below casing: 7.87"		
Water from at 0' to 3'.		
Static level 9' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL		
Location source: Location from the driller		

Permit Date: Permit #: none

COMPANY Fox Drilling
FARM White & Brewer
DATE DRILLED NO. G119B
ELEVATION 622GL **COUNTY NO.** 23620
LOCATION 1080'N line, 750'E line of section
LATITUDE 39.068397 **LONGITUDE** -89.383229
COUNTY Montgomery **API** 121352362000



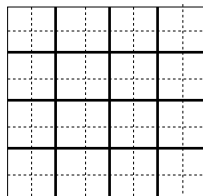
11 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Monitoring	Top	Bottom
clayey silt	0	4
silty clay	4	18
sandy silt	18	25
sand	25	31
clayey silt	31	33
Total Depth		33
Casing: 2" SCH 5 STAINLESS STL from 0' to 21'		
Screen: 10' of 2" diameter .01 slot		
Grout: CEMENT/BENT 5% from 0 to 16.		
Size hole below casing: 7.87"		
Water from at 0' to 2'.		
Static level 4' below casing top which is 2' above GL		
Owner Address: % White & Brewer Landfill Coffeen, IL		
Location source: Location from the driller		

Permit Date: Permit #: none

COMPANY Fox Drilling
 FARM White & Brewer
 DATE DRILLED NO. G-120A
 ELEVATION 623GL COUNTY NO. 23621
 LOCATION 1990'N line, 730'E line of section
 LATITUDE 39.065884 LONGITUDE -89.38313
 COUNTY Montgomery API 121352362100



11 - 7N - 3W

ILLINOIS STATE GEOLOGICAL SURVEY

Private Water Well	Top	Bottom
black dirt	0	1
clay	1	3
gravelly clay	3	13
hardpacked gravelly clay & layered gvl	13	19
layered gray gravelly sand & gry gvly cl	19	27
Total Depth		27
Casing: 6" ASTM F480 SDR 21 from 0' to 11' 36" CONCRETE from 11' to 27'		
Grout: BENTONITE PLUG from 10 to 11.		
Water from gravelly clay at 11' to 13'.		
Static level 12' below casing top which is 1' above GL		
Owner Address: 1756 Red Ball Trail Coffeen, IL		
Address of well: same as above		
Location source: Location from permit		

Permit Date: June 25, 1998

Permit #: 135-001

COMPANY Walters, Steven

FARM Combs, Jack

DATE DRILLED July 15, 1998

NO.

ELEVATION 0

COUNTY NO. 23873

LOCATION NE NW SE

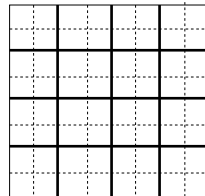
LATITUDE 39.077695

LONGITUDE -89.386518

COUNTY Montgomery

API 121352387300

2 - 7N - 3W



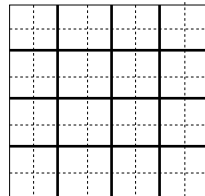
ILLINOIS STATE GEOLOGICAL SURVEY

Semi-Private Water Well	Top	Bottom
coal mine mat coal, concrete, wood, slag	0	5
yellow & gray clay mix	5	7
yellow & gray sandy clay	7	10
gravelly clay	10	17
hardpacked gravelly clay	17	21
gray gravelly clay	21	30
layered gray sand & gray gravelly clay	30	38
gray gravelly clay	38	53
Total Depth		53
Casing: 6" ASTM F480 SDR 21 from 0' to 12' 36" CONCRETE from 0' to 53'		
Grout: CONCRETE from 9 to 12.		
Grout: BENT HOLE PLUG from 11 to 12.		
Water from gravelly clay at 13' to 17'.		
Static level 16' below casing top which is 3' above GL		
Owner Address: % Fairmont Minerals 796 CIPS Trail Coffeen, IL		
Address of well: same as above		
Location source: Location from permit		

Permit Date: June 9, 1999

Permit #: 135-001

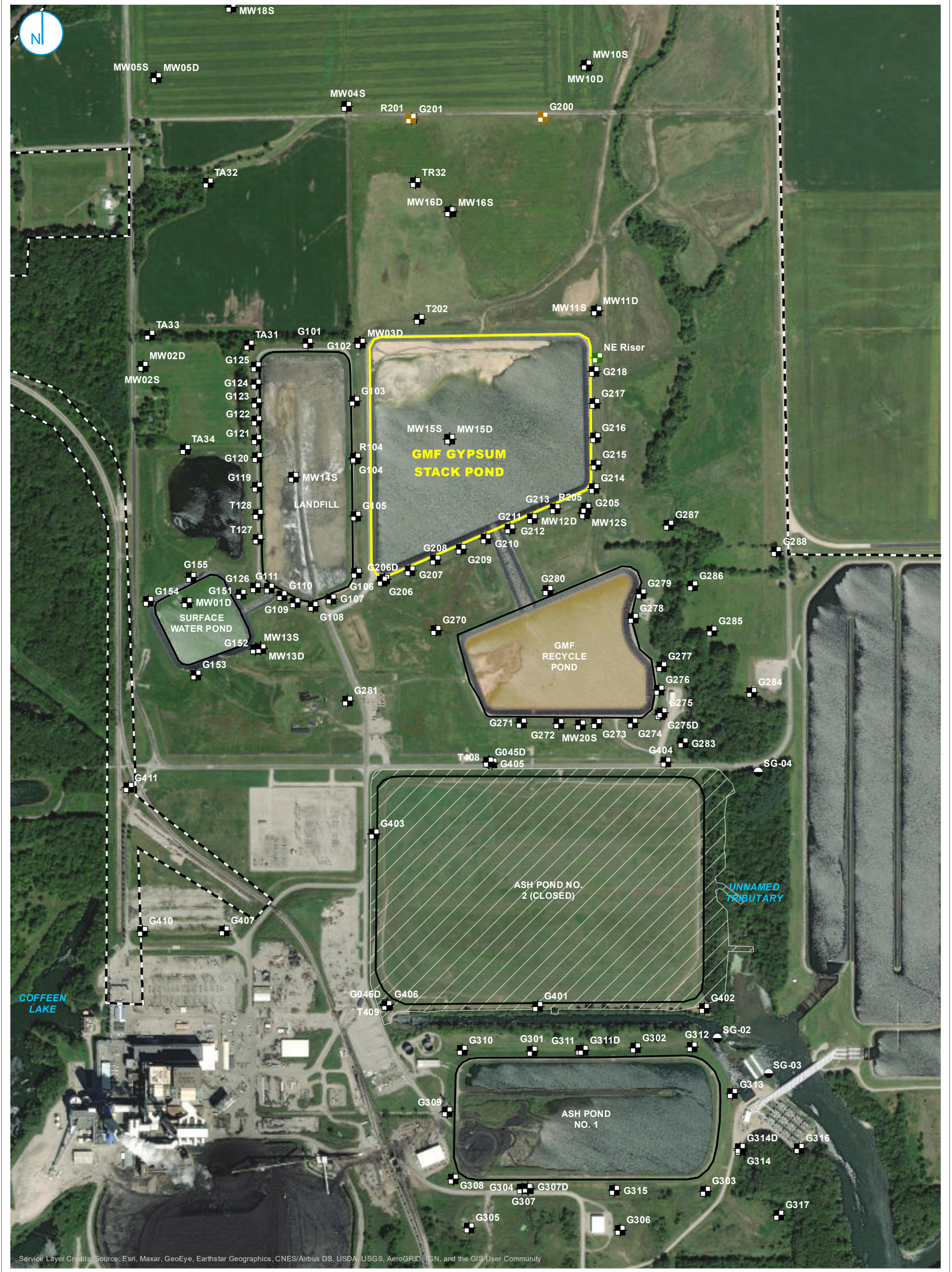
COMPANY Walters, Steven
 FARM Stan Blast Abrasives
 DATE DRILLED June 14, 1999 NO. 1
 ELEVATION 0 COUNTY NO. 23908
 LOCATION SW SE NE
 LATITUDE 39.079557 LONGITUDE -89.402861
 COUNTY Montgomery API 121352390800



3 - 7N - 3W

**APPENDIX C
BORING AND WELL CONSTRUCTION LOGS**

BORING AND WELL LOCATIONS MAP



MONITORING WELL LOCATION MAP

FIGURE C1

BORING LOGS

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/16/2016
Finish: 8/17/2016
WEATHER: Cloudy, rain, (hi-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G405D
Well ID: G45D
Surface Elev: 620.94 ft. MSL
Completion: 42.00 ft. BGS
Station: 873,998.03N
 2,515,322.23E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen Township: East Fork Section 11, Tier 7 N.; Range 3 W.	▼ = 16.00 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0%	BD							2	Black (10YR2/1), moist, soft, clayey SILT (TOPSOIL)		620	
	0/60 0%	BD							4	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY		618	
									6	Gray (10YR6/1), moist, firm, silty CLAY slight trace sand		616	
	0/60 0%	BD							6	Gray (10YR5/1), very moist, soft, clayey, very fine- to fine-grained SAND			
									8	Gray (10YR6/1) with 25% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel		614	
									10	Yellowish brown (10YR5/8) with 40% gray (10YR6/1) mottles, moist, firm, silty CLAY with sand and trace gravel		612	
	0/48 0%	BD							12	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND		610	
									12	Dark brown (10YR3/3), very moist, soft, clayey, fine- to very coarse-grained SAND with slight trace gravel			
									12	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND			
									13	Dark yellowish brown (10YR4/4), moist, soft, sandy SILT with trace gravel		608	
									13	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND			
1A	24/24 100%	SS	6-23 37-44 N=60		11	1.50			14	Gray (10YR5/1), moist, very hard, very silty CLAY with sand and gravel		606	
									16	Dark gray (10YR4/1), moist, hard, SILT with few fine- to coarse-grained sand, little clay and trace small gravel.		604	
2A	23/24 96%	SS	14-32 41-45 N=73		7	4.50			18	Dark gray (10YR4/1), dry, hard, SILT with few fine- to coarse-grained sand, little clay and trace small gravel.		602	
3A	14/17 82%	SS	16-47 50/5"		7	4.50			20				

NOTE(S): G45D installed in boring.
 Boring was blind drilled to 14.0 feet bgs. Blind drill lithologies from boring G405.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/16/2016
Finish: 8/17/2016
WEATHER: Cloudy, rain, (hi-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G405D
Well ID: G45D
Surface Elev: 620.94 ft. MSL
Completion: 42.00 ft. BGS
Station: 873,998.03N
 2,515,322.23E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
4A	19/24 79%	SS	15-36 32-36 N=68	8	4.50		22	Dark gray (10YR4/1), dry, hard, SILT with few fine-to coarse-grained sand, little clay and little small gravel. Dark gray (10YR4/1), moist, dense, fine- to coarse-grained SAND.		600	
5A	19/24 79%	SS	10-18 30-33 N=48	8	4.50		24	Dark gray (10YR4/1), dry, hard, SILT with few fine- to coarse-grained sand, little clay and little small gravel.		598	
6A	11/24 46%	SS	22-42 34-36 N=76	8	4.50		26			596	
7A	11/24 46%	SS	28-26 23-26 N=49	11	4.50		28	Dark gray (10YR4/1) with 5% light brownish gray (10YR6/2) mottles, dry, hard, SILT with few fine- to coarse-grained sand, little clay and little small gravel.		594	
8A	23/24 96%	SS	7-11 16-25 N=27	10	4.50		30	Dark gray (10YR4/1) with 5% light brownish gray (10YR6/2) and dark greenish gray (10YR4/2) mottles, dry, hard, SILT with few fine- to coarse-grained sand, little clay and little small gravel.		592	
9A				16			30	Dark gray (10YR4/1), dry, hard, SILT with few fine- to coarse-grained sand, little clay and little small gravel.			
9B	17/24 71%	SS	7-14 12-12 N=26	14			30	Dark gray (10YR4/1), dry, very stiff, SILT with some very fine-grained SAND.		590	
9C				9			32	Very dark gray (10YR3/1), moist, very stiff, CLAY with few silt and little medium-grained sand.			
10A	24/24 100%	SS	2-5 8-13 N=13	15	3.25		34	Very dark gray (10YR3/1), moist, very stiff, CLAY with little silt and little medium-grained sand.		588	
11-1				15			34				
11-2	22/24 92%	SH		15			36			586	
11-3							36				
11-4							36				
12A	24/24 100%	SS	2-5 8-10 N=13	16	2.00		38	Very dark gray (10YR3/1), moist, very stiff, CLAY with little silt, little medium-grained sand, and trace small gravel.		584	
13A	22/24 92%	SS	2-5 7-8 N=12	16	2.00		40	Very dark gray (10YR3/1) with 5% black (10YR2/1) mottles, moist, very stiff, CLAY with little silt, little medium-grained sand, and trace small gravel.		582	

NOTE(S): G45D installed in boring.
 Boring was blind drilled to 14.0 feet bgs. Blind drill lithologies from boring G405.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/16/2016
Finish: 8/17/2016
WEATHER: Cloudy, rain, (hi-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G405D
Well ID: G45D
Surface Elev: 620.94 ft. MSL
Completion: 42.00 ft. BGS
Station: 873,998.03N
 2,515,322.23E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
14A	23/24 96%	ss	1-3 7-7 N=10	16		3.50	42	Very dark gray (10YR3/1) with 5% black (10YR2/1) mottles, moist, very stiff, CLAY with little silt, little medium-grained sand, and trace small gravel. [Continued from previous page]		580	
End of Boring = 42.0 ft. BGS											

NOTE(S): G45D installed in boring.
 Boring was blind drilled to 14.0 feet bgs. Blind drill lithologies from boring G405.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 5/1/2007
Finish: 5/1/2007
WEATHER: Partly sunny, warm

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" Hollow stem auger with split spoon sampler
FIELD STAFF: Driller: A. Rachford
Helper: M. Brown
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB21 Formerly MW21S
Well ID: G405
Surface Elev: 620.90 ft. MSL
Completion: 14.21 ft. BGS
Station: 873,996.79N
 2,515,335.70E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
							Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		Water Level Information: ▼ = 1.28 - 05/10/2007 ▼ = ▼ =		
1A	19/24 79%	ss	43		0.78		0	Black (10YR2/1), moist, soft, clayey SILT (TOPSOIL)		620	
1B			26		1.94		2				
2A	12/12 100%	ss	27		2.52		4	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY		618	
3A			24		3.92		4				
3B	24/24 100%	ss	24		2.33		6	Gray (10YR6/1), moist, firm, silty CLAY slight trace sand		616	
4A	24/24 100%	ss	20		2.33		6	Gray (10YR5/1), very moist, soft, clayey, very fine- to fine-grained SAND		614	
5A	24/24 100%	ss	24		1.55		8	Gray (10YR6/1) with 25% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel		612	
6A	24/24 100%	ss	19				10	Yellowish brown (10YR5/8) with 40% gray (10YR6/1) mottles, moist, firm, silty CLAY with sand and trace gravel		610	
6B			18				10	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND		610	
7A	24/24 100%	ss	9		7.42		12	Dark brown (10YR3/3), very moist, soft, clayey, fine- to very coarse-grained SAND with slight trace gravel		608	
							12	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND			
							12	Dark yellowish brown (10YR4/4), moist, soft, sandy SILT with trace gravel			
							12	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND			
							14	Gray (10YR5/1), moist, very hard, very silty CLAY with sand and gravel			

End of Boring = 14.2 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G406D
Well ID: G46D
Surface Elev: 621.91 ft. MSL
Completion: 52.00 ft. BGS
Station: 872,519.70N
 2,514,697.78E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	4-3 4-6 N=7	11		1.50	1.50	Dark brown (10YR3/3), moist, stiff, SILT with few clay and trace organics.			
1B							3.00				
2A	12/24 50%	ss	4-6 3-3 N=9	12		2.50	2.50	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay and trace small gravel.		620	
2B							2.00				
3A	3/24 13%	ss	3-3 4-7 N=7	24			4.00	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay. Grayish brown (10YR5/2) with 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with few silt and little fine-grained sand.		618	
							6.00	Very pale brown (10YR7/4) with 25% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with trace silt.		616	
4A	20/24 83%	ss	2-3 4-5 N=7	21		1.25	8.00	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt and trace fine- to medium-grained sand.		614	
5A	19/24 79%	ss	1-3 4-6 N=7	18		1.75	10.00	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt, little fine- to medium-grained sand and trace small gravel.		612	
6A	23/24 96%	ss	2-2 4-5 N=6	18		2.50	12.00	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		610	
7A	21/24 88%	ss	1-3 4-5 N=7	16		1.00	14.00	Gray (10YR6/1) with 25% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		608	
8A	23/24 96%	ss	1-2 2-2 N=4	18		0.75	16.00	Gray (10YR6/1) with 10% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with some fine- to medium-grained sand, little silt, and trace small gravel.		606	
8B							16.00	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, wet, loose, fine-grained SAND with some clay.		606	
9A	22/24 92%	ss	4-13 27-23 N=40	8			18.00	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, moist, loose, fine-grained SAND with some clay.		604	
9B							18.00			604	
10A	17/24 71%	ss	13-31 33-42 N=64	7		4.50	20.00	Gray (10YR5/1), dry, hard, SILT with few clay, few fine- to coarse-grained sand and trace small gravel.		602	

NOTE(S): G46D installed in boring.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G406D
Well ID: G46D
Surface Elev: 621.91 ft. MSL
Completion: 52.00 ft. BGS
Station: 872,519.70N
 2,514,697.78E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen Township: East Fork Section 11, Tier 7 N.; Range 3 W.	▼ = Dry - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11-1	3/3 100%	SH											
	8/24 33%	DS											
12-1									22	Gray (10YR5/1), dry, hard, SILT with few clay, few fine- to coarse-grained sand and trace small gravel. <i>[Continued from previous page]</i>		600	
13-1													
13-2	24/24 100%	DS											
13-3													
13-4									24			598	
14A				19						Pale brown (10YR6/3), wet, loose, fine-grained SAND.			
14B	21/21 100%	SS	7-30 32-20 N=62	7		4.50				Pale brown (10YR6/3), wet, loose, fine-grained SAND and small GRAVEL.			
14C				8					26	Gray (10YR5/1), dry, hard, SILT with some clay, trace fine- to coarse-grained sand and trace small gravel.		596	
15A	12/24 50%	SS	15-16 10-11 N=26	10		4.50			28	Dark gray (10YR4/1), dry, hard, CLAY with some silt, trace fine- to coarse-grained sand and trace small gravel.		594	
16A	17/24 71%	SS	2-5 7-11 N=12	16		3.00			30	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		592	
17A	17/24 71%	SS	2-5 7-12 N=12	15		2.50			32	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small to large gravel.		590	
18A	12/24 50%	SS	3-7 12-20 N=19	17		2.50			34	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		588	
19A	23/24 96%	SS	4-4 6-9 N=10	25		1.00			36	Dark gray (10YR4/1) with 5% very dark gray (10YR3/1) and brown (10YR5/3) mottles, moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		586	
20A	22/24 92%	SS	2-2 5-7 N=7	17		1.00			38	Gray (10YR5/1) with 5% very dark gray (10YR3/1) and brown (10YR5/3) mottles, moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		584	
21A	24/24 100%	SS	2-3 4-6 N=7	17		1.00			40	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		582	

NOTE(S): G46D installed in boring.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G406D
Well ID: G46D
Surface Elev: 621.91 ft. MSL
Completion: 52.00 ft. BGS
Station: 872,519.70N
 2,514,697.78E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
22A	24/24 100%	SS	1-3 5-6 N=8	17	1.50		42			580	
23-1											
23-2	24/24 100%	SH		16							
23-3											
23-4							44			578	
24A	24/24 100%	SS	3-4 6-7 N=10	17	1.50		46	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel. <i>[Continued from previous page]</i>		576	
25A	24/24 100%	SS	2-2 5-7 N=7	18	1.25		48			574	
26A	24/24 100%	SS	2-5 6-8 N=11	17	1.75		50			572	
27A	23/24 96%	SS	2-3 9-12 N=12	15	3.50		52			570	
End of Boring = 52.0 ft. BGS											

NOTE(S): G46D installed in boring.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 2/2/2010
Finish: 2/2/2010
WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G101
Well ID: G101
Surface Elev: 625.27 ft. MSL
Completion: 21.92 ft. BGS
Station: 876,551.76N
 2,514,214.31E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	1-1 2-3 N=3	24			0	TOPSOIL - Brown (10YR5/3), moist, soft, silty CLAY with slight trace sand and gravel, roots. Dark grayish brown (10YR4/2), moist, soft, silty CLAY with slight trace sand, trace roots.		624	
2A	18/24 75%	ss	1-3 3-5 N=6	30			2	Dark grayish brown (10YR4/2) with 15% yellowish brown (10YR5/4) mottles, moist, medium, silty CLAY, slight trace roots. Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/4) mottles, moist, medium, silty CLAY, slight trace roots.		622	
3A	19/24 79%	ss	2-3 4-4 N=7	26			4	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY, slight trace roots.		620	
4A	19/24 79%	ss	1-3 4-3 N=7	21			6	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.		618	
5A	22/24 92%	ss	1-3 3-4 N=6	23			8	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
6A	20/24 83%	ss	1-2 2-3 N=4	24			10	Gray (10YR6/1) with 35% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with trace sand and slight trace gravel.		614	
7A	22/24 92%	ss	1-2 3-2 N=5	17			12	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, very moist, medium, silty, sandy CLAY with trace gravel.		612	
8A 8B	14/24 58%	ss	1-2 5-8 N=7	15 13			14	Brown (10YR5/3), very moist, medium, silty, clayey, very fine- to coarse-grained SAND with slight trace gravel.		610	
9A	16/24 67%	ss	2-5 15-25 N=20	16			16	Brown (10YR5/3), very moist, loose, silty, very fine- to coarse-grained SAND with slight trace gravel.		608	
10A 10B	17/24 71%	ss	19-20 22-18 N=42	14 8			18	Brown (10YR5/3), very moist, dense, silty, very fine- to coarse-grained SAND with slight trace gravel.		606	

NOTE(S): G101 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/2/2010
Finish: 2/2/2010
WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G101
Well ID: G101
Surface Elev: 625.27 ft. MSL
Completion: 21.92 ft. BGS
Station: 876,551.76N
 2,514,214.31E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	16/23 70%	ss	2-16 42-60/5" N=58	8				Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 15.50 - While drilling ▽ = 12.38 - Upon Completion ▽ = 7.31 - 3/1/2010			
								Brown (10YR5/3), slightly moist, hard, clayey SILT with slight trace sand and gravel. Dark gray (10YR4/1), slightly moist, hard, clayey SILT with slight trace sand and slight trace gravel.		604		

End of Boring = 21.9 ft. BGS

NOTE(S): G101 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/28/2006
Finish: 4/28/2006
WEATHER: Partly cloudy, mild (mid-60's)

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-03a
Well ID: G102 (MW3S)
Surface Elev: 625.70 ft. MSL
Completion: 17.15 ft. BGS
Station: 876,554.77N
 2,514,531.48E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 14.00 - While drilling ▽ = 7.03 - 6/1/06 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24	SS	2-2 4-6 N=6	17	2.07	SP	Very dark grayish brown (10YR3/2), lean CLAY Grayish brown (10YR5/2), lean CLAY, trace sand Yellowish brown (10YR5/6), lean CLAY, trace sand Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, lean CLAY, trace sand Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, sandy SILT, trace gravel Yellowish brown (10YR5/6) with 50% gray (N5/1) mottles, lean CLAY, trace sand and gravel Yellowish brown (10YR5/6), lean CLAY, little sand, trace gravel Dark gray (10YR4/1), sandy CLAY, trace gravel Gray (10YR6/1) with 50% yellowish brown (10YR5/8) mottles, lean CLAY, little sand, trace gravel Yellowish brown (10YR5/8), silty, fine SAND, little medium sand, trace gravel, wet Yellowish brown (10YR5/8), silty, fine SAND, little clay, wet Yellowish brown (10YR5/6), lean CLAY, little sand, trace gravel Yellowish brown (10YR5/6), silty SAND, trace gravel, wet Yellowish brown (10YR5/4), clayey SILT, trace sand and gravel End of Boring = 17.15 ft. BGS		624 622 620 618 616 614 612 610				
1B	83%												
1C		26	3.30	B	2								
2A	17/24	71%	2-3 4-5 N=7	25	3.05	B					4		
3A	20/24	83%	2-3 3-5 N=6	16	1.96	B					6		
4A	24/24	100%	4-3 5-6 N=8	21	2.27	B					8		
5A	21/24	88%	1-3 3-4 N=6	20	2.18	B					10		
5B				19									
6A	18/24	75%	1-2 2-4 N=4	24	0.87	B					12		
7A	23/24	96%	3-2 2-4 N=4	19							14		
7B	16/24	67%		12									
8A	23/24	96%	3-12 29-50 N=41	13									
8B				13									
8C				12									
9A	12/12	100%	8-82	10	6.98	B					16		

NOTE(S): G102 (MW03S) installed in blind-drilled borehole within 10 ft of SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/27/2006
Finish: 4/27/2006
WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-03
Well ID: MW3D
Surface Elev: 626 ft. MSL
Completion: 58 ft. BGS
Station: 876,554.5N
 2,514,535.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 14.00 - While drilling ▽ = 7.03 - MW03S on 6/1/06 ▽ = 55.40 - MW03D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24	ss	2-2	17	2.07	SP	Very dark grayish brown (10YR3/2), lean CLAY Grayish brown (10YR5/2), lean CLAY, trace sand Yellowish brown (10YR5/6), lean CLAY, trace sand Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, lean CLAY, trace sand Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, sandy SILT, trace gravel Yellowish brown (10YR5/6) with 50% gray (N5/1) mottles, lean CLAY, trace sand and gravel Yellowish brown (10YR5/6), lean CLAY, little sand, trace gravel Dark gray (10YR4/1), sandy CLAY, trace gravel Gray (10YR6/1) with 50% yellowish brown (10YR5/8) mottles, lean CLAY, little sand, trace gravel Yellowish brown (10YR5/8), silty, fine SAND, little medium sand, trace gravel, wet Yellowish brown (10YR5/8), silty, fine SAND, little clay, wet Yellowish brown (10YR5/6), lean CLAY, little sand, trace gravel Yellowish brown (10YR5/6), silty SAND, trace gravel, wet Yellowish brown (10YR5/4), clayey SILT, trace sand and gravel Gray (10YR5/1), sandy SILT, trace gravel	2	624				
1B	83%		4-6 N=6	20									
1C				26	3.30	B							
2A	17/24	ss	2-3 4-5 N=7	25	3.05	B							
3A	20/24	ss	2-3 3-5 N=6	16	1.96	B							
4A	24/24	ss	4-3 5-6 N=8	21	2.27	B							
5A	21/24	ss	1-3 3-4 N=6	20	2.18	B							
5B				19									
6A	18/24	ss	1-2 2-4 N=4	24	0.87	B							
7A	23/24	SH	3-2	19									
7B	16/24	SS	2-4 N=4	12									
8A	23/24	ss	3-12 29-50 N=41	13									
8B				13									
8C				12									
9A	14/24	ss	8-82 85-72 N=167	10	6.98	B							
9B				13	6.18	BSh							
10A	18/24	ss	6-21 32-49 N=53	8	11.95	Sh							

Shelby tube taken from shallow well borehole at indicated depth.

NOTE(S): MW03D installed in SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/27/2006

WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-03

Well ID: MW3D

Surface Elev: 626 ft. MSL

Completion: 58 ft. BGS

Station: 876,554.5N

2,514,535.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 14.00 - While drilling ▽ = 7.03 - MW03S on 6/1/06 ▽ = 55.40 - MW03D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	19/24 79%	ss	3-20 35-29 N=55	10	5.36 Sh				22			604	Wood fragments
12A	20/24 83%	ss	15-25 89-69 N=114	8	13.00 Sh				24	Gray (10YR5/1), sandy SILT, trace gravel [Continued from previous page]		602	
13A	23/24 96%	ss	14-19 24-22 N=43	9					26	Yellowish brown (10YR5/4), lean CLAY, trace sand and gravel		600	
13B				13	6.98 BSh								
14A	24/24 100%	ss	19-21 26-32 N=47	15	7.01 BSh				28	Gray (10YR4/1), clayey SILT, trace sand and gravel		598	
15A	21/24 88%	ss	10-25 25-23 N=50	12						Light gray (10YR6/1), SILT, trace sand			
15B				13	8.53 BSh				30			596	
16A	24/24 100%	ss	7-12 19-30 N=31	13	9.16 BSh				32			594	
17A	24/24 100%	ss	29-35 39-42 N=74	14	6.59 B				34	Gray (10YR4/1), lean CLAY, trace sand and gravel		592	
17B				14	3.49 Sh								
18A	23/24 96%	ss	6-8 11-17 N=19	13	7.21 BSh				36			590	
19A	24/24 100%	ss	19-21 31-31 N=52	13	6.98 BSh				38	Gray (10YR4/1), clayey SILT, trace sand and gravel		588	
20A	24/24 100%	ss	6-10 15-24 N=25	14	6.76 BSh				40	Gray (10YR4/1), lean CLAY, trace sand and gravel		586	

NOTE(S): MW03D installed in SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/27/2006
Finish: 4/27/2006
WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-03
Well ID: MW3D
Surface Elev: 626 ft. MSL
Completion: 58 ft. BGS
Station: 876,554.5N
 2,514,535.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = 14.00 - While drilling ▽ = 7.03 - MW03S on 6/1/06 ▽ = 55.40 - MW03D on 6/1/06		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	19/24 79%	ss	4-10 16-23 N=26	13	8.04 BSh		42	Gray (10YR4/1), lean CLAY, trace sand and gravel [Continued from previous page]		584	
22A	21/24 88%	ss	19-27 28-32 N=55	13	7.56 BSh		44			582	
23A	23/24 96%	ss	4-9 14-18 N=23	14	6.98 BSh		46	Gray (10YR4/1), clayey SILT, trace sand and gravel		580	
24A	24/24 100%	ss	20-26 30-33 N=56	14	6.59 BSh		48			578	
25A	24/24 100%	ss	6-10 13-12 N=23	20	3.30 BSh		50			576	
26A	24/24 100%	ss	4-7 6-7 N=13	22	2.91 BSh		52	Gray (10YR4/1), lean CLAY, trace sand and gravel		574	
27A	24/24 100%	ss	7-18 37-85 N=55	16	4.05 BSh		54			572	
27B				13			54	Gray (10Y4/1), silty, fine to medium SAND, wet			
28A	23/24 96%	ss	15-34 34-19 N=68	11	3.22 SP		56	Gray (10YR4/1), silty, fine SAND, trace clay, wet		570	
28B				13	5.82 B						
29A	24/24 100%	ss	19-22 28-18 N=50	20	4.36 BSh		58	Very dark gray (10YR3/1), lean CLAY, trace sand and gravel		568	

End of Boring = 58.0 ft. BGS

NOTE(S): MW03D installed in SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/15/2010
 Finish: 2/15/2010
WEATHER: Cold, snowy (lo-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: G103
Well ID: G103
Surface Elev: 627.94 ft. MSL
Completion: 18.03 ft. BGS
Station: 876,199.48N
 2,514,501.19E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = Dry - While drilling ▼ = ▼ = 16.99 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	5-4 5-7 N=9	16					2	FILL - Brown (10YR5/3) with 10% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with trace sand and slight trace gravel.		626	
2A	16/24 67%	ss	3-4 6-7 N=10	25					4			624	
3A	17/24 71%	ss	3-5 7-8 N=12	21					6	Grayish brown (10YR5/2) with 25% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand, slight trace roots.		622	
4A	16/24 67%	ss	2-3 5-6 N=8	24					8	Grayish brown (10YR5/2) with 25% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.		620	
5A	24/24 100%	ss	3-4 4-4 N=8	22					10	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		618	
6A	20/24 83%	ss	1-1 3-3 N=4	23					12			616	
7A	22/24 92%	ss	1-2 2-4 N=4	26					14	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, soft, medium, silty CLAY with trace sand and slight trace gravel.		614	
8A	20/24 83%	ss	1-2 3-3 N=5	22					16			612	
9A	22/24 92%	ss	1-10 21-33 N=31	14					18	Brown (10YR5/3) with 15% gray (10YR6/1) mottles, very moist, medium dense, silty, very fine- to medium-grained SAND.		610	
9B				10						Brown (10YR5/3), slightly moist, hard, clayey SILT with trace sand and gravel.			

End of Boring = 18.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/15/2010
Finish: 2/15/2010
WEATHER: Overcast, cold, windy (10-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G104
Well ID: G104
Surface Elev: 627.96 ft. MSL
Completion: 20.00 ft. BGS
Station: 875,849.26N
 2,514,504.98E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss	9-11 12-14 N=23	16			2	FILL - Brown (10YR5/3) with 5% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY with slight trace sand and gravel.		626	
2A	20/24 83%	ss	3-5 9-12 N=14	21			4			624	
3A	19/24 79%	ss	3-6 8-9 N=14	21			6	Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand, slight trace roots.		622	
4A	17/24 71%	ss	2-3 4-7 N=7	25			8			620	
5A	22/24 92%	ss	1-2 5-8 N=7	24			10	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.		618	
6A	18/24 75%	ss	1-2 3-5 N=5	21			12			616	
7A	23/24 96%	ss	woh-2 3-4 N=5	23			14	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with trace sand and slight trace gravel.		614	
8A	22/24 92%	ss	1-3 3-3 N=6	24			16	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with trace sand and slight trace gravel.		612	
9A				16			16	Gray (10YR6/1), moist, very soft, clayey, very fine- to coarse-grained SAND with trace gravel.		612	
9B	17/24 71%	ss	woh-6 27-40 N=33	13			18	Brown (10YR5/3), slightly moist, hard, clayey SILT with trace sand and gravel.		610	
10A	22/24 92%	ss	10-24 44-66 N=68	7			20	Gray (10YR4/1), slightly moist, hard, clayey SILT with sand and gravel.		608	

End of Boring = 20.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/16/2010
Finish: 2/16/2010

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: G105
Well ID: G105
Surface Elev: 626.86 ft. MSL
Completion: 19.83 ft. BGS
Station: 875,499.70N
 2,514,509.15E

WEATHER: Sunny, cold, windy (mid-20's)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = Dry - While drilling ▽ = ▽ = 16.08 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	4-4 4-7 N=8	20					2	FILL - Brown (10YR5/3) with 10% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, medium, silty CLAY with slight trace sand and gravel.		626	
2A	19/24 79%	ss	2-4 6-10 N=10	19					4			624	
3A	19/24 79%	ss	2-4 5-6 N=9	29					6	Grayish brown (10YR5/2) with 40% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY, slight trace roots.		622	
4A	20/24 83%	ss	1-4 4-5 N=8	29					8	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.		620	
5A	22/24 92%	ss	1-3 3-3 N=6	20					10	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.		618	
6A	24/24 100%	ss	1-2 3-4 N=5	25					12	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
7A	24/24 100%	ss	1-2 3-5 N=5	22					14			614	
8A	19/24 79%	ss	1-2 2-2 N=4	18					16	Gray (10YR6/1) with 15% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		612	
9A	18/24 75%	ss	1-2 8-10 N=10	14				▽	18	Gray (10YR5/1), very moist to wet, loose, silty, fine- to coarse-grained SAND with slight trace gravel.		610	
9B									18	Brown (10YR5/3), very moist to wet, loose, silty, very fine- to coarse-grained SAND with slight trace gravel.			
10A	19/22 86%	ss	11-40 53-60/4" N=93	7					18	Brown (10YR5/3), slightly moist, hard, clayey SILT with trace sand and gravel. Gray (10YR4/1), slightly moist, hard, clayey SILT with sand and gravel.		608	

End of Boring = 19.8 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/16/2010
Finish: 2/16/2010
WEATHER: Overcast, cold, windy (mid-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G106
Well ID: G106
Surface Elev: 625.96 ft. MSL
Completion: 18.00 ft. BGS
Station: 875,149.76N
 2,514,512.79E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	SS	7-8 13-16 N=21	18							
2A	16/24 67%	SS	2-4 5-7 N=9	16			2	FILL - Brown (10YR5/3) with 15% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with trace sand and slight trace gravel.		624	
3A	17/24 71%	SS	2-4 6-7 N=10	21			4	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with slight trace sand, slight trace roots.		622	
4A	20/24 83%	SS	2-3 5-6 N=8	24			6	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, medium, silty CLAY with slight trace sand.		620	
5A	22/24 92%	SS	1-3 3-5 N=6	21			8	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		618	
6A	22/24 92%	SS	1-2 4-4 N=6	24			10	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
7A	23/24 96%	SS	1-3 4-4 N=7	22			12	Yellowish brown (10YR5/6) with 10% gray (10YR6/1) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		614	
8A	19/24 79%	SS	1-4 7-7 N=11	20			14	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with trace sand.		612	
8B				14			14	Brown (10YR5/3), moist, medium dense, clayey, very fine-to medium-grained SAND with slight trace gravel.			
9A	23/24 96%	SS	7-15 22-22 N=37	8			16	Yellowish brown (10YR5/6) with 5% gray (10YR6/1) mottles, moist, medium, clayey SILT with trace sand and gravel.		610	
							16	Brown (10YR5/3), slightly moist, hard, clayey SILT with trace sand and gravel.			
							18	Gray (10YR4/1), slightly moist, hard, clayey SILT with trace sand and gravel.		608	

End of Boring = 18.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/17/2010
 Finish: 2/17/2010

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: G107
Well ID: G107
Surface Elev: 627.11 ft. MSL
Completion: 20.00 ft. BGS
Station: 874,994.33N
 2,514,358.25E

WEATHER: Overcast, cold, windy (mid-20's)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/18 0%	BD						GRAVEL FILL		626	
1A	5/6 83%	SS	3	12			2				
2A	14/24 58%	SS	3-3 6-8 N=9	17			4	FILL - Brown (10YR5/3) with 10% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with trace sand and slight gravel.		624	
3A	16/24 67%	SS	3-5 5-7 N=10	20			6	Grayish brown (10YR5/2) with 30% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel.		622	
4A	18/24 75%	SS	2-3 6-7 N=9	27			8			620	
5A	22/24 92%	SS	2-3 4-5 N=7	18			10			618	
6A	22/24 92%	SS	1-3 3-4 N=6	23			12	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
7A	24/24 100%	SS	1-2 3-4 N=5	27			14			614	
8A	2/24 8%	SS	1-3 2-3 N=5	23			16	Brown (10YR5/3), very moist to wet, very loose, clayey, very fine- to coarse-grained SAND.		612	
9A	20/24 83%	SS	woh-3 8-12 N=11	22			18	Brown (10YR5/3), slightly moist, stiff, clayey SILT with trace sand and gravel.		610	
9B	20/24 83%	SS	8-25 26-58 N=51	11			20	Brown (10YR5/3), slightly moist, hard, clayey SILT with trace sand and gravel.		608	
10A	20/24 83%	SS	8-25 26-58 N=51	11			20	Gray (10YR4/1), slightly moist, hard, clayey SILT with trace sand and gravel.		608	

End of Boring = 20.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/12/2010
 Finish: 2/12/2010
WEATHER: Overcast, cold ~25F

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
 Helper: M. Herbst/S. Hamby
 Eng/Geo: D. Lamb

BOREHOLE ID: G108
Well ID: G108
Surface Elev: 625.58 ft. MSL
Completion: 20.00 ft. BGS
Station: 874,948.81N
 2,514,248.25E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	ss	24-25 13-13 N=38		16				2	FILL - Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/6) mottles, moist, hard, silty CLAY with slight trace gravel.		624	
2A	13/24 54%	ss	4-5 8-11 N=13		26				4	FILL - Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with trace sand and slight trace gravel.		622	
3A	20/24 83%	ss	2-2 5-7 N=7		28				6	Grayish brown (10YR5/2) with 25% yellowish brown (10YR5/8) and 5% very dark brown (10YR2/2) mottles, moist, medium, clayey SILT with trace sand and slight trace gravel.		620	
4A	24/24 100%	ss	2-3 5-6 N=8		18				8			618	
5A	23/24 96%	ss	1-2 3-4 N=5		20				10	Gray (10YR5/1) with 10% brownish yellow (10YR6/8) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
6A	24/24 100%	ss	1-3 3-5 N=6		19				12	Grayish brown (10YR5/2) with brownish yellow (10YR6/8) mottles, moist, soft, sandy CLAY with trace gravel.		614	
7A	19/24 79%	ss	1-1 1-2 N=2		19				14	Brownish yellow (10YR6/8), very moist, soft, sandy CLAY with trace gravel.		612	
8A	23/24 96%	ss	2-4 7-10 N=11		19				16	Light yellowish brown (10YR6/4) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, clayey SILT with trace sand and gravel.		610	
8B					13				18	Gray (10YR5/1), slightly moist, hard, SILT with gravel.		608	
9A	22/24 92%	ss	10-24 25-10 N=49		11				18	Gray (10YR5/1), wet, hard, SILT with sand and gravel.		606	
9B					8				20	Gray (10YR5/1), very moist, hard, SILT with gravel.		606	
10A	24/24 100%	ss	10-25 40-40 N=65		10								

End of Boring = 20.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/11/2010
Finish: 2/11/2010
WEATHER: Sunny, cold ~32F

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: D. Lamb

BOREHOLE ID: G109
Well ID: G109
Surface Elev: 624.79 ft. MSL
Completion: 18.00 ft. BGS
Station: 874,970.10N
 2,514,137.84E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 14.20 - While drilling ▽ = 11.50 - Upon completion ▽ = 8.85 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	6-7 7-8 N=14	22						2	FILL - Grayish brown (10YR5/2) with 40% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace gravel.		624	
2A	19/24 79%	ss	3-5 5-6 N=10	27						4	Light yellowish brown (10YR6/4) with 50% brownish yellow (10YR6/8) mottles, moist, medium, silty CLAY, slight trace roots.		622	
3A	20/24 83%	ss	2-5 6-8 N=11	24						6			620	
4A	24/24 100%	ss	2-4 5-6 N=9	19						8	Light brownish gray (10YR6/2) with 10% brownish yellow (10YR6/6) mottles, moist, medium, silty CLAY with slight trace sand and gravel.		618	
5A	22/24 92%	ss	2-3 4-5 N=7	20						10	Light brownish gray (10YR6/2) with 10% brownish yellow (10YR6/6) and 2% very dark gray (10YR3/1) mottles, moist, medium, silty CLAY with slight trace sand and gravel.		616	
6A	24/24 100%	ss	1-3 3-4 N=6	19						12	Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, moist, medium, sandy CLAY with slight trace gravel.		614	
7A	23/24 96%	ss	1-1 2-2 N=3	19						14	Brownish yellow (10YR6/6), wet, dense, silty SAND with trace gravel.		612	
8A	22/24 92%	ss	8-15 15-21 N=30	14						16	Brownish yellow (10YR6/8), wet, dense, SAND with trace gravel.		610	
9A	24/24 100%	ss	12-29 44-45 N=73	7						18	Grayish brown (10YR5/2), slightly moist, hard, gravelly SILT with sand.		608	

End of Boring = 18.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/11/2010
Finish: 2/11/2010
WEATHER: Sunny, cold 10-20F

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: .

BOREHOLE ID: G110
Well ID: G110
Surface Elev: 624.81 ft. MSL
Completion: 18.00 ft. BGS
Station: 875,015.42N
 2,514,057.73E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 15.00 - While drilling ▽ = ▽ = 9.50 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	8-6 6-8 N=12	17					2	FILL - Yellowish brown (10YR5/6) with 20% light brownish gray (10YR6/2) mottles, moist, stiff, silty CLAY with slight trace sand.		624	
2A	14/24 58%	SS	2-4 5-7 N=9	25					4			622	
3A	22/24 92%	SS	1-4 5-9 N=9	22					6	Grayish brown (10YR5/2) with 20% yellowish brown (10YR5/6) and 5% very dark brown (10YR2/2) mottles, moist, medium, silty CLAY.		620	
4A	24/24 100%	SS	3-6 8-9 N=14	18					8			618	
5A	24/24 100%	SS	1-3 4-6 N=7	21				▽	10	Grayish brown (10YR5/2) with 20% yellowish brown (10YR5/6) and 5% very dark brown (10YR2/2) mottles, moist, medium, silty CLAY with slight trace sand and gravel.		616	
6A	24/24 100%	SS	2-4 4-6 N=8	21					12	Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, moist, medium, clayey SILT with trace sand and gravel.		614	
7A	24/24 100%	SS	1-2 3-3 N=5	22					14	Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, very moist, medium, sandy CLAY with trace gravel.		612	
8A	19/24 79%	SS	1-2 2-1 N=4	24				▽	16	Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, moist, soft, clayey SILT with trace sand and slight trace gravel.		610	
9A	24/24 100%	SS	7-26 49-60 N=75	6					18	Gray (10YR6/1) with 30% brownish yellow (10YR6/8) mottles, wet, soft, sandy CLAY.		608	
End of Boring = 18.0 ft. BGS													

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/10/2010
 Finish: 2/11/2010
WEATHER: Sunny, breezy ~25F

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: D. Lamb

BOREHOLE ID: G111
Well ID: G111
Surface Elev: 625.28 ft. MSL
Completion: 18.00 ft. BGS
Station: 875,058.70N
 2,513,981.72E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	SS	7-7 6-9 N=13		18				0			624	
2A	13/24 54%	SS	3-5 7-8 N=12		20				2	FILL - Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel.		622	
3A	18/24 75%	SS	2-4 6-8 N=10		20				4			620	
4A	16/24 67%	SS	4-12 20-17 N=32		18				6	Grayish brown (10YR5/2) with 20% yellowish brown (10YR5/6) and 5% dark brown (10YR3/3) mottles, moist, medium, clayey SILT with slight trace sand, slight trace roots.		618	
5A	22/24 92%	SS	2-3 4-5 N=7		21				8	Grayish brown (10YR5/2) with 5% yellowish brown (10YR5/6) mottles, moist, medium, clayey SILT with trace sand.		616	
6A	24/24 100%	SS	2-3 6-6 N=9		23				10			614	
7A	24/24 100%	SS	1-4 5-6 N=9		20				12			612	
8A	24/24 100%	SS	1-2 2-2 N=4		17				14	Grayish brown (10YR5/2) with 30% yellowish brown (10YR5/8) mottles, moist, stiff, clayey SILT with trace sand and gravel.		610	
9A	18/18 100%	SS	12-50 66 N=116		7				16	Yellowish brown (10YR5/8), wet, soft, clayey SAND with trace gravel.		608	
	0/6 0%	BD							18	Grayish brown (10YR5/2) with 20% brownish yellow (10YR6/6) mottles, slightly moist, hard, clayey SILT with gravel.			

End of Boring = 18.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/9/2010
Finish: 2/9/2010
WEATHER: Cold, snow, windy (10-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: G119
Well ID: G119
Surface Elev: 626.57 ft. MSL
Completion: 20.10 ft. BGS
Station: 875,675.04N
 2,513,907.73E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = Dry - While drilling ▽ = ▽ = 11.26 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss	3-2 3-3 N=5	21						0			626	
2A	16/24 67%	ss	2-4 5-5 N=9	20						2	FILL - Brown (10YR5/3) with 10% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		624	
3A	10/24 42%	ss	2-5 5-6 N=10	25						4			622	
4A	17/24 71%	ss	2-5 4-5 N=9	18						6			620	
5A	20/24 83%	ss	2-3 4-5 N=7	19						8	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand.		618	
6A	19/24 79%	ss	1-3 4-6 N=7	21						10			616	
7A	20/24 83%	ss	1-3 3-5 N=6	20						12			614	
8A	18/24 75%	ss	1-2 2-2 N=4	18						14			612	
9A	22/24 92%	ss	3-9 13-16 N=22	13						16			610	
9B				12						16	Brown (10YR5/3), moist, medium dense, silty, very fine-grained SAND.			
										17	Brown (10YR5/3), slightly moist, hard, clayey SILT with trace sand and gravel.			
10A	17/24 71%	ss	6-25 33-39 N=58	8						18			608	
										20	Dark gray (10YR4/1), slightly moist, hard, clayey SILT with trace sand and gravel.			
End of Boring = 20.1 ft. BGS														

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/8/2010
Finish: 2/8/2010
WEATHER: Cold, snow, windy (mid-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G120
Well ID: G120
Surface Elev: 627.21 ft. MSL
Completion: 20.00 ft. BGS
Station: 875,854.43N
 2,513,905.84E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▽ = Dry - While drilling ▽ = ▽ = 13.85 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	3-3 4-5 N=7		16								626	
2A	19/24 79%	ss	3-4 7-9 N=11		20						FILL - Brown (10YR5/3) with 10% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		624	
3A	16/24 67%	ss	2-3 4-5 N=7		26						Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel.		622	
4A	17/24 71%	ss	3-5 6-6 N=11		24						Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel, slight trace roots.		620	
5A	14/24 58%	ss	2-4 4-6 N=8		22						Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with slight trace sand.		618	
6A	22/24 92%	ss	1-3 3-4 N=6		23						Gray (10YR5/1) with brownish yellow (10YR6/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
7A	22/24 92%	ss	1-2 3-5 N=5		21						Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		614	
8A	19/24 79%	ss	woh-2 3-3 N=5		25						Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		612	
9A	24/24 100%	ss	woh-3 12-21 N=15		21						Brown (10YR5/3), very moist, soft, clayey SAND with slight trace gravel.		610	
9B					10						Brown (10YR5/3), slightly moist, stiff, clayey SILT with trace sand and slight trace gravel.		610	
10A	24/24 100%	ss	13-36 46-70 N=82		7						Dark gray (10YR4/1), slightly moist, hard, clayey SILT with trace sand and gravel.		608	

End of Boring = 20.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/4/2010
Finish: 2/4/2010
WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: G121
Well ID: G121
Surface Elev: 627.94 ft. MSL
Completion: 22.00 ft. BGS
Station: 875,964.59N
 2,513,904.35E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
							Quadrangle: Coffeen, IL	Township: East Fork	Section 10, Tier 7N; Range 3W	▼ = Dry - While drilling	▽ =	▽ = 14.44 - 3/1/2010
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	22/24 92%	ss	4-4 7-8 N=11		15							
2A	20/24 83%	ss	3-5 8-12 N=13		16		FILL - Brown (10YR4/3) with 10% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel.					
3A	18/24 75%	ss	1-4 5-6 N=9		27							
4A	18/24 75%	ss	2-3 5-6 N=8		25		Dark yellowish brown (10YR4/4), moist, medium, silty CLAY with slight trace sand.					
5A	18/24 75%	ss	2-2 3-4 N=5		24		Gray (10YR5/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, medium, clayey SILT with trace sand.					
6A	13/24 54%	ss	2-2 4-4 N=6		23		Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.					
7A	19/24 79%	ss	woh-2 3-4 N=5		23							
8A	18/24 75%	ss	1-2 2-2 N=4		23		Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.					
9A	23/24 96%	ss	woh-woh 1-2		21		Brown, (10YR5/3), very moist, very soft, clayey SAND with slight trace gravel.					
10A	22/24 92%	ss	4-12 26-30 N=38		8		Brown (10YR5/3), slightly moist, hard, very silty CLAY with trace sand and gravel.					

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/4/2010
 Finish: 2/4/2010
WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: G121
Well ID: G121
Surface Elev: 627.94 ft. MSL
Completion: 22.00 ft. BGS
Station: 875,964.59N
 2,513,904.35E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	24/24 100%	ss	13-23 31-48 N=54	7			22	Dark gray (10YR4/1), slightly moist, hard, clayey SILT with sand and gravel.		606		
							End of Boring = 22.0 ft. BGS					

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/4/2010
Finish: 2/4/2010
WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: D. Lamb

BOREHOLE ID: G122
Well ID: G122
Surface Elev: 628.05 ft. MSL
Completion: 20.00 ft. BGS
Station: 876,080.14N
 2,513,902.82E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	SS	5-5 6-11 N=11		16				2	FILL - Grayish brown (10YR5/2) with 15% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with trace sand and slight trace gravel.		626	
2A	20/24 83%	SS	3-3 5-5 N=8		22				4			624	
3A	20/24 83%	SS	3-4 5-6 N=9		25				6	Dark yellowish brown (10YR4/4), moist, medium, silty CLAY.		622	
4A	19/24 79%	SS	1-5 5-6 N=10		29				8			620	
5A	20/24 83%	SS	1-3 3-3 N=6		21				10	Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, medium, silty CLAY.		618	
6A	19/24 79%	SS	1-2 3-3 N=5		22				12	Grayish brown (10YR5/2), moist, medium, clayey SILT with sand.		616	
7A	16/24 67%	SS	1-2 3-4 N=5		21				14	Yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace gravel.		614	
8A	19/24 79%	SS	1-1 2-2 N=3		21				16	Gray (10YR5/1), moist, medium, clayey SILT with fine sand.		612	
9A	20/24 83%	SS	1-1 4-16 N=5		14				18	Brown (10YR4/3), moist, medium, silty CLAY with trace gravel.		610	
10A	24/24 100%	SS	1-1 51-58 N=89		6				20	Grayish brown (10YR5/2), moist, soft, sandy SILT. Yellowish brown (10YR5/4) with 40% dark yellowish brown (10YR4/6) mottles, moist, medium, silty CLAY with slight trace sand. Dark yellowish brown (10YR4/6), wet, medium dense, silty SAND.			
									20	Brownish yellow (10YR6/6), slightly moist, hard, clayey SILT with trace sand and gravel. Gray (10YR5/1), slightly moist, hard, clayey SILT with gravel.			

End of Boring = 20.00 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/3/2010
Finish: 2/4/2010
WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4¼" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: D. Lamb

BOREHOLE ID: G123
Well ID: G123
Surface Elev: 628.12 ft. MSL
Completion: 24.00 ft. BGS
Station: 876,189.60N
 2,513,901.46E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 16.50 - While drilling ▽ = ▽ = 15.98 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	22/24 92%	ss	6-8 9-11 N=17		16								628	
2A	19/24 79%	ss	4-5 6-7 N=11		18			FILL - Grayish brown (10YR5/2) with 10% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with trace sand and slight trace gravel.					626	
3A	17/24 71%	ss	2-3 5-6 N=8		24								624	
4A	18/24 75%	ss	1-3 5-8 N=8		24			Dark grayish brown (10YR4/2) with 20% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand, slight trace roots.					622	
5A	20/24 83%	ss	2-3 4-5 N=7		18			Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.					620	
6A	19/24 79%	ss	1-2 3-5 N=5		21			Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.					618	
7A	23/24 96%	ss	1-3 4-4 N=7		19			Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with sand and slight trace gravel.					616	
8A	22/24 92%	ss	1-2 3-3 N=5		17			Dark yellowish brown (10YR3/6) with dark yellowish brown (10YR4/6) mottles, moist, medium, silty CLAY with trace gravel.					614	
								Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, moist, medium, sandy SILT.					612	
9A	22/24 92%	ss	1-1 2-2 N=3		19			Dark yellowish brown (10YR4/6), wet, soft, clayey SAND.					610	
10A	12/24 50%	ss	3-3 3-4 N=6		16			Yellowish brown (10YR5/6), wet, medium, sandy SILT with trace gravel.						

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/3/2010
Finish: 2/4/2010
WEATHER: Overcast, cold (lo-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: D. Lamb

BOREHOLE ID: G123
Well ID: G123
Surface Elev: 628.12 ft. MSL
Completion: 24.00 ft. BGS
Station: 876,189.60N
 2,513,901.46E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	20/24 83%	ss	6-12 12-12 N=24	11			22	Yellowish brown (10YR5/6), wet, medium, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		608		
12A	19/24 79%	ss	3-8 13-9 N=21	9			24	Dark gray (10YR4/1), moist, very stiff, sandy SILT with trace clay and gravel.		606		
End of Boring = 24.0 ft. BGS												

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/3/2010
Finish: 2/3/2010
WEATHER: Sunny, cold (mid-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: G124
Well ID: G124
Surface Elev: 628.70 ft. MSL
Completion: 20.00 ft. BGS
Station: 876,304.85N
 2,513,900.34E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	6-4 5-6 N=9	17			0			628	
2A	22/24 92%	ss	4-5 7-8 N=12	21			2	FILL - Brown (10YR5/3) with 10% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel.		626	
3A	19/24 79%	ss	2-4 6-7 N=10	25			4			624	
4A	13/24 54%	ss	3-3 6-6 N=9	28			6	Dark grayish brown (10YR4/2) with 5% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY.		622	
5A	20/24 83%	ss	1-3 5-6 N=8	21			8	Dark grayish brown (10YR4/2) with 5% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY, slight trace roots.		620	
6A	17/24 71%	ss	1-3 3-4 N=6	22			10	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand and gravel.		618	
7A	18/24 75%	ss	1-3 3-4 N=6	23			12	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
8A	20/24 83%	ss	woh-woh 2-3	28			14			614	
8B				22			16	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, very soft, silty CLAY with trace sand and slight trace gravel.		612	
9A	19/24 79%	ss	1-2 3-3 N=5	22			18	Gray (10YR6/1) with 5% yellowish brown (10YR5/6) mottles, moist, medium, sandy CLAY with slight trace gravel.		610	
9B				17			18	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, medium, sandy CLAY with slight trace gravel.			
10A	20/24 83%	ss	4-10 17-23 N=27	12			20	Yellowish brown (10YR5/4), moist, medium, clayey SILT with trace sand and slight trace gravel. Yellowish brown (10YR5/4), slightly moist, stiff, clayey SILT with trace sand and slight trace gravel. Gray (10YR5/1), slightly moist, stiff, clayey SILT with trace sand and slight trace gravel.		610	

End of Boring = 20.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/2/2010
Finish: 2/3/2010
WEATHER: Sunny, cold (mid-30's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G125
Well ID: G125
Surface Elev: 628.85 ft. MSL
Completion: 20.13 ft. BGS
Station: 876,409.47N
 2,513,899.12E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 17.00 - While drilling ▽ = Dry - Upon completion ▽ = 8.58 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	5-6 7-9 N=13	17					2	FILL - Brown (10YR5/3) with 5% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel.		628	
2A	16/24 67%	ss	3-4 6-9 N=10	17					4			626	
3A	20/24 83%	ss	2-5 5-7 N=10	25					6	Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand and gravel, slight trace roots.		624	
4A	19/24 79%	ss	2-4 5-5 N=9	23					8			622	
5A	14/24 58%	ss	3-4 4-6 N=8	25				▽	10	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand and gravel, slight trace roots.		620	
6A	18/24 75%	ss	2-3 3-5 N=6	22					12	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.		618	
7A	20/24 83%	ss	3-4 3-4 N=7	23					14	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		616	
8A	24/24 100%	ss	3-3 3-3 N=6	23					16	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		614	
9A	24/24 100%	ss	woh-1 2-1 N=3	25				▽	18	Brown (10YR5/3), very moist, very soft, clayey SAND with slight trace gravel.		612	
9B				18					20			610	
10A				22									
10B	19/24 79%	ss	woh-1 7-19 N=8	11						Brown (10YR5/3), slightly moist, medium, clayey SILT with trace sand and slight trace gravel.		610	

End of Boring = 20.1 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/10/2010
Finish: 2/10/2010

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4¼" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: D. Lamb

BOREHOLE ID: G126
Well ID: G126
Surface Elev: 622.96 ft. MSL
Completion: 18.00 ft. BGS
Station: 875,062.44N
 2,513,895.37E

WEATHER: Partly cloudy, wind 10 mph, ~25F

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	9-7 12-10 N=19		15		0	FILL - Dark yellowish brown (10YR4/6), moist, very stiff, sandy CLAY with silt and trace gravel.		622	
2A	19/24 79%	ss	2-4 6-9 N=10		26		2	Light grayish brown (10YR6/2), slightly moist, stiff, silty CLAY.		620	
3A	22/24 92%	ss	2-4 5-7 N=9		20		4	Light grayish brown (10YR6/2) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY, slight trace roots.		618	
4A	24/24 100%	ss	2-3 4-5 N=7		22		6	Dark brown (10YR3/3) with 10% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY.		616	
5A	24/24 100%	ss	1-3 4-5 N=7		22		8			614	
6A	22/24 92%	ss	1-3 4-5 N=7		18		10	Light grayish brown (10YR6/2) with 30% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		612	
7A	20/24 83%	ss	1-2 3-4 N=5		19		12	Grayish brown (10YR5/2) with dark yellowish brown (10YR4/6) mottles, moist, medium, clayey SILT with sand and slight trace gravel.		610	
8A	24/24 100%	ss	4-15 12-29 N=27		20		14	Gray (10YR6/1), very moist, medium, sandy CLAY with trace gravel.		608	
9A	23/24 96%	ss	29-39 39-29 N=78		8		16	Yellowish brown (10YR5/8) with 40% dark yellowish brown (10YR4/6) mottles, slightly moist, very stiff, clayey SILT with trace gravel. Brownish yellow (10YR6/8), moist, very stiff, sandy SILT with gravel.		606	
							18	Grayish brown (10YR5/2), moist, hard, sandy SILT with gravel.		606	

End of Boring = 18.0 ft. BGS

NOTE(S): G126 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 12/19/2011
Finish: 12/19/2011
WEATHER: Cloudy, rain (mid-60's)

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. McCuan
Eng/Geo: R. Fiorito

BOREHOLE ID: G151
Well ID: G151
Surface Elev: 622.82 ft. MSL
Completion: 20.46 ft. BGS
Station: 875,023.67N
 2,513,805.93E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	ss	2-3 4-5 N=7				0	FILL - Light yellowish brown (10YR6/4), silty CLAY with some sand and gravel.		622	
2A	19/24 79%	ss	2-3 6-9 N=9				2	FILL - Brown (10YR5/3), silty CLAY with some sand and slight trace gravel.		620	
	19/24 79%	ss	2-4 6-8 N=10				4	FILL - Brown (10YR5/3), silty CLAY with some sand and slight trace gravel.		618	
3A							6	Brown (10YR5/3) with 30% gray (10YR6/1) mottles, clayey SILT with trace sand and slight trace gravel.		616	
4A	19/24 79%	ss	15-15 18-11 N=33				8			614	
5A	18/24 75%	ss	1-3 4-4 N=7				10	Brown (10YR5/3) with 30% gray (10YR6/1) mottles, silty CLAY with sand and gravel.		612	
6A	16/24 67%	ss	3-3 4-6 N=7				12	Light gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, sandy CLAY with some silt and slight trace gravel.		610	
7A	22/24 92%	ss	9-9 10-9 N=19				14	Light brown (10YR5/3) silty SAND with slight trace gravel.		608	
8A	19/24 79%	ss	15-15 35-70 N=50				16	Light gray (10YR6/1), silty SAND with slight trace gravel.		606	
9A	4/4 100%	ss	50/4"				18	Light brown (10YR6/4), silty SAND with some gravel.		604	
10A	10/23 43%	ss	12-18 70-99/5" N=88				20	Dark gray (10YR5/1), very silty CLAY with sand and trace gravel.			

End of Boring = 20.5 ft. BGS

NOTE(S): G151 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 12/20/2011
Finish: 12/20/2011
WEATHER: Cloudy, (mid-40's), rainy later.

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. McCuan
Eng/Geo: R. Fiorito

BOREHOLE ID: G152
Well ID: G152
Surface Elev: 623.06 ft. MSL
Completion: 18.57 ft. BGS
Station: 874,687.53N
 2,513,894.46E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL	Township: East Fork	Section 10, Tier 7N; Range 3W	▽ = 14.50 - While drilling	▽ = 3.05 - Upon completion	▽ = 17.00 - 12/21/2011
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	12/24 50%	SS	2-2 2-3 N=4				Dark brown (10YR4/2), silty CLAY with trace sand and slight trace gravel.		622			
2A	22/24 92%	SS	2-4 9-7 N=13				Light yellowish brown (10YR6/4), silty CLAY with trace sand and slight trace gravel.		620			
2B							Gray (10YR6/1), silty CLAY with trace sand and slight trace gravel.					
3A	15/24 63%	SS	3-4 5-6 N=9				Gray (10YR6/1) and 25% brown (10YR5/6) mottles, silty CLAY with trace sand and slight trace gravel.		618			
3B												
5A	19/24 79%	SS	10-10 12-10 N=22				Gray (10YR5/1), clayey SILT silty CLAY with sand and slight trace gravel.		616			
5B	20/24 83%	SS	2-4 6-7 N=10				Gray (10YR5/1) with 25% brown (10YR5/6) mottles, silty CLAY with trace sand and slight trace gravel.		614			
6A	21/24 88%	SS	2-3 5-7 N=8				Gray (10YR6/1) with 25% yellowish brown (10YR5/6) mottles, silty CLAY with trace sand and slight trace gravel.		612			
7A	20/24 83%	SS	5-7 9-10 N=16				Gray (10YR6/1) with 25% yellowish brown (10YR5/6) mottles, clayey SAND with some silt and slight trace gravel.		610			
8A	21/24 88%	SS	3-4 7-11 N=11				Yellowish brown (10YR5/6) with 20% gray (10YR6/1), mottles, clayey SAND with some silt and slight trace gravel.		608			
8B							Yellowish brown (10YR5/6), silty SAND.					
							Yellowish brown (10YR5/6), silty SAND with slight trace gravel.					
9A	23/24 96%	SS	14-35 60-70 N=95				Dark grayish brown (10YR4/2), very silty CLAY with sand and trace gravel.		606			
							Grayish brown (10YR5/2) silty CLAY with sand and trace gravel.					

End of Boring = 18.6 ft. BGS

NOTE(S): G152 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 12/15/2011
Finish: 12/15/2011
WEATHER: Cloudy, windy, (mid-high 30's)

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. McCuan
Eng/Geo: R. Fiorito

BOREHOLE ID: G153
Well ID: G153
Surface Elev: 623.30 ft. MSL
Completion: 20.76 ft. BGS
Station: 874,532.71N
 2,513,532.68E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 16.00 - While drilling ▽ = 15.70 - Upon completion ▽ = 17.55 - 12/21/2011	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss					FILL - Light yellowish brown (10YR6/4), silty CLAY with sand and slight trace gravel.		0		622		
1B									2	FILL - Gray (10YR6/1), silty CLAY with sand and slight trace gravel.		620	
2A	17/24 71%	ss	woh-3 4-6 N=7						4	Gray (10YR6/1) with 50% brownish yellow (10YR6/8) mottles, silty CLAY with trace sand.		618	
3A	18/24 75%	ss	3-4 5-5 N=9						6	Gray (10YR5/1), silty CLAY with trace sand.		616	
4A	21/24 88%	ss	4-5 6-8 N=11						8			614	
5A	20/24 83%	ss	1-4 4-5 N=8						10	Gray (10YR6/1), silty CLAY with sand and slight trace gravel.		612	
6A	20/24 83%	ss	1-4 4-6 N=8						12	Gray (10YR6/1) with 50% brownish yellow (10YR6/8) mottles, silty CLAY with sand and slight trace gravel.		610	
7A	23/24 96%	ss	7-6 6-15 N=12						14	Gray (10YR6/1) with 25% brownish yellow (10YR6/8) mottles, clayey SAND with trace silt.		608	
8A	19/24 79%	ss	15-23 37-50 N=60						16	Yellowish brown (10YR5/8), silty SAND with slight trace gravel.		606	
9A	9/12 75%	ss	50-99						18	Brownish yellow (10YR6/8), silty CLAY with sand and trace gravel.		604	
10A	20/24 83%	ss	23-50 58-109 N=108						20	Dark gray (10YR4/1), silty CLAY with sand and trace gravel.			
11A	6/7 86%	ss	75-99/1*										

End of Boring = 20.8 ft. BGS

NOTE(S): G153 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 12/16/2011
Finish: 12/16/2011
WEATHER: Ptly. cloudy (mid-30's)

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. McCuan
Eng/Geo: R. Fiorito

BOREHOLE ID: G154
Well ID: G154
Surface Elev: 623.52 ft. MSL
Completion: 20.00 ft. BGS
Station: 874,978.38N
 2,513,243.10E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 13.45 - While drilling ▽ = ▽ = 11.10 - 12/21/2011	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	14/24 58%	ss	3-4 4-5 N=8						0	FILL - Light yellowish brown (10YR6/4), silty CLAY with trace sand and slight trace gravel.		622	
2A	10/24 42%	ss	woh-1 4-5 N=5						2	Brownish yellow (10YR6/6) with 25% gray (10YR6/1) mottles, silty CLAY with trace sand.		620	
3A	14/24 58%	ss	2-4 6-7 N=10						4	Gray (10YR5/1) with 25% yellowish brown (10YR5/4) mottles, silty CLAY with trace sand.		618	
4A	21/24 88%	ss	6-8 10-10 N=18						6			616	
5A	16/24 67%	ss	1-4 6-8 N=10						8	Gray (10YR6/1) with 25% brownish yellow (10YR6/6) mottles, silty CLAY with trace sand and slight trace gravel.		614	
6A	13/24 54%	ss	1-4 6-8 N=10					▽	10			612	
7A	20/24 83%	ss	4-6 8-12 N=14					▼	12	Gray (10YR6/1) with 10% brownish yellow (10YR6/8) mottles, silty CLAY with trace sand and slight trace gravel.		610	
7B									13	Gray (10YR6/1), silty CLAY with trace sand and slight trace gravel.		610	
8A									14	Gray (10YR6/1), clayey SAND with trace silt.			
8B	23/24 96%	ss	4-18 30-37 N=48						15	Gray (10YR6/1) with 50% yellowish brown (10YR5/6) mottles, silty SAND with slight trace gravel.			
8C									16	Dark yellowish brown (10YR4/4), silty CLAY with sand and trace gravel.		608	
9A	21/24 88%	ss	40-75 86-84 N=161						17	Brown (10YR4/3), silty CLAY with sand and trace gravel.		606	
10A	20/24 83%	ss	28-28 30-34 N=58						18	Dark gray (10YR4/1), silty CLAY with sand and trace gravel.		604	
									20	End of Boring = 20.0 ft. BGS			

NOTE(S): G154 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 12/19/2011
Finish: 12/19/2011
WEATHER: Cloudy, rainy, (mid-40's)

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. McCuan
Eng/Geo: R. Fiorito

BOREHOLE ID: G155
Well ID: G155
Surface Elev: 622.89 ft. MSL
Completion: 20.23 ft. BGS
Station: 875,127.65N
 2,513,501.75E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	4-6 8-6 N=14				0	FILL - Yellow (10YR7/6), silty CLAY with trace sand and slight trace gravel.		622	
2A	20/24 83%	ss	2-4 6-7 N=10				2	FILL - Brown (10YR5/3), silty CLAY with sand and trace gravel.		620	
3A	17/24 71%	ss	2-4 5-8 N=9				4	FILL - Brownish yellow (10YR6/6), silty CLAY with trace sand and slight trace gravel.		618	
4A	24/24 100%	ss	6-7 11-11 N=18				6	FILL - Dark grayish brown (10YR4/2), silty CLAY with trace sand and slight trace gravel.		616	
5A	22/24 92%	ss	woh-3 6-6 N=9				8	Gray (10YR5/1), silty CLAY with trace sand and slight trace gravel.		614	
6A	17/24 71%	ss	2-4 6-6 N=10				10	Brownish yellow (10YR6/6) with 10% gray (10YR6/1) mottles, silty CLAY with trace sand and slight trace gravel.		612	
7A	24/24 100%	ss	6-8 10-12 N=18				12	Dark gray (10YR4/1) with 10% brown (10YR4/3) mottles, silty CLAY with trace sand and slight trace gravel.		610	
8A	23/24 96%	ss	woh-woh 4-15				14	Dark yellowish brown (10YR4/6) with 30% grayish brown (10YR5/2) mottles, silty CLAY with trace sand and slight trace gravel.		608	
8B	16/16 100%	ss	15-50 99/4"				16	Yellowish brown (10YR6/4), clayey SAND with trace silt.		606	
9A	15/17 88%	ss	24-68 99/5"				18	Yellowish brown (10YR5/6), silty SAND with slight trace gravel.		604	
10A							20	Light brownish gray (10YR6/2), silty CLAY with sand and trace gravel.			
							20	Light gray (10YR6/2), silty CLAY with sand and trace gravel.			

End of Boring = 20.2 ft. BGS

NOTE(S): G155 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/25/2008
Finish: 2/25/2008
WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: .

BOREHOLE ID: G200
Well ID: G200
Surface Elev: 624.20 ft. MSL
Completion: 18.00 ft. BGS
Station: 877,930.59N
 2,515,649.96E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	3-3 3-3 N=5		31	1.36 B	2	Very dark grayish brown (10YR3/2), moist, firm, friable, clayey SILT		624	
2A	19/24 79%	SS	3-3 6-6 N=9		26	1.94 BSh	2	Dark gray (10YR4/1) with 5% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY		622	
2B					26	2.33 Sh	4	Dark gray (10YR4/1) with 70% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY		620	
3A	19/24 79%	SS	3-3 4-5 N=7		26	1.59 B	4	Dark gray (10YR4/1) with 70% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand		620	
3B					23	1.55 B	6	Very dark gray (10YR3/1), moist, firm, silty CLAY, slight trace sand		618	
4A	22/24 92%	SS	5-5 5-5 N=10		29	0.31 B	8	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, trace coarse sand		616	
5A	20/24 83%	SS	2-2 3-5 N=5		25	1.09 B	10	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, sand and slight trace gravel		614	
6A	22/24 92%	SS	1-3 2-3 N=5		22	1.01	12	Yellowish brown (10YR5/8), moist, soft, sandy CLAY		612	
7A	24/24 100%	SS	3-3 5-6 N=8		15	0.50 B	14	Gray (10YR5/1), wet, soft, fine- to coarse-grained SAND		610	
7B					18						
8A	19/24 79%	SS	0-3 5-8 N=8		17	0.27 B	14	Gray (10YR5/1), wet, soft, silty CLAY, trace sand and gravel		610	
8B					17		16	Yellowish brown (10YR5/4), wet, soft, fine- to coarse-grained SAND, trace gravel		608	
9A					13						
9B	24/24 100%	SS	8-15 30-50 N=45		8		18	Gray (10YR5/1), moist, hard, silty CLAY, trace sand and gravel			

End of Boring = 18.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/25/2008
Finish: 2/25/2008
WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: .

BOREHOLE ID: G201
Well ID: G201
Surface Elev: 623.90 ft. MSL
Completion: 18.15 ft. BGS
Station: 877,924.94N
 2,514,849.47E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▼ = 10.20 - While drilling ▽ = 2.17 - 3/12/08 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	22/24 92%	SS	3-3 2-3 N=5	23						Very dark grayish brown (10YR3/2), moist, soft, friable, clayey SILT, slight trace sand and gravel			
1B				33					1.16	Dark brown (10YR3/3), moist, soft, silty CLAY		622	
2A	22/24 92%	SS	2-3 5-6 N=8	26		2.33 BSh			2.33	Yellowish brown (10YR5/8) with 20% gray (10YR5/1) mottles, moist, firm, silty CLAY, trace sand, slight trace gravel		620	
3A	20/24 83%	SS	2-4 5-5 N=9	15		1.94 B			1.94	Gray (10YR5/1) with 5% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY, sand and trace gravel		618	
4A	24/24 100%	SS	7-7 7-6 N=14	19		1.24 B			1.24	Gray (10YR5/1), moist, firm, sandy CLAY, trace silt and slight trace gravel		616	
5A	24/24 100%	SS	1-2 3-3 N=5	23		1.16 B			1.16	Yellowish brown (10YR5/8) with 10% gray (10YR5/1) mottles, moist, firm, sandy CLAY, trace gravel		614	
6A	23/24 96%	SS	0-1 1-2 N=2	20						Yellowish brown (10YR5/8), wet, soft, silty SAND, trace gravel		612	
7A	20/24 83%	SS	3-6 6-12 N=12	22		2.72 BSh				Yellowish brown (10YR5/8), moist, firm, clayey SILT			
8A	24/24 100%	SS	4-7 7-10 N=14	23		1.59 Sh				Greenish gray (5GY6/1), moist, firm, interbedded clayey SILT and SILT		610	
8B				19						Yellowish brown (10YR5/8), wet, soft, fine- to coarse-grained SAND, slight trace gravel		608	
9A				15						Yellowish brown (10YR5/8), wet, firm, very fine- to fine-grained silty SAND			
9B	24/24 100%	SS	7-12 25-30 N=37	20						Gray (10YR5/1), wet, soft, SILT			
9C				11						Gray (10YR5/1), wet, soft, fine- to coarse-grained SAND, slight trace gravel			
										Gray (10YR5/1), moist, hard, silty CLAY, trace sand and gravel		606	

End of Boring = 18.15 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/21/2008
Finish: 2/21/2008
WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: .

BOREHOLE ID: G205
Well ID: G205
Surface Elev: 622.15 ft. MSL
Completion: 16.00 ft. BGS
Station: 875,550.19N
 2,515,914.87E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	4-3 4-4 N=7	22				Very dark grayish brown (10YR3/2), moist, firm, clayey SILT		622	
1B				30	1.67	Sh	2	Yellowish brown (10YR5/4) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand		620	
2A	19/24 79%	SS	3-5 6-5 N=11	24	1.86	B	4	Yellowish brown (10YR5/4) with 30% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY, slight trace sand		618	
3A	20/24 83%	SS	2-2 5-5 N=7	19	1.55	B	6	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand, slight trace gravel		616	
4A	19/24 79%	SS	5-6 7-8 N=13	20	1.12	B	8	Yellowish brown (10YR5/6), moist, firm, silty CLAY with sand, trace gravel		614	
5A	16/24 67%	SS	2-2 3-5 N=5	20	0.62	BSh	10	Yellowish brown (10YR5/6), moist, soft, sandy SILT, slight trace gravel		612	
6A	22/24 92%	SS	1-2 3-3 N=5	17	0.62	BSh	12	Yellowish brown (10YR5/6), moist, soft, sandy SILT, slight trace gravel		610	
7A				17							
7B	23/24 96%	SS	3-3 5-6 N=8	17	1.36	BSh	14	Yellowish brown (10YR 5/4), moist, soft, silty SAND, trace gravel		608	
8A				15							
8B	17/24 71%	SS	5-19 26-35 N=45	10			16	Gray (10YR5/1), moist, hard, silty CLAY, trace sand and gravel			

End of Boring = 16.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/14/2010
Finish: 10/14/2010
WEATHER: Sunny, warm, breezy (lo-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G206
Well ID: G206
Surface Elev: 630.54 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,103.91N
 2,514,669.16E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 22.00 - While drilling ▽ = 21.54 - Upon completion ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	12/24 50%	ss	2-2 3-2 N=5	18					0	FILL - Grayish brown (10YR5/2), moist, firm, silty CLAY with trace sand and gravel.		630	
2A	20/24 83%	ss	2-2 3-5 N=5	16					2			628	
3A	20/24 83%	ss	4-9 6-8 N=15	19					4	FILL - Dark gray (10YR4/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		626	
4A	19/24 79%	ss	2-4 5-6 N=9	20					6			624	
5A	17/24 71%	ss	2-3 4-5 N=7	30					8	Very dark gray (10YR3/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand, trace roots.		622	
									10	Dark grayish brown (10YR4/2) with 35% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.		620	
6A	22/24 92%	ss	2-3 4-6 N=7	19					12	Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with trace sand and gravel.		618	
7A	23/24 96%	ss	1-2 3-4 N=5	23					14	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		616	
8A	22/24 92%	ss	1-1 3-3 N=4	22					16	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		614	
9A	24/24 100%	ss	1-1 2-2 N=3	21					18	Dark yellowish brown (10YR4/6) with 30% gray (10YR5/1) mottles, moist, soft, silty CLAY with trace sand and gravel.		612	
									18	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, silty CLAY with trace sand and gravel.			
10A	24/24 100%	ss	woh-woh 1-5	25					20	Gray (10YR5/1), moist, very soft, very fine- to fine-grained sandy CLAY with trace gravel.			
									20	Gray (10YR5/1), moist, firm, very fine- to fine-grained			

NOTE(S): G206 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 10/14/2010
Finish: 10/14/2010
WEATHER: Sunny, warm, breezy (10-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G206
Well ID: G206
Surface Elev: 630.54 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,103.91N
 2,514,669.16E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	22/24 92%	ss	19-6 13-19 N=19	13				sandy CLAY with trace gravel.		610	
11B				16			22	Dark yellowish brown (10YR4/6), wet dense, silty, fine- to coarse-grained SAND with trace gravel.			
								Dark yellowish brown (10YR4/6), moist, hard, clayey SILT with sand and gravel.			
								Grayish brown (10YR5/2), moist, dense, silty, very fine- to fine-grained SAND.			
12A	20/24 83%	ss	11-20 19-13 N=39	10				Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.		608	
12B				10				Dark gray (10YR4/1), wet, dense, silty, fine- to coarse-grained SAND with gravel.			
								Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.			
End of Boring = 24.0 ft. BGS											

NOTE(S): G206 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/8/2010
Finish: 10/8/2010
WEATHER: Sunny, mild

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smal
Eng/Geo: R. Hasenyager

BOREHOLE ID: G207
Well ID: G207
Surface Elev: 630.61 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,166.36N
 2,514,837.94E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	ss	6-5 5-8 N=10	15			1	FILL - Yellowish brown (10YR5/6) with 10% gray (10YR6/1) and 5% black (10YR2/1) mottles, slightly moist, hard, silty CLAY with trace sand and slight trace gravel.		630	
2A	24/24 100%	ss	4-4 7-8 N=11	15			2	FILL - Gray (10YR5/1) with 20% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY with sand and trace gravel.		628	
3A	23/24 96%	ss	3-6 7-9 N=13	17			4	FILL - Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, moist, hard, silty CLAY with sand and trace gravel.		626	
4A	24/24 100%	ss	3-4 6-7 N=10	16			6	FILL - Dark yellowish brown (10YR4/4), moist, hard, silty CLAY with slight trace sand and gravel.		624	
5A	24/24 100%	ss	2-2 3-4 N=5	22			8	FILL - Gray (10YR5/1) with 20% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY with slight trace sand and gravel.		622	
5B				24			10	Yellowish brown (10YR5/6) with 25% gray (10YR5/1) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		620	
6A	22/24 92%	ss	2-2 2-5 N=4	27			10	Gray (10YR5/1) with 15% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		620	
7A	24/24 100%	ss	2-2 2-3 N=4	27			12	Gray (10YR5/1) with 40% yellowish brown (10YR5/8) mottles, very moist, firm, silty CLAY with slight trace sand and gravel.		618	
8A	24/24 100%	ss	woh-1 2-3 N=3	25			14	Yellowish brown (10YR5/8) with 30% gray (10YR6/1) mottles, very moist, soft, silty CLAY with trace sand and gravel.		616	
9A	23/24 96%	ss	woh-2 2-3 N=4	22			16	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, very moist, soft, silty CLAY with sand and slight trace gravel.		614	
10A	24/24 100%	ss	woh-woh 2-3	19			18	Yellowish brown (10YR5/6) with 25% gray (10YR6/1) mottles, very moist, very soft, sandy, silty CLAY with trace gravel.		612	
							20	Yellowish brown (10YR5/6) with 30% gray (10YR6/1) mottles, very moist, very soft, sandy CLAY with silt and trace gravel.			

NOTE(S): G207 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/8/2010
Finish: 10/8/2010
WEATHER: Sunny, mild

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: R. Hasenyager

BOREHOLE ID: G207
Well ID: G207
Surface Elev: 630.61 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,166.36N
 2,514,837.94E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = Dry - While drilling ▽ = Dry - Upon completion ▽ =		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	22/24 92%	ss	woh-1 2-2 N=3	18			22	Yellowish brown (10YR5/6) with 30% gray (10YR6/1) mottles, very moist, very soft, sandy CLAY with silt and trace gravel. <i>[Continued from previous page]</i> Gray (10YR6/1) with 40% yellowish brown (10YR5/6) mottles, very moist to wet, loose, silty, very fine- to fine-grained SAND with slight trace gravel.		610	
12A	24/24 100%	ss	10-24 26-30 N=50				24	Gray (10YR5/1), slightly moist, hard, very silty CLAY with trace sand and slight trace gravel.		608	
End of Boring = 24.0 ft. BGS											

NOTE(S): G207 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 10/7/2010
Finish: 10/7/2010
WEATHER: Sunny, warm (lo-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G208
Well ID: G208
Surface Elev: 630.57 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,231.46N
 2,514,993.57E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = Dry - While drilling ▼ = 23.92 - Upon completion ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	ss	4-3 3-5 N=6	23					0	FILL - Brown (10YR4/3) with 5% dark gray (10YR4/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		630	
2A	24/24 100%	ss	2-3 4-6 N=7	14					2			628	
3A	24/24 100%	ss	2-4 4-7 N=8	21					4	FILL - Brown (10YR4/3) with 15% dark gray (10YR4/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		626	
4A	24/24 100%	ss	2-4 6-8 N=10	17					6			624	
5A	20/24 83%	ss	2-2 4-5 N=6	24					8	Very dark gray (10YR3/1), moist, firm, silty CLAY with trace sand and gravel, trace roots.		622	
6A	23/24 96%	ss	1-2 4-4 N=6	26					10	Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand, trace roots.		620	
7A	19/24 79%	ss	1-2 2-3 N=4	23					12	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand, trace roots.		618	
8A	22/24 92%	ss	1-1 2-3 N=3	24					14	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel, trace roots.		616	
9A	24/24 100%	ss	1-1 2-3 N=3	24					16	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel, trace roots.		614	
9B				20					18	Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, very moist, soft, silty, very fine- to fine-grained sandy CLAY with trace gravel.		612	
10A	22/24 92%	ss	woh-woh 1-2	20					20	Dark yellowish brown (10YR4/6) with 15% gray (10YR5/1) mottles, very moist to wet, soft, clayey, very fine- to medium-grained SAND with trace gravel.			
10B				17					20				

NOTE(S): G208 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/7/2010
Finish: 10/7/2010
WEATHER: Sunny, warm (lo-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G208
Well ID: G208
Surface Elev: 630.57 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,231.46N
 2,514,993.57E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = Dry - While drilling ▽ = 23.92 - Upon completion ▽ =		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	20/24 83%	ss	woh-2 5-12 N=7	13			22	Yellowish brown (10YR5/4), moist, firm, very silty CLAY with sand and gravel.		610	
12A	24/24 100%	ss	6-11 18-24 N=29	9			24	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.		608	
							▽	24	End of Boring = 24.0 ft. BGS		

NOTE(S): G208 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/7/2010
Finish: 10/7/2010
WEATHER: Sunny, cool (10-50's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G209
Well ID: G209
Surface Elev: 630.57 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,298.23N
 2,515,149.56E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	4-4 4-6 N=8	21			0			630	
2A	24/24 100%	ss	3-4 6-6 N=10	13			2	FILL - Brown (10YR5/3) with 10% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		628	
3A	24/24 100%	ss	2-3 6-8 N=9	19			4			626	
4A	22/24 92%	ss	2-3 6-8 N=9	17			6			624	
5A	18/24 75%	ss	2-3 3-5 N=6	20			8	Grayish brown (10YR5/2), moist, firm, clayey SILT with trace sand and gravel.		622	
6A	24/24 100%	ss	1-2 2-5 N=4	26			10	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.		620	
7A	22/24 92%	ss	1-3 4-4 N=7	22			12	Dark gray (10YR4/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.		618	
8A	24/24 100%	ss	woh-1 2-3 N=3	25			14			616	
9A	19/24 79%	ss	woh-1 2-3 N=3	24			16			614	
10A	14/24 58%	ss	woh-2 3-3 N=5	20			18	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		612	

NOTE(S): G209 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/7/2010
Finish: 10/7/2010
WEATHER: Sunny, cool (10-50's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G209
Well ID: G209
Surface Elev: 630.57 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,298.23N
 2,515,149.56E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	1/24 4%	ss	woh-1 1-1 N=2	21			21	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel. <i>[Continued from previous page]</i>		610		
12A	20/24 83%	ss	9-16 17-26 N=33	7			22	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.		608		
							24	End of Boring = 24.0 ft. BGS				

NOTE(S): G209 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/6/2010
Finish: 10/6/2010
WEATHER: Sunny, warm (mid-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G210
Well ID: G210
Surface Elev: 630.48 ft. MSL
Completion: 25.00 ft. BGS
Station: 875,359.71N
 2,515,298.97E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	4-3 5-6 N=8	16			0			630	
2A	24/24 100%	ss	3-4 7-7 N=11	15			2	FILL - Brown (10YR4/3) with 20% dark gray (N5/1) mottles, moist, firm, silty CLAY with trace sand and gravel.		628	
3A	24/24 100%	ss	3-5 10-9 N=15	19			4			626	
4A	24/24 100%	ss	3-6 9-11 N=15	17			6	FILL - Dark grayish brown (10YR4/2), slightly moist, firm, clayey SILT with trace sand and gravel.		624	
5A	24/24 100%	ss	3-4 5-7 N=9	15			8	Gray (10YR5/1) with 10% dark grayish brown (10YR4/2) mottles, moist, firm, silty CLAY with trace sand.		622	
6A	22/24 92%	ss	2-2 4-6 N=6	26			10	Dark gray (10YR4/1), moist, firm, silty CLAY with trace sand, trace roots.		620	
7A	19/24 79%	ss	1-3 3-5 N=6	23			12	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with trace sand.		618	
8A	24/24 100%	ss	2-2 2-4 N=4	26			14	Gray (10YR5/1) with 5% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with sand and trace gravel.		616	
9A	24/24 100%	ss	1-1 2-3 N=3	24			16	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with trace sand.		614	
10A	20/24 83%	ss	1-1 2-2 N=3	24			18	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with sand and trace gravel.		612	
10B				31			20	Gray (N6/1), very moist, very soft, silty, very fine- to fine-grained sandy CLAY with trace gravel.			

NOTE(S): G210 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/6/2010
Finish: 10/6/2010
WEATHER: Sunny, warm (mid-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G210
Well ID: G210
Surface Elev: 630.48 ft. MSL
Completion: 25.00 ft. BGS
Station: 875,359.71N
 2,515,298.97E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = 20.00 - While drilling ▽ = 19.90 - Upon completion ▽ =		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	14/24 58%	ss	woh-5 6-18 N=11	12			22	Yellowish brown (10YR5/4), moist, medium dense, silty, fine- to coarse-grained SAND with trace gravel. Gray (N6/1), moist, medium dense, silty, very fine- to fine-grained SAND. Yellowish brown (10YR5/4), slightly moist, hard, clayey SILT with sand and gravel.		610	
12A	19/24 79%	ss	7-18 26-36 N=44	14			24	Brown (10YR5/3), very moist, dense, very fine- to fine-grained sandy SILT with trace gravel. Gray (N5/1), slightly moist, hard, very silty CLAY with sand and gravel.		608	
13A	10/12 83%	ss	27-50/6"	9						606	

End of Boring = 25.0 ft. BGS

NOTE(S): G210 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/11/2010
Finish: 10/11/2010
WEATHER: Sunny, warm (lo-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G211
Well ID: G211
Surface Elev: 630.31 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,424.49N
 2,515,449.06E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 20.00 - While drilling ▽ = 20.60 - Upon completion ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	5-3 4-5 N=7	23					2	FILL - Brown (10YR4/3) with 20% dark gray (10YR4/1) mottles, moist, firm, silty CLAY with trace sand and gravel.		630	
2A	20/24 83%	ss	3-5 5-8 N=10	17					4			628	
3A	24/24 100%	ss	2-3 5-7 N=8	24					6	FILL - Dark gray (10YR4/1) with 20% brown (10YR4/3) and 5% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		626	
4A	24/24 100%	ss	3-5 7-9 N=12	29					8			624	
5A	24/24 100%	ss	2-2 3-5 N=5	31					10	Dark gray (10YR4/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY, slight trace roots.		622	
6A	17/24 71%	ss	1-2 4-4 N=6	19					12	Dark gray (10YR4/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.		620	
7A	24/24 100%	ss	1-2 2-4 N=4	22					14	Dark gray (10YR4/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		618	
8A	24/24 100%	ss	1-1 3-2 N=4	28					16			616	
9A	22/24 92%	ss	1-1 1-2 N=2	19					18	Dark yellowish brown (10YR4/6) with 30% gray (10YR5/1) mottles, very moist, soft, very fine- to fine-grained sandy CLAY with trace gravel.		614	
10A	19/24 79%	ss	1-4 5-11 N=9	13					20	Dark yellowish brown (10YR4/4) with 15% grayish brown (10YR5/2) mottles, moist, firm, clayey SILT with sand and gravel.		612	

NOTE(S): G211 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 10/11/2010
Finish: 10/11/2010
WEATHER: Sunny, warm (lo-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G211
Well ID: G211
Surface Elev: 630.31 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,424.49N
 2,515,449.06E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	18/24 75%	ss	7-17 23-21 N=40	10			20.00	Grayish brown (10YR5/2), slightly moist, hard, very silty CLAY with sand and gravel.		610	
12A	24/24 100%	ss	4-14 15-17 N=29	13			24.00	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.		608	
End of Boring = 24.0 ft. BGS											

NOTE(S): G211 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/11/2010
Finish: 10/11/2010
WEATHER: Sunny, warm (lo-80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G212
Well ID: G212
Surface Elev: 630.59 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,486.50N
 2,515,583.03E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 19.00 - While drilling ▽ = 20.72 - Upon completion ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	4-3 3-6 N=6	17					1	FILL - Brown (10YR4/3), slightly moist, firm, silty CLAY with trace sand and gravel.		630	
2A	24/24 100%	ss	2-3 4-5 N=7	21					2	FILL - Dark gray (10YR4/1) with 20% brown (10YR4/3) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.		628	
3A	24/24 100%	ss	2-5 6-7 N=11	13					4	FILL - Brown (10YR4/3) with 15% dark gray (10YR4/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		626	
4A	24/24 100%	ss	2-5 7-10 N=12	15					6	FILL - Brown (10YR4/3) with 15% dark gray (10YR4/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		624	
5A	24/24 100%	ss	2-2 4-7 N=6	29					8	Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY, slight trace roots.		622	
6A	18/24 75%	ss	2-3 4-6 N=7	23					10	Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.		620	
7A	17/24 71%	ss	1-2 2-2 N=4	25					12	Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.		618	
8A	24/24 100%	ss	woh-1 2-3 N=3	27					14	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		616	
9A	22/24 92%	ss	1-1 2-2 N=3	25					16	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		614	
10A	24/24 100%	ss	woh-woh 1-2	19					18	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, very moist, very soft, silty, very fine- to fine-grained sandy CLAY with trace gravel.		612	
10B				22					20	Gray (10YR5/1), loose, wet, silty, very fine- to			

NOTE(S): G212 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/11/2010
Finish: 10/11/2010
WEATHER: Sunny, warm (lo-80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G212
Well ID: G212
Surface Elev: 630.59 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,486.50N
 2,515,583.03E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = 19.00 - While drilling ▽ = 20.72 - Upon completion ▽ =		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	7/24 29%	ss	1-6 10-22 N=16	19			20	medium-grained SAND. Brown (10YR5/3), moist, medium dense, SILT with trace sand and gravel.		610	
12A	20/24 83%	ss	5-21 18-27 N=39	12			22	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.		608	
24 End of Boring = 24.0 ft. BGS											

NOTE(S): G212 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/12/2010
Finish: 10/12/2010
WEATHER: Partly cloudy, mild (mid-50's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G213
Well ID: G213
Surface Elev: 630.34 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,544.37N
 2,515,723.51E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 20.00 - While drilling ▽ = 19.92 - Upon completion ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	ss	4-3 4-5 N=7	15					0			630	
2A	22/24 92%	ss	2-4 6-8 N=10	21					2			628	
3A	22/24 92%	ss	2-4 7-8 N=11	17			FILL - Brown (10YR4/3) with 10% dark gray (10YR4/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, silty CLAY with trace sand and gravel.		4			626	
4A	22/24 92%	ss	2-4 4-8 N=8	16					6			624	
5A	20/24 83%	ss	1-3 6-6 N=9	12					8			622	
6A	20/24 83%	ss	2-2 5-7 N=7	24			Dark gray (10YR4/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand, trace roots.		10			620	
7A	20/24 83%	ss	2-3 3-5 N=6	19			Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT, trace roots.		12			618	
8A	22/24 92%	ss	1-2 2-3 N=4	24			Dark gray (10YR4/1) with 3% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		14			616	
9A	24/24 100%	ss	woh-1 2-2 N=3	24			Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		16			614	
10A	18/24 75%	ss	woh-woh 1-2	24			Gray (10YR5/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		18			612	
							Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, soft, very fine- to fine-grained sandy CLAY with trace gravel.		20				
							Dark yellowish brown (10YR4/6), very moist, soft, sandy CLAY with trace gravel.		24				

NOTE(S): G213 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/12/2010
Finish: 10/12/2010
WEATHER: Partly cloudy, mild (mid-50's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G213
Well ID: G213
Surface Elev: 630.34 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,544.37N
 2,515,723.51E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
							Quadrangle: Coffeen, IL	Township: East Fork	Section 11, Tier 7N; Range 3W	▼ = 20.00 - While drilling	▽ = 19.92 - Upon completion	▽ =
11A	18/24 75%	ss	1-1 1-2 N=2	18			20	Dark yellowish brown (10YR4/6), moist, soft, clayey, very fine- to coarse-grained SAND with trace gravel.		610		
							22	Dark yellowish brown (10YR4/6), moist, firm, very silty CLAY with sand and gravel.				
12A	22/24 92%	ss	10-13 18-22 N=31	11			24	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.		608		
End of Boring = 24.0 ft. BGS												

NOTE(S): G213 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/14/2010
Finish: 10/14/2010
WEATHER: Sunny, cool (10-40's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G214
Well ID: G214
Surface Elev: 630.39 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,668.02N
 2,515,960.84E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	6-7 7-9 N=14	15			2	FILL - Brown (10YR4/3) with 10% dark gray (10YR4/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		630	
2A	24/24 100%	SS	3-3 6-5 N=9	22			4	FILL - Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		628	
3A	24/24 100%	SS	3-4 6-8 N=10	18			6	FILL - Brown (10YR4/3) with 10% dark yellowish brown (10YR4/6) and 5% dark gray (10YR4/1) mottles, moist, firm, silty CLAY with trace sand and gravel.		626	
4A	24/24 100%	SS	3-4 7-10 N=11	17			8			624	
5A	24/24 100%	SS	3-2 4-5 N=6	19			10	Grayish brown (10YR5/2) with 15% dark gray (10YR4/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with trace sand and gravel.		622	
6A	24/24 100%	SS	2-3 4-7 N=7	24			12	Brown (10YR4/3) with 15% dark gray (10YR4/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		620	
7A	24/24 100%	SS	2-3 4-6 N=7	22			14	Gray (10YR6/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY.		618	
7B				16							
8A	24/24 100%	SS	woh-2 3-4 N=5	22			16	Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		616	
9A	22/24 92%	SS	1-2 2-3 N=4	21			18	Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		614	
10A	22/24 92%	SS	woh-2 2-2 N=4	15			20	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		612	
10B				21				Gray (10YR5/1), wet, loose, silty, very fine- to medium-grained SAND with trace gravel and clayey seams.			

NOTE(S): G214 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 10/14/2010
Finish: 10/14/2010
WEATHER: Sunny, cool (10-40's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G214
Well ID: G214
Surface Elev: 630.39 ft. MSL
Completion: 24.00 ft. BGS
Station: 875,668.02N
 2,515,960.84E

SAMPLE			TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	20/24 83%	ss	woh-woh 3-12	24			22	Gray (10YR5/1), wet, loose, silty, very fine- to medium-grained SAND with trace gravel and clayey seams. <i>[Continued from previous page]</i>		610		
11B				14				Dark yellowish brown (10YR4/6), moist, firm, very silty CLAY with trace sand and gravel.				
12A	24/24 100%	ss	12-28 32-28 N=60	7			24	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with trace sand and gravel.		608		
End of Boring = 24.0 ft. BGS												

NOTE(S): G214 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 10/13/2010
Finish: 10/13/2010
WEATHER: Sunny, warm, windy (hi-60's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smal
Eng/Geo: .

BOREHOLE ID: G215
Well ID: G215
Surface Elev: 630.48 ft. MSL
Completion: 24.31 ft. BGS
Station: 875,810.19N
 2,515,971.55E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = Dry - While drilling ▽ = Dry - Upon completion ▽ = 22.52 - 10/14/10	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	ss	5-3 3-5 N=6	18					0			630	
2A	19/24 79%	ss	3-3 5-6 N=8	17					2	FILL - Brown (10YR4/3) with 30% dark gray (10YR4/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		628	
3A	20/24 83%	ss	2-3 7-7 N=10	13					4			626	
4A	23/24 96%	ss	3-6 6-7 N=12	16					6	FILL - Dark grayish brown (10YR4/2), moist, firm, silty CLAY with trace sand and gravel.		624	
4B				27					8	FILL - Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand.			
5A	20/24 83%	ss	3-3 3-5 N=6	20					8	Very dark gray (10YR3/1), moist, firm, silty CLAY with trace sand, trace roots.		622	
6A	13/24 54%	ss	2-2 3-5 N=5	24					10	Dark gray (10YR4/1) with 30% dark yellowish brown (10YR4/6) moist, firm, silty CLAY with trace sand.		620	
7A	19/24 79%	ss	2-3 4-6 N=7	17					12	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with trace sand.		618	
8A	20/24 83%	ss	2-3 4-5 N=7	19					14	Dark gray (10YR4/1), moist, firm, clayey SILT with trace sand.		616	
9A	22/24 92%	ss	1-3 3-4 N=6	19					16	Dark gray (10YR4/1) with 30% Dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		614	
10A	24/24 100%	ss	woh-1 2-2 N=3	17					18	Dark gray (10YR4/1) with 30% Dark yellowish brown (10YR4/6) mottles, moist, soft, sandy CLAY with trace gravel.		612	

NOTE(S): G215 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers

BOREHOLE ID: G215
Well ID: G215
Surface Elev: 630.48 ft. MSL
Completion: 24.31 ft. BGS
Station: 875,810.19N
 2,515,971.55E

DATES: Start: 10/13/2010
Finish: 10/13/2010

FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

WEATHER: Sunny, warm, windy (hi-60's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = Dry - While drilling ▽ = Dry - Upon completion ▽ = 22.52 - 10/14/10		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	20/24 83%	SS	2-4 4-4 N=8	17			22	Dark yellowish brown (10YR4/6), moist, medium dense, clayey SILT with sand and trace gravel.		610	
12A	24/24 100%	SS	7-11 17-19 N=28	11			▽	Yellowish brown (10YR5/6), moist, medium dense, silty, very fine- to fine-grained SAND.			
								Dark yellowish brown (10YR4/6) with 30% dark gray (10YR4/1) mottles, moist, firm, sandy CLAY with trace gravel.			
12B	0/4 0%	BD		9				Grayish brown (10YR5/2), slightly moist, very firm, very silty CLAY with sand and gravel.		608	
								Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.			

End of Boring = 24.3 ft. BGS

NOTE(S): G215 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers

BOREHOLE ID: G216
Well ID: G216
Surface Elev: 630.28 ft. MSL
Completion: 26.00 ft. BGS
Station: 875,976.05N
 2,515,968.53E

DATES: Start: 10/13/2010
 Finish: 10/13/2010

FIELD STAFF: Driller: D. Mahurin
 Helper: J. Litsch/D. Smail
 Eng/Geo: .

WEATHER: Partly cloudy, mild, windy (10-60's)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 21.00 - While drilling ▽ = Dry - Upon completion ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	3-2 2-4 N=4	22									630	
2A	19/24 79%	ss	2-2 5-4 N=7	28							FILL - Brown (10YR4/3) with 20% dark gray (10YR4/6) and 5% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		628	
3A	24/24 100%	ss	2-2 4-5 N=6	19							FILL - Dark gray (10YR4/1) with 15% brown (10YR4/3) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with trace sand and gravel.		624	
4A	22/24 92%	ss	3-4 6-6 N=10	19							FILL - Dark grayish brown (10YR4/2) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		622	
5A	20/24 83%	ss	2-3 3-6 N=6	18							Dark gray (10YR4/1) with 40% gray (10YR6/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with trace sand.		620	
6A	18/24 75%	ss	2-3 3-4 N=6	17									618	
7A	16/24 67%	ss	1-2 3-4 N=5	20							Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		616	
8A	22/24 92%	ss	2-2 5-5 N=7	20							Very dark gray (10YR3/1), moist, firm, silty CLAY with trace sand.		614	
9A	23/24 96%	ss	woh-2 3-3 N=5	18							Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		612	
10A	24/24 100%	ss	woh-woh 1-2	17							Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, sandy CLAY with trace gravel. Dark yellowish brown (10YR4/6) and 40% gray			

NOTE(S): G216 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers

BOREHOLE ID: G216
Well ID: G216
Surface Elev: 630.28 ft. MSL
Completion: 26.00 ft. BGS
Station: 875,976.05N
 2,515,968.53E

DATES: Start: 10/13/2010
Finish: 10/13/2010

FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

WEATHER: Partly cloudy, mild, windy (10-60's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	20/24 83%	ss	2-9 9.7 N=18	14			21.00	(10YR5/1) mottles, moist, soft, sandy CLAY with trace gravel. [Continued from previous page] Light brownish gray (10YR6/2), wet, medium dense, very fine- to fine-grained sandy SILT with trace gravel. Yellowish brown (10YR5/6), wet, medium dense, silty, very fine- to medium-grained SAND. Gray (10YR5/1), very moist, medium dense, very fine- to fine-grained sandy SILT.		610	
12A	22/24 92%	ss	5-10 21-25 N=31	19			22	Gray (10YR5/1), wet, medium dense, very fine- to fine-grained sandy SILT.		608	
12B				16			24	Dark gray (10YR4/1), wet, dense, silty, fine- to coarse-grained SAND with gravel.		606	
13A	20/24 83%	ss	14-25 27-27 N=52	9			26	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.			

End of Boring = 26.0 ft. BGS

NOTE(S): G216 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/12/2010
Finish: 10/12/2010
WEATHER: Sunny, warm (lo-80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G217
Well ID: G217
Surface Elev: 630.67 ft. MSL
Completion: 26.00 ft. BGS
Station: 876,185.57N
 2,515,963.02E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	5-2 3-4 N=5	21			0			630	
2A	19/24 79%	ss	2-3 5-6 N=8	28			2	FILL - Brown (10YR4/3) with 10% dark gray (10YR4/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		628	
3A	19/24 79%	ss	2-3 6-7 N=9	14			4	FILL - Dark gray (10YR4/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		626	
4A	23/24 96%	ss	5-6 7-8 N=13	15			6	FILL - Brown (10YR4/3) with 10% dark gray (10YR4/1) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		624	
5A	20/24 83%	ss	3-5 7-6 N=12	13			8	FILL - Dark grayish brown (10YR4/2) with 5% dark yellowish brown (10YR4/6) slightly moist, firm, clayey SILT with trace sand and gravel.		622	
6A	19/24 79%	ss	3-3 4-5 N=7	27			10	FILL - Very dark gray (10YR3/1), moist, firm, silty CLAY with trace sand and gravel. Dark gray (10YR4/1), moist, firm, silty CLAY with trace sand and gravel.		620	
7A	18/24 75%	ss	3-4 6-8 N=10	28			12	Dark grayish brown (10YR4/2) with 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY trace sand, trace roots.		618	
8A	20/24 83%	ss	2-4 6-8 N=10	16			14	Dark gray (10YR4/1), moist, firm, silty CLAY with trace sand and gravel.		616	
9A	19/24 79%	ss	2-3 4-5 N=7	26			16	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		614	
10A	19/24 79%	ss	1-2 2-3 N=4	18			18	Gray (10YR5/1) with 5% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with sand and trace gravel.		612	
							20				

NOTE(S): G217 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/12/2010
Finish: 10/12/2010
WEATHER: Sunny, warm (lo-80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G217
Well ID: G217
Surface Elev: 630.67 ft. MSL
Completion: 26.00 ft. BGS
Station: 876,185.57N
 2,515,963.02E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = 23.00 - While drilling ▽ = 24.82 - Upon completion ▽ = 23.98 - 10/13/10		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	22/24 92%	ss	woh-woh 1-2				18	Gray (10YR5/1) with 5% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with sand and trace gravel. <i>[Continued from previous page]</i>		610	
12A	10/24 42%	ss	4-6 7-10 N=13				22	Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with sand and trace gravel.		608	
13A	22/24 92%	ss	8-18 17-17 N=35				24	Yellowish brown (10 YR5/4), wet, medium dense, silty, very fine- to coarse-grained SAND with trace gravel.		606	
							26	Dark gray (10YR4/1), slightly moist, hard, very silty CLAY with sand and gravel.			
End of Boring = 26.0 ft. BGS											

NOTE(S): G217 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/12/2010
Finish: 10/12/2010
WEATHER: Partly cloudy, warm (lo-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G218
Well ID: G218
Surface Elev: 630.64 ft. MSL
Completion: 26.00 ft. BGS
Station: 876,380.92N
 2,515,962.16E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 24.00 - While drilling ▽ = 24.76 - Upon completion ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	4-1 2-1 N=3	20					0	FILL - Brown (10YR4/3) with 15% dark gray (10YR4/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		630	
2A	22/24 92%	ss	2-2 3-5 N=5	20					2			628	
3A	19/24 79%	ss	2-3 4-8 N=7	17					4	FILL - Dark gray (10YR4/1) with 30% brown (10YR4/3) and 10% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand and gravel.		626	
4A	22/24 92%	ss	2-5 6-8 N=11	14					6			624	
5A	20/24 83%	ss	3-4 8-7 N=12	17					8	FILL - Brown (10YR5/3) with 10% dark gray (10YR4/1) mottles, slightly moist, firm, clayey SILT with trace sand and gravel.		622	
6A				19					10	Dark grayish brown (10YR4/2) with 5% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with trace sand.		620	
6B	19/24 79%	ss	2-2 3-5 N=5	25					12	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with trace sand, slight trace roots.		618	
7A	22/24 92%	ss	2-3 5-7 N=8	22					14	Dark gray (10YR4/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with trace sand.		616	
8A	18/24 75%	ss	2-3 4-5 N=7	19					16			614	
9A	24/24 100%	ss	2-2 2-4 N=4	19					18	Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.		612	
10A	24/24 100%	ss	1-2 2-3 N=4	18					20				

NOTE(S): G218 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 10/12/2010
Finish: 10/12/2010
WEATHER: Partly cloudy, warm (lo-70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: .

BOREHOLE ID: G218
Well ID: G218
Surface Elev: 630.64 ft. MSL
Completion: 26.00 ft. BGS
Station: 876,380.92N
 2,515,962.16E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = 24.00 - While drilling ▽ = 24.76 - Upon completion ▽ =		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	22/24 92%	ss	woh-woh woh-woh	16			22	Gray (10YR5/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, very soft, clayey, very fine- to coarse-grained SAND with trace gravel.		610	
12A	24/24 100%	ss	1-1 1-3 N=2	10			24	Yellowish brown, wet, loose, silty, very fine- to coarse-grained SAND with trace gravel.		608	
12B	24/24 100%	ss	1-5 9-13 N=14	16			24	Dark gray (10YR4/1) with 10% dark yellowish brown (10YR4/6) mottles, moist, soft, silty CLAY with trace sand and gravel.			
13A	24/24 100%	ss	1-5 9-13 N=14	20			24	Gray (10YR5/1), wet, loose, silty, very fine- to coarse-grained SAND with trace gravel.		606	
13B				17			26	Dark gray (10YR4/1), slightly moist, very firm, very silty CLAY with sand and gravel.			
End of Boring = 26.0 ft. BGS											

NOTE(S): G218 installed in borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/26/2008
Finish: 2/26/2008
WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: .

BOREHOLE ID: G270
Well ID: G270
Surface Elev: 622.92 ft. MSL
Completion: 18.27 ft. BGS
Station: 874,801.92N
 2,514,996.84E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	2-2 2-4 N=4		24			Dark grayish brown (10YR4/2), moist, firm, clayey SILT		622	
2A	19/24 79%	ss	3-4 5-9 N=9		22	2.33 B		Dark grayish brown (10YR4/2), moist, firm, silty CLAY		620	
2B					20	5.04 Sh	4	Dark grayish brown (10YR4/2) with 5% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand			
3A	20/24 83%	ss	14-5 7-8 N=12		17	2.52 Sh	6	Gray (10YR5/1) with 70% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand and gravel		618	
4A	24/24 100%	ss	8-6 7-5 N=13		21	1.24 BSh		Dark gray (10YR4/1) with 5% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, trace sand, slight trace gravel		616	
4B					21	1.20 B	8	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, trace sand, slight trace gravel		614	
5A	22/24 92%	ss	2-3 4-4 N=7		21	1.36 B	10	Gray (10YR5/1) with 60% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, trace sand, slight trace gravel		612	
6A	24/24 100%	ss	1-2 2-3 N=4		21	0.74 BSh		Gray (10YR5/1), moist, soft, sandy CLAY		610	
6B					24	0.78 B	12	Gray (10YR5/1), moist, soft, fine- to coarse-grained SAND, trace gravel		608	
7A	17/24 71%	ss	2-2 2-3 N=4		21			Dark yellowish brown (10YR4/4), moist, soft, sandy CLAY		606	
8A					20		14	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand and gravel			
8B	19/24 79%	ss	1-3 5-6 N=8		17	4.46 Sh		Yellowish brown (10YR5/4), wet, soft, fine to coarse SAND		606	
9A	24/24 100%	ss	6-8 30-35 N=38		20			Gray (10YR5/1), moist, hard, silty CLAY, trace sand and gravel			
9B					8		18				

End of Boring = 18.27 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/9/2009
Finish: 9/10/2009
WEATHER: Sunny, warm (70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G271
Well ID: G271
Surface Elev: 622.89 ft. MSL
Completion: 16.00 ft. BGS
Station: 874,239.38N
 2,515,517.12E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
							Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		Water Level Information: ▼ = 12.70 - While drilling ▼ = 12.50 - 9/21/09 ▼ =		
1A	20/24 83%	SS	2-5 5-6 N=10	27			0	FILL - Yellowish brown (10YR5/4), moist, firm, silty CLAY with trace sand. Grayish brown (10YR5/2), dry, friable, clayey SILT.		622	
2A	24/24 100%	SS	2-6 5-5 N=11	23			2	Yellowish brown (10YR5/6) with 10% gray (10YR6/1) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel.		620	
3A	23/24 96%	SS	4-5 4-5 N=9	18			4	Gray (10YR6/1), moist, firm, very silty CLAY with slight trace sand.		618	
4A	24/24 100%	SS	2-4 4-5 N=8	17			6	Gray (10YR5/1) with 30% brownish yellow (10YR6/6) mottles, moist, firm, silty CLAY with sand and trace gravel.		616	
5A	24/24 100%	SS	2-4 4-6 N=8	20			8	Very dark gray (10YR3/1), organic-rich (PEAT), silty CLAY and trace sand.		614	
6A	24/24 100%	SS	2-4 4-5 N=8	22			10	Brownish yellow (10YR6/8) with 20% gray (10YR5/1) mottles, moist, firm, silty CLAY with sand and trace gravel.		612	
6B				20			12	Gray (10YR6/1) with 20% brownish yellow (10YR6/8) mottles, very moist, soft, sandy CLAY with silt and slight trace gravel.			
7A				20			12				
7B	20/24 83%	SS	2-2 3-7 N=5	19			13	Brownish yellow (10YR6/6), very moist to wet, soft, sandy CLAY with silt and slight trace gravel.		610	
							14	Gray (10YR6/1), wet, loose, very fine to medium SAND with silt.			
8A	24/24 100%	SS	10-19 30-33 N=49	7			16	Gray (10YR5/1), slightly moist, hard, very silty CLAY with sand and gravel.		608	

End of Boring = 16.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/10/2009
Finish: 9/10/2009
WEATHER: Sunny, warm (70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G272
Well ID: G272
Surface Elev: 620.72 ft. MSL
Completion: 14.32 ft. BGS
Station: 874,234.83N
 2,515,744.99E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	SS	3-2 3-4 N=5	18			0	FILL - Brownish yellow (10YR6/6) with 30% gray (10YR5/1) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel.		620	
2A	24/24 100%	SS	4-5 6-7 N=11	25			2	Brownish yellow (10YR6/6) with 20% brownish yellow (10YR6/8) and 20% gray (10YR5/1) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel.		618	
3A	23/24 96%	SS	2-4 4-6 N=8	18			4			616	
3B				17			6				
4A	24/24 100%	SS	3-4 4-4 N=8	20			8	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with sand and trace gravel.		614	
5A	23/24 96%	SS	2-3 3-5 N=6	21			10	Yellowish brown (10YR5/8) with 15% gray (10YR6/1) mottles, moist, soft, silty CLAY with sand and slight trace gravel.		612	
6A	22/24 92%	SS	2-3 3-3 N=6	23			12	Gray (10YR6/1) with 30% brownish yellow (10YR6/6) mottles, very moist, soft, silty CLAY with sand and slight trace gravel.		610	
7A	18/24 75%	SS	2-9 15-21 N=24	14			14	Gray (10YR6/1), very moist, loose, SILT and very fine sand.		608	
7B				10			14	Gray (10YR6/1), wet, loose, SILT and very fine sand.			
							14	Yellowish brown (10YR5/6), moist, hard, very silty CLAY with sand and trace gravel.			

End of Boring = 14.3 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/10/2009
Finish: 9/10/2009
WEATHER: Sunny, warm (70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G273
Well ID: G273
Surface Elev: 620.17 ft. MSL
Completion: 16.00 ft. BGS
Station: 874,235.24N
 2,515,975.49E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 13.50 - While drilling ▽ = 9.89 - 9/21/09 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	SS	3-3 3-3 N=6	24					0	FILL -Dark yellowish brown (10YR4/6), moist, firm, silty CLAY with slight trace sand.		620	
2A	24/24 100%	SS	3-4 5-7 N=9	28					2	FILL - Gray (10YR5/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		618	
3A				18					4	Brownish yellow (10YR6/8) with 40% gray (10YR5/1) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel.		616	
3B	24/24 100%	SS	3-5 6-8 N=11	25					6	Gray (10YR6/1) with 10% brownish yellow(10YR6/6) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel.		614	
4A				19					8	Gray (10YR5/1), moist, firm, silty CLAY with sand and trace gravel.		612	
4B	24/24 100%	SS	3-5 5-6 N=10	16					10	Yellowish brown (10YR5/8) with 30% gray (10YR6/1) mottles, moist, soft, sandy CLAY with silty and slight trace gravel.		610	
5A	23/24 96%	SS	2-4 5-4 N=9	19					12	Brownish yellow (10YR6/8) with 10% gray (10YR6/1) mottles, very moist, soft, sandy CLAY with silt and slight trace gravel.		608	
5B				21					14	Gray (10YR6/1), moist, hard, very silty CLAY with sand and trace gravel.		606	
6A	24/24 100%	SS	1-2 3-4 N=5	19					16	Light yellowish brown (10YR6/4), wet, loose, very fine- to very coarse-grained SAND with trace silt.		606	
7A	24/24 100%	SS	4-8 17-24 N=25	11					16	Light yellowish brown (10YR6/4), wet, dense, sandy, silty CLAY.		606	
7B				11					16	Gray (10YR6/1), moist, hard, very silty CLAY with sand and trace gravel.		606	
8A	22/24 92%	SS	9-22 22-23 N=44	8					16				

End of Boring = 16.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/16/2009
Finish: 9/16/2009
WEATHER: Sunny, warm (80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G274
Well ID: G274
Surface Elev: 621.67 ft. MSL
Completion: 18.06 ft. BGS
Station: 874,239.25N
 2,516,195.60E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 16.00 - While drilling ▽ = 13.12 - 9/21/09 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	SS	2-3 3-4 N=6		17				2	FILL - Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel.		620	
2A	24/24 100%	SS	4-6 7-9 N=13		25				4			618	
3A	24/24 100%	SS	3-4 6-9 N=10		26				6			616	
3B					21				6			616	
4A	16/24 67%	SS	3-6 6-8 N=12		24				8	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		614	
5A	24/24 100%	SS	2-4 4-6 N=8		20				10	Dark gray (10YR4/1) with 15% yellowish brown (10YR5/8) mottles, moist, soft, silty CLAY with trace sand and slight trace gravel.		612	
6A	22/24 92%	SS	1-3 4-6 N=7		19				12			610	
7A	23/24 96%	SS	1-2 4-4 N=6		21				14	Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, soft, silty CLAY with sand and trace gravel.		608	
8A	22/24 92%	SS	1-3 3-6 N=6		17				16	Yellowish brown (10YR5/8), very moist, soft, silty CLAY with sand and trace gravel.		606	
9A	14/24 58%	SS	wor-4 9-11 N=13		13				16	Brownish yellow (10YR6/6), wet, loose, very fine- to very coarse-grained SAND.		604	
9B					13				18	Brownish yellow (10YR6/6), moist, firm, very silty CLAY with sand and gravel.		604	
					13				18	Gray (10YR6/1), moist, hard, very silty CLAY with sand and gravel.		604	

End of Boring = 18.1 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/16/2009
Finish: 9/16/2009
WEATHER: Sunny, warm (80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G275
Well ID: G275
Surface Elev: 616.14 ft. MSL
Completion: 13.19 ft. BGS
Station: 874,298.94N
 2,516,375.86E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	22/24 92%	SS	2-4 4-6 N=8	22			0	FILL - Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel.		616	
2A	18/24 75%	SS	3-6 7-7 N=13	19			2	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		614	
3A	24/24 100%	SS	2-4 4-5 N=8	22			4	Gray (10YR6/1), moist, soft, silty CLAY with sand and slight trace gravel.		612	
4A	24/24 100%	SS	2-3 4-6 N=7	17			6	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, soft, silty CLAY with sand and slight trace gravel.		610	
5A	24/24 100%	SS	1-2 3-3 N=5	18			8	Yellowish brown (10YR5/8) with 15% gray (10YR6/1) mottles, very moist, soft, silty CLAY with sand and trace gravel.		608	
5B				21			10	Gray (10YR6/1) with 20% yellowish brown (10YR5/8) mottles, very moist, very soft, silty CLAY with sand and trace gravel.		606	
6A				16			10				
6B	20/24 83%	SS	woh-4 5-2 N=9	13			11	Gray (10YR6/1), wet, loose, very fine- to very coarse-grained SAND with trace gravel.		604	
6C				15			12	Yellowish brown (10YR5/6), moist, hard, very silty CLAY with sand and gravel.		604	
7A	12/12 100%	SS	10-35	8			13.2				

End of Boring = 13.2 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/16/2009
Finish: 9/16/2009
WEATHER: Sunny, mild (70'S)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G276
Well ID: G276
Surface Elev: 629.14 ft. MSL
Completion: 28.00 ft. BGS
Station: 874,438.60N
 2,516,358.83E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	SS	5-8 9-10 N=17	10			2			628	
2A	19/24 79%	SS	7-7 10-14 N=17	15			4	FILL - Yellowish brown (10YR5/4) with 20% gray (10YR5/1) mottles, moist, hard, silty CLAY with trace sand and slight trace gravel.		626	
3A	11/24 46%	SS	5-10 14-27 N=24	14			6			624	Rock fragment in split spoon shoe
4A	24/24 100%	SS	5-9 10-14 N=19	8			8	FILL - Yellowish brown (10YR5/4) with 10% gray (10YR5/1) mottles, slightly moist, hard, friable, clayey SILT with sand and trace gravel.		622	
4B				5			8				
5A	17/24 71%	SS	4-4 8-19 N=12	22			10	FILL - Yellowish brown (10YR5/4) with 25% gray (10YR5/1) mottles, slightly moist, firm, silty CLAY with slight trace sand and gravel.		620	
6A	17/24 71%	SS	4-5 8-14 N=13	14			12			618	
7A	16/24 67%	SS	6-7 2-4 N=9	20			14			616	
8A	20/24 83%	SS	2-4 6-6 N=10	21			16	Gray (10YR6/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		614	
9A				17			18				
9B	22/24 92%	SS	1-4 5-7 N=9	13			18	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, sandy CLAY with silt and slight trace gravel.		612	
10A	23/24 96%	SS	2-3 8-12 N=11	20			20	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and slight trace gravel.		610	

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/16/2009
Finish: 9/16/2009
WEATHER: Sunny, mild (70'S)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G276
Well ID: G276
Surface Elev: 629.14 ft. MSL
Completion: 28.00 ft. BGS
Station: 874,438.60N
 2,516,358.83E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%	ss	1-3 5-7 N=8	21			21	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and slight trace gravel. <i>[Continued from previous page]</i>		608	
12A	24/24 100%	ss	1-4 6-7 N=10	16			22	Yellowish brown (10YR5/8), moist, firm, silty CLAY with sand and slight trace gravel.		606	
13A	24/24 100%	ss	2-3 4-6 N=7	16			24	Gray (10YR6/1) with 25% yellowish brown (10YR5/6) mottles, very moist, soft, silty CLAY with sand and trace gravel.		604	
14A	24/24 100%	ss	1-5 15-29 N=20	16			26	Gray (10YR6/1), very moist, loose, very fine- to fine-grained, SAND		602	
							26	Gray (10YR6/1) with 25% yellowish brown (10YR5/6) mottles, very moist, soft, silty CLAY with sand and trace gravel.			
							26	Gray (10YR6/1), very moist, firm, clayey SILT with trace very fine-grained sand.			
							27	Gray (10YR6/1) with 40% yellowish brown (10YR5/4) mottles, moist, hard, very silty CLAY with sand and trace gravel.			
							28	Yellowish brown (10YR5/4), moist, hard, very silty CLAY with sand and trace gravel.			

End of Boring = 28.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/14/2009
Finish: 9/14/2009
WEATHER: Sunny, mild (70'S)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G277
Well ID: G277
Surface Elev: 620.79 ft. MSL
Completion: 20.00 ft. BGS
Station: 874,581.80N
 2,516,370.51E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 16.40 - While drilling ▽ = 18.23 - 9/21/09 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	ss	2-2 4-6 N=6		21				0	FILL - Yellowish brown (10YR5/4) with 10% yellowish brown (10YR5/8) mottles, very moist, soft, silty CLAY with slight trace sand and gravel.		620	
2A	24/24 100%	ss	2-4 6-7 N=10		22				2	FILL - Yellowish brown (10YR5/4) with 20% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		618	
2B					16				4	Yellowish brown (10YR5/4) with 20% gray (10YR5/1) and 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with slight trace sand.			
3A					11				4	Light brownish gray (10YR6/2), dry, friable, clayey SILT.			
3B	17/24 71%	ss	4-7 9-11 N=16		22				6	Light brownish gray (10YR6/2) with 25% yellowish brown (10YR5/8) mottles, moist, firm, clayey SILT with slight trace sand.		616	
4A	18/24 75%	ss	4-8 8-6 N=16		13				8	Gray (10YR6/1) with 50% very dark grayish brown (10YR3/2) mottles, moist, hard, clayey SILT with slight trace sand and gravel.		614	
5A	19/24 79%	ss	3-7 8-9 N=15		12				10	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, hard, very silty CLAY with sand and slight trace gravel.		612	
6A	22/24 92%	ss	3-9 9-10 N=18		14				12			610	
6B					16				14				
7A	18/24 75%	ss	3-5 7-9 N=12		17				16	Yellowish brown (10YR5/8) with 30% gray (10YR6/1) mottles, moist, hard, silty CLAY with sand and trace gravel.		608	
8A	22/24 92%	ss	2-5 5-8 N=10		12				18	Gray (10YR6/1), moist, slightly dense, silty, very fine- to very coarse-grained SAND.		606	
9A	22/24 92%	ss	1-2 3-3 N=5		14				18	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel.		604	
10A					11				18	Gray (10YR6/1) with 20% brownish yellow (10YR6/6) mottles, wet, very soft, silty, very fine- to fine-grained SAND with trace gravel.			
10B	23/24 96%	ss	1-3 19-47 N=22		9				18	Gray (10YR6/1) with 30% grayish brown (10YR5/2) mottles, moist, soft, sandy CLAY with slight trace gravel.		602	
10									20	Brownish yellow (10YR6/6) with 25% gray (10YR6/1) mottles, moist, hard, very silty CLAY with sand and trace gravel.			

End of Boring = 20.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/11/2009
Finish: 9/11/2009
WEATHER: Sunny, warm (70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G278
Well ID: G278
Surface Elev: 628.85 ft. MSL
Completion: 24.06 ft. BGS
Station: 874,875.37N
 2,516,200.66E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss	3-4 6-7 N=10	19			0			628	
2A	24/24 100%	ss	4-7 8-11 N=15	21			2			626	
3A	22/24 92%	ss	9-10 9-35 N=19	10			4			624	Rock fragment in split spoon shoe
4A	4/24 17%	ss	20-7 10-8 N=17				6	FILL - Yellowish brown (10YR5/4) with 15% yellowish brown(10YR5/8) mottles, moist, hard, silty CLAY with trace sand and slight trace gravel.		622	Rock fragment in split spoon shoe
5A	14/24 58%	ss	11-6 8-8 N=14	15			8			620	Rock fragment in split spoon shoe
6A	20/24 83%	ss	6-4 8-9 N=12	26			10			618	
7A	24/24 100%	ss	2-4 8-11 N=12	18			12			616	
8A	20/24 83%	ss	4-7 10-11 N=17	12			14	Gray (10YR6/1) with 30% brownish yellow (10YR6/6) mottles, moist, firm, clayey SILT with slight trace sand.		614	
8B				22			16	Very dark gray (10YR3/1), moist, firm, clayey SILT with slight trace sand and trace roots.		614	
9A	22/24 92%	ss	4-6 6-9 N=12	17			18	Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel.		612	
10A	20/24 83%	ss	2-4 5-8 N=9	21			20	Gray (10YR6/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel.		610	
							22	Yellowish brown (10YR5/8) with 20% gray (10YR5/1) mottles, moist, firm, silty CLAY with sand and trace gravel.			

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/11/2009
Finish: 9/11/2009
WEATHER: Sunny, warm (70's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G278
Well ID: G278
Surface Elev: 628.85 ft. MSL
Completion: 24.06 ft. BGS
Station: 874,875.37N
 2,516,200.66E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▮ = Dry - While drilling ▮ = 23.98 - 9/21/09 ▮ =			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	18/24 75%	ss	2-4 7-7 N=11	19			20	Yellowish brown (10YR5/8) with 20% gray (10YR5/1) mottles, moist, firm, silty CLAY with sand and trace gravel. [Continued from previous page]		608		
11B							22	Yellowish brown (10YR5/6), very moist, soft, silty, very fine- to medium-grained SAND.				
12A				16								
12B	20/24 83%	ss	1-5 10-18 N=15	10			24	Yellowish brown (10YR5/4), moist, hard, very silty CLAY with sand and trace gravel.		606		
							▮ 24	End of Boring = 24.1 ft. BGS				

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/10/2009
Finish: 9/10/2009
WEATHER: Sunny, warm (80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G279
Well ID: G279
Surface Elev: 629.19 ft. MSL
Completion: 28.00 ft. BGS
Station: 875,028.06N
 2,516,245.60E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	3-3 5-6 N=8	18			2	FILL - Brown (10YR4/3) with 30% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with sand and trace gravel.		628	
2A	24/24 100%	SS	5-9 10-11 N=19	14			4			626	
3A	24/24 100%	SS	5-9 9-10 N=18	17			6			624	
4A	24/24 100%	SS	4-5 7-6 N=12	21			8			622	
5A	24/24 100%	SS	3-3 5-7 N=8	19			10	FILL - dark gray (10YR4/1) with 10% brownish yellow (10YR6/6) mottles, moist, hard, silty CLAY with sand and trace gravel.		620	
6A	24/24 100%	SS	3-4 6-9 N=10	17			12			618	
7A	23/24 96%	SS	2-5 5-6 N=10	23			14			616	
8A	24/24 100%	SS	2-3 7-6 N=10	23			16	Brownish yellow (10YR6/8) with 30% gray (10YR5/1) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		614	
9A	18/24 75%	SS	4-7 8-9 N=15	25			18	Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, moist, firm, silty CLAY with slight trace sand and gravel.		612	
10A	24/24 100%	SS	3-6 7-10 N=13	17			20	Gray (10YR6/1) with 25% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel.		610	

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 9/10/2009
Finish: 9/10/2009
WEATHER: Sunny, warm (80's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: G. Mills
Helper: J. Twellman
Eng/Geo: R. Hasenyager

BOREHOLE ID: G279
Well ID: G279
Surface Elev: 629.19 ft. MSL
Completion: 28.00 ft. BGS
Station: 875,028.06N
 2,516,245.60E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = 23.60 - While drilling ▽ = 24.68 - 9/21/09 ▽ =		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	23/24 96%	ss	2-4 5-7 N=9	18			22	Gray (10YR6/1) with 25% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel. <i>[Continued from previous page]</i>		608	
12A	19/24 79%	ss	4-9 8-9 N=17	13			24	Yellowish brown (10YR5/8), moist, firm, clayey SILT and very fine-grained SAND with slight trace gravel.		606	
12B				12							
13A	17/24 71%	ss	1-5 5-7 N=10	18			26	Light brownish gray (10YR6/2), wet, loose, very fine- to coarse-grained SAND.		604	
14A				16							
14B	24/24 100%	ss	10-10 18-18 N=28	14			28	Brownish yellow (10YR6/6), moist, hard, very silty CLAY with sand and trace gravel. Gray (10YR6/1), moist, hard, very silty CLAY with sand and trace gravel.		602	
							End of Boring = 28.0 ft. BGS				

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/26/2008
 Finish: 2/26/2008
WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: .

BOREHOLE ID: G280
Well ID: G280
Surface Elev: 622.95 ft. MSL
Completion: 17.98 ft. BGS
Station: 875,045.11N
 2,515,679.48E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 15.60 - While drilling ▽ = 4.34 - 3/12/08 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	5-3 4-4 N=7	23					0	Dark grayish brown (10YR4/2), moist, firm, clayey SILT		622	
1B				26		2.33 B			2	Brown (10YR4/3) with 20% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY			
2A	24/24 100%	SS	3-4 4-6 N=8	30		1.28 BSh			2	Dark yellowish brown (10YR4/4), moist, firm, silty CLAY		620	
2B				25					4	Dark gray (10YR4/1) with 40% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY			
3A	19/24 79%	SS	3-4 6-6 N=10	14		3.10 Sh			6	Dark gray (10YR4/1) with 40% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand		618	
4A	22/24 92%	SS	9-11 10-8 N=21	18		1.67 BSh			8	Dark gray (10YR4/1) with 40% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, trace sand, slight trace gravel		616	
5A	19/24 79%	SS	2-2 4-4 N=6	20		1.47 B			10	Dark gray (10YR4/1) with 40% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, sand, trace gravel		614	
5B				21		1.28 B			10				
6A	22/24 92%	SS	2-3 3-3 N=6	20					12	Yellowish brown (10YR5/8) with 20% light gray (10YR6/1) mottles, moist, soft, sandy CLAY		612	
7A	23/24 96%	SS	3-14 23-21 N=37	13					14	Yellowish brown (10YR5/8), moist, soft, fine to coarse SAND, trace gravel Yellowish brown (10YR5/8), moist, firm, sandy CLAY, trace gravel		610	
8A	23/24 96%	SS	12-17 24-26 N=41	9					16	Yellowish brown (10YR5/4), moist, firm, clayey SILT, trace sand and gravel		608	
8B				15					16				
9A	24/24 100%	SS	11-27 54-43 N=81	26					16	Yellowish brown (10YR5/4), wet, soft, fine- to coarse-grained SAND, trace gravel		606	
9B				7					16	Gray (10YR5/1), moist, hard, silty CLAY, trace sand and gravel		606	

End of Boring = 17.98 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: Natural Resource Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 9/8/2015
Finish: 9/8/2015
WEATHER: Sunny, hi 70's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: Hollow Stem Auger (3 1/4" overdrill / 4 1/4")
FIELD STAFF: Driller: D. Crump
Helper: D. Groves
Eng/Geo: K. Theesfeld

BOREHOLE ID: G281
Well ID: G281
Surface Elev: 623.82 ft. MSL
Completion: 20.29 ft. BGS
Station: 2,514,455.48N
 874,375.37E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 14.00 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	SS	15-10 7-6 N=17	14					0	Light gray (10YR7/2), dry, very stiff, SILT with little clay and trace gravel.			
2A	19/24 79%	SS	2-4 5-5 N=9	25	1.50				2	Yellowish brown (10YR5/4) with 5% dark brown (10YR3/3) mottles, dry, very stiff, SILT with few clay and trace gravel.		622	
3A	22/24 92%	SS	2-2 3-4 N=5	23	0.40				4	Yellowish brown (10YR5/4) with 15% dark yellowish brown (10YR4/6) and 5% dark brown (10YR3/3) mottles, moist, stiff, SILT with few clay.		620	
4A	24/24 100%	SS	5-5 6-6 N=11	19	1.20				6	Yellowish brown (10YR5/4) with 15% dark yellowish brown (10YR4/6) mottles, moist, medium, CLAY with some silt and trace fine-grained sand and small gravel.		618	
5A	20/24 83%	SS	2-2 3-4 N=5	21	1.40				8	Yellowish brown (10YR5/4) with 15% dark yellowish brown (10YR4/6) and 5% dark brown (10YR3/3) mottles, moist, stiff, CLAY with some silt and trace fine-grained sand and small gravel.		616	
6A	22/24 92%	SS	2-2 3-3 N=5	18	0.50				10	Yellowish brown (10YR5/4) with 30% dark yellowish brown (10YR4/6) and 5% dark brown (10YR3/3) mottles, moist, stiff, SILT with some clay and trace very fine- to fine-grained sand and small gravel.		614	
7A	17/24 71%	SS	3-4 5-5 N=9	19	0.30				12	Dark yellowish brown (10YR4/6) with 30% yellowish brown (10YR5/4) mottles, moist, soft, SILT with few clay and little fine- to coarse-grained sand and small gravel, trace wood fragments.		612	
									14	Dark yellowish brown (10YR4/6) with 15% yellowish brown (10YR5/4) mottles, moist, soft, SILT with few clay and very fine- to fine-grained sand and trace small gravel.		610	
	19/24 79%	SS	3-11 21-28 N=32						16	Dark yellowish brown (10YR4/4), wet, dense, very fine- to fine-grained SAND with some silt, few clay and trace small gravel.		608	
	24/24 100%	SS	21-36 39-50 N=75						18	Dark yellowish brown (10YR4/4), wet, dense, very fine- to fine-grained SAND with few silt, little clay and trace small gravel.		606	
8A	11/24 46%	SS	16-9 30-50 N=39	7	4.50				20	Yellowish brown (10YR5/6) with 5% strong brown (7.5YR5/6) mottles, moist, hard, SILT with few clay and little fine-grained sand and small gravel.		604	
	0/3 0%	BD								Dark grayish brown (10YR4/2) with 5% strong brown (7.5YR5/6) mottles, moist, hard, SILT with few clay and little fine-grained sand and small gravel.			

End of boring = 20.29 feet

NOTE(S): G281 installed in borehole.

FIELD BORING LOG



CLIENT: Natural Resource Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 9/4/2015
Finish: 9/4/2015
WEATHER: Sunny, hi 70's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: Hollow Stem Auger (3/4" overdrill / 4/4")
FIELD STAFF: Driller: D. Crump
Helper: D. Groves
Eng/Geo: K. Theesfeld

BOREHOLE ID: G301
Well ID: G301
Surface Elev: 620.27 ft. MSL
Completion: 16.21 ft. BGS
Station: 2,515,582.97N
 872,234.82E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 12.00 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	SS	4-4 3-5 N=7	18		1.80			1.80	Brown (10YR4/3), moist, stiff, SILT with few clay, trace organics.		620	
2A	22/24 92%	SS	2-4 4-6 N=8	26		2.40			2.40	Brown (10YR5/3) with 30% yellowish brown (10YR5/6) mottles, moist, stiff, SILT with few clay, trace very fine- to medium-grained sand.		618	
3A	21/24 88%	SS	2-2 3-4 N=5	22		1.30			4.00	Brown (10YR5/3) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, CLAY with some silt and trace very fine- to medium-grained sand.		616	
4A	24/24 100%	SS	6-4 6-6 N=10	19		1.10			6.00	Brown (10YR5/3) with 10% yellowish brown (10YR5/6) mottles, moist, medium to stiff, CLAY with some silt and little very fine- to coarse-grained sand and small gravel.		614	
5A	21/24 88%	SS	1-2 3-4 N=5	21					8.00	Brown (10YR5/3) with 5% yellowish brown (10YR5/6) mottles, moist, stiff, SILT with some clay and little very fine- to coarse-grained sand and small gravel.		612	
6A	24/24 100%	SS	3-2 3-3 N=5	19		0.80			10.00	Brown (10YR5/3) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, SILT with some clay and little very fine- to coarse-grained sand and small gravel.		610	
7A	24/24 100%	SS	2-4 6-21 N=10	13		1.60			12.00	Yellowish brown (10YR5/6) with 10% grayish brown (10YR5/2) and 5% yellowish brown (10YR5/4) mottles, wet, stiff, SILT with few clay and little fine- to coarse-grained sand and small gravel.		608	
8A	21/24 88%	SS	20-27 50 N=77	7		4.50			14.00	Grayish brown (10YR5/2) with 5% brown (10YR5/3) mottles, dry, hard, SILT with few clay, very fine- to coarse-grained sand and small gravel.		606	
	0/3 0%	BD							16.00				

End of boring = 16.21 feet

NOTE(S): G301 installed in borehole.

FIELD BORING LOG



CLIENT: Natural Resource Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 9/3/2015
Finish: 9/4/2015
WEATHER: Sunny, hi 70's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: Hollow Stem Auger (3/4" overdrill / 4/4")
FIELD STAFF: Driller: D. Crump
Helper: D. Groves
Eng/Geo: K. Theesfeld

BOREHOLE ID: G302
Well ID: G302
Surface Elev: 617.95 ft. MSL
Completion: 18.39 ft. BGS
Station: 2,516,214.19N
 872,252.95E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	SS	5-5 5-6 N=10	17	1.50		1.50	Yellowish brown (10YR5/6) to dark yellowish brown (10YR4/6), moist, hard, SILT with some clay and trace fine-grained sand and small gravel.			
2A	20/24 83%	SS	2-3 4-4 N=7	27	2.60		2.60	Very dark brown (10YR3/1), moist, hard, SILT with some clay and trace fine-grained sand and small gravel.		616	
3A	21/24 88%	SS	1-2 3-4 N=5	26	1.80		4.00	Dark grayish brown (10YR4/2) with very dark gray (10YR3/1) mottles, moist, stiff, CLAY with some silt and trace sand.		614	
4A	24/24 100%	SS	4-7 8-8 N=15	18	1.60		6.00	Brown (10YR5/3) with dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with some silt and trace sand.		612	
5A	24/24 100%	SS	2-3 5-5 N=8	17	1.80		8.00	Brown (10YR5/3) with dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with some silt and few very fine- to fine-grained sand.		610	
6A	18/24 75%	SS	2-2 4-5 N=6	19			10.00	Brown (10YR4/3) with dark yellowish brown (10YR4/6) mottles, moist, stiff, SILT with some clay and few sand.		608	
7A	24/24 100%	SS	7-7 8-8 N=15	16	1.70		12.00	Dark gray (10YR4/1), moist, stiff, SILT with some clay and few sand.		606	
8A							13.00	Yellowish brown (10YR5/8) with reddish brown (5YR4/4) inclusions, moist, stiff, SILT with some clay and few sand.			
8B	22/24 92%	SS	2-5 25-32 N=30	12	4.50		14.00	Dark gray (10YR4/1), moist, stiff, SILT with some clay, few sand and trace small gravel.		604	
9A	24/24 100%	SS	7-24 48-38 N=72	8	4.50		16.00	Grayish brown (10YR5/2), wet, hard, SILT with little clay and very fine-grained sand and trace small gravel.		602	
	0/5 0%	BD					18.00	Brown (10YR5/3) grading to yellowish brown (10YR5/4), moist, hard, SILT with some clay, few small gravel.		600	

End of boring = 18.39 feet

NOTE(S): G302 installed in borehole.

Surface Elevation: 619.10

Completion Date: 8/26/10

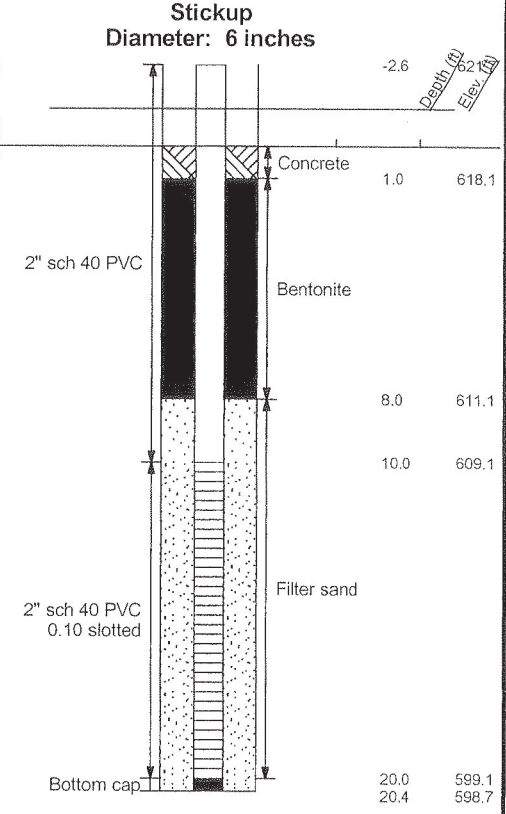
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Easting: 2516641.06

Datum msl

WELL DIAGRAM

DEPTH IN FEET	DESCRIPTION OF MATERIAL	GRAPHIC LOG	DRY UNIT WEIGHT (pcf) SPT BLOW COUNTS CORE RECOVERY/RQD	SAMPLES
	FILL: brown, silty clay			
			1-2-3	SS1
	Medium stiff, grayish-brown, silty CLAY with lignite - CL			
5			1-3-3	SS2
			1-3-4	SS3
	Soft, brown, silty CLAY - CL			
10	Stiff to hard, brown to grayish-brown, silty CLAY - CL (TILL)			
			2-3-3	SS4
			27-44 -50/1"	SS5
15				
			44-50/2"	SS6
20	Boring terminated at 20 feet.			
25				
30				
35				



NOTE: STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARIES BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. GRAPHIC LOG FOR ILLUSTRATION PURPOSES ONLY.

GROUNDWATER DATA

ENCOUNTERED AT 9 FEET ∇

DRILLING DATA

 AUGER 4 1/4" HOLLOW STEM
WASHBORING FROM FEET
 MVU DRILLER SWG LOGGER
 CME 55TRK DRILL RIG
HAMMER TYPE Auto

REMARKS:

Drawn by: KSA Checked by: SK App'vd. by: KBS
Date: 9/10/10 Date: 1-4-11 Date: 1/4/11



Ameren-Coffeen Ash Pond Evaluation

Renamed G303

LOG OF BORING: APW-3

Project No. J017150.01

Surface Elevation: 623.46

Completion Date: 8/26/10

Datum msl

Northing: 871397.48

Easting: 2515520.23

WELL DIAGRAM

DEPTH IN FEET

DESCRIPTION OF MATERIAL

GRAPHIC LOG

DRY UNIT WEIGHT (pcf)
SPT BLOW COUNTS
CORE RECOVERY/RQD

SAMPLES

Stickup
Diameter: 6 inches

Depth (ft)
Elev. (ft)

Medium stiff, grayish-brown, silty CLAY - CL

0-1-1 SS1

1-2-5 SS2

1-3-4 SS3

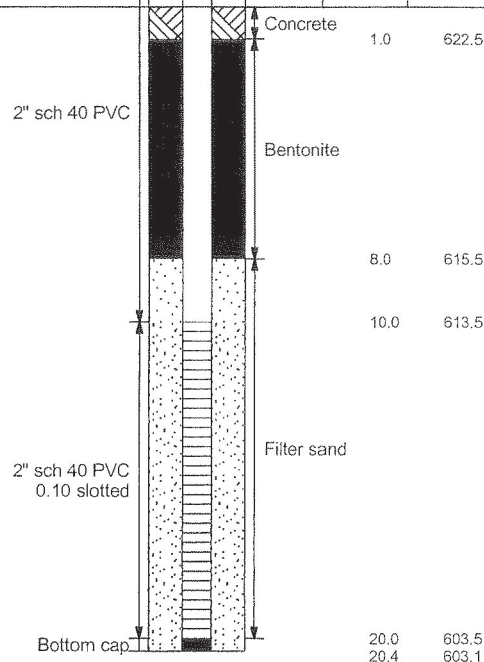
1-2-3 SS4

Medium to fine SAND - SP
Hard, gray, silty CLAY - CL (TILL)

1-2-1 SS5

14-47 SS6

Boring terminated at 20 feet.



NOTE: STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARIES BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. GRAPHIC LOG FOR ILLUSTRATION PURPOSES ONLY.

LOG OF BORING 2002 WL J017150.01 - COFFEEN.GPJ GTINC 0638301.GPJ 12/13/10

GROUNDWATER DATA

ENCOUNTERED AT 14 FEET ∇

DRILLING DATA

 AUGER 4 1/4" HOLLOW STEM
WASHBORING FROM FEET
MVU DRILLER SWG LOGGER
CME 55TRK DRILL RIG
HAMMER TYPE Auto

REMARKS:

Drawn by: KSA Checked by: DK App'vd. by: RS
Date: 9/10/10 Date: 1-4-11 Date: 1/7/11



Ameren-Coffeen Ash Pond Evaluation

Renamed G304
LOG OF BORING: APW-4

Project No. J017150.01

FIELD BORING LOG



CLIENT: Natural Resource Technology, Inc.
Site: Coffeen Power Station
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 5/3/2016
Finish: 5/3/2016
WEATHER: Cloudy, breezy, warm, lo 60s

CONTRACTOR: Ramsey Geotechnical Engineering LLC
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: 4 1/4" HSA, split spoon sampler
FIELD STAFF: Driller: B. Williamson
Helper: D. Crump
Eng/Geo: S. Keim

BOREHOLE ID: G305
Well ID: G305
Surface Elev: 622.54 ft. MSL
Completion: 18.45 ft. BGS
Station: 2,515,199.36N
 871,156.33E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 14, Tier 7N; Range 3W	▼ = 3.00 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	14/24 58%	SS	2-2 5-7 N=7	12					0	FILL - Grayish brown (10YR5/2), moist, soft, silty CLAY with few small to coarse sand and few small to large gravel.		622	
2									2	FILL - Black (10YR2/1), moist, loose, silty, fine- to coarse-grained SAND with little coal fragments.			
2A	17/24 71%	SS	6-7 7-6 N=14	19				▼	3	Gray (10YR6/1) and light yellowish brown (10YR6/4), moist, very stiff, SILT with little clay.		620	
3A	18/24 75%	SS	3-3 6-4 N=9	28					4	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY with trace very fine-grained sand.		618	
4A	16/24 67%	SS	6-6 7-8 N=13	24					6	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with trace very fine- to coarse-grained sand.		616	
5A	23/24 96%	SS	1-3 5-6 N=8	21					8	Dark gray (10YR4/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY with few fine- to coarse-grained sand and trace small gravel.		614	
6A	21/24 88%	SS	3-4 6-6 N=10	21					10	Gray (10YR5/1) with 35% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY with few fine- to coarse-grained sand and trace small gravel.		612	
7A	24/24 100%	SS	8-8 9-9 N=17	18					12	Yellowish brown (10YR5/8) with 15% gray (10YR5/1) mottles, moist, very stiff, silty CLAY with few fine- to coarse-grained sand and trace small gravel.		610	
8A	19/24 79%	SS	3-3 4-4 N=7	18					14	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with little fine- to coarse-grained sand and trace small gravel.		608	
9A	22/24 92%	SS	1-3 7-14 N=10	19					16	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with some fine- to coarse-grained sand and trace small gravel.		606	
9B	0/5 0%	BD		15					18	Brown (10YR5/3), wet, loose, very silty, very fine- to coarse-grained SAND with trace small gravel.		606	
									18	Brown (10YR5/3) with 40% yellowish brown (10YR5/8) mottles, moist, very stiff, SILT with little clay and trace fine- to coarse-grained sand.			

End of boring = 18.45 feet

NOTE(S): G305 installed in borehole.

FIELD BORING LOG



CLIENT: Natural Resource Technology, Inc.
Site: Coffeen Power Station
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 5/3/2016
Finish: 5/3/2016
WEATHER: Sunny, calm, warm, lo 60s

CONTRACTOR: Ramsey Geotechnical Engineering LLC
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: 4 1/4" HSA, split spoon sampler
FIELD STAFF: Driller: B. Williamson
Helper: D. Crump
Eng/Geo: S. Keim

BOREHOLE ID: G306
Well ID: G306
Surface Elev: 622.84 ft. MSL
Completion: 18.00 ft. BGS
Station: 2,516,120.41N
 871,140.98E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 14, Tier 7N; Range 3W	▼ = 5.50 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	12/24 50%	ss	1-3 3-4 N=6	14					0	Very dark brown (10YR2/2), moist, medium, SILT with little clay and few very fine- to medium-grained sand, roots, trace coal fragments.		622	
2A	24/24 100%	ss	5-4 5-4 N=9	21					2	Dark gray (10YR4/1) with 5% dark yellowish brown (10YR3/6) mottles, moist, stiff, SILT with little clay and trace very fine- to medium-grained sand.		620	
2B				19					4	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, very stiff, SILT with little clay and trace very fine-grained sand.			
3A	22/24 92%	ss	2-2 3-3 N=5	30				▼	6	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, very stiff, SILT with some clay and trace very fine-grained sand.		618	
4A	20/24 83%	ss	3-4 6-6 N=10	26					8			616	
5A	24/24 100%	ss	2-2 3-3 N=5	23					10	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY with trace very fine- to coarse-grained sand.		614	
6A	22/24 92%	ss	1-2 3-4 N=5	20					12			612	
7A	20/24 83%	ss	5-6 6-6 N=12	21					14	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY with few very fine- to coarse-grained sand.		610	
8A	20/24 83%	ss	2-2 8-14 N=10	15					16	Yellowish brown (10YR5/6), wet, soft, very fine- to coarse-grained sandy CLAY with little silt.		608	
8B				12					18	Yellowish brown (10YR5/6), wet, medium dense, silty, very fine- to medium-grained SAND with trace coarse-grained sand.			
9A	23/24 96%	ss	14-17 28-50/5" N=45	10					18	Yellowish brown (10YR5/6), moist, dense, fine- to coarse-grained SAND with little silt, little very fine-grained sand, and trace small gravel.		606	
9B				13					18	Brown (10YR5/3) with 20% dark yellowish brown (10YR4/6) mottles, moist, hard, SILT with little clay, few very fine- to coarse-grained sand, and trace small gravel.			

End of boring = 18.0 feet

NOTE(S): G306 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Power Station Ash Pond 1
Location: Coffeen, Illinois
Project: 16E0108
DATES: Start: 07/26/2016
Finish: 07/27/2016

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME 55LC Track Drill
Drilling Method: 4 1/4" Hollow Stem Auger w/Continuous Split Spoon
FIELD STAFF: Driller: J. Gates
Helper: C. Clines
Eng/Geo: R. Hasenyager

BOREHOLE ID: G307
Well ID: G307
Surface Elev: 622.08 ft. MSL
Completion: 18.22 ft. BGS
Station: 871,398.55N
 2,515,553.26E

WEATHER: Overcast, warm & humid (mid-80s)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Quadrangle: Coffeen Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 14.00 - during drilling ▽ = -1.76 - 7/27/2016 @ 07:30 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	SS	1-3 3-2 N=6	22						0	Brown (10YR5/3), moist, stiff, SILT with few clay, trace sand, gravel and roots.			
2A	22/24 92%	SS	2-1 3-3 N=4	28						2	Gray (10YR5/1), moist, soft, CLAY, with some silt, trace sand, gravel and roots.		620	
3A	24/24 100%	SS	1-2 3-4 N=5	26						4	Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, soft, CLAY with some silt, trace sand, gravel and roots.		618	
4A	24/24 100%	SS	1-3 3-3 N=6	18						6			616	
5A	24/24 100%	SS	3-3 4-5 N=7	19						8	Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, medium CLAY, with some silt, few very fine- to medium-grained sand, and trace gravel.		614	
6A	24/24 100%	SS	3-3 4-5 N=7	20						10			612	
7A	24/24 100%	SS	3-3 4-5 N=7	20						12	Gray (10YR5/1), moist, medium, CLAY with little silt and very fine to very coarse sand, trace gravel. Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, soft, CLAY with some silt, trace sand and gravel.		610	
7B	24/24 100%	SS	woh-2 5-13 N=7	11						14	Gray (10YR6/1), wet, medium dense, very fine- to very coarse-grained SAND with few silt and trace clay.		608	
8A	24/24 100%	SS	12-9 6-9 N=15	20						16	Yellow brown (10YR5/6), wet, medium dense, very fine- to very coarse-grained SAND, with little silt and trace gravel.		606	
9A	18/18 100%	SS	8-30 50 N=80	8						18	Yellowish brown (10YR5/8), moist, hard, SILT with some clay, little very fine- to very coarse-grained sand and trace gravel. Gray (10YR5/1), moist, hard, SILT with some clay, little very fine- to very coarse-grained sand and trace gravel.		604	
	0/9 0%	BD								18			604	

End of boring = 18.2 feet

NOTE(S):

FIELD BORING LOG



CLIENT: Natural Resource Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 9/14/2015
Finish: 9/14/2015
WEATHER: Sunny, hi 60's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: Hollow Stem Auger (3/4" overdrill / 4/4")
FIELD STAFF: Driller: D. Crump
Helper: D. Groves
Eng/Geo: R. Hasenyager

BOREHOLE ID: G401
Well ID: G401
Surface Elev: 623.03 ft. MSL
Completion: 19.30 ft. BGS
Station: 2,515,614.84N
 872,510.57E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss	2-2 3-7 N=5		17			Dark grayish brown (10YR4/2), moist, soft, CLAY with little silt and trace very fine- to fine-grained sand - FILL.		622	
2								Yellowish brown (10YR5/6) moist, medium, CLAY with some silt and trace very fine- to coarse-grained sand - FILL.			
2A	21/24 88%	ss	8-11 8-9 N=19		17	1.80		Dark gray (10YR4/1), moist, stiff, SILT with little clay and trace very fine-grained sand.		620	
2B					25			Yellowish brown (10YR5/6), moist, stiff, CLAY with some silt and trace very fine- to fine-grained sand.			
3A	23/24 96%	ss	3-4 7-8 N=11		23	2.50		Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with some silt and trace very fine- to fine-grained sand.		618	
4A	24/24 100%	ss	8-9 12-14 N=21		21	3.30		Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, stiff, SILT and very fine-grained SAND with trace clay.		616	
4B					19	2.80					
5A	24/24 100%	ss	2-3 4-5 N=7		21	1.30				614	
6A	24/24 100%	ss	2-4 5-6 N=9		17	2.50		Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with some silt and trace very fine- to fine-grained sand.		612	
7A	24/24 100%	ss	9-7 8-9 N=15		21	1.40				610	
8A	24/24 100%	ss	2-3 2-4 N=5		17	1.30		Gray (10YR6/1), moist soft, CLAY with very fine- to fine-grained sand and little silt.		608	
8B					19			Yellowish brown (10YR5/6), wet, loose, very fine- to fine-grained SAND with trace silt.			
9A	20/24 83%	ss	5-4 5-10 N=9		21			Yellowish brown (10YR5/6), wet, medium, SILT with some very fine-grained sand and little clay.		606	
9B					16			Yellowish brown (10YR5/6), wet, loose, very fine- to medium-grained SAND with trace silt.			
10A	12/16 75%	ss	23-41 50/4"		6	4.50		Gray (10YR5/1), moist, very hard, SILT with few clay and little very fine- to very coarse sand.		604	

End of boring = 19.3 feet

NOTE(S): G401 installed in borehole.

Surface Elevation: 610.56

Completion Date: 8/27/10

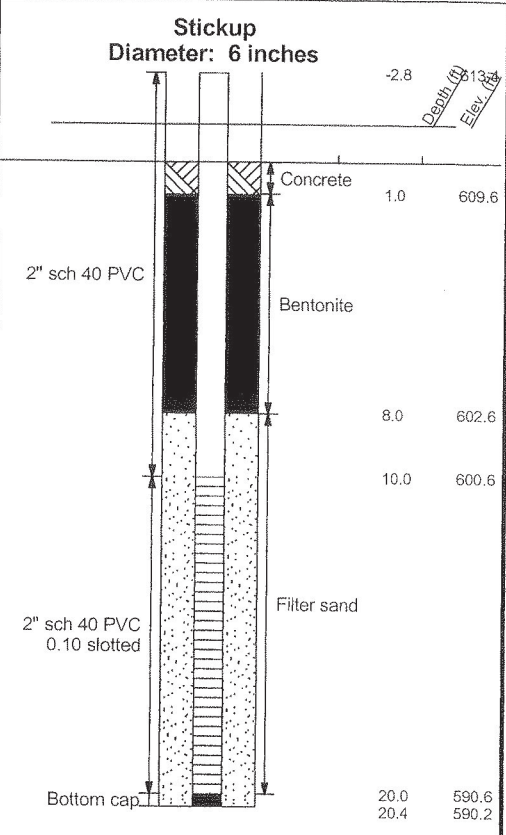
Datum msl

Northing: 872502.26

Easting: 2516632.59

WELL DIAGRAM

DEPTH IN FEET	DESCRIPTION OF MATERIAL	GRAPHIC LOG	DRY UNIT WEIGHT (pcf) SPT BLOW COUNTS CORE RECOVERY/ROD	SAMPLES
	Medium stiff, brown, silty CLAY - CL			
			6-3-3	SS1
			1-2-4	SS2
5	Medium stiff, gray, silty CLAY, trace sand - CL			
			1-2-5	SS3
10	Hard to stiff, brown to gray, silty CLAY, trace sand seams - CL (TILL)			
			9-22-28	SS4
15			1-6-12	SS5
20	Boring terminated at 20 feet.		3-4-9	SS6



NOTE: STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARIES BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. GRAPHIC LOG FOR ILLUSTRATION PURPOSES ONLY.

GROUNDWATER DATA

DRILLING DATA

ENCOUNTERED AT 7 FEET ∇

 AUGER 4 1/4" HOLLOW STEM
WASHBORING FROM FEET
MVU DRILLER SWG LOGGER
CME 55TRK DRILL RIG
HAMMER TYPE Auto

REMARKS:

Drawn by: KSA Checked by: DTK App'vd. by: KR8
Date: 9/10/10 Date: 1-4-11 Date: 1/4/11



Ameren-Coffeen Ash Pond Evaluation

Renamed G402
LOG OF BORING: APW-2

Project No. J017150.01

FIELD BORING LOG



CLIENT: Natural Resource Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 9/11/2015
Finish: 9/11/2015
WEATHER: Raining, hi 60's

CONTRACTOR: Ramsey Geotechnical Engineering, LLC
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: Hollow Stem Auger (3/4" overdrill / 4/4")
FIELD STAFF: Driller: D. Crump
Helper: D. Groves
Eng/Geo: K. Theesfeld

BOREHOLE ID: G403
Well ID: G403
Surface Elev: 623.81 ft. MSL
Completion: 18.15 ft. BGS
Station: 2,514,616.63N
 873,561.34E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 15.00 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	SS	2-2 2-2 N=4	25	0.80		Very dark brown (10YR2/2) grading to dark grayish brown (10YR4/2), moist, medium, SILT with some clay, trace roots and grass.		2			622	
2A	22/24 92%	SS	2-2 3-4 N=5	29	1.50		Very dark brown (10YR2/2) grading to dark grayish brown (10YR4/2), moist, stiff, SILT with some clay, trace wood.		3				
2B				26			Yellowish brown (10YR5/4) with 10% very dark brown (10YR2/2) mottles, moist, stiff, CLAY with little silt, trace very fine-grained sand seams (<1/16" thick).		4			620	
3A	8/24 33%	SS	2-3 4-4 N=7	25	1.50		Yellowish brown (10YR5/4), moist, stiff, CLAY with little silt and trace very fine-grained sand.		6			618	
4A	21/24 88%	SS	8-7 8-7 N=15	20	1.30		Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/6) and 5% very dark grayish brown (10YR3/2) mottles, moist, stiff, CLAY with little silt and trace very fine-grained sand.		8			616	
5A	20/24 83%	SS	2-2 3-3 N=5	22	0.70		Grayish brown (10YR5/2) with 5% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with little silt and trace very fine-grained sand.		10			614	
6A	24/24 100%	SS	2-2 3-4 N=5	23	1.40		Grayish brown (10YR5/2) with 30% yellowish brown (10YR5/6) mottles, moist, very stiff, CLAY with little silt, few very fine- to medium-grained sand, and trace gravel.		12			612	
7A	21/24 88%	SS	5-5 6-5 N=11	20	0.90		Grayish brown (10YR5/3) with 45% yellowish brown (10YR5/6) and 5% dark brown (10YR3/3) mottles, moist, stiff, SILT with some clay, few very fine- to coarse-grained sand, and trace gravel.		14			610	
8A	24/24 100%	SS	3-2 3-6 N=5	17			Grayish brown (10YR5/3) with 40% yellowish brown (10YR5/6) mottles, moist, medium, SILT with little clay, few very fine- to coarse-grained sand, and trace gravel. Yellowish brown (10YR5/6) with 30% grayish brown (10YR5/2) mottles, moist, medium, SILT with little clay, few very fine- to coarse-grained sand, and trace gravel.		16			608	
9A	19/24 79%	SS	8-12 21-25 N=33	8	4.50		Yellowish brown (10YR5/6) with 30% grayish brown (10YR5/2) mottles, moist, stiff, SILT with few clay, very fine- to coarse-grained sand, and gravel. Yellowish brown (10YR5/6), moist, very stiff, SILT with some clay and few sand and gravel.		18			606	
	0/2 0%	BD					Very dark grayish brown (10YR3/2), dry, hard, SILT with little clay and few very fine- to coarse-grained sand and gravel.		18.15				

End of boring = 18.15 feet

NOTE(S): G403 installed in borehole.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 5/1/2007
Finish: 5/1/2007
WEATHER: Partly sunny, warm

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" Hollow stem auger with split spoon sampler
FIELD STAFF: Driller: A. Rachford
Helper: M. Brown
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB22 Formerly MW22S
Well ID: G404
Surface Elev: 613.10 ft. MSL
Completion: 12.00 ft. BGS
Station: 873,999.77N
 2,516,397.85E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 2.07 - 05/10/2007 ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	ss		26	0.85 B				0	Black (10YR2/1), moist, firm, clayey SILT (TOPSOIL)		612	
1B				26					2				
2A	19/24 79%	ss		16	2.47 B				4	Gray (10YR5/1) with 35% dark yellowish brown (10YR4/6) mottles, moist, firm, clayey SILT with sand and trace gravel		610	
3A	18/24 75%	ss		19	2.18 B				6	Gray (10YR6/1) with 20% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel		608	
3B				18	2.33 B				8	Yellowish brown (10YR5/8), moist, soft, sandy CLAY with slight trace gravel		606	
4A	24/24 100%	ss		23	0.58 B				10	Yellowish brown (10YR5/8), very moist to wet, very soft, clayey, very fine- to medium-grained SAND with trace gravel		604	
4B				18					12	Yellowish brown (10YR5/4) with 30% yellowish brown (10YR5/8) mottles, moist, hard, clayey SILT with sand and trace gravel		602	
5A	23/24 96%	ss		10						Dark yellowish brown (10YR4/6) with 40% yellowish brown (10YR5/8) mottles, moist, very hard, very silty CLAY with sand and gravel			
5B				19									
6A	19/24 79%	ss		19									
6B				11									

End of Boring = 12.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Energy Center
Location: Coffeen, Illinois
Project: 15E0030
DATES: Start: 5/1/2007
Finish: 5/1/2007
WEATHER: Partly sunny, warm

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" Hollow stem auger with split spoon sampler
FIELD STAFF: Driller: A. Rachford
Helper: M. Brown
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB21 Formerly MW21S
Well ID: G405
Surface Elev: 620.90 ft. MSL
Completion: 14.21 ft. BGS
Station: 873,996.79N
 2,515,335.70E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 1.28 - 05/10/2007 ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	ss	43		0.78				0	Black (10YR2/1), moist, soft, clayey SILT (TOPSOIL)		620	
1B			26		1.94				2				
2A	12/12 100%	ss	27		2.52				4	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY		618	
3A			24		3.92				6				
3B	24/24 100%	ss	24		2.33				6	Gray (10YR6/1), moist, firm, silty CLAY slight trace sand		616	
4A	24/24 100%	ss	20		2.33				6	Gray (10YR5/1), very moist, soft, clayey, very fine- to fine-grained SAND		614	
5A	24/24 100%	ss	24		1.55				8	Gray (10YR6/1) with 25% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel		612	
6A	24/24 100%	ss	19						10	Yellowish brown (10YR5/8) with 40% gray (10YR6/1) mottles, moist, firm, silty CLAY with sand and trace gravel		610	
6B			18						10	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND			
7A	24/24 100%	ss	9		7.42				12	Dark brown (10YR3/3), very moist, soft, clayey, fine- to very coarse-grained SAND with slight trace gravel		608	
									12	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND			
									12	Dark yellowish brown (10YR4/4), moist, soft, sandy SILT with trace gravel			
									12	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND			
									14	Gray (10YR5/1), moist, very hard, very silty CLAY with sand and gravel			

End of Boring = 14.2 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G406D
Well ID: G46D
Surface Elev: 621.91 ft. MSL
Completion: 52.00 ft. BGS
Station: 872,519.70N
 2,514,697.78E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	4-3 4-6 N=7	11	1.50		1.50	Dark brown (10YR3/3), moist, stiff, SILT with few clay and trace organics.			
1B							3.00				
2A	12/24 50%	ss	4-6 3-3 N=9	12	2.50		2.50	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay and trace small gravel.		620	
2B							2.00				
3A	3/24 13%	ss	3-3 4-7 N=7	24			4.00	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay. Grayish brown (10YR5/2) with 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with few silt and little fine-grained sand.		618	
							6.00	Very pale brown (10YR7/4) with 25% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with trace silt.		616	
4A	20/24 83%	ss	2-3 4-5 N=7	21	1.25		8.00	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt and trace fine- to medium-grained sand.		614	
5A	19/24 79%	ss	1-3 4-6 N=7	18	1.75		10.00	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt, little fine- to medium-grained sand and trace small gravel.		612	
6A	23/24 96%	ss	2-2 4-5 N=6	18	2.50		12.00	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		610	
7A	21/24 88%	ss	1-3 4-5 N=7	16	1.00		14.00	Gray (10YR6/1) with 25% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		608	
8A	23/24 96%	ss	1-2 2-2 N=4	18	0.75		16.00	Gray (10YR6/1) with 10% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with some fine- to medium-grained sand, little silt, and trace small gravel.		606	
8B							16.00	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, wet, loose, fine-grained SAND with some clay.		606	
9A	22/24 92%	ss	4-13 27-23 N=40	8			18.00	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, moist, loose, fine-grained SAND with some clay.		604	
9B							18.00			604	
10A	17/24 71%	ss	13-31 33-42 N=64	7	4.50		20.00	Gray (10YR5/1), dry, hard, SILT with few clay, few fine- to coarse-grained sand and trace small gravel.		602	

NOTE(S): G46D installed in boring.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G406D
Well ID: G46D
Surface Elev: 621.91 ft. MSL
Completion: 52.00 ft. BGS
Station: 872,519.70N
 2,514,697.78E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen Township: East Fork Section 11, Tier 7 N.; Range 3 W.	▼ = Dry - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11-1	3/3 100%	SH											
	8/24 33%	DS											
12-1									22	Gray (10YR5/1), dry, hard, SILT with few clay, few fine- to coarse-grained sand and trace small gravel. <i>[Continued from previous page]</i>		600	
13-1													
13-2	24/24 100%	DS											
13-3													
13-4									24			598	
14A				19						Pale brown (10YR6/3), wet, loose, fine-grained SAND.			
14B	21/21 100%	SS	7-30 32-20 N=62	7						Pale brown (10YR6/3), wet, loose, fine-grained SAND and small GRAVEL.			
14C				8	4.50				26	Gray (10YR5/1), dry, hard, SILT with some clay, trace fine- to coarse-grained sand and trace small gravel.		596	
15A	12/24 50%	SS	15-16 10-11 N=26	10	4.50				28	Dark gray (10YR4/1), dry, hard, CLAY with some silt, trace fine- to coarse-grained sand and trace small gravel.		594	
16A	17/24 71%	SS	2-5 7-11 N=12	16	3.00				30	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		592	
17A	17/24 71%	SS	2-5 7-12 N=12	15	2.50				32	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small to large gravel.		590	
18A	12/24 50%	SS	3-7 12-20 N=19	17	2.50				34	Dark gray (10YR4/1) with 5% very dark gray (10YR3/1) and brown (10YR5/3) mottles, moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		588	
19A	23/24 96%	SS	4-4 6-9 N=10	25	1.00				36	Gray (10YR5/1) with 5% very dark gray (10YR3/1) and brown (10YR5/3) mottles, moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		586	
20A	22/24 92%	SS	2-2 5-7 N=7	17	1.00				38	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel.		584	
21A	24/24 100%	SS	2-3 4-6 N=7	17	1.00				40			582	

NOTE(S): G46D installed in boring.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Sunny, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: G406D
Well ID: G46D
Surface Elev: 621.91 ft. MSL
Completion: 52.00 ft. BGS
Station: 872,519.70N
 2,514,697.78E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
22A	24/24 100%	SS	1-3 5-6 N=8	17		1.50	42			580	
23-1											
23-2	24/24 100%	SH		16							
23-3											
23-4							44			578	
24A	24/24 100%	SS	3-4 6-7 N=10	17		1.50	46	Dark gray (10YR4/1), moist, very stiff, CLAY with few silt, trace fine- to coarse-grained sand and trace small gravel. <i>[Continued from previous page]</i>		576	
25A	24/24 100%	SS	2-2 5-7 N=7	18		1.25	48			574	
26A	24/24 100%	SS	2-5 6-8 N=11	17		1.75	50			572	
27A	23/24 96%	SS	2-3 9-12 N=12	15		3.50	52			570	
							End of Boring = 52.0 ft. BGS				

NOTE(S): G46D installed in boring.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
 Site: Coffeen Power Station - Ash Pond 2
 Location: 134 CIPS Lane, Coffeen, IL 62017
 Project: 16E0080
 DATES: Start: 8/16/2016
 Finish: 8/16/2016
 WEATHER: Rain, (mid-70s)

CONTRACTOR: Bulldog Drilling, Inc.
 Rig mfg/model: CME-750 ATV Drill
 Drilling Method: 4 1/4" Hollow Stem Auger
 FIELD STAFF: Driller: J. Dittmaier
 Helper: M. Hill
 Eng/Geo: K. Theesfeld

BOREHOLE ID: G407
 Well ID: G407
 Surface Elev: 618.35 ft. MSL
 Completion: 20.00 ft. BGS
 Station: 2,513,705.87N
 2,513,705.87E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	12/24 50%	ss	4-3 3-3 N=6	14		3.50	0	Very dark gray (10YR3/1), wet, medium, SILT with some organics. [Fill]		618	
2A	20/24 83%	ss	2-2 4-4 N=6	18		1.50	2	Gray (10YR6/1), wet, loose, SAND with some gravel and little clay. [Fill]		616	
3A	23/24 96%	ss	1-2 3-4 N=5	19		1.75	4	Yellowish brown (10YR5/6) with 5% dark yellowish brown (10YR3/6) mottles, moist, very stiff, SILT with some clay and trace very fine- to fine-grained sand. Brown (10YR5/3) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, SILT with some clay, little fine- to coarse-grained sand, and trace small gravel.		614	
4A	24/24 100%	ss	1-3 3-5 N=6	19		1.50	6	Brown (10YR5/3) with 25% yellowish brown (10YR5/6) mottles, moist, stiff, CLAY with some silt, trace fine-grained sand and trace small gravel.		612	
5A	21/24 88%	ss	1-2 4-4 N=6	19		0.50	8	Brown (10YR5/3) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, CLAY with some silt, little fine- to coarse-grained sand and trace small gravel.		610	
6A	22/24 92%	ss	1-2 2-1 N=4	17			10	Yellowish brown (10YR5/8) with 5% gray (10YR5/1) mottles, moist, very loose, fine-grained SAND with some clay and trace small gravel.		608	
7A	24/24 100%	ss	7-29 33-17 N=62	8			12	Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, very dense, fine-grained SAND		606	
8A	24/24 100%	ss	3-7 12-17 N=19	12		4.50	14	Brown (10YR5/3), moist, hard, SILT with some clay and little fine- to coarse-grained sand.		604	
9A	24/24 100%	ss	4-9 14-20 N=23	13		4.00	16	Yellowish brown (10YR5/4) with 5% yellowish brown (10YR5/6) and 5% black (10YR2/1) mottles, SILT with some clay and little fine- to coarse-grained sand.		602	
10A	24/24 100%	ss	2-8 14-19 N=22	14		4.50	18	Yellowish brown (10YR5/4) with 5% yellowish brown (10YR5/6), 5% dark gray (10YR4/1) and 5% black (10YR2/1) mottles, moist, hard, SILT with little fine- to coarse-grained sand and trace small gravel.		600	
							20	Dark grayish brown (10YR4/2) with 10% dark yellowish brown (10YR3/6) mottles, moist, hard, CLAY with some silt, little fine- to coarse-grained sand and trace small gravel.			

End of Boring = 20.0 ft. BGS

NOTE(S): G407 installed in boring.

FIELD BORING LOG



CLIENT: Illinois Power Generating Company
Site: Coffeen Power Station - Ash Pond 2
Location: Coffeen, Illinois
Project: 16E0031A
DATES: Start: 2/23/2018
 Finish: 2/23/2018
WEATHER: Overcast, mild (mid-40s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA with continuous split spoon
FIELD STAFF: Driller: C. Dutton
 Helper: M. Baetje
 Eng/Geo: R. Hasenyager

BOREHOLE ID: G410
Well ID: G410
Surface Elev: 617.21 ft. MSL
Completion: 14.09 ft. BGS
Station: 872,968.54N
 2,513,206.33E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 11.00 - During drilling ▽ = 13.30 - Upon completion ▽ = 4.82 - 1 March 2018		
						Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	14/24 58%	SS	2-3 1-5 N=4	26.7			FILL - Light gray (10YR7/1), moist, loose, very fine- to very coarse-grained SAND with few small to medium gravel. Very dark grayish brown (10YR3/2), moist, soft, CLAY with some silt and trace very fine- to fine-grained sand. Yellowish brown (10YR5/4), moist, soft, CLAY with some silt and trace very fine- to fine-grained sand.		616	
2A	15/24 63%	SS	4-4 4-5 N=8	25.2					614	
3A	17/24 71%	SS	2-3 4-6 N=7	17.9					612	
4A	20/24 83%	SS	3-4 5-6 N=9	17.2			Gray (10YR6/1) with 25% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with some silt and trace very fine- to fine-grained sand.		610	
5A	11/24 46%	SS	3-3 5-6 N=8	17.5			Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, medium, CLAY with some silt, little very fine- to very coarse-grained sand, and trace small to medium gravel.		608	
6A	22/24 92%	SS	2-3 4-6 N=7	16.9			Gray (10YR6/1), moist, loose, very fine- to medium-grained SAND with few silt and few clay.		606	
6B				16.8			Yellowish brown (10YR5/8) with 10% gray (10YR5/1) mottles, moist, medium, CLAY with some silt, little very fine- to very coarse-grained sand, and trace small to medium gravel.		606	
7A	24/24 100%	SS	6-28 36-42 N=64	10.3			Yellowish brown (10YR5/6), moist, hard, SILT with some clay, few very fine- to very coarse-grained sand, and trace small to medium gravel.		604	
	0/1 0%	BD								
						14	End of Boring = 14.09 ft bgs			

NOTE(S):

FIELD BORING LOG



CLIENT: Illinois Power Generating Company
Site: Coffeen Power Station - Ash Pond 2
Location: Coffeen, Illinois
Project: 16E0031A
DATES: Start: 2/22/2018
 Finish: 2/22/2018
WEATHER: Overcast, cool (lo-40s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA with continuous split spoon
FIELD STAFF: Driller: C. Dutton
 Helper: M. Baetje
 Eng/Geo: R. Hasenyager

BOREHOLE ID: G411
Well ID: G411
Surface Elev: 620.49 ft. MSL
Completion: 16.47 ft. BGS
Station: 873,844.81N
 2,513,122.38E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 13.00 - During drilling ▽ = 8.00 - Upon completion ▽ = 2.65 - 1 March 2018			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	SS	5-2 4-4 N=6	15.6						620	
2A	17/24 71%	SS	4-4 4-6 N=8	28.9						618	
3A	20/24 83%	SS	2-3 4-5 N=7	18.8						616	
4A	24/24 100%	SS	2-3 4-4 N=7	19.0						614	
5A	20/24 83%	SS	2-3 4-4 N=7	21.0						612	
6A	24/24 100%	SS	2-4 5-6 N=9	14.4						610	
7A	24/24 100%	SS	1-3 2-4 N=5	18.9						608	
	23/24 96%	SS	3-17 20-20 N=37							606	
	0/6 0%	BD									

End of Boring = 16.47 ft bgs

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/3/2006
Finish: 5/3/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-01
Well ID: MW1D
Surface Elev: 607 ft. MSL
Completion: 40 ft. BGS
Station: 874,972.6N
 2,513,478.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 34.00 - While drilling ▽ = ▽ = 36.28 - MW01D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss	1-1 1-2 N=2	22	0.78 B		Very dark gray (10YR3/1), clayey SILT, trace sand		0			606	
2A	20/24 83%	ss	2-3 5-5 N=8	13	3.71 BSh		Light gray (10YR7/1) with 40% yellowish brown (10YR5/8) mottles, clayey SILT, trace sand and gravel		2			604	
3A	22/24 92%	ss	2-2 3-6 N=5	14	2.62 BSh		Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, clayey SILT, trace sand and gravel		4			602	
4A	23/24 96%	ss	8-12 19-19 N=31	13	3.30 BSh		Yellowish brown (10YR5/6) with 20% black (10YR2/1) mottles, clayey SILT, little sand and gravel		6			600	
5A	24/24 100%	ss	4-9 13-19 N=22	13	4.80 BSh		Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, clayey SILT, trace sand and gravel		8			598	
6A	22/24 92%	ss	3-6 12-15 N=18	12	8.73 B		Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, clayey SILT, trace sand and gravel		10			596	
7A	24/24 100%	ss	14-19 23-30 N=42	12	7.86 B		Dark gray (N4/1) with 25% yellowish brown (10YR5/6) mottles, clayey SILT, trace sand and gravel		12			594	
8A	24/24 100%	ss	4-8 12-14 N=20	13	7.56 B		Dark gray (N4/1), clayey SILT, trace sand and gravel		14			592	
9A	24/24 100%	ss	16-16 20-21 N=36	14	7.01 B		Dark gray (N4/1), clayey SILT, trace sand and gravel		16			590	
10A	24/24 100%	ss	3-5 8-11 N=13	14	5.24 B		Dark gray (N4/1), clayey SILT, trace sand and gravel		18			588	

NOTE(S): MW01D installed in SB-01.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/3/2006
Finish: 5/3/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-01
Well ID: MW1D
Surface Elev: 607 ft. MSL
Completion: 40 ft. BGS
Station: 874,972.6N
 2,513,478.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%	SH		15	3.69 B		22			586	Shelby tube taken from shallow well borehole at indicated depth.
12A	24/24 100%	SS	10-11 12-16 N=23	14	5.24 B		24			584	Shelby tube taken from shallow well borehole at indicated depth.
13A	6/24 25%	SH		14	3.69 B		26			582	
14A	24/24 100%	SS	10-12 18-18 N=30	15	4.27 B		28	Dark gray (N4/1), clayey SILT, trace sand and gravel [Continued from previous page]		580	
15A	24/24 100%	SS	5-9 11-16 N=20	14	4.27 B		30			578	
16A	24/24 100%	SS	5-8 10-12 N=18	17	2.72 BSh		32			576	
17A	24/24 100%	SS	8-14 14-16 N=28	14	5.62 B		34			574	
18A	23/24 96%	SS	8-28 40-65 N=68	14			36	Gray (N4/1), silty, fine to medium SAND, little coarse sand, trace gravel, wet		572	
19A	24/24 100%	SS	24-14 17-16 N=31	19	5.43 B		38	Very dark gray (10YR3/1), silty CLAY		570	
20A	24/24 100%	SS	3-5 6-10 N=11	24	3.50 BSh		40	Dark gray (N4/1) with 30% dark yellowish brown (10YR4/6) mottles, silty CLAY		568	

NOTE(S): MW01D installed in SB-01.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/5/2006
Finish: 5/5/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 4¼" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

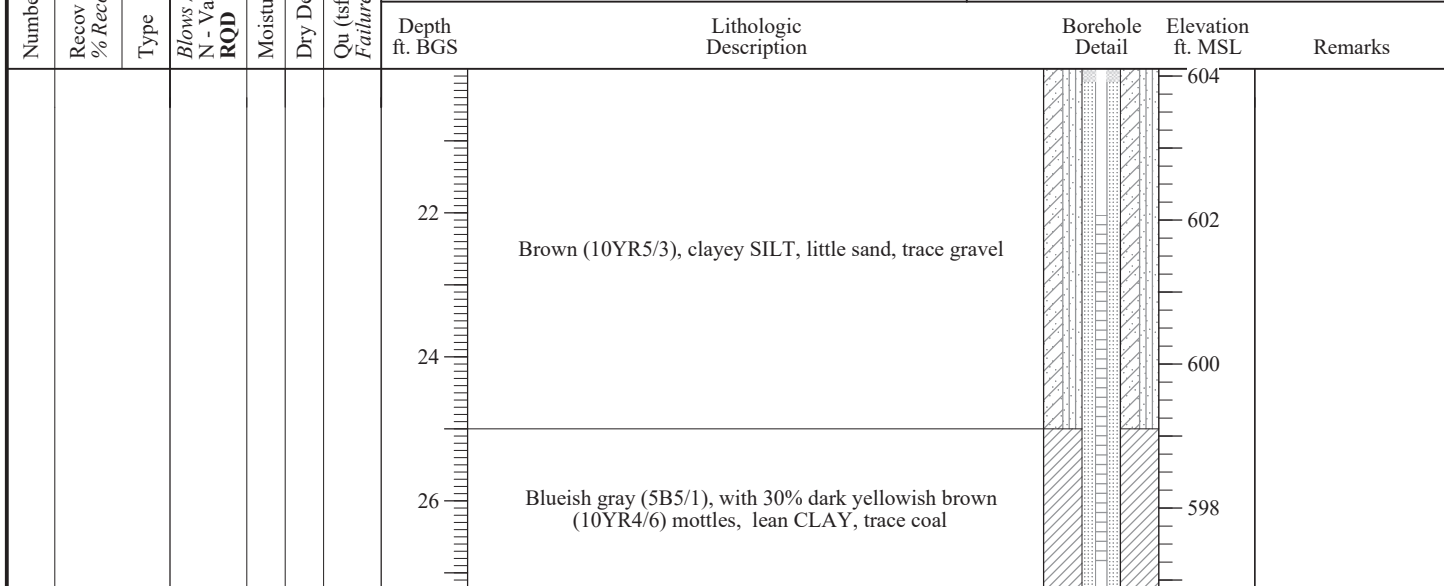
BOREHOLE ID: SB-02b
Well ID: MW2D
Surface Elev: 624 ft. MSL
Completion: 27 ft. BGS
Station: 876,414.0N
 2,513,209.7E

WEATHER: Partly cloudy, mild (high-50's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	

Quadrangle: Coffeen, IL
Township: East Fork
Section 10, Tier 7N; Range 3W

▽ = 12.80 - While drilling
 ▽ = 7.42 - MW02S on 6/1/06
 ▽ = 7.36 - MW02D on 6/1/06



End of Boring = 27.22 ft. BGS
 See SB-02 for sample & testing details

NOTE(S): MW02D installed in blind-drilled borehole within 10 ft of SB-02.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-02

Well ID: n/a

Surface Elev: 624 ft. MSL

Completion: 50 ft. BGS

Station: 876,410.0N

2,513,210.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24	ss	3-3	23				Dark grayish brown (10YR4/2), clayey SILT, trace sand		624	
1B	100%		4-5			1.96 B		Gray (10YR6/1), clayey SILT, trace sand			
1C			N=7	18				Yellowish brown (10YR5/4) with 20% gray (10YR6/1) mottles, lean CLAY, trace sand			
				29		1.94 B	2			622	
2A	24/24	ss	3-4	25		2.89 B	4	Yellowish brown (10YR5/8) with 15% gray (10YR6/1) mottles, lean CLAY, trace sand			
	100%		4-6							620	
			N=8								
3A	24/24	ss	3-5	20		2.91 B	6	Yellowish brown (10YR5/8) with 40% gray (10YR6/1) mottles, lean CLAY, trace sand			
	100%		5-7							618	
			N=10								
4A	24/24	ss	10-8	17		2.91 B	8	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, lean CLAY, little sand			
	100%		8-10							616	
			N=16								
5A	24/24	ss	3-3	19		1.94 B	10	Gray (10YR5/1) with 50% yellowish brown (10YR5/6) mottles, lean CLAY, little sand			
	100%		4-5							614	
			N=7								
6A	24/24	ss	2-3	18		2.13 B	12	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, clayey SAND, trace gravel			
	100%		6-5							612	
			N=9								
7A	24/24	ss	4-4	14		2.06 B	14	Gray (10YR6/1) with 30% white (10YR8/1) mottles, sandy CLAY, trace gravel			
	100%		7-10							610	
			N=11								
8A	24/24	ss	15-23	10			16	Pale brown (10YR6/3), silty, fine SAND, trace gravel, wet			
	100%		33-68							608	
			N=56								
9A	10/10	ss	48-62/4"	10		3.92 Sh	18			606	
	100%										
10A	12/12	ss	15-45	9		8.07 BSh	20	Dark gray (10YR4/1), clayey SILT, little sand and gravel			
	100%										

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-02

Well ID: n/a

Surface Elev: 624 ft. MSL

Completion: 50 ft. BGS

Station: 876,410.0N

2,513,210.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	36/60 60%	cs		9				22	Brown (10YR5/3), clayey SILT, little sand, trace gravel		604	
11B				11			24	602			600	
12A	60/60 100%	cs		21				26	Blueish gray (5B5/1), with 30% dark yellowish brown (10YR4/6) mottles, lean CLAY, trace coal		598	
12B				23			28	596			594	
13A	60/60 100%	cs		14				32	Very dark gray (10Y3/1), lean CLAY, trace sand and gravel		592	
14A	60/60 100%	cs		14			34	590			588	
								36			586	
								38				
								40				

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-02

Well ID: n/a

Surface Elev: 624 ft. MSL

Completion: 50 ft. BGS

Station: 876,410.0N

2,513,210.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
15A	60/60 100%	cs		14			42	Very dark gray (10Y3/1), lean CLAY, trace sand and gravel [Continued from previous page]		584	
						44			582		
						46			580		
						48			578		
						50			576		
16A	60/60 100%	cs		13							
							End of Boring = 50.0 ft. BGS				

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



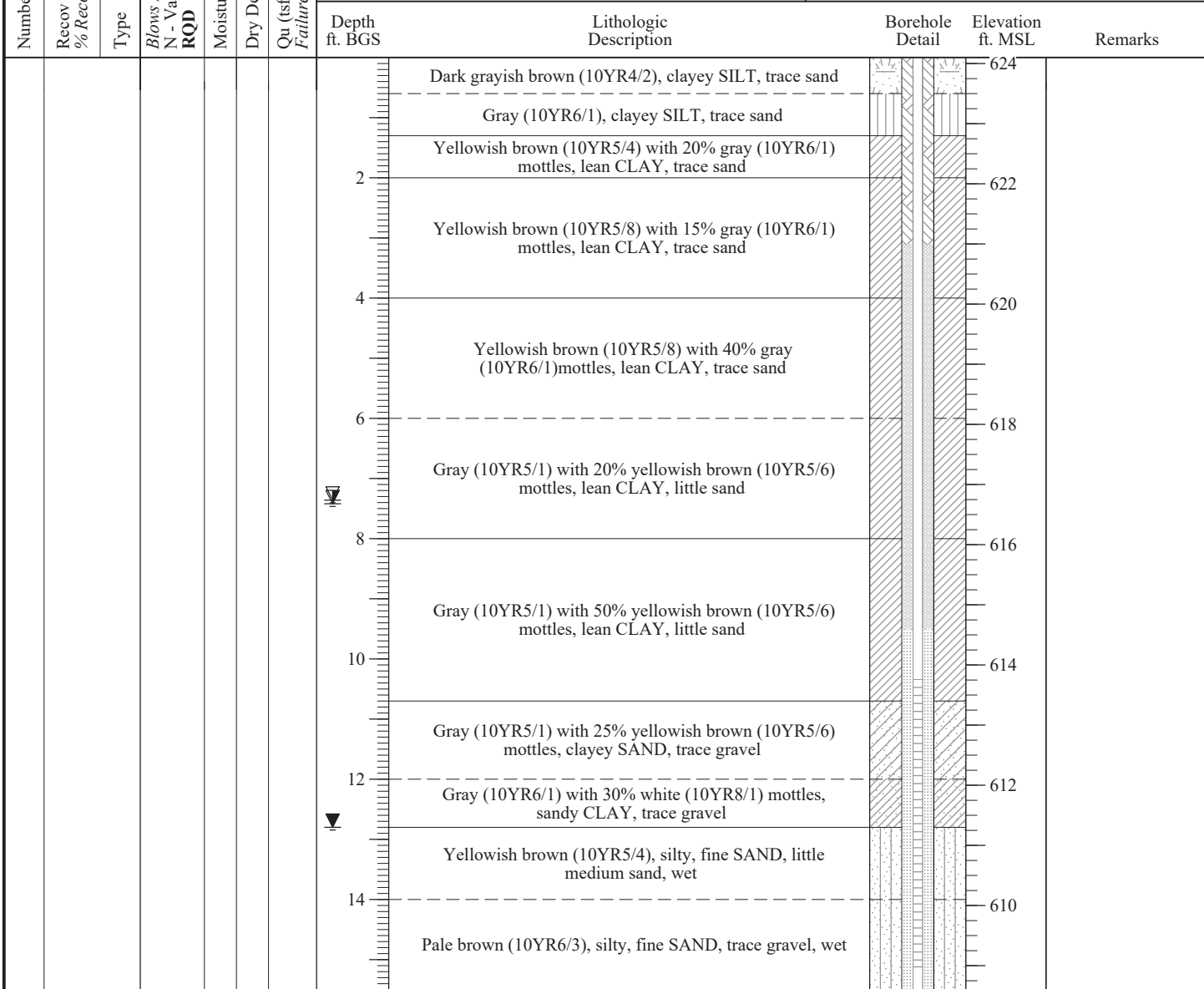
CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/5/2006
Finish: 5/5/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-02a
Well ID: MW2S
Surface Elev: 624 ft. MSL
Completion: 16 ft. BGS
Station: 876,408.9N
 2,513,210.0E

WEATHER: Partly cloudy, mild (high-50's)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▽ = 12.80 - While drilling
								Township: East Fork	▽ = 7.42 - MW02S on 6/1/06
								Section 10, Tier 7N; Range 3W	▽ = 7.36 - MW02D on 6/1/06



End of Boring = 15.51 ft. BGS
 See SB-02 for sample & testing details

NOTE(S): MW02S installed in blind-drilled borehole within 10 ft of SB-02.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-02

Well ID: n/a

Surface Elev: 624 ft. MSL

Completion: 50 ft. BGS

Station: 876,410.0N

2,513,210.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24	ss	3-3	23				Dark grayish brown (10YR4/2), clayey SILT, trace sand		624	
1B	100%		4-5		1.96	B		Gray (10YR6/1), clayey SILT, trace sand			
1C			N=7	29	1.94	B	2	Yellowish brown (10YR5/4) with 20% gray (10YR6/1) mottles, lean CLAY, trace sand		622	
2A	24/24	ss	3-4	25	2.89	B	4	Yellowish brown (10YR5/8) with 15% gray (10YR6/1) mottles, lean CLAY, trace sand		620	
	100%		4-6								
			N=8								
3A	24/24	ss	3-5	20	2.91	B	6	Yellowish brown (10YR5/8) with 40% gray (10YR6/1) mottles, lean CLAY, trace sand		618	
	100%		5-7								
			N=10								
4A	24/24	ss	10-8	17	2.91	B	8	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, lean CLAY, little sand		616	
	100%		8-10								
			N=16								
5A	24/24	ss	3-3	19	1.94	B	10	Gray (10YR5/1) with 50% yellowish brown (10YR5/6) mottles, lean CLAY, little sand		614	
	100%		4-5								
			N=7								
6A	24/24	ss	2-3	18	2.13	B	12	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, clayey SAND, trace gravel		612	
	100%		6-5								
			N=9								
7A	24/24	ss	4-4	14	2.06	B	14	Gray (10YR6/1) with 30% white (10YR8/1) mottles, sandy CLAY, trace gravel		610	
	100%		7-10								
			N=11								
8A	24/24	ss	15-23	10			16	Yellowish brown (10YR5/4), silty, fine SAND, little medium sand, wet		608	
	100%		33-68								
			N=56								
9A	10/10	ss	48-62/4"	10	3.92	Sh	18	Pale brown (10YR6/3), silty, fine SAND, trace gravel, wet		606	
	100%										
10A	12/12	ss	15-45	9	8.07	BSh	20	Dark gray (10YR4/1), clayey SILT, little sand and gravel			
	100%										

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-02

Well ID: n/a

Surface Elev: 624 ft. MSL

Completion: 50 ft. BGS

Station: 876,410.0N

2,513,210.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	36/60 60%	cs		9				22	Brown (10YR5/3), clayey SILT, little sand, trace gravel		604	
11B				11			24	602				
12A	60/60 100%	cs		21				26	Blueish gray (5B5/1), with 30% dark yellowish brown (10YR4/6) mottles, lean CLAY, trace coal		600	
12B				23			28	598				
13A	60/60 100%	cs		14			30	596				
14A	60/60 100%	cs		14			32	594	Very dark gray (10Y3/1), lean CLAY, trace sand and gravel		592	
							34	590				
							36	588				
							38	586				
							40					

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/5/2006

Finish: 5/5/2006

WEATHER: Partly cloudy, mild (high-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-02

Well ID: n/a

Surface Elev: 624 ft. MSL

Completion: 50 ft. BGS

Station: 876,410.0N

2,513,210.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
15A	60/60 100%	cs		14			42	Very dark gray (10Y3/1), lean CLAY, trace sand and gravel [Continued from previous page]		584		
						44	582					
						46	580					
						48	578					
						50	576					
16A	60/60 100%	cs		13								

End of Boring = 50.0 ft. BGS

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
 Site: CCB Management Facility
 Location: Coffeen, Illinois
 Project: 05S3004A
 DATES: Start: 4/27/2006
 Finish: 4/27/2006
 WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation
 Rig mfg/model: CME-650 Track Rig
 Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
 FIELD STAFF: Driller: B. Williamson
 Helper: R. Keedy
 Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-03
 Well ID: MW3D
 Surface Elev: 626 ft. MSL
 Completion: 58 ft. BGS
 Station: 876,554.5N
 2,514,535.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:							
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qtz (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 14.00 - While drilling ▽ = 7.03 - MW03S on 6/1/06 ▽ = 55.40 - MW03D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks			
1A	20/24	ss	2-2	17	2.07	SP	Very dark grayish brown (10YR3/2), lean CLAY Grayish brown (10YR5/2), lean CLAY, trace sand Yellowish brown (10YR5/6), lean CLAY, trace sand Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, lean CLAY, trace sand Yellowish brown (10YR5/6) with 40% gray (N5/1) mottles, sandy SILT, trace gravel Yellowish brown (10YR5/6) with 50% gray (N5/1) mottles, lean CLAY, trace sand and gravel Yellowish brown (10YR5/6), lean CLAY, little sand, trace gravel Dark gray (10YR4/1), sandy CLAY, trace gravel Gray (10YR6/1) with 50% yellowish brown (10YR5/8) mottles, lean CLAY, little sand, trace gravel Yellowish brown (10YR5/8), silty, fine SAND, little medium sand, trace gravel, wet Yellowish brown (10YR5/8), silty, fine SAND, little clay, wet Yellowish brown (10YR5/6), lean CLAY, little sand, trace gravel Yellowish brown (10YR5/6), silty SAND, trace gravel, wet Yellowish brown (10YR5/4), clayey SILT, trace sand and gravel Gray (10YR5/1), sandy SILT, trace gravel		626 624 622 620 618 616 614 612 610 608 606							
1B	83%	4-6 N=6	20			2										
1C				26	3.30	B					2					
2A	17/24	ss	2-3 4-5 N=7	25	3.05	B					4					
3A	20/24	ss	2-3 3-5 N=6	16	1.96	B					6					
4A	24/24	ss	4-3 5-6 N=8	21	2.27	B					8					
5A	21/24	ss	1-3 3-4 N=6	20	2.18	B					10					
5B				19							10					
6A	18/24	ss	1-2 2-4 N=4	24	0.87	B					12					
7A	23/24	SH	3-2	19							14					
7B	16/24	SS	2-4 N=4	12							14					
8A	23/24	ss	3-12 29-50 N=41	13							16					
8B				13							16					
8C				12							16					
9A	14/24	ss	8-82 85-72 N=167	10	6.98	B					18					
9B				13	6.18	BSh					18					
10A	18/24	ss	6-21 32-49 N=53	8	11.95	Sh					20					

Shelby tube taken from shallow well borehole at indicated depth.

NOTE(S): MW03D installed in SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

CONTRACTOR: Testing Service Corporation

Site: CCB Management Facility

Rig mfg/model: CME-650 Track Rig

BOREHOLE ID: SB-03

Location: Coffeen, Illinois

Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill

Well ID: MW3D

Project: 05S3004A

DATES: Start: 4/27/2006

FIELD STAFF: Driller: B. Williamson

Surface Elev: 626 ft. MSL

Finish: 4/27/2006

Helper: R. Keedy

Completion: 58 ft. BGS

Station: 876,554.5N

WEATHER: Sunny, mild (high-50's)

Eng/Geo: R. Hasenyager

2,514,535.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:						
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL	▼ = 14.00 - While drilling	▽ = 7.03 - MW03S on 6/1/06	▽ = 55.40 - MW03D on 6/1/06					
							Township: East Fork				Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
							Section 11, Tier 7N; Range 3W								
11A	19/24 79%	ss	3-20 35-29 N=55	10		5.36 Sh							604	Wood fragments	
12A	20/24 83%	ss	15-25 89-69 N=114	8		13.00 Sh	Gray (10YR5/1), sandy SILT, trace gravel [Continued from previous page]						602		
13A	23/24 96%	ss	14-19 24-22 N=43	9											
13B				13		6.98 BSh	Yellowish brown (10YR5/4), lean CLAY, trace sand and gravel						600		
14A	24/24 100%	ss	19-21 26-32 N=47	15		7.01 BSh	Gray (10YR4/1), clayey SILT, trace sand and gravel						598		
15A	21/24 88%	ss	10-25 25-23 N=50	12			Light gray (10YR6/1), SILT, trace sand								
15B				13		8.53 BSh							596		
16A	24/24 100%	ss	7-12 19-30 N=31	13		9.16 BSh							594		
17A	24/24 100%	ss	29-35 39-42 N=74	14		6.59 B	Gray (10YR4/1), lean CLAY, trace sand and gravel								
17B				14		3.49 Sh							592		
18A	23/24 96%	ss	6-8 11-17 N=19	13		7.21 BSh							590		
19A	24/24 100%	ss	19-21 31-31 N=52	13		6.98 BSh	Gray (10YR4/1), clayey SILT, trace sand and gravel						588		
20A	24/24 100%	ss	6-10 15-24 N=25	14		6.76 BSh	Gray (10YR4/1), lean CLAY, trace sand and gravel						586		

NOTE(S): MW03D installed in SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/27/2006
Finish: 4/27/2006
WEATHER: Sunny, mild (high-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-03
Well ID: MW3D
Surface Elev: 626 ft. MSL
Completion: 58 ft. BGS
Station: 876,554.5N
 2,514,535.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 14.00 - While drilling ▽ = 7.03 - MW03S on 6/1/06 ▽ = 55.40 - MW03D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	19/24 79%	ss	4-10 16-23 N=26	13	8.04 BSh		Gray (10YR4/1), lean CLAY, trace sand and gravel <i>[Continued from previous page]</i>		42			584	
22A	21/24 88%	ss	19-27 28-32 N=55	13	7.56 BSh		Gray (10YR4/1), clayey SILT, trace sand and gravel		44			582	
23A	23/24 96%	ss	4-9 14-18 N=23	14	6.98 BSh		Gray (10YR4/1), lean CLAY, trace sand and gravel		46			580	
24A	24/24 100%	ss	20-26 30-33 N=56	14	6.59 BSh		Gray (10YR4/1), lean CLAY, trace sand and gravel		48			578	
25A	24/24 100%	ss	6-10 13-12 N=23	20	3.30 BSh		Gray (10YR4/1), lean CLAY, trace sand and gravel		50			576	
26A	24/24 100%	ss	4-7 6-7 N=13	22	2.91 BSh		Gray (10YR4/1), lean CLAY, trace sand and gravel		52			574	
27A	24/24 100%	ss	7-18 37-85 N=55	16	4.05 BSh		Gray (10Y4/1), silty, fine to medium SAND, wet		54			572	
27B				13			Gray (10YR4/1), silty, fine SAND, trace clay, wet						
28A	23/24 96%	ss	15-34 34-19 N=68	11	3.22 SP				56			570	
28B				13	5.82 B								
29A	24/24 100%	ss	19-22 28-18 N=50	20	4.36 BSh		Very dark gray (10YR3/1), lean CLAY, trace sand and gravel		58			568	

End of Boring = 58.0 ft. BGS

NOTE(S): MW03D installed in SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/11/2006
Finish: 5/11/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-04a
Well ID: MW4S
Surface Elev: 622 ft. MSL
Completion: 15 ft. BGS
Station: 877,999.7N
 2,514,450.6E

WEATHER: Partly sunny, cool (mid-50's)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 8.00 - While drilling	▼ = 5.67 - MW04S on 6/1/06	▼ =
								Section 2, Tier 7N; Range 3W			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
								Very dark gray (10YR3/1), clayey SILT, trace sand			622
								Dark grayish brown (10YR4/2) with 15% yellowish brown (10YR5/6) mottles, lean CLAY			
							2				620
								Yellowish brown (10YR5/8) with 50% grayish brown (10YR5/2) mottles, lean CLAY			
							4				618
								Dark gray (10YR4/1), clayey SILT, trace sand			
							▼ 6				616
								Gray (10YR5/1), lean CLAY, trace sand and gravel			
							▼ 8				614
								Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, sandy SILT			
							10				612
								Very dark gray (10YR3/1) with 35% gray (10YR6/1) mottles, clayey SILT, wet			
							12				610
								Yellowish brown (10YR5/6), clayey SAND, trace gravel			
							14				608
								Light gray (10YR7/1) with 20% brown (10YR5/3) mottles, clayey SILT, trace sand and gravel			
								Yellowish brown (10YR5/8), silty, fine to medium SAND, trace coarse sand, wet			
								Gray (10YR4/1), sandy SILT, trace gravel			

End of Boring = 14.77 ft. BGS
 See SB-04 for sample & testing details

NOTE(S): MW04S installed in blind-drilled borehole within 10 ft of SB-03.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/11/2006

Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-04

Well ID: n/a

Surface Elev: 622 ft. MSL

Completion: 55 ft. BGS

Station: 878,000.0N

2,514,445.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▼ = 8.00 - While drilling ▽ = 5.67 - MW04S on 6/1/06 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	ss	1-2 2-4 N=4	24		1.09 B			0	Very dark gray (10YR3/1), clayey SILT, trace sand		622	
1B				29					1	Dark grayish brown (10YR4/2) with 15% yellowish brown (10YR5/6) mottles, lean CLAY			
2A	18/24 75%	ss	2-4 5-6 N=9	27		2.72 BSh			2	Yellowish brown (10YR5/8) with 50% grayish brown (10YR5/2) mottles, lean CLAY		620	
3A	16/24 67%	ss	2-3 4-6 N=7	23		1.71 B			4	Dark gray (10YR4/1), clayey SILT, trace sand		618	
4A	24/24 100%	ss	4-5 5-7 N=10	20		1.40 BSP			6	Gray (10YR5/1), lean CLAY, trace sand and gravel		616	
5A	24/24 100%	ss	1-2 2-2 N=4	23		0.70 B			8	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, sandy SILT		614	
6A	24/24 100%	ss	0-1 2-3 N=3	24		0.31 BSh			10	Very dark gray (10YR3/1) with 35% gray (10YR6/1) mottles, clayey SILT, wet		612	
6B				28					12	Yellowish brown (10YR5/6), clayey SAND, trace gravel		610	
7A	22/24 92%	ss	0-1 3-7 N=4	16		0.08 B			14	Light gray (10YR7/1) with 20% brown (10YR5/3) mottles, clayey SILT, trace sand and gravel			
7B				12					14	Yellowish brown (10YR5/8), silty, fine to medium SAND, trace coarse sand, wet		608	
7C				14					16			606	
8A	23/24 96%	ss	4-9 22-35 N=31	11		4.36 SP			18	Gray (10YR4/1), sandy SILT, trace gravel		604	
8B				7					20				
9A	24/24 100%	ss	27-38 54-50 N=92	9		2.18 BSh							
10A	24/24 100%	ss	24-29 39-34 N=68	9									

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/11/2006
Finish: 5/11/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-04
Well ID: n/a
Surface Elev: 622 ft. MSL
Completion: 55 ft. BGS
Station: 878,000.0N
 2,514,445.0E

WEATHER: Partly sunny, cool (mid-50's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
									Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
15A	60/60 100%	cs			13				42	Gray (10YR4/1), sandy SILT, trace gravel [Continued from previous page]		582		
								44					580	
								46					578	
16A	60/60 100%	cs			14				48	Greenish gray (10BG5/1) with 20% dark yellowish brown (10YR4/6) mottles, lean CLAY, trace sand		574		
16B					21			50					572	
								52					570	
17A	60/60 100%	cs			24				54			568		

End of Boring = 55.0 ft. BGS

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/12/2006
Finish: 5/17/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05
Well ID: MW5D
Surface Elev: 623 ft. MSL
Completion: 54 ft. BGS
Station: 878,174.8N
 2,513,290.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 3, Tier 7N; Range 3W		▽ = 10.00 - While drilling ▽ = 6.74 - MW05S on 6/1/06 ▽ = 50.44 - MW05D on 6/1/06		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	ss	0-2 3-4 N=5	29			0	Dark grayish brown (10YR4/2), clayey SILT, trace sand		622	
1B				28			2	Gray (10YR5/1 with 50% yellowish brown (10YR5/6) mottles, clayey SILT		620	
2A	22/24 92%	ss	2-5 5-7 N=10	27	2.47 BSh		4	Very dark brown (10YR2/2) with 20% dark gray (10YR4/1) mottles, clayey SILT, trace sand		618	
2B				22	2.13 B		6	Dark gray (10YR4/1) with 30% light gray (10YR7/1) mottles, lean CLAY		616	
3A	24/24 100%	ss	2-2 3-6 N=5	21	2.33 BSP		8	Gray (10YR6/1), lean CLAY, trace sand		614	
4A	24/24 100%	ss	7-6 6-8 N=12	21	1.90 BSh		10	Yellowish brown (10YR5/6), clayey SAND, trace gravel, wet		612	
5A	18/24 75%	ss	1-3 4-5 N=7	22	1.78 B		12	Yellowish brown (10YR5/6) with 50% gray 10YR5/1 mottles, sandy CLAY		610	
6A	20/24 83%	ss	0-1 3-4 N=4	22	0.70 BSh		14	Gray 10YR6/1, clayey, fine to medium SAND, trace gravel, wet		608	
6B				22			16	Brownish yellow (10YR6/6), silty, fine SAND, trace medium sand		606	
7A	24/24 100%	ss	3-6 17-20 N=23	19			18	Yellowish brown (10YR5/6), silty, fine SAND, wet		604	
7B				16			20	Brown (10YR5/3), silty SAND and GRAVEL, wet			
8A	20/24 83%	ss	4-16 25-25 N=41	20				Dark gray (10YR4/1), sandy SILT, trace clay and gravel			
8B				11							
9A	24/24 100%	ss	14-18 38-62 N=56	12							
9B				8							
10A	18/24 75%	ss	14-39 77 N=116	9	3.27 Sh						

NOTE(S): MW05D installed in SB-05.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

CONTRACTOR: Testing Service Corporation

Site: CCB Management Facility

Rig mfg/model: CME-650 Track Rig

BOREHOLE ID: SB-05

Location: Coffeen, Illinois

Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill

Well ID: MW5D

Project: 05S3004A

FIELD STAFF: Driller: B. Williamson

Surface Elev: 623 ft. MSL

DATES: Start: 5/12/2006

Helper: R. Keedy

Completion: 54 ft. BGS

Finish: 5/17/2006

Eng/Geo: R. Hasenyager

Station: 878,174.8N

WEATHER: Sunny, mild (mid-60's)

2,513,290.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	60/60 100%	cs		7				22	Dark gray (10YR4/1), silty, fine SAND, trace medium sand		600	
11B				7				24			598	
12A	60/60 100%	cs		7				26			596	
								28			594	
13A	60/60 100%	cs		8				30	Dark gray (10YR4/1), sandy SILT, trace clay and gravel		592	
								32			590	
14A	48/60 80%	cs		7				34			588	
								36			586	
14B				14				38	Gray (10YR6/1), fine to medium SAND		584	
								40	Dark gray (10YR4/1), sandy SILT, trace clay and gravel			

NOTE(S): MW05D installed in SB-05.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/12/2006

Finish: 5/17/2006

WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05

Well ID: MW5D

Surface Elev: 623 ft. MSL

Completion: 54 ft. BGS

Station: 878,174.8N

2,513,290.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	Township: East Fork	Section 3, Tier 7N; Range 3W	▽ = 10.00 - While drilling	▽ = 6.74 - MW05S on 6/1/06	▽ = 50.44 - MW05D on 6/1/06
									Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
15A	60/60 100%	cs												
16A	60/60 100%	cs												
16B														
17A	24/24 100%	ss	7-10 15-16 N=25											
18A	24/24 100%	ss	17-18 19-21 N=37											
									582	Dark gray (10YR4/1), sandy SILT, trace clay and gravel [Continued from previous page]				
									42	Gray (10YR4/1), clayey SILT, trace sand		580		
									44			578		
									46	Dark gray (10YR4/1), sandy SILT, trace clay and gravel		576		
									48			574		
									50	Gray(10YR4/1), silty, fine to medium SAND, trace organics and coal		572		
									52	Dark gray (10YR4/1), sandy SILT, trace clay and gravel		570		
									54	Greenish gray (5G5/1) with 40% yellowish brown (10YR5/6)mottles, lean CLAY				
End of Boring = 54.0 ft. BGS														

NOTE(S): MW05D installed in SB-05.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/17/2006
Finish: 5/17/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05a
Well ID: MW5S
Surface Elev: 623 ft. MSL
Completion: 18 ft. BGS
Station: 878,175.6N
 2,513,285.5E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	
						Quadrangle: Coffeen, IL Township: East Fork Section 3, Tier 7N; Range 3W		Water Level Information: ▽ = 10.00 - While drilling ▽ = 6.74 - MW05S on 6/1/06 ▽ = 50.44 - MW05D on 6/1/06	

Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
0	Dark grayish brown (10YR4/2), clayey SILT, trace sand		622	
2	Gray (10YR5/1 with 50% yellowish brown (10YR5/6) mottles, clayey SILT		620	
4	Very dark brown (10YR2/2) with 20% dark gray (10YR4/1) mottles, clayey SILT, trace sand		618	
6	Dark gray (10YR4/1) with 30% light gray (10YR7/1) mottles, lean CLAY		616	
8	Gray (10YR6/1), lean CLAY, trace sand		614	
10	Yellowish brown (10YR5/6), clayey SAND, trace gravel, wet		612	
12	Yellowish brown (10YR5/6) with 50% gray 10YR5/1) mottles, sandy CLAY		610	
14	Gray 10YR6/1), clayey, fine to medium SAND, trace gravel, wet		608	
16	Brownish yellow (10YR6/6), silty, fine SAND, trace medium sand		606	
17.71	Dark gray (10YR4/1), silty SAND and GRAVEL, wet		606	
17.71	Dark gray (10YR4/1), sandy SILT, trace clay and gravel		606	

End of Boring = 17.71 ft. BGS
 See SB-05 for sample & testing details

NOTE(S): MW05S installed in blind-drilled borehole within 10 ft of SB-05.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/12/2006

Finish: 5/17/2006

WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05

Well ID: MW5D

Surface Elev: 623 ft. MSL

Completion: 54 ft. BGS

Station: 878,174.8N

2,513,290.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	ss	0-2 3-4 N=5	29				Dark grayish brown (10YR4/2), clayey SILT, trace sand		622	
1B				28			2	Gray (10YR5/1 with 50% yellowish brown (10YR5/6) mottles, clayey SILT		620	
2A	22/24 92%	ss	2-5 5-7 N=10	27	2.47 BSh		4	Very dark brown (10YR2/2) with 20% dark gray (10YR4/1) mottles, clayey SILT, trace sand		618	
2B				22	2.13 B		6	Dark gray (10YR4/1) with 30% light gray (10YR7/1) mottles, lean CLAY		616	
3A	24/24 100%	ss	2-2 3-6 N=5	21	2.33 BSP		8	Gray (10YR6/1), lean CLAY, trace sand		614	
4A	24/24 100%	ss	7-6 6-8 N=12	21	1.90 BSh		10	Yellowish brown (10YR5/6), clayey SAND, trace gravel, wet		612	
5A	18/24 75%	ss	1-3 4-5 N=7	22	1.78 B		12	Yellowish brown (10YR5/6) with 50% gray 10YR5/1 mottles, sandy CLAY		610	
6A	20/24 83%	ss	0-1 3-4 N=4	22	0.70 BSh		14	Gray 10YR6/1, clayey, fine to medium SAND, trace gravel, wet		608	
6B				19			16	Brownish yellow (10YR6/6), silty, fine SAND, trace medium sand		606	
7A	24/24 100%	ss	3-6 17-20 N=23	16			18	Yellowish brown (10YR5/6), silty, fine SAND, wet		604	
7B				16				Brown (10YR5/3), silty SAND and GRAVEL, wet			
8A	20/24 83%	ss	4-16 25-25 N=41	20				Dark gray (10YR4/1), sandy SILT, trace clay and gravel			
8B				11							
9A	24/24 100%	ss	14-18 38-62 N=56	12							
9B				8							
10A	18/24 75%	ss	14-39 77 N=116	9	3.27 Sh						

NOTE(S): MW05D installed in SB-05.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/12/2006

Finish: 5/17/2006

WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05

Well ID: MW5D

Surface Elev: 623 ft. MSL

Completion: 54 ft. BGS

Station: 878,174.8N

2,513,290.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	60/60 100%	cs		7				22	Dark gray (10YR4/1), silty, fine SAND, trace medium sand		600	
11B				7				24			598	
12A	60/60 100%	cs		7				28			594	
13A				8				34			588	
14A	48/60 80%	cs		7				38	Gray (10YR6/1), fine to medium SAND		584	
14B				14				40			584	

NOTE(S): MW05D installed in SB-05.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/12/2006
Finish: 5/17/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-05
Well ID: MW5D
Surface Elev: 623 ft. MSL
Completion: 54 ft. BGS
Station: 878,174.8N
 2,513,290.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
15A	60/60 100%	cs					42	Dark gray (10YR4/1), sandy SILT, trace clay and gravel [Continued from previous page]		582	
							42	Gray (10YR4/1), clayey SILT, trace sand		580	
16A	60/60 100%	cs					44	Dark gray (10YR4/1), sandy SILT, trace clay and gravel		578	
							46	Dark gray (10YR4/1), sandy SILT, trace clay and gravel		576	
16B							48	Gray(10YR4/1), silty, fine to medium SAND, trace organics and coal		574	
17A	24/24 100%	ss	7-10 15-16 N=25				50	Dark gray (10YR4/1), sandy SILT, trace clay and gravel		572	
18A	24/24 100%	ss	17-18 19-21 N=37				52	Greenish gray (5G5/1) with 40% yellowish brown (10YR5/6)mottles, lean CLAY		570	
							54	End of Boring = 54.0 ft. BGS			

NOTE(S): MW05D installed in SB-05.

FIELD BORING LOG



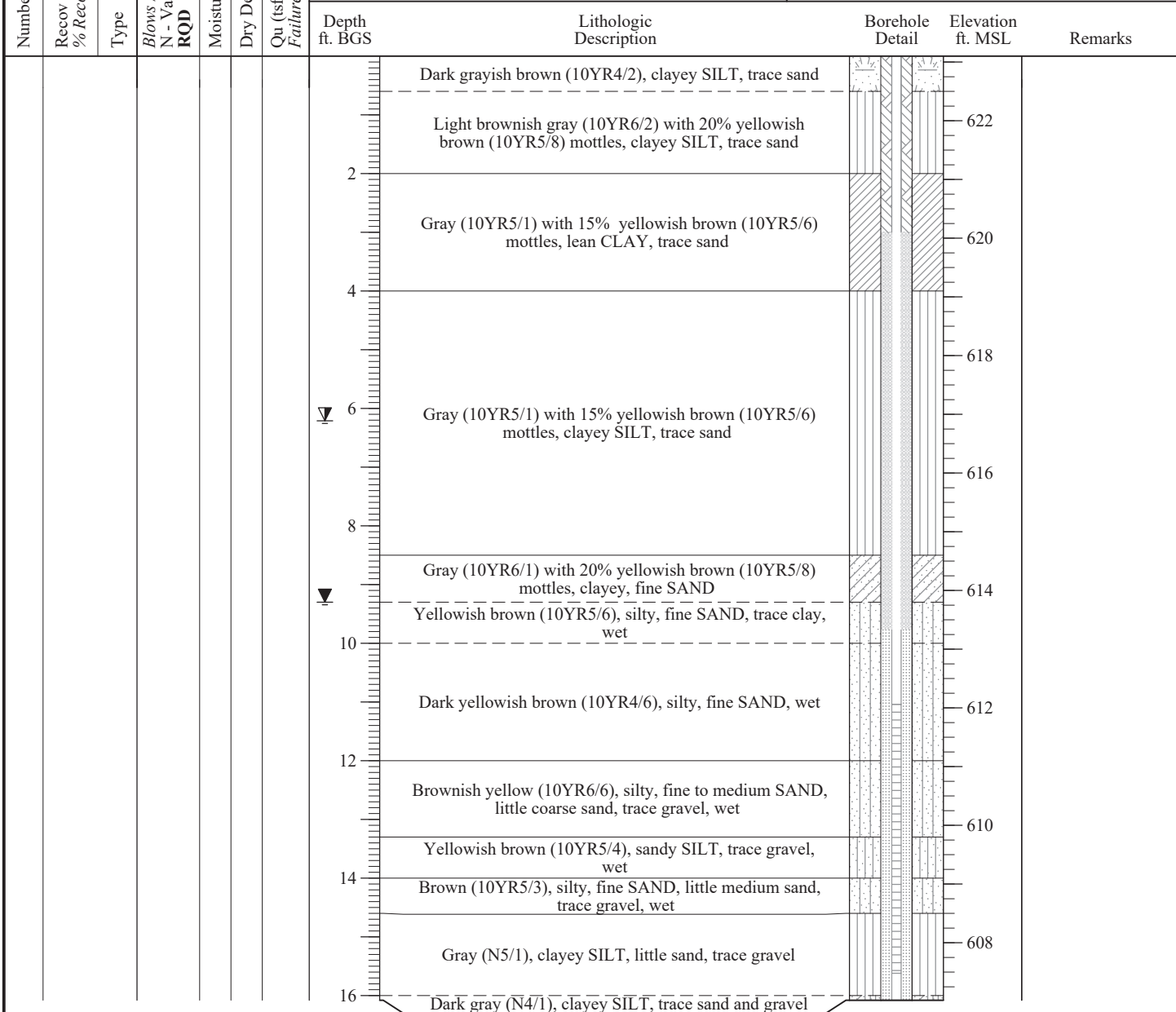
CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/4/2006
Finish: 5/4/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-06a
Well ID: MW6S
Surface Elev: 623 ft. MSL
Completion: 16 ft. BGS
Station: 879,021.2N
 2,513,189.4E

WEATHER: Partly sunny, cool (mid-50's)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▽ = 9.30 - While drilling
								Township: East Fork	▽ = 6.21 - MW06S on 6/1/06
								Section 3, Tier 7N; Range 3W	▽ =



End of Boring = 16.08 ft. BGS
 See SB-06 for sample & testing details

NOTE(S): MW06S installed in blind-drilled borehole within 10 ft of SB-06.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/4/2006

Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS sampler

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-06

Well ID: n/a

Surface Elev: 623 ft. MSL

Completion: 60 ft. BGS

Station: 879,015.0N

2,513,190.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	2-3 3-5 N=6	25	1.22 SP		0	Dark grayish brown (10YR4/2), clayey SILT, trace sand			
1B				28	1.96 B		2	Light brownish gray (10YR6/2) with 20% yellowish brown (10YR5/8) mottles, clayey SILT, trace sand		622	
2A	24/24 100%	ss	2-4 5-6 N=9	27	1.94 BSh		4	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand		620	
2B				19	2.52 BSh						
3A	24/24 100%	ss	1-3 4 N=7	21	1.36 B		6	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, clayey SILT, trace sand		618	
4A	24/24 100%	ss	4-6 6 N=12	17	1.78 B		8			616	
5A				18	0.85 BSh						
5B	22/24 92%	ss	1-2 3-3 N=5	17	0.81 None		10	Gray (10YR6/1) with 20% yellowish brown (10YR5/8) mottles, clayey, fine SAND		614	
5C				21	0.31 None			Yellowish brown (10YR5/6), silty, fine SAND, trace clay, wet			
6A	20/24 83%	ss	0-1 2-4 N=3	25			12	Dark yellowish brown (10YR4/6), silty, fine SAND, wet		612	
7A	24/24 100%	ss	2-4 7-8 N=11	15			14	Brownish yellow (10YR6/6), silty, fine to medium SAND, little coarse sand, trace gravel, wet		610	
7B				12	1.48 BSh			Yellowish brown (10YR5/4), sandy SILT, trace gravel, wet			
8A	22/24 92%	ss	11-33 57-35 N=90	8	7.18 Sh		16	Brown (10YR5/3), silty, fine SAND, little medium sand, trace gravel, wet		608	
8B				9	11.35 Sh			Gray (N5/1), clayey SILT, little sand, trace gravel		606	
9A	22/24 92%	ss	30-39 46-55 N=85	11			18			604	
8A				7	11.64 Sh			Dark gray (N4/1), clayey SILT, trace sand and gravel			
10A	24/24 100%	ss	4-23 50-51 N=73	7							

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/4/2006
Finish: 5/4/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-06
Well ID: n/a
Surface Elev: 623 ft. MSL
Completion: 60 ft. BGS
Station: 879,015.0N
 2,513,190.0E

WEATHER: Partly sunny, cool (mid-50's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	20/24 83%	ss	6-16 33-58 N=49	11		3.49 BSh	22			602	
12A	20/24 83%	ss	45-56 54-50/3" N=110	9		11.64 Sh	24			600	
13A	12/24 50%	ss	26-78	12		2.84 Sh	26			598	
14A	8/24 33%	ss	52-48/2"	9		5.43 BSh	28			596	
15A	24/24 100%	ss	10-24 30-40 N=54	13		4.95 BSh	30	Dark gray (N4/1), clayey SILT, trace sand and gravel [Continued from previous page]		594	
16A	21/24 88%	ss	10-16 37-38 N=53	8		10.91 BSh	32			592	
17A	17/24 71%	ss	36-47 61/5"	9			34			590	
18A	22/24 92%	ss	11-36 45-60 N=81	9		10.04 Sh	36			588	
19A	22/24 92%	ss	40-35 34-29 N=69	10		9.60 Sh	38			586	
19B				13		8.92 B					
20A	24/24 100%	ss	3-8 12-15 N=20	14		8.53 BSh	40	Very dark greenish gray (10Y3/1), lean CLAY, trace sand		584	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/4/2006
Finish: 5/4/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-06
Well ID: n/a
Surface Elev: 623 ft. MSL
Completion: 60 ft. BGS
Station: 879,015.0N
 2,513,190.0E

WEATHER: Partly sunny, cool (mid-50's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	23/24 96%	ss	3-5 8-12 N=13	15	4.27 B		42			582	
22A	22/24 92%	ss	6-6 7-8 N=13	14	3.49 B		44			580	
23A	24/24 100%	ss	5-7 9-12 N=16	16	2.72 B		46	Very dark greenish gray (10Y3/1), lean CLAY, trace sand and gravel		578	
24A	24/24 100%	ss	4-8 10-11 N=18	15	4.07 B		48			576	
25A	24/24 100%	ss	5-6 9-13 N=15	15	3.10 B		50			574	
26A	24/24 100%	ss	5-6 8-12 N=14	22	1.94 B		52			572	
27A	24/24 100%	ss	5-6 8-8 N=14	23	2.13 B		54	Dark greenish gray (10BG4/1), lean CLAY		570	
28A	24/24 100%	ss	3-5 6-8 N=11	24	2.33 BSh		56			568	
29A	24/24 100%	ss	12-10 12-14 N=22	24	3.30 BSh		58	Greenish gray (5G4/1) with 10% dark yellowish brown (10YR3/4) mottles, lean CLAY, trace sand		566	
30A	24/24 100%	ss	5-8 12-13 N=20	27	2.13 BSh		60	Dark greenish gray (10G4/1), lean CLAY, trace sand		564	
End of Boring = 60.0 ft. BGS											

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: P. McIntire
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-07a
Well ID: MW7S
Surface Elev: 625 ft. MSL
Completion: 14 ft. BGS
Station: 879,181.1N
 2,514,397.5E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▽ = 10.80 - While drilling
								Township: East Fork	▽ = 4.90 - MW07S on 6/1/06
								Section 2, Tier 7N; Range 3W	▽ =

Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
0	Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		624	
2	Dark gray (10YR4/1), moist, soft, silty CLAY with trace and and trace gravel.		622	
4	Gray (10YR5/1) with 20% yellowish brown mottles, moist, soft, silty CLAY with trace sand and trace gravel.		620	
4.90	Black (10YR2/1), very moist, soft, clayey SILT.		620	
5.5	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist soft, silty CLAY with trace sand and trace gravel.		618	
6	Black (10YR2/1), moist, soft, silty CLAY with trace sand and trace gravel.		618	
8	Dark gray (10YR4/1), very moist, soft, silty CLAY with some sand and trace gravel.		616	
10	Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, soft, silty CLAY with sand and gravel.		614	
12	Yellowish brown (10YR5/8) wet, very soft, clayey, very fine- to fine-grained, SAND with trace gravel.		612	
12.5	Yellowish brown (10YR5/8) wet, soft, clayey, very fine- to fine-grained, SAND with trace gravel.		612	
14	Gray (10YR4/1), moist, very hard, sandy, clayey SILT with gravel.			

End of Boring = 14.39 ft. BGS
 See SB-07 for sample & testing details

NOTE(S): MW07S installed in blind-drilled borehole within 10 ft of SB-07.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/5/2006
Finish: 5/8/2006

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: P. McIntire
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-07
Well ID: n/a
Surface Elev: 625 ft. MSL
Completion: 54 ft. BGS
Station: 879,180.0N
 2,514,390.0E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 10.80 - While drilling ▽ = 4.90 - MW07S on 6/1/06 ▽ =	

Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	1-1 1-1 N=2	23	2.40 B		0	Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		624	
1B				24			1	Dark gray (10YR4/1), moist, soft, silty CLAY with trace and trace gravel.			
2A	24/24 100%	ss	0-0 1-1 N=1	26	2.33 B		2	Gray (10YR5/1) with 20% yellowish brown mottles, moist, soft, silty CLAY with trace sand and trace gravel.		622	
3A				25	4.33 BSh		3	Black (10YR2/1), very moist, soft, clayey SILT.		620	
3B	24/24 100%	ss	0-1 1-2 N=2	25	2.52 BSh		4	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist soft, silty CLAY with trace sand and trace gravel.			
3C				22	3.05 B		5	Black (10YR2/1), moist, soft, silty CLAY with trace sand and trace gravel.			
4A	24/24 100%	ss	0-1 1-2 N=2	22	1.75 B		6	Dark gray (10YR4/1), very moist, soft, silty CLAY with some sand and trace gravel.		618	
5A	24/24 100%	ss	0-0 1-1 N=1	22	1.24 B		7			616	
6A	24/24 100%	ss	0-0 0-1 N=0	24	0.54 B		8	Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, soft, silty CLAY with sand and gravel.		614	
6B				20	1.65 B		9	Yellowish brown (10YR5/8) wet, very soft, clayey, very fine- to fine-grained, SAND with trace gravel.			
7A				24			10	Yellowish brown (10YR5/8) wet, soft, clayey, very fine- to fine-grained, SAND with trace gravel.		612	
7B	24/24 100%	ss	0-0 1-2 N=1	13	2.89 B		11				
8A	24/24 100%	ss	2-5 7-9 N=12	8	5.04 BSh		12			610	
9A	24/24 100%	ss	3-6 6-8 N=12	9	9.27 BSh		13	Gray (10YR4/1), moist, very hard, sandy, clayey SILT with gravel.		608	
10A	24/24 100%	ss	3-6 8-11 N=14	9	11.13 BSh		14			606	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/5/2006

Finish: 5/8/2006

WEATHER: Partly cloudy, mild (mid-70s)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: P. McIntire

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-07

Well ID: n/a

Surface Elev: 625 ft. MSL

Completion: 54 ft. BGS

Station: 879,180.0N

2,514,390.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:							
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 10.80 - While drilling	▼ = 4.90 - MW07S on 6/1/06	▼ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A				8				Section 2, Tier 7N; Range 3W Township: East Fork				604	Gray (10YR4/1), moist, very hard, sandy, clayey SILT with gravel. <i>[Continued from previous page]</i>			
11B	35/48 73%	cs		14					22	Gray (10YR6/1), wet, loose, fine- to medium-grained SAND.			602			
									24				600			
	50/60 83%	cs							26				598			
12A				7					28	Gray (10YR5/1), moist, very hard, sandy, clayey SILT with gravel.			596			
13A				6					30				594			
13B	56/60 93%	cs		6					32	Gray (10YR5/1), wet, loose, medium- to very coarse-grained SAND and GRAVEL.			592			
13C				8						Gray (10YR5/1), wet, dense, very fine- to fine-grained SAND.						
									34	Gray (10YR5/1), moist, very hard, sandy, clayey SILT with gravel.			590			
									36				588			
14A	60/60 100%	cs		14					38	Gray (10YR5/1), moist, firm, sandy, clayey SILT with gravel.			586			
									40							

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/5/2006

Finish: 5/8/2006

WEATHER: Partly cloudy, mild (mid-70s)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: P. McIntire

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-07

Well ID: n/a

Surface Elev: 625 ft. MSL

Completion: 54 ft. BGS

Station: 879,180.0N

2,514,390.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL		▽ = 10.80 - While drilling		
									Township: East Fork		▽ = 4.90 - MW07S on 6/1/06		
									Section 2, Tier 7N; Range 3W				
									Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
15A	60/60 100%	cs			14				42	Gray (10YR5/1), moist, firm, sandy, clayey SILT with gravel. <i>[Continued from previous page]</i>		584	
								44	582				
									46			580	
	60/60 100%	cs							48			578	
16A					15				50	Dark greenish gray (10BG4/1), moist, soft, silty CLAY with trace sand and trace gravel.		576	
									52	Very dark gray (10YR3/1), moist firm, clayey SILT.		574	
17A	60/60 100%	cs			24				54	Dark greenish gray (10BG4/1), moist, soft, silty CLAY with trace sand and trace gravel.		572	
17B													
End of Boring = 54.0 ft BGS													

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG

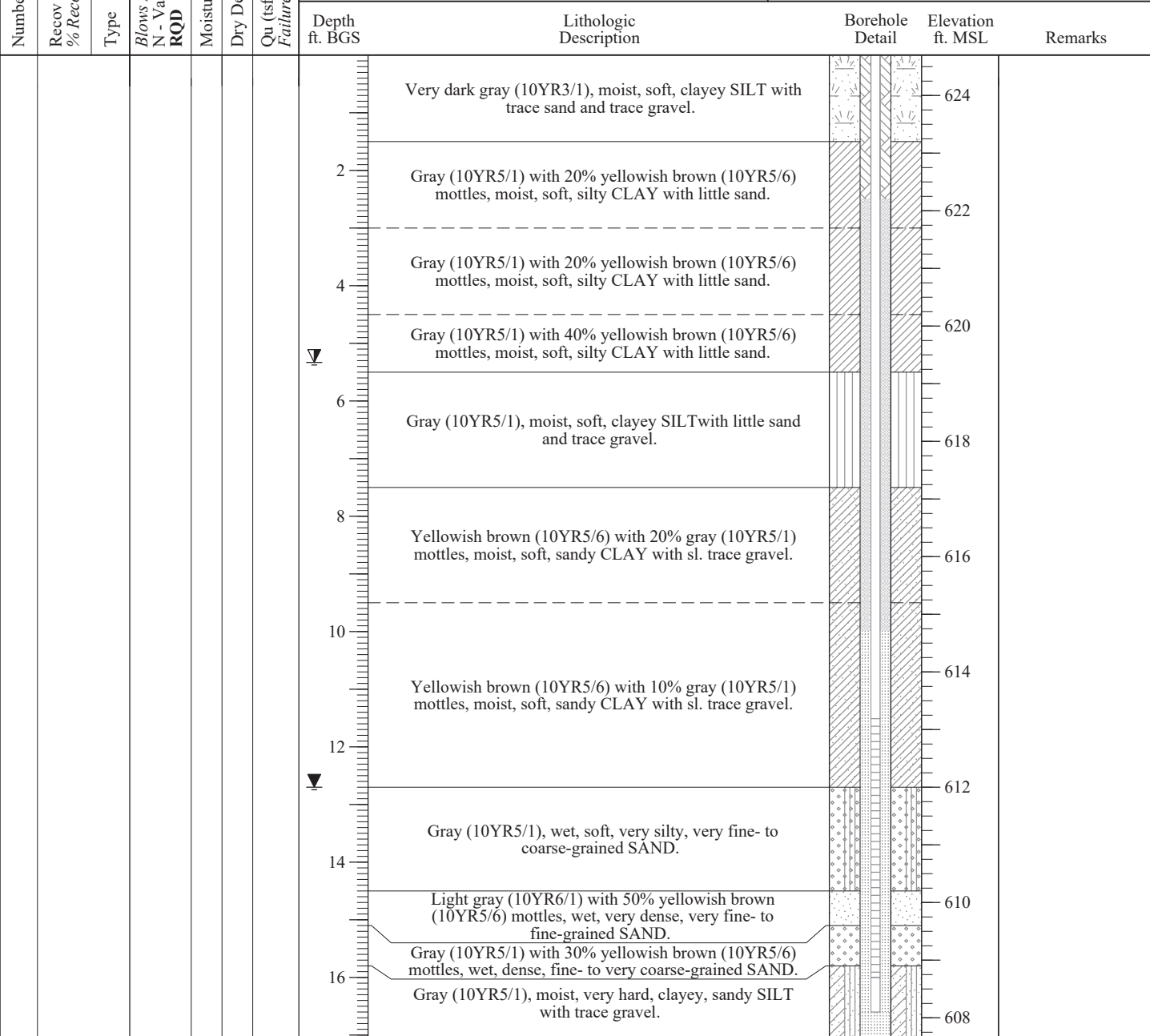


CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/10/2006
Finish: 5/10/2006

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-08a
Well ID: MW8S
Surface Elev: 625 ft. MSL
Completion: 17 ft. BGS
Station: 879,776.6N
 2,514,478.8E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▽ = 12.70 - While drilling
								Township: East Fork	▽ = 5.33 - MW08S on 6/1/06
								Section 2, Tier 7N; Range 3W	▽ =



End of Boring = 17.08 ft. BGS
 See SB-08 for sample & testing details

NOTE(S): MW08S installed in blind-drilled borehole within 10 ft of SB-08.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/9/2006

Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-08

Well ID: n/a

Surface Elev: 625 ft. MSL

Completion: 59 ft. BGS

Station: 879,770.0N

2,514,480.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL	▼ = 12.70 - While drilling	▼ = 5.33 - MW08S on 6/1/06	▼ =	
							Section 2, Tier 7N; Range 3W				
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
2A	24/24 100%	ss	1-1 1 N=2				Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		624		
	24/24 100%	ss	1-1 1 N=2			1.59 B	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		622		
3A	24/24 100%	ss	1-1 1 N=2			1.86 B	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		620		
3B	24/24 100%	ss	0-1 1 N=2			1.20 B	Gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		618		
4A	24/24 100%	ss	0-1 1 N=2			1.01 None	Gray (10YR5/1), moist, soft, clayey SILT with little sand and trace gravel.		616		
5A	24/24 100%	ss	0-1 1 N=2				Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, soft, sandy CLAY with sl. trace gravel.		614		
6A	24/24 100%	ss	0-1 1 N=2			0.70 BSh	Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, moist, soft, sandy CLAY with sl. trace gravel.		612		
6B	24/24 100%	ss	0-1 1 N=2				Gray (10YR5/1), wet, soft, very silty, very fine- to coarse-grained SAND.		610		
7A	20/24 83%	ss	0-1 3 N=4				Light gray (10YR6/1) with 50% yellowish brown (10YR5/6) mottles, wet, very dense, very fine- to fine-grained SAND.		608		
7B	20/24 83%	ss	0-1 3 N=4				Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, wet, dense, fine- to very coarse-grained SAND.		606		
8A	23/24 96%	ss	7-11 16 N=27			5.45 Sh	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		606		
8B	23/24 96%	ss	7-11 16 N=27								
9A	24/30 80%	cs									

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/9/2006

Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-08

Well ID: n/a

Surface Elev: 625 ft. MSL

Completion: 59 ft. BGS

Station: 879,770.0N

2,514,480.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
10A	60/60 100%	cs		7			22			604	
11A	36/60 60%	cs		10			24			602	
							26			600	
							28			598	
							30	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		596	
12A	60/60 100%	cs		6			32			594	
							34			592	
							36			590	
13A	60/60 100%	cs		7			38			588	
							40			586	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/9/2006

Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-08

Well ID: n/a

Surface Elev: 625 ft. MSL

Completion: 59 ft. BGS

Station: 879,770.0N

2,514,480.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
	60/60 100%	cs						Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 12.70 - While drilling ▽ = 5.33 - MW08S on 6/1/06 ▽ =			
14A				14			42	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		584		
14B				11			43	Greenish gray (10BG5/1) with 10% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with little sand and trace gravel		582		
							44	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.				
15A				12			44	Dark yellowish brown (10YR4/4), moist, firm, clayey SILT with some sand and trace gravel.		580		
15B	60/60 100%	cs		15			46	Gray (10YR4/1), moist, hard, clayey, sandy SILT with trace gravel.		578		
15C				13			48	Gray (10YR4/1) with 50% very dark grayish brown (10YR3/2), moist, firm, silty CLAY with sand and trace gravel.		576		
16A	60/60 100%	cs		14			50	Gray (10YR4/1), moist, hard, clayey, sandy SILT with trace gravel.		574		
16B				19			52			572		
17A				24			54	Greenish gray (10BG4/1), moist, soft, silty CLAY with little sand and sl. trace gravel.		570		
17B	60/60 100%	cs		19			56	Yellowish brown (10YR5/6) with 20% Greenish gray (10BG4/1) mottles, moist, soft, silty CLAY with little sand and sl. trace gravel.		568		
							58					

End of Boring = 58.5 ft.

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/3/2006

Finish: 5/3/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09

Well ID: MW9D

Surface Elev: 625 ft. MSL

Completion: 54 ft. BGS

Station: 879,679.7N

2,515,666.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▼ = 14.00 - While drilling ▽ = 5.23 - MW09S on 6/1/06 ▽ = 52.46 - MW09D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	22/24 92%	ss	0-1 1-1 N=2	24	1.65 B				1	Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		624	
2A	20/24 83%	ss	1-2 1-1 N=3	27	2.06 B				2	Yellowish brown (10YR5/6), moist, soft, silty CLAY with little sand.		622	
3A	24/24 100%	ss	1-1 1-1 N=2	24	1.65 B			▽	3			620	
4A	24/24 100%	ss	1-1 1-2 N=2	23	1.57 B				4	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		618	
5A	24/24 100%	ss	0-1 1-1 N=2	27	1.85 B				5	Gray (10YR5/1) with 50% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with sand.		616	
6A	24/24 100%	ss	0-0 0-0 N=0	22	1.44 B				6	Yellowish brown (10YR5/6) with 10% Gray (10YR5/1) mottles, moist, soft, silty CLAY with sand.		614	
7A				21	0.93 BSh				7	Yellowish brown (10YR5/8), very moist, soft, sandy CLAY with trace gravel.		612	
7B	24/24 100%	ss	0-0 1-2 N=1	18	0.97 BSh				8	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist to very moist, clayey SILT with sand and trace gravel.		610	
7C				15				▽	9	Gray (10YR5/1), wet, soft, clayey very fine- to fine-grained SAND.		610	
8A				18					10	Gray (10YR5/1), wet, loose, fine- to very coarse-grained SAND with trace gravel.		610	
8B	24/24 100%	ss	1-3 6-6 N=9	18					11	Gray (10YR5/1), wet, dense, silty very fine- to fine-grained SAND.		608	
9A	24/24 100%	ss	7-13 19-25 N=32	8	7.86 Sh				12			608	
	12/12 100%	cs							13	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		606	

NOTE(S): MW09D installed in SB-09.
CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/3/2006

Finish: 5/3/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09

Well ID: MW9D

Surface Elev: 625 ft. MSL

Completion: 54 ft. BGS

Station: 879,679.7N

2,515,666.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	▽ = 14.00 - While drilling
11A	60/60 100%	cs		2			22				604	
12A	49/60 82%	cs		3			26	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]			602	
						28						600
							30				598	2" Sand stringer
13A	60/60 100%	cs		3			32				596	
							34				594	
14A	60/60 100%	cs		10			36	Yellowish brown (10YR5/6) with 40% gray (10YR5/1) mottles, moist, very hard, clayey SILT with little sand and occasional dry, silt stringers (<1").			592	
							38	Gray (10YR5/1), moist, very hard, clayey SILT with little sand and trace gravel.			590	
14B				11			40	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.			588	
											586	DRILLER NOTE: Appears more plastic

NOTE(S): MW09D installed in SB-09.
CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/3/2006
Finish: 5/3/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09
Well ID: MW9D
Surface Elev: 625 ft. MSL
Completion: 54 ft. BGS
Station: 879,679.7N
 2,515,666.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
									Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W		▽ = 14.00 - While drilling ▽ = 5.23 - MW09S on 6/1/06 ▽ = 52.46 - MW09D on 6/1/06		
15A	60/60 100%	cs			12				42	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		584	
									44	Very dark grayish brown (10YR3/2), moist, hard, silty CLAY with trace sand and trace organic matter. Very dark grayish brown (10YR3/2), moist, firm, PEAT.		580	
19	60/60 100%	cs			22				46	Dark greenish gray (10BG4/1), moist, firm, silty CLAY with little sand and trace gravel.		578	
16A					17				48	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. Dark gray (10YR3/1), moist, hard, clayey SILT.		576	
17A	60/60 100%	cs							50			574	
									52	Dark greenish gray (10BG4/1), moist, firm, silty CLAY with little sand and trace gravel.		572	
									54	End of Boring = 54.0 ft.			

NOTE(S): MW09D installed in SB-09.
 CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/3/2006
Finish: 5/3/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4¼" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09a
Well ID: MW9S
Surface Elev: 625 ft. MSL
Completion: 16 ft. BGS
Station: 879,684.9N
 2,515,666.2E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
									Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		624	
								2	Yellowish brown (10YR5/6), moist, soft, silty CLAY with little sand.		622	
								4			620	
								6	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		618	
								8	Gray (10YR5/1) with 50% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with sand.		616	
								10	Yellowish brown (10YR5/6) with 10% Gray (10YR5/1) mottles, moist, soft, silty CLAY with sand.		614	
								12	Yellowish brown (10YR5/8), very moist, soft, sandy CLAY with trace gravel.		612	
								13	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist to very moist, clayey SILT with sand and trace gravel.		612	
								14	Gray (10YR5/1), wet, soft, clayey very fine- to fine-grained SAND.		610	
								14.5	Gray (10YR5/1), wet, loose, fine- to very coarse-grained SAND with trace gravel.		610	
								15	Gray (10YR5/1), wet, dense, silty very fine- to fine-grained SAND.		610	
								16	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		610	

End of Boring = 16.20 ft. BGS
 See SB-09 for sample & testing details

NOTE(S): MW09S installed in blind-drilled borehole within 10 ft of SB-09.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
 Site: CCB Management Facility
 Location: Coffeen, Illinois
 Project: 05S3004A
 DATES: Start: 5/3/2006
 Finish: 5/3/2006
 WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
 Rig mfg/model: CME-1050 ATV Rig
 Drilling Method: 4 1/4" HSA w/SS & CME samplers
 FIELD STAFF: Driller: K. Doetzel
 Helper: S. McCartney
 Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09
 Well ID: MW9D
 Surface Elev: 625 ft. MSL
 Completion: 54 ft. BGS
 Station: 879,679.7N
 2,515,666.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	22/24 92%	ss	0-1 1-1 N=2	24	1.65 B		1.65	Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		624	
2A	20/24 83%	ss	1-2 1-1 N=3	27	2.06 B		2.06	Yellowish brown (10YR5/6), moist, soft, silty CLAY with little sand.		622	
3A	24/24 100%	ss	1-1 1-1 N=2	24	1.65 B		1.65	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		620	
4A	24/24 100%	ss	1-1 1-2 N=2	23	1.57 B		1.57	Gray (10YR5/1) with 50% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with sand.		618	
5A	24/24 100%	ss	0-1 1-1 N=2	27	1.85 B		1.85	Yellowish brown (10YR5/6) with 10% Gray (10YR5/1) mottles, moist, soft, silty CLAY with sand.		616	
6A	24/24 100%	ss	0-0 0-0 N=0	22	1.44 B		1.44	Yellowish brown (10YR5/8), very moist, soft, sandy CLAY with trace gravel.		614	
7A	24/24 100%	ss	0-0 1-2 N=1	18	0.93 BSh		0.93	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist to very moist, clayey SILT with sand and trace gravel.		612	
7B	24/24 100%	ss	0-0 1-2 N=1	18	0.97 BSh		0.97	Gray (10YR5/1), wet, loose, fine- to very coarse-grained SAND with trace gravel.		610	
7C	24/24 100%	ss	1-3 6-6 N=9	18			14	Gray (10YR5/1), wet, soft, clayey very fine- to fine-grained SAND.		610	
8A	24/24 100%	ss	1-3 6-6 N=9	18			14	Gray (10YR5/1), wet, loose, fine- to very coarse-grained SAND with trace gravel.		610	
8B	24/24 100%	ss	1-3 6-6 N=9	18			14	Gray (10YR5/1), wet, dense, silty very fine- to fine-grained SAND.		610	
9A	24/24 100%	ss	7-13 19-25 N=32	8	7.86 Sh		7.86	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		608	
	12/12 100%	cs					18			606	

NOTE(S): MW09D installed in SB-09.
 CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/3/2006
Finish: 5/3/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-09
Well ID: MW9D
Surface Elev: 625 ft. MSL
Completion: 54 ft. BGS
Station: 879,679.7N
 2,515,666.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
									Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W		Water Level Information: ▽ = 14.00 - While drilling ▽ = 5.23 - MW09S on 6/1/06 ▽ = 52.46 - MW09D on 6/1/06		
15A	60/60 100%	cs			12				42	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		584	
									44	Very dark grayish brown (10YR3/2), moist, hard, silty CLAY with trace sand and trace organic matter. Very dark grayish brown (10YR3/2), moist, firm, PEAT.		580	
19	60/60 100%	cs			22				46	Dark greenish gray (10BG4/1), moist, firm, silty CLAY with little sand and trace gravel.		578	
16A					17				48	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. Dark gray (10YR3/1), moist, hard, clayey SILT.		576	
17A	60/60 100%	cs							50			574	
									52	Dark greenish gray (10BG4/1), moist, firm, silty CLAY with little sand and trace gravel.		572	
									54	End of Boring = 54.0 ft.			

NOTE(S): MW09D installed in SB-09.
 CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/1/2006

Finish: 5/1/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10

Well ID: MW10D

Surface Elev: 621 ft. MSL

Completion: 49 ft. BGS

Station: 878,245.1N

2,515,914.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	ss	1-1 1-1 N=2	23	2.33	B	2.33	Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		620	
2A	24/24 100%	ss	1-1 2-2 N=3	25	2.47	BSh	2.47	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		618	
3A	24/24 100%	ss	1-2 1-2 N=3	24	2.33	B	2.33	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		616	
4A	24/24 100%	ss	1-1 1-1 N=2	23	1.55	B	1.55	Gray (10YR5/1), moist, soft, clayey, very fine- to fine-grained SAND.		614	
4B	24/24 100%	ss	1-1 1-1 N=2	23	1.55	B	1.55	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand and trace gravel.		612	
5A	17/24 71%	ss	1-1 1-2 N=2	23	2.06	B	2.06	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, clayey, very fine- to medium-grained SAND with trace gravel.		610	
6A	24/24 100%	ss	0-1 1-1 N=2	17	1.16	B	1.16	Grayish brown (10YR5/2), wet, loose, very fine- to medium-grained SAND with trace coarse- to very coarse-grained sand.		608	
7A	23/24 96%	ss	1-1 3-3 N=4	17	2.84	B	2.84	Yellowish brown (10YR5/6), moist, firm, silty CLAY with little sand and trace gravel.		606	
7B	23/24 96%	ss	1-1 3-3 N=4	18	2.84	B	2.84	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		604	
8A	24/24 100%	ss	3-5 9-9 N=14	10	7.64	BSP	7.64			602	
10A	12/33 36%	cs		16						602	

NOTE(S): MW10D installed in SB-10.
CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/1/2006
Finish: 5/1/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10
Well ID: MW10D
Surface Elev: 621 ft. MSL
Completion: 49 ft. BGS
Station: 878,245.1N
 2,515,914.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	56/60 93%	cs		12			22	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		600	
11B				13			24			598	
11C				196			26			596	2" Gravel stringer
12A	60/60 100%	cs		13			28	Dark grayish brown (10YR4/2), very moist, soft, PEAT.		594	
13A	60/60 100%	cs		12			30			592	
							32	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.		590	
							34			588	
							36			586	
							38			584	
							40			582	

NOTE(S): MW10D installed in SB-10.
 CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/1/2006
Finish: 5/1/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10
Well ID: MW10D
Surface Elev: 621 ft. MSL
Completion: 49 ft. BGS
Station: 878,245.1N
 2,515,914.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W		▽ = 11.80 - While drilling ▽ = 4.91 - MW10S on 6/1/06 ▽ = 47.48 - MW10D on 6/1/06			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
14A	60/60 100%	cs		14			42	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		580		
14B				23			44	Greenish gray (5G5/1), moist, firm, silty CLAY with little sand and sl. trace gravel.		578	1" Gravel stringer	
15A				13			46	Greenish gray (5G5/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with little sand and sl. trace gravel.		576		
15B	60/60 100%	cs		25			48	Greenish gray (5G5/1), moist, firm, silty CLAY with little sand and sl. trace gravel.		574		
								End of Boring = 48.75 ft.				

NOTE(S): MW10D installed in SB-10.
 CME-1050 had 280# hammer for SPT.

FIELD BORING LOG

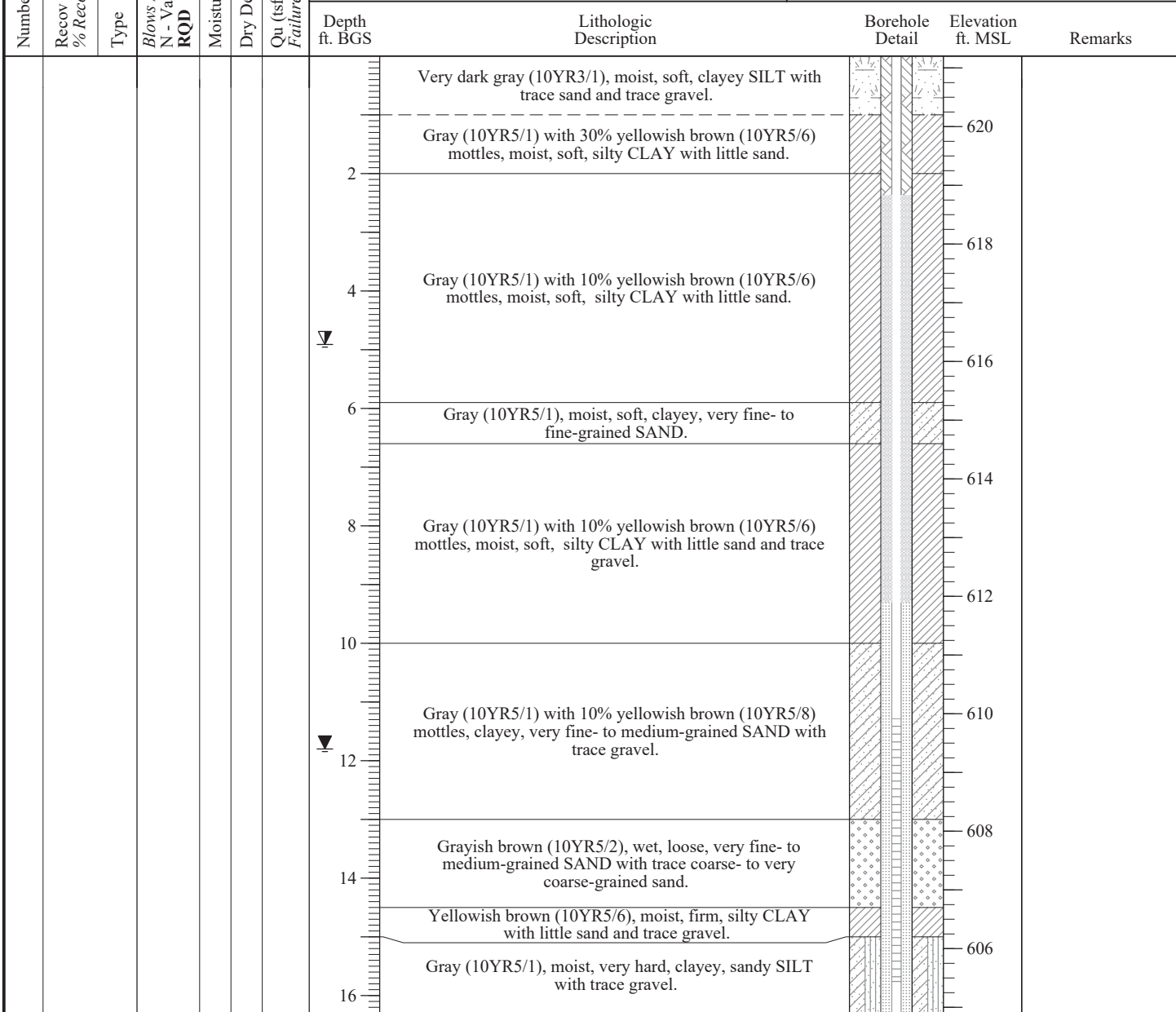


CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/2/2006
Finish: 5/2/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10a
Well ID: MW10S
Surface Elev: 621 ft. MSL
Completion: 16 ft. BGS
Station: 878,250.5N
 2,515,914.4E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 11.80 - While drilling ▽ = 4.91 - MW10S on 6/1/06 ▽ = 47.48 - MW10D on 6/1/06



End of Boring = 16.30 ft. BGS
 See SB-10 for sample & testing details

NOTE(S): MW10S installed in blind-drilled borehole within 10 ft of SB-10.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/1/2006

Finish: 5/1/2006

WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10

Well ID: MW10D

Surface Elev: 621 ft. MSL

Completion: 49 ft. BGS

Station: 878,245.1N

2,515,914.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	ss	1-1 1-1 N=2	23	2.33	B	2.33	Very dark gray (10YR3/1), moist, soft, clayey SILT with trace sand and trace gravel.		620	
2A	24/24 100%	ss	1-1 2-2 N=3	25	2.47	BSh	2.47	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		618	
3A	24/24 100%	ss	1-2 1-2 N=3	24	2.33	B	2.33	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand.		616	
4A	24/24 100%	ss	1-1 1-1 N=2	23	1.55	B	1.55	Gray (10YR5/1), moist, soft, clayey, very fine- to fine-grained SAND.		614	
4B	24/24 100%	ss	1-1 1-1 N=2	23	1.55	B	1.55	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with little sand and trace gravel.		614	
5A	17/24 71%	ss	1-1 1-2 N=2	23	2.06	B	2.06	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, clayey, very fine- to medium-grained SAND with trace gravel.		612	
6A	24/24 100%	ss	0-1 1-1 N=2	17	1.16	B	1.16	Grayish brown (10YR5/2), wet, loose, very fine- to medium-grained SAND with trace coarse- to very coarse-grained sand.		610	
7A	23/24 96%	ss	1-1 3-3 N=4	17	2.84	B	2.84	Yellowish brown (10YR5/6), moist, firm, silty CLAY with little sand and trace gravel.		608	
7B	23/24 96%	ss	1-1 3-3 N=4	18	2.84	B	2.84	Yellowish brown (10YR5/6), moist, firm, silty CLAY with little sand and trace gravel.		608	
8A	24/24 100%	ss	3-5 9-9 N=14	10	7.64	BSP	7.64	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		606	
10A	12/33 36%	cs		16				Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		604	

NOTE(S): MW10D installed in SB-10.
CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/1/2006
Finish: 5/1/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-10
Well ID: MW10D
Surface Elev: 621 ft. MSL
Completion: 49 ft. BGS
Station: 878,245.1N
 2,515,914.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	56/60 93%	cs		12			22	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>		600	
11B				13			24			598	
11C				196			26			596	2" Gravel stringer
12A	60/60 100%	cs		13			28	Dark grayish brown (10YR4/2), very moist, soft, PEAT.		594	
13A	60/60 100%	cs		12			30			592	
							32	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.		590	
							34			588	
							36			586	
							38			584	
							40			582	

NOTE(S): MW10D installed in SB-10.
 CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/28/2006

WEATHER: Partly cloudy, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-11

Well ID: MW11D

Surface Elev: 622 ft. MSL

Completion: 36 ft. BGS

Station: 876,749.6N

2,515,976.7E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	3-3 4-4 N=7	16				Very dark grey (10YR3/1), moist, soft, clayey SILT.			
1B						1.09 B		Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, soft, silty CLAY.		620	
2A	24/24 100%	ss	4-7 14-21 N=21	27		2.13 BSh		Yellowish brown (10YR5/6) with 40% gray (10YR5/1) mottles, moist, soft, silty CLAY.		618	
3A	24/24 100%	ss	4-6 7-8 N=13	24		2.27 BSh		Gray (10YR5/1), moist, soft, clayey SILT.			
3B						2.33 B				616	
4A	24/24 100%	ss	7-8 13-14 N=21	21		2.13 None		Gray (10YR4/1), moist, firm, silty CLAY with little sand and trace gravel.		614	
5A	24/24 100%	ss	3-4 4-4 N=8	20		1.55 Sh				612	
6A	24/24 100%	ss	3-2 2-3 N=4	19		0.78 BSh					
6B								Gray (10YR4/1), wet, soft, sandy CLAY.		610	
7A	24/24 100%	ss	3-6 14-21 N=20	16				Yellowish brown (10YR5/6) with 50% gray (10YR5/1) mottles, wet, soft, sandy CLAY with trace gravel.			
7B						5.45 BSP		Gray (10YR5/1), moist, very hard, sandy, clayey SILT with gravel.		608	
8A	11/12 92%	ss	41-61/5"	7				Gray (10YR5/1), moist, hard, silty CLAY with little sand and trace gravel.			
								Light gray (10YR7/1) with 10% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY with little sand and trace gravel.		606	
9A	48/54 89%	cs		8				Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		604	

NOTE(S): MW11D installed in SB-11.
CME-1050 had 280# hammer for SPT.

DRILLER NOTE:
sampler recovered wet.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/28/2006

WEATHER: Partly cloudy, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-11

Well ID: MW11D

Surface Elev: 622 ft. MSL

Completion: 36 ft. BGS

Station: 876,749.6N

2,515,976.7E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 11.70 - While drilling	▽ = 5.42 - MW11S on 6/1/06	▽ = 6.03 - MW11D on 6/1/06	
								Section 2, Tier 7N; Range 3W				
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
10A	54/60 90%	cs		6								DRILLER NOTE: soft drilling 20.5' to 21.0' Wet, gravelly zone from 22.4' to 22.8'
								Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]				
11A	58/60 97%	cs		8								
12A	32/60 53%	cs		9				Gray (10YR5/1), wet, loose, clayey, very fine- to medium-grained SAND with little coarse-grained sand and trace gravel.				
12B				9				Gray (10YR6/1) with occasional black (10YR2/1) varves, dry, dense, SILT with trace sand.				
13B	22/22 100%	ss	7-19 41-50/4" N=60	14		4.85	BSh	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.				
								End of Boring = 36.33 ft.				

NOTE(S): MW11D installed in SB-11.
CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/28/2006
Finish: 4/28/2006

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-11a
Well ID: MW11S
Surface Elev: 622 ft. MSL
Completion: 14 ft. BGS
Station: 876,749.4N
 2,515,971.2E

WEATHER: Partly cloudy, mild (mid-60's)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 11.70 - While drilling	▽ = 5.42 - MW11S on 6/1/06	▽ = 6.03 - MW11D on 6/1/06
								Section 2, Tier 7N; Range 3W			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
								Very dark grey (10YR3/1), moist, soft, clayey SILT.			
								Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, soft, silty CLAY.			
								Yellowish brown (10YR5/6) with 40% gray (10YR5/1) mottles, moist, soft, silty CLAY.			
								Gray (10YR5/1), moist, soft, clayey SILT.			
								Gray (10YR4/1), moist, firm, silty CLAY with little sand and trace gravel.			
								Gray (10YR4/1), wet, soft, sandy CLAY.			
								Yellowish brown (10YR5/6) with 50% gray (10YR5/1) mottles, wet, soft, sandy CLAY with trace gravel.			
								Gray (10YR5/1), moist, very hard, sandy, clayey SILT with gravel.			

End of Boring = 14.08 ft. BGS
 See SB-11 for sample & testing details

NOTE(S): MW11S installed in blind-drilled borehole within 10 ft of SB-11.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/28/2006

WEATHER: Partly cloudy, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-11

Well ID: MW11D

Surface Elev: 622 ft. MSL

Completion: 36 ft. BGS

Station: 876,749.6N

2,515,976.7E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:						
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 11.70 - While drilling ▽ = 5.42 - MW11S on 6/1/06 ▽ = 6.03 - MW11D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	24/24 100%	ss	3-3 4-4 N=7	16					0	Very dark grey (10YR3/1), moist, soft, clayey SILT.				
1B					1.09 B				2	Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, soft, silty CLAY.		620		
2A	24/24 100%	ss	4-7 14-21 N=21	27	2.13 BSh				4	Yellowish brown (10YR5/6) with 40% gray (10YR5/1) mottles, moist, soft, silty CLAY.		618		
3A	24/24 100%	ss	4-6 7-8 N=13	24	2.27 BSh				6	Gray (10YR5/1), moist, soft, clayey SILT.		616		
3B					2.33 B				8			614		
4A	24/24 100%	ss	7-8 13-14 N=21	21	2.13 None				10	Gray (10YR4/1), moist, firm, silty CLAY with little sand and trace gravel.		612		
5A	24/24 100%	ss	3-4 4-4 N=8	20	1.55 Sh				12			610		
6A	24/24 100%	ss	3-2 2-3 N=4	19	0.78 BSh				14	Gray (10YR4/1), wet, soft, sandy CLAY.		608		
6B									16	Yellowish brown (10YR5/6) with 50% gray (10YR5/1) mottles, wet, soft, sandy CLAY with trace gravel.		606		
7A	24/24 100%	ss	3-6 14-21 N=20	16					18	Gray (10YR5/1), moist, very hard, sandy, clayey SILT with gravel.		604		
7B					5.45 BSP				20					
8A	11/12 92%	ss	41-61/5"	7					16	Gray (10YR5/1), moist, hard, silty CLAY with little sand and trace gravel.		602		
									18	Light gray (10YR7/1) with 10% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY with little sand and trace gravel.				
9A	48/54 89%	cs		8					20	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		602		

NOTE(S): MW11D installed in SB-11.
CME-1050 had 280# hammer for SPT.

DRILLER NOTE:
sampler recovered wet.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/27/2006

Finish: 4/28/2006

WEATHER: Partly cloudy, mild (mid-60's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-11

Well ID: MW11D

Surface Elev: 622 ft. MSL

Completion: 36 ft. BGS

Station: 876,749.6N

2,515,976.7E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 11.70 - While drilling	▼ = 5.42 - MW11S on 6/1/06	▼ = 6.03 - MW11D on 6/1/06	
								Section 2, Tier 7N; Range 3W				
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
10A	54/60 90%	cs		6								DRILLER NOTE: soft drilling 20.5' to 21.0' Wet, gravelly zone from 22.4' to 22.8'
								Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]				
11A	58/60 97%	cs		8								
12A	32/60 53%	cs		9								
12B				9								
	22/22 100%	ss	7-19 41-50/4" N=60									
13B				14		4.85	BSh					
								End of Boring = 36.33 ft.				

NOTE(S): MW11D installed in SB-11.
CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/10/2006
Finish: 5/10/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-12
Well ID: MW12D
Surface Elev: 622 ft. MSL
Completion: 50 ft. BGS
Station: 875,515.1N
 2,515,900.6E

WEATHER: Foggy to partly sunny, mild (hi-60's)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24"/24	ss	2-3 4-5 N=7	22	1.27	Sh	0	Very dark gray (10YR3/1), clayey SILT, trace sand		622	
1B				27	2.33	B	2	Dark gray (10YR4/1) with 15% yellowish brown (10YR5/6) mottles, lean CLAY		620	
2A	19"/24	ss	2-4 5-7 N=9	24	2.91	B	4	Yellowish brown (10YR5/8) with 40% grayish brown (10YR5/2) mottles, lean CLAY		618	
3A	20"/24	ss	2-2 3-4 N=5	21	2.13	B	6	Gray (10YR5/1), lean CLAY, trace sand and gravel		616	
4A	24/24 100%	ss	4-5 5-6 N=10	21	1.36	BSh	8	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand		614	
5A	24"/24	ss	1-2 2-5 N=4	20	1.47	BSh	10	Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, lean CLAY, trace sand and gravel		612	
6A	20"/24	ss	0-1 3-3 N=4	21	0.62	B	12	Yellowish brown (10YR 5/8) with 25% gray (10YR6/1) mottles, clayey SAND, trace gravel		610	
7A	21"/24	ss	2-2 3-5 N=5	22	0.19	B	14	Gray (10YR6/1), clayey SAND, trace gravel, wet		608	
7B				22				Dark yellowish brown (10YR4/6), clayey SAND, trace gravel, wet			
7C				15	3.71	B		Light yellowish brown (10YR6/4) with 30% brownish yellow (10YR6/6) mottles, clayey SILT, trace sand and gravel			
8A	24"/24	ss	4-13 18-29 N=31	9	5.15	BSh	16			606	
9A	24"/24	ss	26-32 46-50 N=78	9	6.59	Sh	18	Dark greenish gray (N4/1), clayey SILT, trace sand and gravel		604	
10A	24"/24	ss	21-31 63-71 N=94	11	6.39	Sh	20				

NOTE(S): MW12D installed in SB-12.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
 Site: CCB Management Facility
 Location: Coffeen, Illinois
 Project: 05SS3004A
 DATES: Start: 5/10/2006
 Finish: 5/10/2006

CONTRACTOR: Testing Service Corporation
 Rig mfg/model: CME-650 Track Rig
 Drilling Method: 3/4" HSA w/SS & CME samplers
 FIELD STAFF: Driller: B. Williamson
 Helper: R. Keedy
 Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-12
 Well ID: MW12D
 Surface Elev: 622 ft. MSL
 Completion: 50 ft. BGS
 Station: 875,515.1N
 2,515,900.6E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	60/60 100%	cs	6				22	Dark greenish gray (N4/1), clayey SILT, trace sand and gravel [Continued from previous page]		602	
							24				600
12A	60/60 100%	cs	7				26	Dark greenish gray (N4/1), sandy SILT, trace gravel		598	
							28				596
13A	60/60 100%	cs	13				30	Very dark gray (N3/1), clayey SILT, trace sand and gravel		594	
							32				592
14A	60/60 100%	cs	16				34			590	
							36			588	
							38			586	
							40			584	

NOTE(S): MW12D installed in SB-12.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/10/2006

Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-12

Well ID: MW12D

Surface Elev: 622 ft. MSL

Completion: 50 ft. BGS

Station: 875,515.1N

2,515,900.6E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
15A	60/60 100%	cs		14			42	Very dark gray (N3/1), clayey SILT, trace sand and gravel <i>[Continued from previous page]</i>		582	
							44			580	
							46	Very dark gray (N3/1), PEAT		578	
							48	Gray (N5/1) with 30% yellowish brown (10YR5/6) mottles, lean CLAY		576	
16A	60/60 100%	cs		45			50	End of Boring = 50.0 ft. BGS		574	

NOTE(S): MW12D installed in SB-12.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/10/2006
Finish: 5/10/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-12a
Well ID: MW12S
Surface Elev: 622 ft. MSL
Completion: 16 ft. BGS
Station: 875,520.1N
 2,515,900.5E

WEATHER: Foggy to partly sunny, mild (hi-60's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL
							Very dark gray (10YR3/1), clayey SILT, trace sand		622	
							Dark gray (10YR4/1) with 15% yellowish brown (10YR5/6) mottles, lean CLAY		620	
							Yellowish brown (10YR5/8) with 40% grayish brown (10YR5/2) mottles, lean CLAY		618	
							Gray (10YR5/1), lean CLAY, trace sand and gravel		616	
							Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand		614	
							Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, lean CLAY, trace sand and gravel		612	
							Yellowish brown (10YR 5/8) with 25% gray (10YR6/1) mottles, clayey SAND, trace gravel		610	
							Gray (10YR6/1), clayey SAND, trace gravel, wet		608	
							Dark yellowish brown (10YR4/6), clayey SAND, trace gravel, wet			
							Light yellowish brown (10YR6/4) with 30% brownish yellow (10YR6/6) mottles, clayey SILT, trace sand and gravel			
							Dark greenish gray (N4/1), clayey SILT, trace sand and gravel			

End of Boring = 15.61 ft. BGS
 See SB-12 for sample & testing details

NOTE(S): MW12S installed in blind-drilled borehole within 10 ft of SB-12.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/10/2006

Finish: 5/10/2006

WEATHER: Foggy to partly sunny, mild (hi-60's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-12

Well ID: MW12D

Surface Elev: 622 ft. MSL

Completion: 50 ft. BGS

Station: 875,515.1N

2,515,900.6E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24"/24	ss	2-3 4-5 N=7	22	1.27	Sh	0	Very dark gray (10YR3/1), clayey SILT, trace sand		622	
1B							2	Dark gray (10YR4/1) with 15% yellowish brown (10YR5/6) mottles, lean CLAY		620	
2A	19"/24	ss	2-4 5-7 N=9	24	2.91	B	4	Yellowish brown (10YR5/8) with 40% grayish brown (10YR5/2) mottles, lean CLAY		618	
3A	20"/24	ss	2-2 3-4 N=5	21	2.13	B	6	Gray (10YR5/1), lean CLAY, trace sand and gravel		616	
4A	24/24 100%	ss	4-5 5-6 N=10	21	1.36	BSh	8	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand		614	
5A	24"/24	ss	1-2 2-5 N=4	20	1.47	BSh	10	Yellowish brown (10YR5/8) with 20% gray (10YR6/1) mottles, lean CLAY, trace sand and gravel		612	
6A	20"/24	ss	0-1 3-3 N=4	21	0.62	B	12	Yellowish brown (10YR 5/8) with 25% gray (10YR6/1) mottles, clayey SAND, trace gravel		610	
7A	21"/24	ss	2-2 3-5 N=5	22	0.19	B	13	Gray (10YR6/1), clayey SAND, trace gravel, wet		608	
7B							14	Dark yellowish brown (10YR4/6), clayey SAND, trace gravel, wet		608	
7C							14	Light yellowish brown (10YR6/4) with 30% brownish yellow (10YR6/6) mottles, clayey SILT, trace sand and gravel		608	
8A	24"/24	ss	4-13 18-29 N=31	9	5.15	BSh	16			606	
9A	24"/24	ss	26-32 46-50 N=78	9	6.59	Sh	18	Dark greenish gray (N4/1), clayey SILT, trace sand and gravel		604	
10A	24"/24	ss	21-31 63-71 N=94	11	6.39	Sh	20			604	

NOTE(S): MW12D installed in SB-12.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/10/2006

Finish: 5/10/2006

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 3/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-12

Well ID: MW12D

Surface Elev: 622 ft. MSL

Completion: 50 ft. BGS

Station: 875,515.1N

2,515,900.6E

WEATHER: Foggy to partly sunny, mild (hi-60's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	60/60 100%	cs	6				22	Dark greenish gray (N4/1), clayey SILT, trace sand and gravel <i>[Continued from previous page]</i>		602	
							24			600	
12A	60/60 100%	cs	7				26	Dark greenish gray (N4/1), sandy SILT, trace gravel		598	
							28			596	
13A	60/60 100%	cs	13				30	Very dark gray (N3/1), clayey SILT, trace sand and gravel		594	
							32			592	
14A	60/60 100%	cs	16				34			590	
							36			588	
							38			586	
							40			584	

NOTE(S): MW12D installed in SB-12.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/10/2006
Finish: 5/10/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-12
Well ID: MW12D
Surface Elev: 622 ft. MSL
Completion: 50 ft. BGS
Station: 875,515.1N
 2,515,900.6E

WEATHER: Foggy to partly sunny, mild (hi-60's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
15A	60/60 100%	cs		14			42	Very dark gray (N3/1), clayey SILT, trace sand and gravel <i>[Continued from previous page]</i>		582	
							44			580	
							46	Very dark gray (N3/1), PEAT		578	
							48	Gray (N5/1) with 30% yellowish brown (10YR5/6) mottles, lean CLAY		576	
16A	60/60 100%	cs		45			50	End of Boring = 50.0 ft. BGS		574	

NOTE(S): MW12D installed in SB-12.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13
Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS
Station: 874,694.3N
 2,513,929.9E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 12.40 - While drilling ▽ = 8.24 - MW12S on 6/1/06 ▽ = 56.03 - MW13D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	22/24	ss	4-5	21						Grayish brown (10YR5/2), clayey SILT, trace sand		622	
1B	92%		4-4 N=9	15						Light gray (10YR7/2), clayey SILT, trace sand			
1C				28		2.13 B			2	Light brownish gray (10YR6/2) with 15% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand		620	
2A	21/24 88%	ss	3-4 5-8 N=9	25		2.13 BSh			4	Gray (10YR6/1) with 40% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand			
3A	24/24 100%	ss	4-5 6-8 N=11	22		2.84 Sh				Dark gray (10YR4/1), lean CLAY, trace sand and gravel		618	
3B				21		2.91 BSh			6				
4A	24/24 100%	ss	9-12 10-10 N=22	23		2.33 B			8			616	
5A	24/24 100%	ss	3-4 6-7 N=10	21		2.72 Sh			10	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand and gravel		614	
6A	19/24 79%	ss	1-3 6-8 N=9	23		1.94 Sh			12			612	
7A	21/24 88%	ss	6-8 10-12 N=18	18		1.94 Sh			14	Yellowish brown (10YR5/6), silty SAND, trace gravel, wet		610	
7B				13		1.55 BSh			16			608	
8A	22/24 92%	ss	7-21 29-30 N=50	11					18	Yellowish brown (10YR5/8) with 30% light brownish gray (10YR6/2) mottles, sandy SILT, trace gravel		606	
8B				9					20			604	
9A	23/24 96%	ss	25-28 28-45 N=56	9		9.16 Sh				Dark gray (10YR4/1), sandy SILT, trace gravel			
10A	24/24 100%	ss	18-27 31-36 N=58	8		12.00 Sh							

NOTE(S): MW13D installed in SB-13.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13
Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS
Station: 874,694.3N
 2,513,929.9E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	60/60 100%	cs		13			22			602	
12A	60/60 100%	cs					24	Dark gray (10YR4/1), sandy SILT, trace gravel [Continued from previous page]		600	
13A	60/60 100%	cs		15			26			598	
14A	60/60 100%	cs					28	Dark gray (10YR4/1), lean CLAY, trace sand and gravel		596	
							30			594	
							32			592	
							34			590	
							36			588	
							38			586	
							40			584	

NOTE(S): MW13D installed in SB-13.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13
Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS
Station: 874,694.3N
 2,513,929.9E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
15A	60/60 100%	cs		15				42			582	
								44			580	
								46	Dark gray (10YR4/1), lean CLAY, trace sand and gravel <i>[Continued from previous page]</i>		578	
								48			576	
16A	60/60 100%	cs		15				50			574	
								52			572	
17A	60/60 100%	cs		14				52	Gray (10YR4/1), silty, fine to medium SAND, wet		570	
17B				20					Gray (10YR4/1), sandy SILT			
17C				14								
								54	Dark greenish gray (5GY4/1) with 25% yellowish brown (10YR5/6) mottles, lean CLAY			
17D				22							568	
End of Boring = 55.0 ft. BGS												

NOTE(S): MW13D installed in SB-13.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13a
Well ID: MW13S
Surface Elev: 623 ft. MSL
Completion: 17 ft. BGS
Station: 874,695.7N
 2,513,925.3E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 12.00 - While drilling ▽ = 6.76 - MW13S on 6/1/06 ▽ = 46.90 - MW12D on 6/1/06			
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
								0	Grayish brown (10YR5/2), clayey SILT, trace sand		622	
								1	Light gray (10YR7/2), clayey SILT, trace sand			
								2	Light brownish gray (10YR6/2) with 15% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand			
								4	Gray (10YR6/1) with 40% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand		620	
								6	Dark gray (10YR4/1), lean CLAY, trace sand and gravel		618	
								8			616	
								10	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand and gravel		614	
								12			612	
								14	Yellowish brown (10YR5/6), silty SAND, trace gravel, wet		610	
								16	Yellowish brown (10YR5/8) with 30% light brownish gray (10YR6/2) mottles, sandy SILT, trace gravel		608	
								16.62	Dark gray (10YR4/1), sandy SILT, trace gravel			

End of Boring = 16.62 ft. BGS
 See SB-13 for sample & testing details

NOTE(S): MW13S installed in blind-drilled borehole within 10 ft of SB-13.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13
Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS
Station: 874,694.3N
 2,513,929.9E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	22/24	ss	4-5	21				Grayish brown (10YR5/2), clayey SILT, trace sand		622	
1B	92%		4-4 N=9	15				Light gray (10YR7/2), clayey SILT, trace sand			
1C				28		2.13 B	2	Light brownish gray (10YR6/2) with 15% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand		620	
2A	21/24 88%	ss	3-4 5-8 N=9	25		2.13 BSh	4	Gray (10YR6/1) with 40% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand			
3A	24/24 100%	ss	4-5 6-8 N=11	22		2.84 Sh		Dark gray (10YR4/1), lean CLAY, trace sand and gravel		618	
3B				21		2.91 BSh	6				
4A	24/24 100%	ss	9-12 10-10 N=22	23		2.33 B	8			616	
5A	24/24 100%	ss	3-4 6-7 N=10	21		2.72 Sh	10	Gray (10YR5/1) with 25% yellowish brown (10YR5/6) mottles, lean CLAY, trace sand and gravel		614	
6A	19/24 79%	ss	1-3 6-8 N=9	23		1.94 Sh	12			612	
7A	21/24 88%	ss	6-8 10-12 N=18	18		1.94 Sh	14	Yellowish brown (10YR5/6), silty SAND, trace gravel, wet		610	
7B				13		1.55 BSh					
8A	22/24 92%	ss	7-21 29-30 N=50	11			16	Yellowish brown (10YR5/8) with 30% light brownish gray (10YR6/2) mottles, sandy SILT, trace gravel		608	
8B				9							
9A	23/24 96%	ss	25-28 28-45 N=56	9		9.16 Sh	18	Dark gray (10YR4/1), sandy SILT, trace gravel		606	
10A	24/24 100%	ss	18-27 31-36 N=58	8		12.00 Sh	20			604	

NOTE(S): MW13D installed in SB-13.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13
Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS
Station: 874,694.3N
 2,513,929.9E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	60/60 100%	cs		13				22	Dark gray (10YR4/1), sandy SILT, trace gravel [Continued from previous page]		602		
								24				600	
								26				598	
								28				596	
12A	60/60 100%	cs						30				594	
								32				592	
								34				590	
13A	60/60 100%	cs		15				36				588	
								38				586	
								40				584	
14A	60/60 100%	cs		15						Dark gray (10YR4/1), lean CLAY, trace sand and gravel			

NOTE(S): MW13D installed in SB-13.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/9/2006
Finish: 5/9/2006
WEATHER: Overcast, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler & 4/4" HSA overdrill
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-13
Well ID: MW13D
Surface Elev: 623 ft. MSL
Completion: 55 ft. BGS
Station: 874,694.3N
 2,513,929.9E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
15A	60/60 100%	cs		15			42			582	
							44			580	
							46	Dark gray (10YR4/1), lean CLAY, trace sand and gravel <i>[Continued from previous page]</i>		578	
							48			576	
16A	60/60 100%	cs		15			50			574	
							52			572	
17A	60/60 100%	cs		14			52	Gray (10YR4/1), silty, fine to medium SAND, wet		570	
17B				20				Gray (10YR4/1), sandy SILT			
17C				14							
							54	Dark greenish gray (5GY4/1) with 25% yellowish brown (10YR5/6) mottles, lean CLAY		568	
17D				22							

End of Boring = 55.0 ft. BGS

NOTE(S): MW13D installed in SB-13.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/2/2006
Finish: 5/2/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: Sb-14a
Well ID: MW14S
Surface Elev: 625 ft. MSL
Completion: 17 ft. BGS
Station: 875,737.8N
 2,514,125.9E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
								Grayish brown (10YR5/2), clayey SILT, trace sand		624	
							2	Gray (10YR6/1) with 30% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand			
								Light gray (10YR7/1) with 30% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand		622	
							4			620	
								Light gray (10YR7/1) with 15% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand		618	
							6			616	
							8				
							10			614	
							12	Yellowish brown (10YR5/8) with 50% light gray (10YR7/1) mottles, sandy CLAY		612	
							14	Yellowish brown (10YR5/6), silty, fine SAND, trace medium sand and gravel, wet		610	
								Yellowish brown (10YR5/6), sandy SILT, trace gravel			
							16	Yellowish brown (10YR5/6) with 40% gray (10YR6/1) mottles, sandy SILT, trace gravel		608	

End of Boring = 17.38 ft. BGS
 See SB-14 for sample & testing details

NOTE(S): MW14S installed in blind-drilled borehole within 10 ft of SB-14.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/1/2006
Finish: 5/2/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-14
Well ID: n/a
Surface Elev: 625 ft. MSL
Completion: 60 ft. BGS
Station: 875,740.0N
 2,514,130.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	23/24 96%	ss	2-3 2-3 N=5	16				Grayish brown (10YR5/2), clayey SILT, trace sand		624	
1B				26		2.33 B	2	Gray (10YR6/1) with 30% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand			
2A	24/24 100%	ss	3-4 5-7 N=9	23		3.10 B	4	Light gray (10YR7/1) with 30% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand		622	
3A	23/24 96%	ss	3-3 5-5 N=8	19		2.33 B	6			620	
4A	24/24 100%	ss	5-6 5-7 N=11	23		2.68 BSh	8	Light gray (10YR7/1) with 15% yellowish brown (10YR6/8) mottles, lean CLAY, trace sand		618	
5A	24/24 100%	ss	2-2 3-4 N=5	26		1.83 B	10			616	
6A	19/24 79%	ss	2-2 3-5 N=5	17		2.18 B	12	Yellowish brown (10YR5/8) with 50% light gray (10YR7/1) mottles, sandy CLAY		614	
7A	20/24 83%	ss	2-3 3-3 N=6	22		1.16 B	14			612	
8A	24/24 100%	ss	5-14 14-20 N=28	16		1.36 B	14	Yellowish brown (10YR5/6), silty, fine SAND, trace medium sand and gravel, wet		610	
8B				11		5.77 BSh	16	Yellowish brown (10YR5/6), sandy SILT, trace gravel			
9A	12/24 50%	ss	57-65	10			16	Yellowish brown (10YR5/6) with 40% gray (10YR6/1) mottles, sandy SILT, trace gravel		608	
10A	24/24 100%	ss	6-8 16-18 N=24	12		5.04 BSh	18	Dark gray (10YR4/1), clayey SILT, trace sand and gravel		606	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/1/2006
Finish: 5/2/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-14
Well ID: n/a
Surface Elev: 625 ft. MSL
Completion: 60 ft. BGS
Station: 875,740.0N
 2,514,130.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Quadrangle: Coffeen, IL	Section 10, Tier 7N; Range 3W	▽ = 14.00 - While drilling	▽ = 4.49 - MW14S on 6/1/06	▽ =	Depth ft. BGS
11A	24/24 100%	ss	2-7 13-30 N=20	13	9.70 B						604	
12A	24/24 100%	ss	26-40 36-40 N=76	9	13.09 BSP						602	
13A	24/24 100%	ss	8-18 28-34 N=46	9	8.73 BSP						600	
14A	22/24 92%	ss	20-18 24-30 N=42	9	7.42 BSP		Dark gray (10YR4/1), clayey SILT, trace sand and gravel [Continued from previous page]				598	
15A	19/24 79%	ss	8-27 33-67 N=60	9							596	
16A	24/24 100%	ss	8-25 27-33 N=52	10	9.60 BSh						594	
17A	20/24 83%	ss	11-15 20-24 N=35	14	6.80 B						592	
18A	24/24 100%	ss	3-4 7-9 N=11	16	3.88 B						590	
19A	24/24 100%	ss	8-12 13-15 N=25	16	6.18 B		Dark gray (N4/1), lean CLAY, trace sand and gravel				588	
20A	24/24 100%	ss	3-7 10-13 N=17	14	3.10 B						586	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/1/2006
Finish: 5/2/2006
WEATHER: Sunny, mild (mid-60's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-14
Well ID: n/a
Surface Elev: 625 ft. MSL
Completion: 60 ft. BGS
Station: 875,740.0N
 2,514,130.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▽ = 14.00 - While drilling ▽ = 4.49 - MW14S on 6/1/06 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	23/24 96%	ss	3-6 8-13 N=14	15	4.80 B				42			584	
22A	24/24 100%	ss	13-15 16-18 N=31	14	5.62 B				44	Dark gray (N4/1), lean CLAY, trace sand and gravel [Continued from previous page]		582	
23A	24/24 100%	ss	4-8 11-13 N=19	15	4.65 B				46			580	
24A	24/24 100%	ss	18-18 20-20 N=38	15	4.65 B				48			578	
25A	24/24 100%	ss	4-7 9-11 N=16	19	2.13 BSh				50	Dark gray (N4/1), clayey SILT, trace sand and gravel		576	
26A	22/24 92%	ss	3-5 6-8 N=11	22	3.30 BSh				50	Gray (N4/1), wet, loose, fine- to medium-grained SAND		574	
27A	24/24 100%	ss	3-5 5-7 N=10	25	2.89 BSh				52	Dark gray (N4/1), clayey SILT, trace sand and gravel		572	
28A	21/24 88% 0/24 0%	ss SH	4-6 7-8 N=13	22	3.71 BSh				54	Greenish gray (5BG5/1), lean CLAY		570	
29A	14/24 58%	ss	0-0 0-0 N=0	22	3.09 BSh				56	Greenish gray (5BG5/1) with 15% yellowish brown (10YR5/6) mottles, lean CLAY		568	
30A	22/24 92%	ss	5-6 8-12 N=14	19	4.46 BSh				58	Greenish gray (5BG5/1) with 25% yellowish brown (10YR5/6) mottles, lean CLAY		566	
									60	Yellowish brown (10YR4/6) with 10% greenish gray (5BG5/1) mottles, lean CLAY			

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15b
Well ID: MW15D
Surface Elev: 624 ft. MSL
Completion: 39 ft. BGS
Station: 875,970.5N
 2,515,080.7E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
										Dark brown (10YR3/3), clayey SILT			
										Dark grayish brown (10YR3/2), clayey SILT, trace sand			
									2	Grayish brown (10YR5/2), with 50% very dark gray (10YR3/1) mottles, lean CLAY, trace sand		622	
									4	Gray (10YR6/1), lean CLAY, trace sand		620	
									5	Yellowish brown (10YR5/6) with 40% gray (10YR6/1) mottles, lean CLAY, trace sand		618	
									8	Gray (10YR6/1), lean CLAY, trace sand		616	
									10	Gray (10YR6/1) with 50% yellowish brown (10YR5/6) mottles, lean CLAY, little sand, trace gravel		614	
									12	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, silty, fine to medium SAND, trace coarse sand and gravel, wet		612	
									14	Pale brown (10YR6/3), clayey SILT, little sand, trace gravel		610	
									16	Yellowish brown (10YR5/4), silty, fine to coarse SAND, wet		608	
									17	Yellowish brown (10YR5/4) silty fine SAND, wet			
									18	Gray (10YR6/1), sandy SILT, trace medium to coarse sand and trace gravel		606	
									20	Dark gray (10YR4/1), clayey SILT, little sand, trace gravel		604	

NOTE(S): MW15D installed in blind-drilled borehole within 10 ft of SB-15.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/24/2006

Finish: 4/25/2006

WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation

Rig mfg/model: CME-650 Track Rig

Drilling Method: 4¼" HSA (blind drill)

FIELD STAFF: Driller: B. Williamson

Helper: R. Keedy

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15b

Well ID: MW15D

Surface Elev: 624 ft. MSL

Completion: 39 ft. BGS

Station: 875,970.5N

2,515,080.7E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
							Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▽ = 13.40 - While drilling ▽ = 4.99 - MW15S on 6/1/06 ▽ = 5.24 - MW15D on 6/1/06			
							22 24 26 28 30 32 34 36 38	Dark gray (10YR4/1), clayey SILT, little sand, trace gravel <i>[Continued from previous page]</i>	602 600 598 596 594 592 590 588 586		
								Dark gray (10YR4/1), silty, fine to medium SAND, trace coarse sand and gravel, wet			
								Very dark gray (10YR3/1), clayey SILT, little sand, trace gravel			

End of Boring = 38.80 ft. BGS

See SB-15 for sample & testing details

NOTE(S): MW15D installed in blind-drilled borehole within 10 ft of SB-15.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 13.40 - While drilling ▽ = 4.99 - MW15S on 6/1/06 ▽ = 5.24 - MW15D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	2-2 3-4 N=5	19					Dark brown (10YR3/3), clayey SILT				
1B				27	1.94 B				Dark grayish brown (10YR3/2), clayey SILT, trace sand			622	
2A	24/24 100%	ss	2-2 4-6 N=6	25	3.10 B				Grayish brown (10YR5/2), with 50% very dark gray (10YR3/1) mottles, lean CLAY, trace sand			620	
3A	20/24 83%	ss	2-3 3-5 N=6	29	2.10 B			▼	Gray (10YR6/1), lean CLAY, trace sand			618	
4A	24/24 100%	ss	4-6 5-5 N=11	24	1.75 B				Yellowish brown (10YR5/6) with 40% gray (10YR6/1) mottles, lean CLAY, trace sand			616	
5A	22/24 92%	ss	1-2 3-4 N=5	26	1.55 B				Gray (10YR6/1), lean CLAY, trace sand			614	
6A	22/24 92%	ss	2-3 3-4 N=6	22	1.85 B				Gray (10YR6/1) with 50% yellowish brown (10YR5/6) mottles, lean CLAY, little sand, trace gravel			612	
7A	19/24 79%	SH							Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, silty, fine to medium SAND, trace coarse sand and gravel, wet			610	Shelby tube taken from shallow well borehole at indicated depth.
7B	24/24 100%	ss	4-4 5-5 N=9	23	1.22 B			▼	Pale brown (10YR6/3), clayey SILT, little sand, trace gravel			608	
8A	21/24 88%	ss	2-6 15-19 N=21	11	3.22 BSP				Yellowish brown (10YR5/4), silty, fine to coarse SAND, wet			606	
9A				20					Yellowish brown (10YR5/4) silty fine SAND, wet				
9B	24/24 100%	ss	18-29 40-50 N=69	21					Gray (10YR6/1), sandy SILT, trace medium to coarse sand and trace gravel			606	
9C				9					Dark gray (10YR4/1), clayey SILT, little sand, trace gravel			604	
10A	17/24 71%	ss	11-43 59/5"	7	7.42 B								

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	14/24 58%	ss	14-55 45/2"		8				22			602	
12A	8/24 33%	ss	100/8"		8		6.76 SP		24			600	
13A	23/24 96%	ss	12-28 43-57/5" N=71		5				26			598	
14A	8/24 33%	ss	59-41/2"		6		7.95 BSh		28	Dark gray (10YR4/1), clayey SILT, little sand, trace gravel [Continued from previous page]		596	
15A	16/24 67%	ss	11-26 74/4"		12		4.74 BSh		30			594	
16A	12/24 50%	ss	39-61		7				32			592	
17A	10/24 42%	ss	49-51/4"		9		5.43 B		34			590	
18A	11/24 46%	ss	100-95		11				36			588	
19A	8/24 33%	ss	61-39/2"		10				38	Dark gray (10YR4/1), silty, fine to medium SAND, trace coarse sand and gravel, wet		586	
20A	24/24 100%	ss	21-41 21-24 N=62		12		16.00 None			Very dark gray (10YR3/1), clayey SILT, little sand, trace gravel			
20B					13		9.38		40	Very dark gray (10YR3/1) with 20% dark grayish brown (10YR4/2) mottles, clayey SILT, trace sand and gravel		584	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	▼ = 13.40 - While drilling	▽ = 4.99 - MW15S on 6/1/06	▽ = 5.24 - MW15D on 6/1/06
							Quadrangle:	Coffeen, IL			
							Township:	East Fork			
							Section	11, Tier 7N; Range 3W			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	22/24 92%	ss	3-7 11-18 N=18	19	6.11	BSh	42	Dark gray (10YR4/1), clayey SILT, trace sand and gravel		582	
22A	24/24 100%	ss	4-7 8-10 N=15	23	4.46	B	44	Dark greenish gray (5GY4/1) with 30% dark gray (N4/1) mottles, lean CLAY, trace sand		580	
23A	24/24 100%	ss	3-6 6-10 N=12	21			46			578	
23B	24/24 100%	ss	10-12 10-15 N=22	16	3.69	B	48	Dark gray (N4/1), lean CLAY, trace sand and gravel		576	
24A	24/24 100%	ss	2-4 7-9 N=11	16	4.58	B	50			574	
25A	24/24 100%	ss	3-5 7-13 N=12	21	4.65	B	52			572	
26A	19/24 79%	ss	8-10 8-13 N=18	25	3.88	B	54	Dark yellowish brown (10YR3/4) with 50% dark grayish brown (10YR4/2) mottles, lean CLAY, trace sand and gravel		570	
27A	24/24 100%	ss	4-5 8-12 N=13	22	3.49	BSh	56	Greenish gray (10YR5/1) with 20% dark yellowish brown (10YR4/4) mottles, lean CLAY, trace sand and gravel		568	
28A	24/24 100%	ss	5-9 15-18 N=24	20	3.49	BSh	58			566	
29A	24/24 100%	ss	8-9 14-18 N=23	18	5.82	BSh	60	Olive (5Y4/3) with 15% greenish gray (10GY5/1) mottles, lean CLAY, trace sand and gravel		564	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
31A	24/24 100%	ss	4-7 13-15 N=20	18	5.42 BSh		62	Olive (5Y4/3) with 15% greenish gray (10GY5/1) mottles, lean CLAY, trace sand and gravel [Continued from previous page]		562	
32A	24/24 100%	ss	10-15 11-16 N=26	20	4.74 BSh	64			560		
33A	24/24 100%	ss	6-10 11-13 N=21	16	6.98 BSh	66	Greenish gray (10Y5), lean CLAY, trace sand and gravel			558	
34A	24/24 100%	ss	11-14 18-31 N=32	18	6.98 BSh	68				556	
35A	23/24 96%	ss	9-18 27-40 N=45	15	11.95 BSh	70				554	
36A	24/24 100%	ss	4-12 18-24 N=30	16	7.15 BSh	72				552	
37A	24/24 100%	ss	17-29 36-47 N=65	17	8.24 BSh	74	Dark yellowish brown (10YR4/4), lean CLAY, trace sand and gravel			550	
38A	20/24 83%	ss	12-18 23-28 N=41	17	6.59 BSh	76				548	
39A	9/24 38%	ss	29-39 48-66 N=87	16		78				546	
40A	24/24 100%	ss	5-9 13-18 N=22	18	6.21 B	80				544	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W		▽ = 13.40 - While drilling ▽ = 4.99 - MW15S on 6/1/06 ▽ = 5.24 - MW15D on 6/1/06		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
41A	24/24 100%	ss	6-8 13-16 N=21	17		5.82 B	82	Dark yellowish brown (10YR4/4), lean CLAY, trace sand and gravel [Continued from previous page]		542	
42A	24/24 100%	ss	18-28 25-25 N=53	18		5.82 BSh	84			540	
End of Boring = 84.0 ft. BGS											

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/25/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15a
Well ID: MW15S
Surface Elev: 624 ft. MSL
Completion: 20 ft. BGS
Station: 875,971.1N
 2,515,076.3E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
										Dark brown (10YR3/3), clayey SILT			
										Dark grayish brown (10YR3/2), clayey SILT, trace sand			
									2	Grayish brown (10YR5/2), with 50% very dark gray (10YR3/1) mottles, lean CLAY, trace sand		622	
									4	Gray (10YR6/1), lean CLAY, trace sand		620	
									6	Yellowish brown (10YR5/6) with 40% gray (10YR6/1) mottles, lean CLAY, trace sand		618	
									8	Gray (10YR6/1), lean CLAY, trace sand		616	
									10	Gray (10YR6/1) with 50% yellowish brown (10YR5/6) mottles, lean CLAY, little sand, trace gravel		614	
									12	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, silty, fine to medium SAND, trace coarse sand and gravel, wet		612	
									14	Pale brown (10YR6/3), clayey SILT, little sand, trace gravel		610	
									16	Yellowish brown (10YR5/4), silty, fine to coarse SAND, wet		608	
									17	Yellowish brown (10YR5/4) silty fine SAND, wet			
									18	Gray (10YR6/1), sandy SILT, trace medium to coarse sand and trace gravel		606	
									19.62	Dark gray (10YR4/1), clayey SILT, little sand, trace gravel			
See SB-15 for sample & testing details End of Boring = 19.62 ft. BGS													

NOTE(S): MW15S installed in blind-drilled borehole within 10 ft of SB-15.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = 13.40 - While drilling ▽ = 4.99 - MW15S on 6/1/06 ▽ = 5.24 - MW15D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	ss	2-2 3-4 N=5	19					Dark brown (10YR3/3), clayey SILT				
1B				27	1.94 B				Dark grayish brown (10YR3/2), clayey SILT, trace sand			622	
2A	24/24 100%	ss	2-2 4-6 N=6	25	3.10 B				Grayish brown (10YR5/2), with 50% very dark gray (10YR3/1) mottles, lean CLAY, trace sand			620	
3A	20/24 83%	ss	2-3 3-5 N=6	29	2.10 B				Gray (10YR6/1), lean CLAY, trace sand			618	
4A	24/24 100%	ss	4-6 5-5 N=11	24	1.75 B				Yellowish brown (10YR5/6) with 40% gray (10YR6/1) mottles, lean CLAY, trace sand			616	
5A	22/24 92%	ss	1-2 3-4 N=5	26	1.55 B				Gray (10YR6/1), lean CLAY, trace sand			614	
6A	22/24 92%	ss	2-3 3-4 N=6	22	1.85 B				Gray (10YR6/1) with 50% yellowish brown (10YR5/6) mottles, lean CLAY, little sand, trace gravel			612	
7A	19/24 79%	SH							Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, silty, fine to medium SAND, trace coarse sand and gravel, wet			610	Shelby tube taken from shallow well borehole at indicated depth.
7B	24/24 100%	ss	4-4 5-5 N=9	23	1.22 B				Pale brown (10YR6/3), clayey SILT, little sand, trace gravel			608	
8A	21/24 88%	ss	2-6 15-19 N=21	11	3.22 BSP				Yellowish brown (10YR5/4), silty, fine to coarse SAND, wet			606	
9A				20					Yellowish brown (10YR5/4) silty fine SAND, wet				
9B	24/24 100%	ss	18-29 40-50 N=69	21					Gray (10YR6/1), sandy SILT, trace medium to coarse sand and trace gravel			606	
9C				9					Dark gray (10YR4/1), clayey SILT, little sand, trace gravel			604	
10A	17/24 71%	ss	11-43 59/5"	7	7.42 B								

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	14/24 58%	ss	14-55 45/2"		8				22			602	
12A	8/24 33%	ss	100/8"		8		6.76 SP		24			600	
13A	23/24 96%	ss	12-28 43-57/5" N=71		5				26			598	
14A	8/24 33%	ss	59-41/2"		6		7.95 BSh		28	Dark gray (10YR4/1), clayey SILT, little sand, trace gravel [Continued from previous page]		596	
15A	16/24 67%	ss	11-26 74/4"		12		4.74 BSh		30			594	
16A	12/24 50%	ss	39-61		7				32			592	
17A	10/24 42%	ss	49-51/4"		9		5.43 B		34			590	
18A	11/24 46%	ss	100-95		11				36			588	
19A	8/24 33%	ss	61-39/2"		10				38	Dark gray (10YR4/1), silty, fine to medium SAND, trace coarse sand and gravel, wet		586	
20A	24/24 100%	ss	21-41 21-24 N=62		12		16.00 None		38	Very dark gray (10YR3/1), clayey SILT, little sand, trace gravel			
20B					13		9.38		40	Very dark gray (10YR3/1) with 20% dark grayish brown (10YR4/2) mottles, clayey SILT, trace sand and gravel		584	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	▼ = 13.40 - While drilling	▽ = 4.99 - MW15S on 6/1/06	▽ = 5.24 - MW15D on 6/1/06
							Quadrangle:				
							Township:				
							Section 11, Tier 7N; Range 3W				
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	22/24 92%	ss	3-7 11-18 N=18	19	6.11	BSh	42	Dark gray (10YR4/1), clayey SILT, trace sand and gravel		582	
22A	24/24 100%	ss	4-7 8-10 N=15	23	4.46	B	44	Dark greenish gray (5GY4/1) with 30% dark gray (N4/1) mottles, lean CLAY, trace sand		580	
23A	24/24 100%	ss	3-6 6-10 N=12	21			46			578	
23B	24/24 100%	ss	10-12 10-15 N=22	16	3.69	B	48	Dark gray (N4/1), lean CLAY, trace sand and gravel		576	
24A	24/24 100%	ss	2-4 7-9 N=11	16	4.58	B	50			574	
25A	24/24 100%	ss	3-5 7-13 N=12	21	4.65	B	52			572	
26A	19/24 79%	ss	8-10 8-13 N=18	25	3.88	B	54	Dark yellowish brown (10YR3/4) with 50% dark grayish brown (10YR4/2) mottles, lean CLAY, trace sand and gravel		570	
27A	24/24 100%	ss	4-5 8-12 N=13	22	3.49	BSh	56	Greenish gray (10YR5/1) with 20% dark yellowish brown (10YR4/4) mottles, lean CLAY, trace sand and gravel		568	
28A	24/24 100%	ss	5-9 15-18 N=24	20	3.49	BSh	58			566	
29A	24/24 100%	ss	8-9 14-18 N=23	18	5.82	BSh	60	Olive (5Y4/3) with 15% greenish gray (10GY5/1) mottles, lean CLAY, trace sand and gravel		564	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (lo-50's)

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
31A	24/24 100%	ss	4-7 13-15 N=20	18	5.42 BSh		62	Olive (5Y4/3) with 15% greenish gray (10GY5/1) mottles, lean CLAY, trace sand and gravel [Continued from previous page]		562	
32A	24/24 100%	ss	10-15 11-16 N=26	20	4.74 BSh	64	560				
33A	24/24 100%	ss	6-10 11-13 N=21	16	6.98 BSh		66	Greenish gray (10Y5), lean CLAY, trace sand and gravel		558	
34A	24/24 100%	ss	11-14 18-31 N=32	18	6.98 BSh	68	556				
35A	23/24 96%	ss	9-18 27-40 N=45	15	11.95 BSh	70	554				
36A	24/24 100%	ss	4-12 18-24 N=30	16	7.15 BSh	72	552				
37A	24/24 100%	ss	17-29 36-47 N=65	17	8.24 BSh	74	550				
38A	20/24 83%	ss	12-18 23-28 N=41	17	6.59 BSh	76	548				
39A	9/24 38%	ss	29-39 48-66 N=87	16		78	546				
40A	24/24 100%	ss	5-9 13-18 N=22	18	6.21 B	80	544				

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/24/2006
Finish: 4/25/2006

CONTRACTOR: Testing Service Corporation
Rig mfg/model: CME-650 Track Rig
Drilling Method: 3/4" HSA w/SS sampler
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-15
Well ID: n/a
Surface Elev: 624 ft. MSL
Completion: 84 ft. BGS
Station: 875,970.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
41A	24/24 100%	ss	6-8 13-16 N=21	17		5.82 B	82	Dark yellowish brown (10YR4/4), lean CLAY, trace sand and gravel [Continued from previous page]		542	
42A	24/24 100%	ss	18-28 25-25 N=53	18		5.82 BSh	84				
End of Boring = 84.0 ft. BGS											

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4¼" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16b
Well ID: MW16D
Surface Elev: 626 ft. MSL
Completion: 51 ft. BGS
Station: 877,354.9N
 2,515,079.4E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
								0	Black (10YR2/1), sl. moist, firm, clayey SILT with trace sand and trace gravel.		626	
								2	Brown (10YR4/3), sl. moist, firm, silty CLAY with trace sand.		624	
								4	Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, very soft, very silty CLAY with trace sand.		622	
								6	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, very silty CLAY with trace sand.		620	
								8			618	
								10	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with some sand and trace gravel.		616	
								12			614	
								13	Dark yellowish brown (10YR4/6), wet, sl. dense, silty, very fine- to fine-grained SAND.			
								14	Dark yellowish brown (10YR4/6), moist, firm, silty CLAY with sand and trace gravel.		612	
								15	Dark yellowish brown (10YR4/6), wet, loose, silty, very fine- to fine-grained SAND.			
								16	Dark yellowish brown (10YR4/6), wet, soft, silty CLAY with sand and trace gravel.		610	
								17	Yellowish brown (10YR5/6), wet, loose, very fine- to very coarse-grained SAND.			
								18	Gray (10YR5/1), wet, loose, fine- to medium-grained SAND.			
								19	Gray (10YR5/1), moist, hard, clayey SILT with sand and trace gravel.		608	
								20	Gray (10YR5/1), wet, loose, very fine- to fine-grained SAND.			
								21	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.			

NOTE(S): MW16D installed in blind-drilled borehole within 10 ft of SB-16.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4¼" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16b
Well ID: MW16D
Surface Elev: 626 ft. MSL
Completion: 51 ft. BGS
Station: 877,354.9N
 2,515,079.4E

WEATHER: Overcast, cool (mid-40's)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
							22				606	
							24				604	
							26				602	
							28				600	
							30	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>			598	
							32				600	
							34				596	
							36				594	
							38				592	
							40				590	
											588	

NOTE(S): MW16D installed in blind-drilled borehole within 10 ft of SB-16.

FIELD BORING LOG

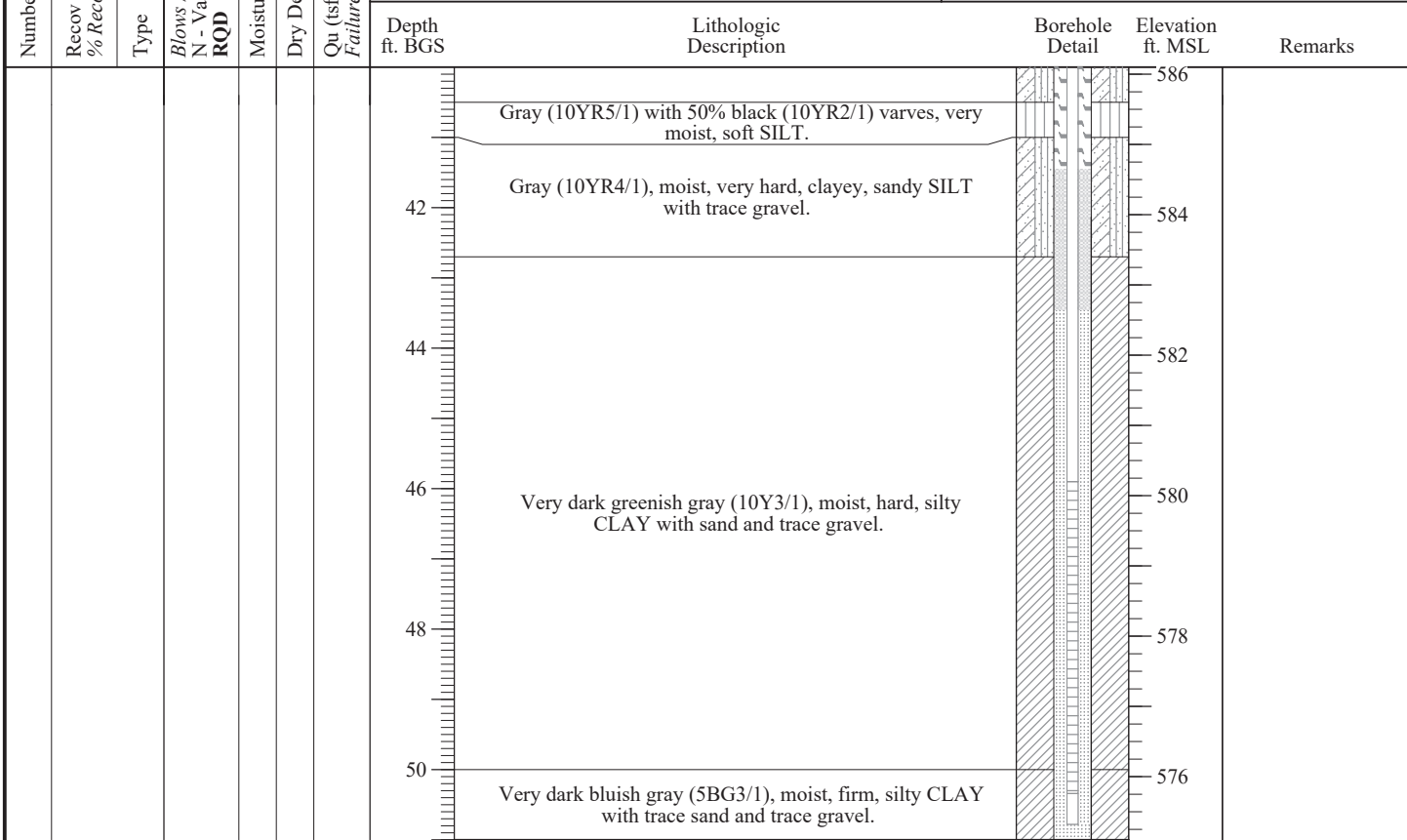


CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4¼" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16b
Well ID: MW16D
Surface Elev: 626 ft. MSL
Completion: 51 ft. BGS
Station: 877,354.9N
 2,515,079.4E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 12.80 - While drilling ▽ = 5.74 - MW16S on 6/1/06 ▽ = 51.37 - MW16D on 6/1/06



End of Boring = 51.00 ft. BGS
 See SB-16 for sample & testing details

NOTE(S): MW16D installed in blind-drilled borehole within 10 ft of SB-16.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 12.80 - While drilling ▽ = 5.74 - MW16S on 6/1/06 ▽ = 51.37 - MW16D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	SS	4-4 6-7 N=10	22					0	Black (10YR2/1), sl. moist, firm, clayey SILT with trace sand and trace gravel.		626	
1B				29	2.13	B			2	Brown (10YR4/3), sl. moist, firm, silty CLAY with trace sand.		624	
2A	24/24 100%	SS	4-6 7-9 N=13	25	2.13	B			4	Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, very soft, very silty CLAY with trace sand.		622	
3A	20/24 83%	SS	3-4 5-7 N=9	21	2.33	B		▽	6	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, very silty CLAY with trace sand.		620	
4A	24/24 100%	SS	2-3 4-6 N=7	25	2.13	B			8			618	
5A	24/24 100%	SS	3-4 5-5 N=9	24	2.33	B			10	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with some sand and trace gravel.		616	
6A	24/24 100% 24/24 100%	SS SH	2-4 4-5 N=8	24	1.75	B			12			614	Shelby tube taken from shallow well borehole at indicated depth.
7A	24/24 100%	SS	4-7 7-7 N=14	22	1.94	BSh		▽	14	Dark yellowish brown (10YR4/6), wet, sl. dense, silty, very fine- to fine-grained SAND.		612	
7B				18					14	Dark yellowish brown (10YR4/6), moist, firm, silty CLAY with sand and trace gravel.		612	
8A	21/24 88%	SS	1-2 2-4 N=4	20					16	Dark yellowish brown (10YR4/6), wet, loose, silty, very fine- to fine-grained SAND.		610	
9A	18/24 75%	SS	4-3 4-10 N=7	14					16	Dark yellowish brown (10YR4/6), wet, soft, silty CLAY with sand and trace gravel.		610	
9B				15					18	Yellowish brown (10YR5/6), wet, loose, very fine- to very coarse-grained SAND.		610	
10A				10					18	Gray (10YR5/1), wet, loose, fine- to medium-grained SAND.		608	
10B	20/24 83%	SS	27-54 59-59 N=113	17					18	Gray (10YR5/1), moist, hard, clayey SILT with sand and trace gravel.		608	
									20	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.		608	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 12.80 - While drilling ▽ = 5.74 - MW16S on 6/1/06 ▽ = 51.37 - MW16D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	10/24 42%	ss	10-96	8						22			606	
12A	14/24 58%	ss	84-132	10		3.10	BSh			24			604	
13A	20/24 83%	ss	41-68 82 N=150	10		7.56	B			26			602	
14A	12/24 50%	ss	58-119	10		9.89	B			28			600	
15A	24/24 100%	ss	30-48 70-71 N=118	9		5.62	B			30	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]		598	Dusky red (7.5YR3/4) staining.
16A	24/24 100%	ss	50-54 68-93 N=122	9						32			596	
17A	35/36 97%	cs		17						34			594	
18A	60/60 100%	cs		10						36			592	
										38			590	
										40			588	Wood fragments.

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 4/21/2006

Finish: 4/25/2006

WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-850 Track Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16

Well ID: n/a

Surface Elev: 626 ft. MSL

Completion: 92 ft. BGS

Station: 877,355.0N

2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
19A	60/60 100%	cs		19			42	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.		584	
19B				11			46	Very dark greenish gray (10Y3/1), moist, hard, silty CLAY with sand and trace gravel.		580	
20A	60/60 100%	cs		13			48			578	
21A	24/24 100%	ss	6-14 18-22 N=32	19		1.94 B	50			576	
	0/48 0%	cs					52	Very dark bluish gray (5BG3/1), moist, firm, silty CLAY with trace sand and trace gravel.		574	Possible rock at end of auger.
22A	24/24 100%	ss	2-7 7-15 N=14	21		3.71 BSh	56			570	
	24/24 100%	ss	4-8 11-13 N=19				58	Greenish gray (5G6/1) with 40% yellowish brown (10YR5/6) mottles, moist, hard, silty CLAY with sl. trace sand.		568	
23A	0/12 0%	BD		26		2.13 BSh	60				

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 12.80 - While drilling	▼ = 5.74 - MW16S on 6/1/06	▼ = 51.37 - MW16D on 6/1/06	
			RQD					Section 2, Tier 7N; Range 3W				
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/48 0%	RC						62	Greenish gray (5G6/1) with 40% yellowish brown (10YR5/6) mottles, moist, hard, silty CLAY with sl. trace sand. [Continued from previous page]		566	
26A	24/24 100%	SS	32-34 42-51 N=76	25		2.72	BSh	64			562	
	0/24 0%	RC						66	Yellowish brown (10YR5/6) with 20% greenish gray (5G6/1) mottles, moist, hard, silty CLAY with trace sand and trace coal fragments.		560	
28A	24/24 100%	SS	15-21 21-21 N=42	18		2.72	BSh	68			558	
29A	24/24 100%	SS	14-17 21-25 N=38	20		2.91	BSh	70			556	70' to 79.5' - possible oxidation rinds.
30A	24/24 100%	SS	12-21 34-35 N=55	18		5.04	BSh	72			554	
31A	24/24 100%	SS	16-21 27-35 N=48	16		8.15	BSh	74			552	
								76	Yellowish brown (10YR5/6) with zones of gray (10YR4/1) mottles, moist, hard, clayey SILT with some sand and trace gravel.		550	
32A	60/60 100%	CS		19				78			548	
								80				

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 12.80 - While drilling	▽ = 5.74 - MW16S on 6/1/06	▽ = 51.37 - MW16D on 6/1/06	
			RQD					Section 2, Tier 7N; Range 3W				
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
33A	60/60 100%	cs		16				82	Yellowish brown (10YR5/6) with zones of gray (10YR4/1) mottles, moist, hard, clayey SILT with some sand and trace gravel. <i>[Continued from previous page]</i>		546	
								84	Yellow brown (10YR5/6), very moist, very soft, clayey, very fine-grained SAND and SILT.		544	
	0/60 0%	cs						86			542	
								88	Yellowish brown (10YR5/6), moist, hard, silty CLAY with sand and trace gravel.		540	
								90			538	
35A	24/24 100%	ss	9 11-16 N=20			2.72 BSh		92	Dark gray (10YR3/1), moist, firm, silty CLAY with sand and trace gravel.		536	Possible rock at end of auger.
End of Boring = 92.0 ft. BGS												

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/25/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16a
Well ID: MW16S
Surface Elev: 626 ft. MSL
Completion: 20 ft. BGS
Station: 877,355.1N
 2,515,088.0E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▽ = 12.80 - While drilling
								Township: East Fork	▽ = 5.74 - MW16S on 6/1/06
								Section 2, Tier 7N; Range 3W	▽ = 51.37 - MW16D on 6/1/06

Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
0	Black (10YR2/1), sl. moist, firm, clayey SILT with trace sand and trace gravel.			
2	Brown (10YR4/3), sl. moist, firm, silty CLAY with trace sand.		624	
4	Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, very soft, very silty CLAY with trace sand.		622	
6	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, very silty CLAY with trace sand.		620	
8			618	
10	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with some sand and trace gravel.		616	
12			614	
14	Dark yellowish brown (10YR4/6), wet, sl. dense, silty, very fine- to fine-grained SAND.		612	
14	Dark yellowish brown (10YR4/6), moist, firm, silty CLAY with sand and trace gravel.		612	
14	Dark yellowish brown (10YR4/6), wet, loose, silty, very fine- to fine-grained SAND.		612	
16	Dark yellowish brown (10YR4/6), wet, soft, silty CLAY with sand and trace gravel.		610	
16	Yellowish brown (10YR5/6), wet, loose, very fine- to very coarse-grained SAND.		610	
18	Gray (10YR5/1), wet, loose, fine- to medium-grained SAND.		608	
18	Gray (10YR5/1), moist, hard, clayey SILT with sand and trace gravel.		608	
18	Gray (10YR5/1), wet, loose, very fine- to fine-grained SAND.		608	
19.90	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.		608	

End of Boring = 19.90 ft. BGS
 See SB-16 for sample & testing details

NOTE(S): MW16S installed in blind-drilled borehole within 10 ft of SB-16.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 12.80 - While drilling ▽ = 5.74 - MW16S on 6/1/06 ▽ = 51.37 - MW16D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	SS	4-4 6-7 N=10	22					0	Black (10YR2/1), sl. moist, firm, clayey SILT with trace sand and trace gravel.		626	
1B				29	2.13	B			2	Brown (10YR4/3), sl. moist, firm, silty CLAY with trace sand.		624	
2A	24/24 100%	SS	4-6 7-9 N=13	25	2.13	B			4	Gray (10YR5/1) with 25% yellowish brown (10YR5/8) mottles, moist, very soft, very silty CLAY with trace sand.		622	
3A	20/24 83%	SS	3-4 5-7 N=9	21	2.33	B			6	Gray (10YR5/1) with 15% dark yellowish brown (10YR4/6) mottles, moist, very soft, very silty CLAY with trace sand.		620	
4A	24/24 100%	SS	2-3 4-6 N=7	25	2.13	B			8			618	
5A	24/24 100%	SS	3-4 5-5 N=9	24	2.33	B			10	Gray (10YR5/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with some sand and trace gravel.		616	
6A	24/24 100% 24/24 100%	SS SH	2-4 4-5 N=8	24	1.75	B			12			614	Shelby tube taken from shallow well borehole at indicated depth.
7A	24/24 100%	SS	4-7 7-7 N=14	22	1.94	BSh			14	Dark yellowish brown (10YR4/6), wet, sl. dense, silty, very fine- to fine-grained SAND.		612	
7B				18					14	Dark yellowish brown (10YR4/6), moist, firm, silty CLAY with sand and trace gravel.		612	
8A	21/24 88%	SS	1-2 2-4 N=4	20					16	Dark yellowish brown (10YR4/6), wet, loose, silty, very fine- to fine-grained SAND.		610	
9A	18/24 75%	SS	4-3 4-10 N=7	14					16	Dark yellowish brown (10YR4/6), wet, soft, silty CLAY with sand and trace gravel.		610	
9B				15					18	Yellowish brown (10YR5/6), wet, loose, very fine- to very coarse-grained SAND.		610	
10A				10					18	Gray (10YR5/1), wet, loose, fine- to medium-grained SAND.		608	
10B	20/24 83%	SS	27-54 59-59 N=113	17					18	Gray (10YR5/1), moist, hard, clayey SILT with sand and trace gravel.		608	
									20	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.		608	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	10/24 42%	ss	10-96	8			22			606	
12A	14/24 58%	ss	84-132	10		3.10 BSh	24			604	
13A	20/24 83%	ss	41-68 82 N=150	10		7.56 B	26			602	
14A	12/24 50%	ss	58-119	10		9.89 B	28			600	
15A	24/24 100%	ss	30-48 70-71 N=118	9		5.62 B	30	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]		598	Dusky red (7.5YR3/4) staining.
16A	24/24 100%	ss	50-54 68-93 N=122	9			32			596	
17A	35/36 97%	cs		17			34			594	
18A	60/60 100%	cs		10			36			592	
							38			590	
							40			588	Wood fragments.

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
19A	60/60 100%	cs		19			38	Gray (10YR5/1) with 50% black (10YR2/1) varves, very moist, soft SILT.		586	
19B				11			42	Gray (10YR4/1), moist, very hard, clayey, sandy SILT with trace gravel.		584	
20A	60/60 100%	cs		13			44			582	
21A	24/24 100%	ss	6-14 18-22 N=32	19		1.94 B	50	Very dark greenish gray (10Y3/1), moist, hard, silty CLAY with sand and trace gravel.		580	
	0/48 0%	cs					52	Very dark bluish gray (5BG3/1), moist, firm, silty CLAY with trace sand and trace gravel.		578	Possible rock at end of auger.
22A	24/24 100%	ss	2-7 7-15 N=14	21		3.71 BSh	54			576	
	24/24 100%	ss	4-8 11-13 N=19				56	Greenish gray (5G6/1) with 40% yellowish brown (10YR5/6) mottles, moist, hard, silty CLAY with sl. trace sand.		574	
23A	0/12 0%	BD		26		2.13 BSh	58			572	
							60			570	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 12.80 - While drilling ▽ = 5.74 - MW16S on 6/1/06 ▽ = 51.37 - MW16D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/48 0%	RC								62	Greenish gray (5G6/1) with 40% yellowish brown (10YR5/6) mottles, moist, hard, silty CLAY with sl. trace sand. [Continued from previous page]		566	
26A	24/24 100%	SS	32-34 42-51 N=76	25		2.72	BSh			64			562	
	0/24 0%	RC								66	Yellowish brown (10YR5/6) with 20% greenish gray (5G6/1) mottles, moist, hard, silty CLAY with trace sand and trace coal fragments.		560	
28A	24/24 100%	SS	15-21 21-21 N=42	18		2.72	BSh			68			558	
29A	24/24 100%	SS	14-17 21-25 N=38	20		2.91	BSh			70			556	70' to 79.5' - possible oxidation rinds.
30A	24/24 100%	SS	12-21 34-35 N=55	18		5.04	BSh			72			554	
31A	24/24 100%	SS	16-21 27-35 N=48	16		8.15	BSh			74			552	
										76	Yellowish brown (10YR5/6) with zones of gray (10YR4/1) mottles, moist, hard, clayey SILT with some sand and trace gravel.		550	
32A	60/60 100%	CS		19						78			548	
										80				

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 4/21/2006
Finish: 4/25/2006
WEATHER: Overcast, cool (mid-40's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-850 Track Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-16
Well ID: n/a
Surface Elev: 626 ft. MSL
Completion: 92 ft. BGS
Station: 877,355.0N
 2,515,080.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 12.80 - While drilling	▽ = 5.74 - MW16S on 6/1/06	▽ = 51.37 - MW16D on 6/1/06	
			RQD					Section 2, Tier 7N; Range 3W				
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
33A	60/60 100%	cs		16				82	Yellowish brown (10YR5/6) with zones of gray (10YR4/1) mottles, moist, hard, clayey SILT with some sand and trace gravel. <i>[Continued from previous page]</i>		546	
								84	Yellow brown (10YR5/6), very moist, very soft, clayey, very fine-grained SAND and SILT.		544	
	0/60 0%	cs						86			542	
								88	Yellowish brown (10YR5/6), moist, hard, silty CLAY with sand and trace gravel.		540	
								90			538	
35A	24/24 100%	ss	9 11-16 N=20			2.72 BSh		92	Dark gray (10YR3/1), moist, firm, silty CLAY with sand and trace gravel.		536	Possible rock at end of auger.
End of Boring = 92.0 ft. BGS												

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/4/2006

Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17

Well ID: MW17D

Surface Elev: 627 ft. MSL

Completion: 54 ft. BGS

Station: 878,659.0N

2,515,090.4E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 11.70 - While drilling ▽ = 6.89 - MW17S on 6/1/06 ▽ = 54.45 - MW17D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	1-1 1-1 N=2	29						Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel.		626	
2A	24/24 100%	ss	1-1 2-2 N=3	26	1.71 None				2	Yellowish brown (10YR5/8), moist, soft, silty CLAY.		624	
3A	24/24 100%	ss	1-1 1-3 N=2	16	2.62 BSh				4	Gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with little sand and trace gravel.		622	
4A	24/24 100%	ss	1-2 2-3 N=4	18	2.33 B				6	Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT.		620	
5A	24/24 100%	ss	1-1 2-3 N=3	18					8	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT.		618	
6A	24/24 100%	ss	1-1 1-1 N=2	21	0.58 B				10	Yellowish brown (10YR5/6), moist, soft, very sandy, clayey SILT.		616	
6B				18					12	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, very moist, soft, very sandy, clayey SILT.			
7A	24/24 100%	ss	1-1 3-7 N=4	21					12	Moderate yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND.			
7B				19					14	Yellowish brown (10YR5/6), wet, dense, silty, very fine-grained SAND.			
8A				15					14	Yellowish brown (10YR5/6) wet, dense, fine- to medium-grained SAND.			
8B	24/24 100%	ss	2-5 5-7 N=10	13					14	Moderate yellowish brown (10YR5/4), wet, dense, SILT and very fine-grained SAND.			
8C				13					16	Yellowish brown (10YR5/6), moist, clayey SILT and very fine-grained SAND with trace gravel.			
9A	24/24 100%	ss	0-6 6-6 N=12	17					16	Yellowish brown (10YR5/6), wet, loose, very fine- to medium-grained SAND.			
10A	24/24 100%	ss	0-2 2-3 N=4	19					18	Yellowish brown (10YR5/6), wet, loose, very fine- to fine-grained SAND.			
10B				23					18	Yellowish brown (10YR5/6), wet, sl. dense, SILT with some very fine-grained SAND.			
									20	Gray (10YR5/1), wet, sl. dense, SILT with some very			

NOTE(S): CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/4/2006

Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17

Well ID: MW17D

Surface Elev: 627 ft. MSL

Completion: 54 ft. BGS

Station: 878,659.0N

2,515,090.4E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:							
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 11.70 - While drilling	▽ = 6.89 - MW17S on 6/1/06	▽ = 54.45 - MW17D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	12/24 50%	ss	0-1 1-2 N=2	21				Section 2, Tier 7N; Range 3W				21	fine-grained SAND.		606	
12A	24/24 100%	ss	2-2 5-7 N=7	18									22	Yellowish brown (10YR5/6), wet, loose, very fine- to fine-grained SAND.		604
12B				9		4.65	BSh					24	Gray (10YR5/1), wet, loose, very fine- to fine-grained SAND.		602	
13A	25/60 42%	cs										26			600	
												28			598	
												30	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		596	
14A	60/60 100%	cs		9								32			594	
												34			592	
												36			590	
15A	60/60 100%	cs		16								38	Gray (10YR5/1), wet, dense, very fine- to fine-grained SAND.		588	
15B				8								40	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.			

NOTE(S): CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/4/2006

Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17

Well ID: MW17D

Surface Elev: 627 ft. MSL

Completion: 54 ft. BGS

Station: 878,659.0N

2,515,090.4E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 11.70 - While drilling ▽ = 6.89 - MW17S on 6/1/06 ▽ = 54.45 - MW17D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
16A	48/60 80%	cs		14					42			586	
16B				8					44			584	
17A	60/60 100%	cs		13			Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]		46			582	
18A	60/60 100%	cs		22					48			580	
18B				16			Dark bluish gray (10BG4/1), moist, hard, silty CLAY with little sand.		50			578	
18C				11			Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		52			576	
							Gray (10YR5/1), moist, firm, silty, very fine-grained to fine-grained SAND.					574	
							Dark bluish gray (10BG4/1), moist, hard, silty CLAY with little sand.						

End of Boring = 53.87 ft.

NOTE(S): CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/4/2006
Finish: 5/4/2006
WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17a
Well ID: MW17S
Surface Elev: 627 ft. MSL
Completion: 24 ft. BGS
Station: 878,658.5N
 2,515,084.8E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
										Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel.		626	
									2	Yellowish brown (10YR5/8), moist, soft, silty CLAY.			
									4	Gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with little sand and trace gravel.		624	
									6	Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT.		622	
									8	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT.		620	
									10	Yellowish brown (10YR5/6), moist, soft, very sandy, clayey SILT.		618	
									12	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, very moist, soft, very sandy, clayey SILT.		616	
									12	Moderate yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND.		614	
									12	Yellowish brown (10YR5/6), wet, dense, silty, very fine- to fine-grained SAND.			
									14	Yellowish brown (10YR5/6), very moist, dense, silty, very fine-grained SAND.			
									14	Yellowish brown (10YR5/6) wet, dense, fine- to medium-grained SAND.			
									14	Moderate yellowish brown (10YR5/4), wet, dense, SILT and very fine-grained SAND.		612	
									16	Yellowish brown (10YR5/6), moist, clayey SILT and very fine-grained SAND with trace gravel.			
									18	Yellowish brown (10YR5/6), wet, loose, very fine- to medium-grained SAND.		610	
									18	Yellowish brown (10YR5/6), wet, loose, very fine- to fine-grained SAND.			
									20	Yellowish brown (10YR5/6), wet, sl. dense, SILT with some very fine-grained SAND.		608	
									20	Gray (10YR5/1), wet, sl. dense, SILT with some very			

NOTE(S): MW17S installed in blind-drilled borehole within 10 ft of SB-17.

FIELD BORING LOG



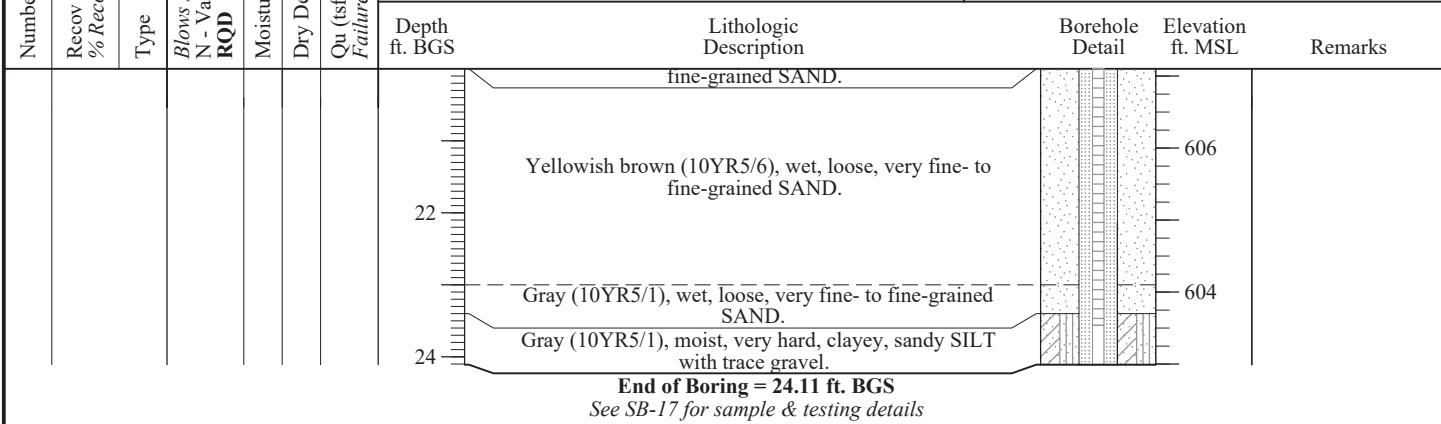
CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/4/2006
Finish: 5/4/2006

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4¼" HSA (blind drill)
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17a
Well ID: MW17S
Surface Elev: 627 ft. MSL
Completion: 24 ft. BGS
Station: 878,658.5N
 2,515,084.8E

WEATHER: Partly sunny, cool (mid-50's)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 11.70 - While drilling ▽ = 6.89 - MW17S on 6/1/06 ▽ = 54.45 - MW17D on 6/1/06	



NOTE(S): MW17S installed in blind-drilled borehole within 10 ft of SB-17.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 5/4/2006
Finish: 5/4/2006

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4 1/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: K. Doetzel
Helper: S. McCartney
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17
Well ID: MW17D
Surface Elev: 627 ft. MSL
Completion: 54 ft. BGS
Station: 878,659.0N
 2,515,090.4E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 11.70 - While drilling ▽ = 6.89 - MW17S on 6/1/06 ▽ = 54.45 - MW17D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	20/24 83%	ss	1-1 1-1 N=2	29						Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel.		626	
2A	24/24 100%	ss	1-1 2-2 N=3	26	1.71 None				2	Yellowish brown (10YR5/8), moist, soft, silty CLAY.		624	
3A	24/24 100%	ss	1-1 1-3 N=2	16	2.62 BSh				4	Gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY with little sand and trace gravel.		622	
4A	24/24 100%	ss	1-2 2-3 N=4	18	2.33 B				6	Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT.		620	
5A	24/24 100%	ss	1-1 2-3 N=3	18					8	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, firm sandy, clayey SILT.		618	
6A	24/24 100%	ss	1-1 1-1 N=2	21	0.58 B				10	Yellowish brown (10YR5/6), moist, soft, very sandy, clayey SILT.		616	
6B				18					12	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, very moist, soft, very sandy, clayey SILT.			
7A	24/24 100%	ss	1-1 3-7 N=4	21					12	Moderate yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND.			
7B				19					14	Yellowish brown (10YR5/6), wet, dense, silty, very fine-grained SAND.			
8A				15					14	Yellowish brown (10YR5/6) wet, dense, fine- to medium-grained SAND.			
8B	24/24 100%	ss	2-5 5-7 N=10	13					14	Moderate yellowish brown (10YR5/4), wet, dense, SILT and very fine-grained SAND.			
8C				13					16	Yellowish brown (10YR5/6), moist, clayey SILT and very fine-grained SAND with trace gravel.			
9A	24/24 100%	ss	0-6 6-6 N=12	17					16	Yellowish brown (10YR5/6), wet, loose, very fine- to medium-grained SAND.			
10A	24/24 100%	ss	0-2 2-3 N=4	19					18	Yellowish brown (10YR5/6), wet, loose, very fine- to fine-grained SAND.			
10B				23					18	Yellowish brown (10YR5/6), wet, sl. dense, SILT with some very fine-grained SAND.			
									20	Gray (10YR5/1), wet, sl. dense, SILT with some very			

NOTE(S): CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
 Site: CCB Management Facility
 Location: Coffeen, Illinois
 Project: 05SS3004A
 DATES: Start: 5/4/2006
 Finish: 5/4/2006

CONTRACTOR: Reynolds Drilling Corp.
 Rig mfg/model: CME-1050 ATV Rig
 Drilling Method: 4 1/4" HSA w/SS & CME samplers
 FIELD STAFF: Driller: K. Doetzel
 Helper: S. McCartney
 Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17
 Well ID: MW17D
 Surface Elev: 627 ft. MSL
 Completion: 54 ft. BGS
 Station: 878,659.0N
 2,515,090.4E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 2, Tier 7N; Range 3W	▽ = 11.70 - While drilling ▽ = 6.89 - MW17S on 6/1/06 ▽ = 54.45 - MW17D on 6/1/06	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	12/24 50%	ss	0-1 1-2 N=2	21							fine-grained SAND.		606	
12A	24/24 100%	ss	2-2 5-7 N=7	18							Yellowish brown (10YR5/6), wet, loose, very fine- to fine-grained SAND.		604	
12B				9		4.65	BSh				Gray (10YR5/1), wet, loose, very fine- to fine-grained SAND.		602	
13A	25/60 42%	cs											600	
													598	
											Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		596	
14A	60/60 100%	cs		9									594	
													592	
													590	
15A	60/60 100%	cs		16							Gray (10YR5/1), wet, dense, very fine- to fine-grained SAND.		590	
15B				8							Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		588	
													588	

NOTE(S): CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/4/2006

Finish: 5/4/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-17

Well ID: MW17D

Surface Elev: 627 ft. MSL

Completion: 54 ft. BGS

Station: 878,659.0N

2,515,090.4E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
16A	48/60 80%	cs		14			42			586	
16B				8			44			584	
17A	60/60 100%	cs		13			46	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]		582	
18A	60/60 100%	cs		22			50			580	
18B				16			52	Dark bluish gray (10BG4/1), moist, hard, silty CLAY with little sand.		576	
18C				11			54	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		574	
							56	Gray (10YR5/1), moist, firm, silty, very fine-grained to fine-grained SAND.			
							58	Dark bluish gray (10BG4/1), moist, hard, silty CLAY with little sand.			

End of Boring = 53.87 ft.

NOTE(S): CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 5/11/2006
Finish: 5/11/2006

CONTRACTOR: Reynolds Drilling Corp.
Rig mfg/model: CME-1050 ATV Rig
Drilling Method: 4¼" HSA (blind drill)
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-18a
Well ID: MW18S
Surface Elev: 626 ft. MSL
Completion: 16 ft. BGS
Station: 878,604.7N
 2,513,745.2E

WEATHER: Partly sunny, cool (mid-50's)

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft³)	Qu (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 12.20 - While drilling	▼ = 6.87 - MW18S on 6/1/06	▼ =
								Section 3, Tier 7N; Range 3W			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
								Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel.			
								Yellowish brown (10YR5/6) with 50% gray (10YR4/1) mottles, moist, silty CLAY with little sand.		624	
								Yellowish brown (10YR5/6) with 10% gray (10YR4/1) mottles, moist, silty CLAY with little sand.		622	
										620	
								Gray (10YR4/1) with 10% Yellowish brown (10YR5/6) mottles, moist, silty CLAY with little sand.		618	
										616	
								Light gray (10YR6/1) moist, soft, clayey, very fine- to fine-grained SAND.		614	
								Light gray (10YR6/1) moist, soft, silty, very fine- to fine-grained SAND.			
								Yellowish brown (10YR5/6), very moist, soft, silty, very fine- to fine-grained, SAND with trace gravel.		612	
								Yellowish brown (10YR5/6), wet, loose, fine- to medium-grained SAND.			
								Yellowish brown (10YR5/6), very moist, soft, sandy (very fine- to fine-grained) SILT.			
								Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		610	

End of Boring = 16.40 ft. BGS
 See SB-18 for sample & testing details

NOTE(S): MW18S installed in blind-drilled borehole within 10 ft of SB-18.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/11/2006

Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-18

Well ID: n/a

Surface Elev: 626 ft. MSL

Completion: 54 ft. BGS

Station: 878,605.0N

2,513,750.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	ss	0-1 1-1 N=2	24	1.31	BSh	0	Dark grayish brown (10YR4/2), moist, soft, clayey SILT with trace sand and trace gravel.		626	
2A	24/24 100%	ss	1-2 2-2 N=4	28	1.78	Sh	2	Yellowish brown (10YR5/6) with 50% gray (10YR4/1) mottles, moist, silty CLAY with little sand.		624	
3A	24/24 100%	ss	1-2 2-2 N=4	23	1.32	BSh	4	Yellowish brown (10YR5/6) with 10% gray (10YR4/1) mottles, moist, silty CLAY with little sand.		622	
4A	24/24 100%	ss	1-1 1-2 N=2	24	1.09	B	6	Gray (10YR4/1) with 10% Yellowish brown (10YR5/6) mottles, moist, silty CLAY with little sand.		620	
5A	24/24 100%	ss	1-1 1-2 N=2	28	0.54	BSh	8				618
6A	24/24 100%	ss	0-0 1-2 N=1	21	0.39	B	10	Light gray (10YR6/1) moist, soft, silty, very fine- to fine-grained SAND.		616	
7A	24/24 100%	ss	3-5 9-15 N=14	17			12				614
7B	24/24 100%	ss	3-5 9-15 N=14	15			14	Light gray (10YR6/1) moist, soft, silty, very fine- to fine-grained SAND.		612	
8A	24/24 100%	ss	8-9 9-10 N=18	14			16	Yellowish brown (10YR5/6), very moist, soft, silty, very fine- to fine-grained, SAND with trace gravel.		610	
8B	24/24 100%	ss	8-9 9-10 N=18	11			18	Yellowish brown (10YR5/6), wet, loose, fine- to medium-grained SAND.		608	
8C	24/24 100%	ss	8-9 9-10 N=18	9			20	Yellowish brown (10YR5/6), very moist, soft, sandy (very fine- to fine-grained) SILT.		606	
9A	24/36 67%	cs						Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		608	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05S3004A

DATES: Start: 5/11/2006

Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-18

Well ID: n/a

Surface Elev: 626 ft. MSL

Completion: 54 ft. BGS

Station: 878,605.0N

2,513,750.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:								
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf)	Failure Type	Quadrangle: Coffeen, IL	▼ = 12.20 - While drilling	▼ = 6.87 - MW18S on 6/1/06	▼ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
10A	60/60 100%	cs						Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. <i>[Continued from previous page]</i>				22			604		
													24			602	
11A	60/60 100%	cs		5									26			600	
													28			598	
12A	60/60 100%	cs			11							30			596		
												32			594	Coal fragment seam	
												34			592		
												36			590		
13A					10							38	Gray (10YR5/1), moist, dense, SILT.		588		
13B					12								Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel.		586		

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: CCB Management Facility

Location: Coffeen, Illinois

Project: 05SS3004A

DATES: Start: 5/11/2006

Finish: 5/11/2006

WEATHER: Partly sunny, cool (mid-50's)

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-1050 ATV Rig

Drilling Method: 4 1/4" HSA w/SS & CME samplers

FIELD STAFF: Driller: K. Doetzel

Helper: S. McCartney

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB-18

Well ID: n/a

Surface Elev: 626 ft. MSL

Completion: 54 ft. BGS

Station: 878,605.0N

2,513,750.0E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
14A	60/60 100%	cs			13				42			584	Appears more clayey
15A	60/60 100%	cs			13				44			582	
16A	60/60 100%	cs			13				46	Gray (10YR5/1), moist, very hard, clayey, sandy SILT with trace gravel. [Continued from previous page]		580	
16B	60/60 100%	cs			22				48			578	
									50			576	
									52	Greenish gray (10BG5/1), moist, firm, silty CLAY with little sand and trace gravel.		574	
									54	End of Boring = 54.0 ft.		572	

NOTE(S): Borehole abandoned using bentonite grout pumped from bottom of borehole. CME-1050 had 280# hammer for SPT.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station

Site: Ash Pond Investigation

Location: Coffeen, IL

Project: 05S3004B

DATES: Start: 5/1/2007

Finish: 5/1/2007

WEATHER: Partly sunny, warm

CONTRACTOR: Reynolds Drilling Corp.

Rig mfg/model: CME-550 ATV Drill

Drilling Method: 4 1/4" Hollow stem auger with split spoon sampler

FIELD STAFF: Driller: A. Rachford

Helper: M. Brown

Eng/Geo: R. Hasenyager

BOREHOLE ID: SB20

Well ID: MW20S

Surface Elev: 620.30 ft. MSL

Completion: 16.00 ft. BGS

Station: 874,226.44N

2,515,867.87E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows/6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	12/24 50%	SS		24.9			0	Black (10YR2/1), moist, firm, clayey SILT with slight trace sand (TOPSOIL)		620		
1B				19.9			2	Gray (10YR6/1), moist, firm, clayey SILT with roots and slight trace sand				
2A	23/24 96%	SS		28.9		2.33 BSh	4	Gray (10YR5/1) with 40% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY		618		
3A	13/24 54%	SS		17.2			6	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with slight trace sand		616		
4A	24/24 100%	SS		20.5		2.52 B	8	Gray (10YR5/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel		614		
4B				20.6		2.33 B	10	Gray (10YR6/1), moist, firm, silty CLAY with trace sand and slight trace gravel		612		
5A	24/24 100%	SS		23.1		1.63 B	12	Yellowish brown (10YR5/8) with 10% gray (10YR6/1) mottles, moist, firm, silty CLAY with trace sand and slight trace gravel		610		
5B				24.0		1.07 B	14	Yellowish brown (10YR5/8), very moist, soft, sandy CLAY with some silt		608		
6A				16.4			16	Yellowish brown (10YR5/8), very moist, soft, clayey, very fine- to medium-grained SAND		606		
6B	24/24 100%	SS		19.1		0.39 B		Gray (10YR6/1), wet, loose, very fine- to medium-grained SAND with trace coarse-grained sand				
6C				17.5				Brown (10YR5/3), moist, very hard, very silty CLAY with sand and gravel				
7A	13/16 81%	SS		19.1				Gray (10YR5/1), moist, very hard, very silty CLAY with sand and gravel				
7B				11.7		1.75 BSh						
8A	22/24 92%	SS		9.0		4.80 BSh						

End of Boring = 16.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/15/2010
Finish: 2/15/2010
WEATHER: Overcast, cold, windy (10-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Suzanna Simpson

BOREHOLE ID: G104
Well ID: G104
Surface Elev: 627.96 ft. MSL
Completion: 20.00 ft. BGS
Station: 875,849.26N
 2,514,504.98E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = Dry - While drilling ▽ = ▽ = 15.40 - 3/1/2010	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	ss	9-11 12-14 N=23	16					2	FILL - Brown (10YR5/3) with 5% gray (10YR5/1) and 5% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY with slight trace sand and gravel.		626	
2A	20/24 83%	ss	3-5 9-12 N=14	21					4			624	
3A	19/24 79%	ss	3-6 8-9 N=14	21					6	Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand, slight trace roots.		622	
4A	17/24 71%	ss	2-3 4-7 N=7	25					8			620	
5A	22/24 92%	ss	1-2 5-8 N=7	24					10	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with slight trace sand.		618	
6A	18/24 75%	ss	1-2 3-5 N=5	21					12			616	
7A	23/24 96%	ss	woh-2 3-4 N=5	23					14	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with trace sand and slight trace gravel.		614	
8A	22/24 92%	ss	1-3 3-3 N=6	24					16	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, soft, silty CLAY with trace sand and slight trace gravel.		612	
9A				16					16	Gray (10YR6/1), moist, very soft, clayey, very fine- to coarse-grained SAND with trace gravel.		612	
9B	17/24 71%	ss	woh-6 27-40 N=33	13					18	Brown (10YR5/3), slightly moist, hard, clayey SILT with trace sand and gravel.		610	
10A	22/24 92%	ss	10-24 44-66 N=68	7					20	Gray (10YR4/1), slightly moist, hard, clayey SILT with sand and gravel.		608	

End of Boring = 20.0 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05SS3004A
DATES: Start: 10/15/2010
Finish: 10/15/2010
WEATHER: Sunny (mid-50's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: R. Hasenyager

BOREHOLE ID: R201
Well ID: R201
Surface Elev: 624.02 ft. MSL
Completion: 17.22 ft. BGS
Station: 877,925.26N
 2,514,841.96E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:				
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
							Very dark grayish brown (10YR3/2), moist, soft, friable, clayey SILT, slight trace sand and gravel				
							Dark brown (10YR3/3), moist, soft, silty CLAY		622		
							Yellowish brown (10YR5/8) with 20% gray (10YR5/1) mottles, moist, firm, silty CLAY, trace sand, slight trace gravel		620		
							Gray (10YR5/1) with 5% yellowish brown (10YR5/6) mottles, moist, firm, silty CLAY, sand and trace gravel		618		
							Gray (10YR5/1), moist, firm, sandy CLAY, trace silt and slight trace gravel		616		
							Yellowish brown (10YR5/8) with 10% gray (10YR5/1) mottles, moist, firm, sandy CLAY, trace gravel		614		
							Yellowish brown (10YR5/8), wet, soft, silty SAND, trace gravel		612		
							Yellowish brown (10YR5/8), moist, firm, clayey SILT		610		
							Greenish gray (5GY6/1), moist, firm, interbedded clayey SILT and SILT		608		
							Yellowish brown (10YR5/8), wet, soft, fine- to coarse-grained SAND, slight trace gravel		608		
							Yellowish brown (10YR5/8), wet, firm, very fine- to fine-grained silty SAND				
							Gray (10YR5/1), wet, soft, SILT				
							Gray (10YR5/1), wet, soft, fine- to coarse-grained SAND, slight trace gravel				
							Gray (10YR5/1), moist, hard, very silty CLAY, trace sand and gravel				
							End of Boring = 17.22 ft. BGS				

NOTE(S): R201 blind drilled in borehole approximately 8 ft. west of G201. Lithology taken from G201.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Power Station - Gypsum Mgmt Facility
Location: Coffeen, IL
Project: 15E0161A
DATES: Start: 3/20/2017
 Finish: 3/20/2017
WEATHER: Sunny, mild, (mid-60s)

CONTRACTOR: Bulldog Drilling
Rig mfg/model: CME-550 ATV Drill
Drilling Method: 4 1/4" HSA (blind drill)
FIELD STAFF: Driller: J. Gates
 Helper: C. Clines
 Eng/Geo: R. Hasenyager

BOREHOLE ID: R205
Well ID: R205
Surface Elev: 621.91 ft. MSL
Completion: 16.42 ft. BGS
Station: 875,548.77N
 2,515,910.12E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W	▼ = Dry - Upon Completion ▽ = 2.50 - 3/21/2017 ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
									0	Very dark grayish brown (10YR3/2), moist, firm, clayey SILT			
								▼	2	Yellowish brown (10YR5/4) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand		620	
									4	Yellowish brown (10YR5/4) with 30% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY, slight trace sand		618	
									6			616	
									8	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand, slight trace gravel		614	
									10	Yellowish brown (10YR5/6), moist, firm, silty CLAY with sand, trace gravel		612	
									12	Yellowish brown (10YR5/6), moist, soft, sandy SILT, slight trace gravel		610	
									14	Yellowish brown (10YR 5/4), moist, soft, silty SAND, trace gravel		608	
									16	Gray (10YR5/1), moist, hard, silty CLAY, trace sand and gravel		606	

End of Boring = 16.42 ft. BGS

NOTE(S): R205 installed in blind drilled boring approximately 5 ft. west of G205. Lithologies from boring G205.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/21/2008
Finish: 2/21/2008
WEATHER: Overcast, cold

CONTRACTOR: Testing Service Corp.
Rig mfg/model: CME-650 Track Drill
Drilling Method: 3/4" HSA w/SS & CME samplers
FIELD STAFF: Driller: B. Williamson
Helper: R. Keedy
Eng/Geo: .

BOREHOLE ID: G205
Well ID: G205
Surface Elev: 622.15 ft. MSL
Completion: 16.00 ft. BGS
Station: 875,550.19N
 2,515,914.87E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	4-3 4-4 N=7	22				Very dark grayish brown (10YR3/2), moist, firm, clayey SILT		622	
1B				30	1.67	Sh	2	Yellowish brown (10YR5/4) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY, slight trace sand		620	
2A	19/24 79%	SS	3-5 6-5 N=11	24	1.86	B	4	Yellowish brown (10YR5/4) with 30% yellowish brown (10YR5/8) mottles, moist, hard, silty CLAY, slight trace sand		618	
3A	20/24 83%	SS	2-2 5-5 N=7	19	1.55	B	6	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand, slight trace gravel		616	
4A	19/24 79%	SS	5-6 7-8 N=13	20	1.12	B	8	Yellowish brown (10YR5/6), moist, firm, silty CLAY with sand, trace gravel		612	
5A	16/24 67%	SS	2-2 3-5 N=5	20	0.62	BSh	10	Yellowish brown (10YR5/6), moist, soft, sandy SILT, slight trace gravel		610	
6A	22/24 92%	SS	1-2 3-3 N=5	17	0.62	BSh	12	Yellowish brown (10YR5/4), moist, soft, silty SAND, trace gravel		608	
7A				17							
7B	23/24 96%	SS	3-3 5-6 N=8	17	1.36	BSh	14	Gray (10YR5/1), moist, hard, silty CLAY, trace sand and gravel		608	
8A				15							
8B	17/24 71%	SS	5-19 26-35 N=45	10							

End of Boring = 16.0 ft. BGS

NOTE(S):

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 2/10/2010
Finish: 2/10/2010

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: .

BOREHOLE ID: T127
Well ID: T127
Surface Elev: 625.53 ft. MSL
Completion: 19.80 ft. BGS
Station: 875,359.21N
 2,513,911.02E

WEATHER: Overcast, wind 15mph, ~10-20F

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 15.30 - While drilling ▽ = 18.00 - Upon completion ▽ = 11.26 - 3/1/10	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1	24/24 100%	SS	8-5 5-8 N=10						2	FILL - yellowish brown (10YR5/6) with 20% grayish brown (10YR5/2) mottles, moist, stiff, silty CLAY with trace sand and gravel.		624	
2	10/24 42%	SS	5-6 7-11 N=13						4			622	
3	20/24 83%	SS	1-4 5-7 N=9						6	Dark yellowish brown (10YR4/6) with 30% brownish yellow (10YR6/6) and 10% very dark gray (10YR3/1) mottles, moist, medium, clayey SILT with trace sand.		620	
4	24/24 100%	SS	2-4 5-6 N=9						8	Grayish brown (10YR5/2) with 20% dark yellowish brown (10YR4/6) mottles, moist, medium, clayey SILT with sand.		618	
5	24/24 100%	SS	2-4 5-6 N=9						10	Grayish brown (10YR5/2) with 20% dark yellowish brown (10YR4/6) and 5% very dark brown (10YR2/2) mottles, moist, medium, clayey SILT with sand and slight trace gravel.		616	
6	24/24 100%	SS	3-3 5-7 N=8					▽	12	Dark yellowish brown (10YR4/6) with 5% light brownish gray (10YR6/2) mottles, moist, medium, sandy SILT with gravel.		614	
7	24/24 100%	SS	1-2 3-3 N=5						14	Light brownish gray (10YR6/2) with 30% yellowish brown (10YR5/6) mottles, very moist, medium, clayey SAND with silt.		612	
8	24/24 100%	SS	1-1 2-1 N=3					▼	16	Light brownish gray (10YR6/2) with 30% brownish yellow (10YR6/8) mottles, wet, very loose, silty SAND trace with gravel.		610	
9	24/24 100%	SS	8-12 28-30 N=40					▽	18	Brownish yellow (10YR6/8), wet, medium dense, silty SAND with gravel.		608	
10	22/22 100%	SS	8-44 65-60/4" N=109							Grayish brown (10YR5/2), slightly moist, hard, clayey SILT with sand and gravel.		606	

End of Boring = 19.8 ft. BGS

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/9/2010
Finish: 2/9/2010
WEATHER: Cold, windy, snow (lo-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: T128
Well ID: T128
Surface Elev: 626.27 ft. MSL
Completion: 22.00 ft. BGS
Station: 875,509.70N
 2,513,909.45E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	12/24 50%	ss	37-17 15-18 N=32	14			0	FILL - Brown (10YR5/3) with 5% gray (10YR5/1) and 5% yellowish brown (10YR5/6) mottles, moist, hard (frozen), silty CLAY with trace sand and slight trace gravel.		626	
2A	7/24 29%	ss	4-7 7-13 N=14				2	FILL - Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, hard (frozen), silty CLAY with slight trace sand and gravel.		624	
3A	12/24 50%	ss	3-4 6-8 N=10	26			4	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with slight trace sand, slight trace roots.		622	
4A	19/24 79%	ss	2-4 5-7 N=9	24			6			620	
5A	22/24 92%	ss	1-3 4-5 N=7	20			8	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY with trace sand and slight trace gravel.		618	
6A	22/24 92%	ss	1-3 5-5 N=8	20			10			616	
7A	22/24 92%	ss	2-3 4-5 N=7	19			12			614	
8A	20/24 83%	ss	2-2 3-3 N=5	18			14	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, moist, medium, silty CLAY with trace sand and slight trace gravel.		612	
9A	22/24 92%	ss	woh-1 8-7 N=9	19			16	Brown (10YR5/3), moist, soft, clayey SAND with slight trace gravel.		610	
9B				18			17	Gray (10YR6/1), wet, loose, silty, very fine- to coarse-grained SAND.			
10A							18	Brown (10YR5/3), wet, loose, silty, very fine- to fine-grained SAND.		608	
10B	18/24 75%	ss	3-10 13-11 N=23	17			20	Brown (10YR5/3), slightly moist, very stiff, clayey SILT with trace sand and gravel.			

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project:
DATES: Start: 2/9/2010
Finish: 2/9/2010
WEATHER: Cold, windy, snow (10-20's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4/4" HSA w/SS samplers
FIELD STAFF: Driller: T. List
Helper: M. Herbst/S. Hamby
Eng/Geo: S. Simpson

BOREHOLE ID: T128
Well ID: T128
Surface Elev: 626.27 ft. MSL
Completion: 22.00 ft. BGS
Station: 875,509.70N
 2,513,909.45E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%	ss	8-16 18-25 N=34	16			22	Dark gray (10YR4/1), slightly moist, hard, clayey SILT with trace sand and gravel.		606	
End of Boring = 22.0 ft. BGS											

NOTE(S): Well completed prior to construction of berm road. Boring surface elevation is as of the well install date and not the final constructed elevation.

FIELD BORING LOG



CLIENT: AEG Coffeen Power Station
Site: CCB Management Facility
Location: Coffeen, Illinois
Project: 05S3004A
DATES: Start: 10/15/2010
Finish: 10/15/2010
WEATHER: Sunny (mid-50's)

CONTRACTOR: Layne-Western Co
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" HSA w/SS samplers
FIELD STAFF: Driller: D. Mahurin
Helper: J. Litsch/D. Smail
Eng/Geo: R. Hasenyager

BOREHOLE ID: T202
Well ID: T202
Surface Elev: 626.22 ft. MSL
Completion: 18.00 ft. BGS
Station: 876,699.42N
 2,514,895.01E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Quadrangle: Coffeen, IL	Township: East Fork	Section 11, Tier 7N; Range 3W	▽ = Dry - While drilling	▽ = 15.42 - Upon completion
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
								FILL - Yellowish brown (10YR5/6), slightly moist, hard, silty CLAY with trace sand and gravel.		626	
1A	23/24 96%	ss	6-4 5-7 N=9				27				
								Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, hard, silty CLAY with slight trace sand and gravel.		624	
2A	24/24 100%	ss	3-4 6-6 N=10				25				
2B							26				
								Yellowish brown (10YR5/8) with 35% gray (10YR5/1) and 15% very dark brown (10YR2/2) mottles, moist, hard, silty CLAY with trace sand and slight trace gravel.		622	
3A	22/24 92%	ss	2-4 5-6 N=9				22				
4A	23/24 96%	ss	1-2 3-3 N=5				21				
5A	23/24 96%	ss	1-1 3-2 N=4				22				
6A	24/24 100%	ss	woh-1 3-3 N=4				23				
6B							21				
7A	24/24 100%	ss	1-2 1-2 N=3				21				
7B							25				
8A	24/24 100%	ss	woh-2 1-8 N=3				17				
								Gray (10YR6/1) with 10% brownish yellow (10YR6/6) mottles, very moist, soft, sandy CLAY with silt and slight trace gravel.		612	
								Gray (10YR5/1), wet, loose, very fine- to medium-grained SAND with trace gravel.		610	
9A	24/24 100%	ss	6-15 20-24 N=35				9				
								Gray (10YR6/1), moist, hard, very silty CLAY with sand and trace gravel.		610	
							18				

End of Boring = 18.0 ft. BGS

NOTE(S): T202 installed in borehole.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/17/2016
Finish: 8/17/2016
WEATHER: Hazy, (low-80s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: T408
Well ID: T408
Surface Elev: 621.09 ft. MSL
Completion: 25.92 ft. BGS
Station: 2,515,314.91N
 2,515,314.91E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0%	BD					2	Black (10YR2/1), moist, soft, clayey SILT (TOPSOIL)		620	
	0/60 0%	BD					4	Gray (10YR6/1) with 30% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY		618	
	0/60 0%	BD					6	Gray (10YR6/1), moist, firm, silty CLAY slight trace sand		616	
	0/60 0%	BD					8	Gray (10YR5/1), very moist, soft, clayey, very fine- to fine-grained SAND		614	
	0/60 0%	BD					10	Gray (10YR6/1) with 25% yellowish brown (10YR5/8) mottles, moist, firm, silty CLAY with sand and trace gravel		612	
	0/60 0%	BD					12	Yellowish brown (10YR5/8) with 40% gray (10YR6/1) mottles, moist, firm, silty CLAY with sand and trace gravel		610	
	0/60 0%	BD					14	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND		608	
	0/60 0%	BD					14	Dark brown (10YR3/3), very moist, soft, clayey, fine- to very coarse-grained SAND with slight trace gravel		608	
	0/60 0%	BD					14	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND		608	
	0/60 0%	BD					14	Dark yellowish brown (10YR4/4), moist, soft, sandy SILT with trace gravel		608	
	0/60 0%	BD					14	Yellowish brown (10YR5/4), wet, loose, very fine- to fine-grained SAND		608	
	0/60 0%	BD					14	Gray (10YR5/1), moist, very hard, very silty CLAY with sand and gravel		608	
4A	22/24 92%	SS	8-16 33-35 N=49			4.50	16	Dark gray (10YR4/1), moist, hard, SILT with few fine- to coarse-grained sand, little clay and trace small gravel.		606	
	0/36 0%	BD					18	Dark gray (10YR4/1), dry, hard, SILT with few fine- to coarse-grained sand, little clay and trace small gravel.		602	

NOTE(S): T408 installed in boring.
 Boring was blind drilled to 26.0 feet bgs. Blind drill lithologies from boring G405 and G405D.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/17/2016
Finish: 8/17/2016
WEATHER: Hazy, (low-80s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: T408
Well ID: T408
Surface Elev: 621.09 ft. MSL
Completion: 25.92 ft. BGS
Station: 2,515,314.91N
 2,515,314.91E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
7-1	0/36 0%	BD							22	Dark gray (10YR4/1), dry, hard, SILT with few fine-to coarse-grained sand, little clay and little small gravel.		600	
7-2	20/24 83%	DS							24	Dark gray (10YR4/1), moist, dense, fine- to coarse-grained SAND.		598	
7-3													
7-4	0/12 0%	BD								Dark gray (10YR4/1), dry, hard, SILT with few fine- to coarse-grained sand, little clay and little small gravel.		596	
End of Boring = 25.92 ft. BGS													

NOTE(S): T408 installed in boring.
 Boring was blind drilled to 26.0 feet bgs. Blind drill lithologies from boring G405 and G405D.

FIELD BORING LOG



CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Cloudy, (70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: T409
Well ID: T409
Surface Elev: 621.85 ft. MSL
Completion: 26.99 ft. BGS
Station: 2,514,693.89N
 2,514,693.89E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
	0/60 0%	BD							2	Dark brown (10YR3/3), moist, stiff, SILT with few clay and trace organics.		620	
	0/60 0%	BD							4	Brown (10YR5/3) with 10% dark brown (10YR3/3) mottles, SILT with some clay and trace small gravel.		618	
	0/60 0%	BD							6	Grayish brown (10YR5/2) with 5% dark yellowish brown (10YR4/6) mottles, moist, stiff, CLAY with few silt and little fine-grained sand.		616	
	0/60 0%	BD							8	Very pale brown (10YR7/4) with 25% yellowish brown (10YR5/6) mottles, moist, medium, CLAY with trace silt.		614	
	0/60 0%	BD							10	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with little silt and trace fine- to medium-grained sand.		612	
	0/60 0%	BD							12	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) and 5% very dark gray (10YR3/1) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		610	
	0/60 0%	BD							14	Gray (10YR6/1) with 25% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with few fine- to medium-grained sand, little silt, and trace small gravel.		608	
	0/60 0%	BD							16	Gray (10YR6/1) with 10% brownish yellow (10YR6/8) and 5% strong brown (7.5YR4/6) mottles, moist, stiff, CLAY with some fine- to medium-grained sand, little silt, and trace small gravel.		606	
	0/60 0%	BD							18	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, wet, loose, fine-grained SAND with some clay.		604	
	0/60 0%	BD							20	Gray (10YR6/1) with 5% brownish yellow (10YR6/6) mottles, moist, loose, fine-grained SAND with some clay.		602	
									22	Gray (10YR5/1), dry, hard, SILT with few clay, few fine- to coarse-grained sand and trace small gravel.			

NOTE(S): T409 installed in boring.
 Boring was blind drilled to 27.0 feet bgs. Blind drill lithologies from boring G406D.

FIELD BORING LOG

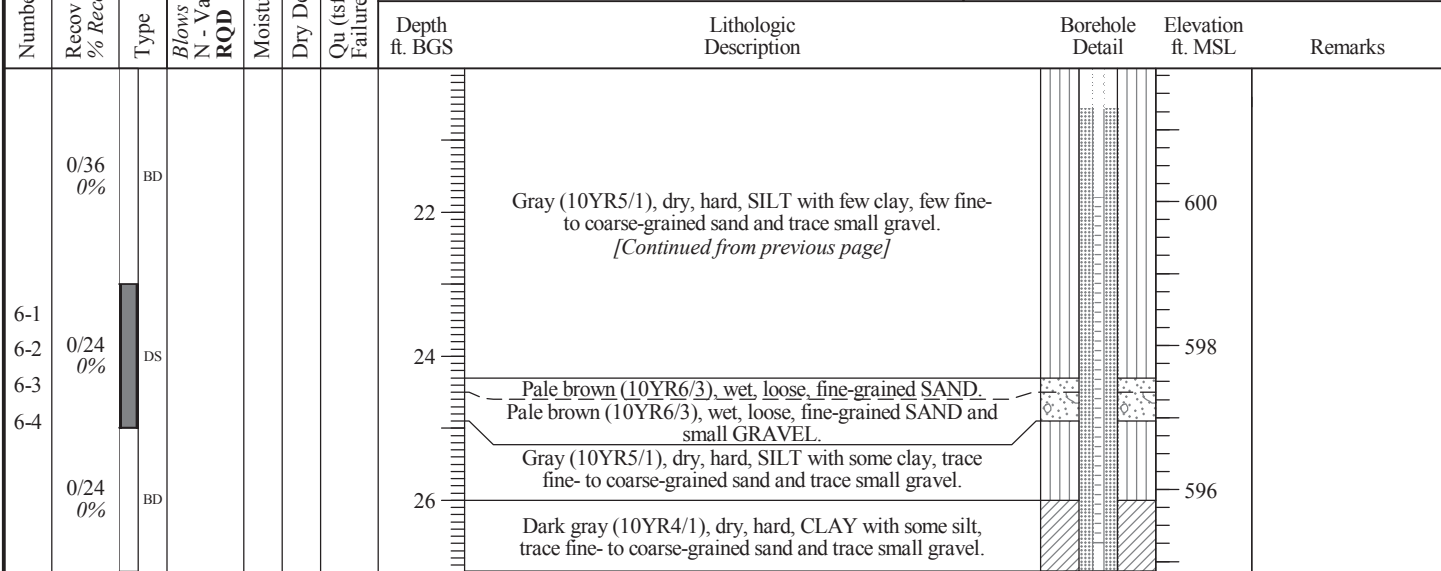


CLIENT: Natural Resources Technology, Inc.
Site: Coffeen Power Station - Ash Pond 2
Location: 134 CIPS Lane, Coffeen, IL 62017
Project: 16E0080
DATES: Start: 8/19/2016
Finish: 8/19/2016
WEATHER: Cloudy, (70s)

CONTRACTOR: Bulldog Drilling, Inc.
Rig mfg/model: CME-750 ATV Drill
Drilling Method: 4 1/4" Hollow Stem Auger
FIELD STAFF: Driller: J. Dittmaier
Helper: M. Hill
Eng/Geo: K. Theesfeld

BOREHOLE ID: T409
Well ID: T409
Surface Elev: 621.85 ft. MSL
Completion: 26.99 ft. BGS
Station: 2,514,693.89N
 2,514,693.89E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:	
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Quadrangle: Coffeen	▼ = Dry - During Drilling
								Township: East Fork	▽ =	
								Section 11, Tier 7 N.; Range 3 W.	▽ =	



End of Boring = 26.99 ft. BGS

NOTE(S): T409 installed in boring.
 Boring was blind drilled to 27.0 feet bgs. Blind drill lithologies from boring G406D.

FIELD BORING LOG



CLIENT: Illinois Power Holdings
Site: Coffeen Power Station
Location: Coffeen, Montgomery County, Illinois
Project: 14E0078
DATES: Start: 10/28/2014
Finish: 10/28/2014
WEATHER: Overcast, mild - mid 50's

CONTRACTOR: Ramsey
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: 4 1/4" Hollow Stem Auger with Split spoon
FIELD STAFF: Driller: B. Williamson
Helper: D. Crump
Eng/Geo: R. Hasenyager

BOREHOLE ID: TA31
Well ID: TA31
Surface Elev: 623.89 ft. MSL
Completion: 20.19 ft. BGS
Station: 876,542.25N
 2,513,856.77E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 10, Tier 7N; Range 3W	▼ = 8.11 - 10/30/2014 ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1	24/24 100%	ss	1-2 2-2 N=4	26					2			622	
2	22/24 92%	ss	2-3 3-4 N=6	28	1.44 B				4	Yellowish brown (10YR5/4), moist, soft, SILT with little clay and trace very fine sand. Brown (10YR4/3), moist, soft, SILT with little clay and trace very fine sand.		620	
3	23/24 96%	ss	2-3 4-4 N=7	20	1.71 Sh				6			618	
4	24/24 100%	ss	6-7 6-6 N=13	22	1.55 Sh				8			616	
5	22/24 92%	ss	2-2 2-4 N=4	22	1.55 Sh				10	Gray (10YR5/1) with 20% yellowish brown (10YR5/8) mottles, moist, soft, CLAY with few silt and trace very fine to fine sand.		614	
6	24/24 100%	ss	2-1 3-2 N=4	20	0.93 B				12	Brownish yellow (10YR6/6) with 20% gray (10YR6/1) mottles, moist, soft, CLAY with few silt and little very fine to fine sand and trace small gravel.		612	
7A	24/24 100%	ss	2-3 3-3 N=6	19					14	Brownish yellow (10YR6/6), wet, slightly dense, SAND (very fine to coarse) with little sand and silt and trace clay and small gravel.		610	
7B				22						Brownish yellow (10YR6/6), moist, soft, CLAY with few silt and trace very fine to fine sand and small gravel.			
8A	24/24 100%	ss	4-5 8-7 N=13	12						Brownish yellow (10YR6/6) wet, loose, SAND (very fine to very coarse) and trace small gravel.			
8B				13						Brownish yellow (10YR6/6) very moist, soft, SAND (very fine to medium) with few clay and silt.			
9	22/24 92%	ss	5-4 4-4 N=8	18					16	Yellowish brown (10YR5/8), moist, soft, CLAY with few silt and trace sand (very fine to very coarse) and small gravel.		608	
									18	Yellowish brown (10YR5/8), wet, loose, very fine to very coarse SAND.		606	
10	16/24 67%	ss	2-4 3-5 N=7	12					20	Gray (10YR5/1), moist, hard, CLAY with few silt and (very fine to very coarse) sand and small gravel.		604	

End of Boring at 20.19 ft.

NOTE(S): Monitoring well TA31 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Holdings
Site: Coffeen Power Station
Location: Coffeen, Montgomery County, Illinois
Project: 14E0078
DATES: Start: 10/27/2014
Finish: 10/27/2014
WEATHER: Partly sunny, mild - mid 70's

CONTRACTOR: Ramsey
Rig mfg/model: D-50 Turbo Tracked MST 800ATV
Drilling Method: 4 1/4" Hollow Stem Auger with Split spoon
FIELD STAFF: Driller: B. Williamson
Helper: D. Crump
Eng/Geo: R. Hasenyager

BOREHOLE ID: TA32
Well ID: TA32
Surface Elev: 618.93 ft. MSL
Completion: 16.47 ft. BGS
Station: 877,532.57N
 2,513,605.19E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Section 3, Tier 7N; Range 3W	▼ = 14.05 - 10/30/2014 ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1	18/24 75%	ss	2-1 2-3 N=3	21					0	Dark yellowish brown (10YR4/4), moist, soft, clayey SILT with trace sand (very fine).		618	
	21/24 88%	ss	2-1 1-1 N=2						2			616	
2				28					4	Black (10YR2/1), moist, soft, clayey SILT with trace sand (very fine).			
3A				24									
3B	14/24 58%	ss	1-1 3-3 N=4	22		1.15 Sh			6	Very dark gray (10YR3/1), moist, soft, CLAY with little silt and trace sand (very fine to fine).		614	
4	20/24 83%	ss	4-5 4-7 N=9	22		0.96 B			8	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, medium, CLAY with little silt and trace sand (very fine to very coarse) and gravel (small).		612	
5	21/24 88%	ss	4-7 12-22 N=19	9		8.45 Sh			10			610	
6	21/24 88%	ss	6-16 27-42 N=43	8		10.47 Sh			12	Yellowish brown (10YR5/8) with 30% Gray (10YR5/1) mottles, slightly moist, very hard, CLAY with silt and little sand (very fine to very coarse) trace gravel (small).		608	
7	12/24 50%	ss	36-55 50/4"	6					14	Yellowish brown (10YR5/8), slightly moist, loose, SAND (very fine to very coarse) and few gravel (small to medium).		606	
									14	Gray (10YR5/1), slightly moist, very hard, CLAY with silt and few gravel (small to medium).			
8	16/24 67%	ss	16-35 69 N=104	8					16	Gray (10YR5/1), moist, dense, SAND (very fine to coarse).		604	
									16	Gray (10YR5/1), slightly moist, very hard, CLAY with silt and few gravel (small to medium).			

End of Boring at 16.47 ft.

NOTE(S): Monitoring well TA32 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Company
Site: Coffeen Well Sealing & Assmt Well Install
Location: Coffeen, Illinois
Project: 14E0078A
DATES: Start: 6/2/2015
Finish: 6/2/2015
WEATHER: Sunny, calm, hi-60s

CONTRACTOR: Ramsey
Rig mfg/model: Diedrich D-50
Drilling Method: 3 1/4" HSA, split spoon sampler
FIELD STAFF: Driller: B. Williamson
Helper: D. Crump
Eng/Geo: S. Keim

BOREHOLE ID: TA33a
Well ID: n/a
Surface Elev: 622.5 ft. MSL
Completion: 30.0 ft. BGS
Station: 876,610.00N
 2,513,248.00E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	ss	2-3 5-5 N=8						0	Yellowish brown (10YR5/4), moist, stiff, clayey SILT, trace very fine-grained sand, roots.		622	
2A	20/24 83%	ss	3-4 4-5 N=8						2	Light brownish gray (10YR6/2) with 35% yellowish brown (10YR5/6) mottles, moist, stiff to very stiff, clayey SILT, trace very fine- to fine-grained sand.		620	
3A	23/24 96%	ss	5-6 6-9 N=12						4	Gray (10YR5/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, very stiff, clayey SILT, little fine- to medium-grained sand, trace coarse-grained sand.		618	
4A	24/24 100%	ss	10-10 10-10 N=20						6	Gray (10YR5/1) with 5-10% dark yellowish brown (10YR4/6) mottles, moist, very stiff, silty CLAY, little fine- to medium-grained sand, trace coarse-grained sand.		616	
5A	21/24 88%	ss	2-4 6-6 N=10						8	Dark yellowish brown (10YR4/6) with 3% gray (10YR6/1) mottles, moist, very stiff, silty CLAY, little fine- to coarse-grained sand, trace small gravel.		614	
6A	24/24 100%	ss	0-3 4-8 N=7						10	Gray (10YR6/1) with 30% dark yellowish brown (10YR4/6) mottles, moist, stiff, silty CLAY, little fine- to coarse-grained sand, trace small gravel.		612	
7A	24/24 100%	ss	15-27 32-41 N=59						12	Brown (10YR5/3) with 5% dark yellowish brown (10YR4/6) mottles, slightly moist, very stiff, very fine- to fine-grained sandy SILT, trace medium- to coarse-grained sand, trace small to large gravel, friable.		610	
7B									14	Dark yellowish brown (10YR4/6), very moist, dense, silty, very fine- to medium-grained SAND.		608	
8A	20/24 83%	ss	5-12 18-22 N=30						16	Brown (10YR5/3), slightly moist, hard, clayey SILT, few fine- to coarse-grained sand, trace small gravel.		606	
9A	16/24 67%	ss	23-44 50/5"						18			604	
10A	22/24 92%	ss	12-12 22-25 N=34						20	Dark gray (10YR4/1), slightly moist, hard, clayey SILT, few fine- to coarse-grained sand, trace small to large gravel.		604	

NOTE(S): Borehole sealed with high-solids bentonite grout.

FIELD BORING LOG



CLIENT: Illinois Power Generating Company
Site: Coffeen Well Sealing & Assmt Well Install
Location: Coffeen, Illinois
Project: 14E0078A
DATES: Start: 6/2/2015
Finish: 6/2/2015
WEATHER: Sunny, calm, hi-60s

CONTRACTOR: Ramsey
Rig mfg/model: Diedrich D-50
Drilling Method: 3 1/4" HSA, split spoon sampler
FIELD STAFF: Driller: B. Williamson
Helper: D. Crump
Eng/Geo: S. Keim

BOREHOLE ID: TA33a
Well ID: n/a
Surface Elev: 622.5 ft. MSL
Completion: 30.0 ft. BGS
Station: 876,610.00N
 2,513,248.00E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
							Quadrangle: Coffeen, IL	Township: East Fork	Section 3, Tier 7N; Range 3W	▼ = 13.80 - during drilling	▽ =
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%	ss	11-17 25-32 N=42				22	Dark gray (10YR4/1), slightly moist, hard, clayey SILT, few fine- to coarse-grained sand, trace small to large gravel. <i>[Continued from previous page]</i>		602	
12A	23/24 96%	ss	17-23 35-44 N=58			24	600				
13A	21/24 88%	ss	26-30 30-37 N=60			26	598				
14A	18/24 75%	ss	19-35 47-50 N=82			28	596				
15A	24/24 100%	ss	18-27 34-35 N=61			30	594				
							End of boring = 30.0 feet				

NOTE(S): Borehole sealed with high-solids bentonite grout.

FIELD BORING LOG



CLIENT: Illinois Power Generating Company
Site: Coffeen Well Sealing & Assmt Well Install
Location: Coffeen, Illinois
Project: 14E0078A
DATES: Start: 6/3/2015
Finish: 6/3/2015
WEATHER: Mostly cloudy, windy, mid-60s

CONTRACTOR: Ramsey
Rig mfg/model: Diedrich D-50
Drilling Method: 3 1/4" HSA, split spoon sampler
FIELD STAFF: Driller: B. Williamson
Helper: D. Crump
Eng/Geo: S. Keim

BOREHOLE ID: TA34
Well ID: TA34
Surface Elev: 624.10 ft. MSL
Completion: 16.10 ft. BGS
Station: 875,906.10N
 2,513,466.73E

SAMPLE		TESTING					TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Moisture (%)	Dry Den. (lb/ft ³)	Q _u (tsf) Q _p (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	SS	3-3 3-3 N=6						0	Gray (10YR5/1) with 10% dark yellowish brown (10YR3/6) mottles, moist, stiff, SILT, some clay, trace very fine-grained, roots.		624	
2A	17/24 71%	SS	2-2 4-6 N=6						2	Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/6) mottles, moist, very stiff, silty CLAY, trace fine-grained sand.		622	
3A	22/24 92%	SS	3-3 3-4 N=6						4	Light grayish brown (10YR6/2) with 15% yellowish brown (10YR5/6) mottles, slightly moist, very stiff, silty CLAY, trace fine-grained sand.		620	
4A	24/24 100%	SS	3-4 6-7 N=10						6	Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY, little fine-grained sand, trace medium- to coarse-grained sand.		618	
5A	24/24 100%	SS	2-2 5-5 N=7						8	Grayish brown (10YR5/2) with 25% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY, little fine-grained sand, trace medium- to coarse-grained sand, trace small gravel.		616	
6A	22/24 92%	SS	3-3 4-5 N=7						10	Grayish brown (10YR5/2) with 25% yellowish brown (10YR5/6) mottles, moist, stiff, silty CLAY, little fine-grained sand, trace medium- to coarse-grained sand, trace small gravel.		614	
7A	21/24 88%	SS	3-3 4-5 N=7						12	Dark yellowish brown (10YR4/6) with 15% light gray (10YR7/1) mottles, very moist, very soft, fine- to medium-grained sandy CLAY, trace coarse-grained sand.		612	
7B									14	Gray (10YR6/1), very moist, loose, clayey, silty, fine- to medium-grained SAND, trace coarse grained sand.		610	
8A									14	Brown (10YR5/3), very moist, very dense, silty, fine- to coarse-grained SAND and small to large GRAVEL.		610	
8B	24/24 100%	SS	18-28 25-32 N=53						14	Yellowish brown (10YR5/6), slightly moist, hard, clayey SILT, little fine- to coarse-grained sand, trace small gravel.		610	
8C									16	Gray (10YR6/1), slightly moist, hard, clayey SILT, few fine- to coarse-grained sand, trace small gravel.		608	
									16	Dark gray (10YR4/1), slightly moist, hard, clayey SILT, few fine- to coarse-grained sand, trace small gravel.		608	

End of boring = 16.1 feet

NOTE(S): TA34 installed in bore hole.



Site #: _____ County: _____ Well #: TR32

Site Name: Coffeen Power Station Borehole #: TR32

State _____
Plan Coordinate: X 2,513,605.0 Y 877,523.7 (or) Latitude: 39° 4' 2663.000" Longitude: 89° 24' 8.070"

Surveyed By: Kyle J. Nolan IL Registration #: 035-003919

Drilling Contractor: Ramsey Geotechnical Engineering, LLC Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow Stem Auger Drilling Fluid (Type): none

Logged By: Rhonald W. Hasenyager Date Started: 7/2/2019 Date Finished: 7/2/2019

Report Form Completed By: Rhonald W. Hasenyager Date: 7/3/2019

ANNULAR SPACE DETAILS

Elevations **Depths** (0.01 ft.)
(MSL)* (BGS)

Type of Surface Seal: Concrete

Type of Annular Sealant: Bentonite

Installation Method: Gravity

Setting Time: 30 min.

Type of Bentonite Seal -- Granular Pellet Slurry
(choose one)

Installation Method: _____

Setting Time: _____

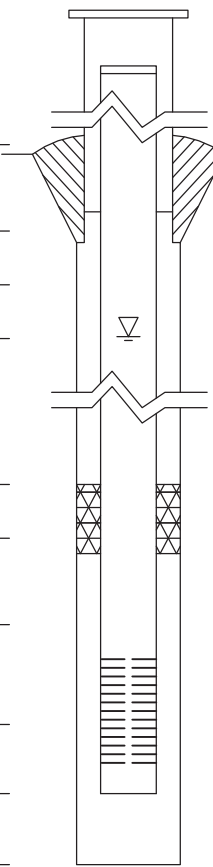
Type of Sand Pack: Quartz Sand

Grain Size: 10/20 (sieve size)

Installation Method: Gravity

Type of Backfill Material: n/a
(if applicable)

Installation Method: _____



621.97 -2.69 Top of Protective Casing

621.68 -2.40 Top of Riser Pipe

619.28 0.00 Ground Surface

616.28 3.00 Top of Annular Sealant

_____ _____ Static Water Level
(After Completion)

n/a n/a Top of Seal

609.77 9.51 Top of Sand Pack

608.28 11.00 Top of Screen

603.60 15.68 Bottom of Screen

603.11 16.17 Bottom of Well

603.11 16.17 Bottom of Borehole

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	13.40
Bottom of Screen to End Cap	(feet)	0.49
Screen Length (1st slot to last slot)	(feet)	4.68
Total Length of Casing	(feet)	18.57
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input type="checkbox"/> PVC	OTHER:

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/28/2021
 Finish: 2/3/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G275D
Well ID: G275
Surface Elev: 617.52 ft. MSL
Completion: 99.70 ft. BGS
Station: 874,285.30N
 2,516,366.50E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 10.90 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	SS	3-4 5-10 N=9						0	Dark yellowish brown (10YR4/4), moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand and small gravel, trace roots. [FILL]		616	
2A	22/24 92%	SS	3-4 6-9 N=10						2	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		614	
3A	19/24 79%	SS	2-4 6-8 N=10						4	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		612	
4A	23/24 96%	SS	2-5 5-6 N=10						6			610	
5A	24/24 100%	SS	2-3 5-6 N=8						8	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) and 5% yellowish red (5YR4/6) mottles, moist, very stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		608	
6A	24/24 100%	SS	0-1 2-2 N=3						10	Dark yellowish brown (10YR3/6), moist, stiff, SILT, with some very fine- to medium-grained sand, few clay and small gravel. Gray (10YR6/1) with 10% dark yellowish brown (10YR3/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, few small gravel.		606	
7A	18/24 75%	SH							12	Dark yellowish brown (10YR3/6), wet, loose, SILT, with some very fine- to fine-grained sand, few clay and small gravel.		604	
8A	14/14 100%	SS	26-43 50/2"						14	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, few clay and small gravel.		602	
9A	24/24 100%	SS	5-12 18-22 N=30						16	Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, few clay and small gravel.		600	
10A	24/24 100%	SS	4-11 13-20 N=24						18	Brown (10YR5/3) with 10% yellowish brown (10YR5/6) mottles, moist, hard, SILT, with some clay and very fine- to fine-grained sand, few small gravel.		598	

NOTE(S): G275 installed in adjacent blind drill borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/28/2021
Finish: 2/3/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G275D
Well ID: G275
Surface Elev: 617.52 ft. MSL
Completion: 99.70 ft. BGS
Station: 874,285.30N
 2,516,366.50E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%	SS	3-9 13-22 N=22						Brown (10YR5/3) with 10% yellowish brown (10YR5/6) mottles, moist, hard, SILT, with some clay and very fine- to fine-grained sand, few small gravel. <i>[Continued from previous page]</i>		596	
12A	24/24 100%	SS	7-14 20-24 N=34					22			594	Vertical fractures with oxidation from 22 to 24 ft, no oxidation below 24 ft.
13A	24/24 100%	SS	6-11 15-21 N=26					24			592	Occasional thin SILT and SAND lenses from 25.3 to 25.8 ft.
14A	18/24 75%	SS	4-8 12-10 N=20					26			590	
15A	24/24 100%	SS	5-7 13-17 N=20					28			588	Trace wood fragments below 28 ft.
16A	23/24 96%	SS	4-7 12-16 N=19					30	Dark gray (10YR4/1) with frequent dark yellowish brown (10YR3/6) oxidation along fractures, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand and small gravel.		586	
17A	2/24 8%	SS	4-10 13-17 N=23					32			584	
18A	21/24 88%	SH						34			582	
19A	24/24 100%	SS	3-6 10-14 N=16					36			580	
20A	4/24 17%	SS	3-8 11-17 N=19					38			578	

NOTE(S): G275 installed in adjacent blind drill borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/28/2021
Finish: 2/3/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G275D
Well ID: G275
Surface Elev: 617.52 ft. MSL
Completion: 99.70 ft. BGS
Station: 874,285.30N
 2,516,366.50E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	24/24 100%	SS	4-8 11-15 N=19					42	Dark gray (10YR4/1) with frequent dark yellowish brown (10YR3/6) oxidation along fractures, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand and small gravel. <i>[Continued from previous page]</i>		576	0.5" lignite fragment seam at 42.8 ft.
22A	24/24 100%	SS	7-8 11-17 N=19					44			574	
23A	24/24 100%	SS	5-8 13-40 N=21					46	Dark gray (10YR4/1), moist, hard, SILT, with some to little clay and very fine- to fine-grained sand, few small to medium gravel.		572	
24A	23/24 96%	SS	22-45 35-23 N=80					48	Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand and small gravel.		570	
25A	24/24 100%	SS	7-9 14-21 N=23					50	Very dark gray (10YR3/1), moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand and small gravel, trace wood fragments.		568	
26A	24/24 100%	SS	3-8 15-15 N=23					52	Dark gray (10YR3/1), moist, SILT, with some very fine-grained sand seams. Dark gray (10YR4/1) with 15% dark grayish brown (10YR4/2) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand, small gravel and wood fragments.		566	Methane deposit encountered at approx. 51 ft.
27A	17/24 71%	SS	12-27 13-15 N=40					54	Gray (10YR5/1), moist, dense, very fine- to medium-grained SAND, with some silt, trace small gravel.		564	
28A	24/24 100%	SS	4-9 11-13 N=20					56	Very dark grayish brown (10YR3/2), moist, hard, lean CLAY, with some silt, trace very fine-grained sand and organics.		562	
29A	24/24 100%	SS	5-9 13-12 N=22					58	Very dark grayish brown (10YR3/2) with 10% dark yellowish brown (10YR4/6) mottles, moist, hard, lean CLAY, with some silt, trace very fine-grained sand and organics.		560	
30A	24/24 100%	SS	3-4 7-14 N=11					60	Very dark grayish brown (10YR3/2), wet, SAND, with some silt. Gray (GLEY15/1) with 20% dark yellowish brown (10YR4/6) mottles, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand and small gravel.		558	

NOTE(S): G275 installed in adjacent blind drill borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/28/2021
Finish: 2/3/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G275D
Well ID: G275
Surface Elev: 617.52 ft. MSL
Completion: 99.70 ft. BGS
Station: 874,285.30N
 2,516,366.50E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
							Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ = 10.90 - During Drilling	▽ =	▽ =
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
31A	24/24 100%	SS	0-4 5-7 N=9				62	Gray (GLEY15/) with 30% dark yellowish brown (10YR4/6) mottles, moist, hard, lean CLAY, with some silt, few very fine-to fine-grained sand and small gravel.		556		
32A	24/24 100%	SS	4-6 8-11 N=14				64			554		
33A	24/24 100%	SH					66	Greenish gray (GLEY15/1) with 15% very dark gray (10YR3/1) mottles, moist, hard, lean CLAY, with some silt, few very fine-to fine-grained sand and small gravel.		552		
34A	24/24 100%	SS	5-10 22-41 N=32				68			550		
35A	24/24 100%	SS	12-24 33-45 N=57				70	Yellowish brown (10YR5/4) with occasional thin greenish gray (GLEY15/1) seams, moist, hard, lean CLAY, with some silt, few small gravel, trace very fine-grained sand.		548	Trace medium gravel at 70 ft.	
36A	23/24 96%	SS	6-14 25-30 N=39				72			546		
37A	24/24 100%	SS	8-18 24-32 N=42				74			544		
38A	24/24 100%	SS	7-16 25-29 N=41				76	Yellowish brown (10YR5/4) with 15% gray (10YR6/1) mottles, moist, hard, lean CLAY, with some silt, few small gravel, trace very fine-grained sand.		542		
39A	24/24 100%	SS	7-15 20-21 N=35				78			540		
40A	19/24 79%	SS	3-5 7-10 N=12				80	Greenish gray (GLEY15/1) with 5% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very		538		

NOTE(S): G275 installed in adjacent blind drill borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/28/2021
Finish: 2/3/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G275D
Well ID: G275
Surface Elev: 617.52 ft. MSL
Completion: 99.70 ft. BGS
Station: 874,285.30N
 2,516,366.50E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ = 10.90 - During Drilling	▽ =	▽ =
						Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks			
41A	22/24 92%	SS	1-5 7-11 N=12										
42A	24/24 100%	SS	4-14 19-20 N=33										
43A													
44A	8/24 33%	SS	6-20 22-23 N=42										
45A	24/24 100%	SS	7-8 16-17 N=24										
46A	24/24 100%	SS	5-13 16-21 N=29										
47A	24/24 100%	SS	4-8 15-9 N=23										
47B	24/24 100%	SS	5-6 8-10 N=14										
48A	24/24 100%	SS	2-4 7-8 N=11										
49A	24/24 100%	SS	2-6 7-11 N=13										
50A	18/20 90%	SS	3-15 28-50/2" N=43										

NOTE(S): G275 installed in adjacent blind drill borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler

BOREHOLE ID: G275D
Well ID: G275
Surface Elev: 617.52 ft. MSL
Completion: 99.70 ft. BGS
Station: 874,285.30N
 2,516,366.50E

DATES: Start: 1/28/2021
 Finish: 2/3/2021

FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

WEATHER: Clear, cold (20s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ = 10.90 - During Drilling	▽ =	▽ =
						Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks		

							fine-grained sand, trace small gravel.				
							End of boring = 99.7 feet				

NOTE(S): G275 installed in adjacent blind drill borehole.



FIELD BORING LOG

CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/25/2021
 Finish: 1/25/2021
WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G282D
Well ID: 282D
Surface Elev: ft. MSL
Completion: 60.00 ft. BGS
Station: N
 E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 18.80 - During Drilling ▽ = 55.90 - During Drilling ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	15/24 63%	SS	3-3 3-4 N=6						1	Gray (10YR6/1), wet, loose, GRAVEL, with some sand. [FILL]			
2A	17/24 71%	SS	3-4 4-5 N=8						2	Brown (10YR5/3), moist, stiff, lean CLAY, with some silt, trace very fine- to fine-grained sand. [FILL]			
3A	22/24 92%	SS	2-3 5-6 N=8						4	Yellowish brown (10YR5/4) with 10% gray (10YR6/1) mottles, moist, stiff, lean CLAY, with some silt, trace very fine- to fine-grained sand and small gravel. [FILL]			
4A	20/24 83%	SS	3-4 4-5 N=8						6	Grayish brown (10YR5/2), moist, stiff, lean CLAY, with some silt, trace small gravel.			
5A	22/24 92%	SS	2-3 5-7 N=8						8	Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/4) mottles, moist, stiff, lean CLAY, with some silt, trace small gravel.			
6A	20/24 83%	SS	3-4 4-7 N=8						10	Grayish brown (10YR5/2) with 10% yellowish brown (10YR5/4) and 5% gray (10YR5/1) mottles, moist, stiff, lean CLAY, with some silt, trace small gravel.			
7A	20/24 83%	SS	2-3 4-5 N=7						12	Grayish brown (10YR5/2) with 20% yellowish brown (10YR5/4) and 5% gray (10YR5/1) mottles, moist, stiff, lean CLAY, with some silt, trace small gravel.			
8A	20/24 83%	SS	1-2 3-4 N=5						14	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.			
9A	21/24 88%	SS	1-2 2-3 N=4						16	Dark gray (10YR4/1), moist, stiff, lean CLAY, with some silt.			
10A	24/24 100%	SS	0-1 1-0 N=2						18	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.			
10B	24/24 100%	SS	0-1 1-0 N=2						20	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.			
									20	Yellowish brown (10YR5/6) with 10% gray (10YR6/1) mottles, wet, very loose, SILT, with some very fine- to fine-grained sand, few small gravel, trace clay.			

NOTE(S): G282D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/25/2021
Finish: 1/25/2021
WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G282D
Well ID: 282D
Surface Elev: ft. MSL
Completion: 60.00 ft. BGS
Station: N
 E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 18.80 - During Drilling ▽ = 55.90 - During Drilling ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%	SS	3-7 11-16 N=18						22	Yellowish brown (10YR5/4) with 20% yellowish brown (10YR5/6) and 5% yellowish red (5YR4/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel.			
12A	24/24 100%	SS	7-12 17-24 N=29						24	Yellowish brown (10YR5/4), wet, fine- to medium-grained SAND.			
13A	24/24 100%	SS	9-15 22-22 N=37						26	Yellowish brown (10YR5/4) with 20% yellowish brown (10YR5/6) and 5% yellowish red (5YR4/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel, trace medium gravel.			
14A	22/24 92%	SS	8-17 16-22 N=33						28	Dark gray (10YR4/1), moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel, trace medium gravel.			
15A	21/24 88%	SS	5-11 15-19 N=26						30	Dark gray (10YR4/1), moist, SAND, little silt and clay.			
16A	22/22 100%	SS	5-25 33-50/4" N=58						32	Dark gray (10YR4/1), moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel, trace medium gravel.			
17A	22/24 92%	SS	7-10 15-20 N=25						34				
18A	24/24 100%	SS	4-8 10-16 N=18						36	Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, little small gravel.			
19A	24/24 100%	SS	5-8 13-15 N=21						38				
20A	21/24 88%	SS	2-4 8-11 N=12						40				

NOTE(S): G282D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/25/2021
 Finish: 1/25/2021
WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G282D
Well ID: 282D
Surface Elev: ft. MSL
Completion: 60.00 ft. BGS
Station: N
 E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
21A	24/24 100%	SS	4-8 11-14 N=19				42					
22A	22/24 92%	SS	3-7 8-12 N=15				44					
23A	24/24 100%	SS	3-6 9-13 N=15				46				Trace wood fragments below 45.7 ft.	
24A	24/24 100%	SS	4-6 9-12 N=15				48	Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, little small gravel. [Continued from previous page]				
25A	24/24 100%	SS	4-6 12-13 N=18				50				0.5" gravel seam at 48.5 ft.	
26A	24/24 100%	SS	2-7 9-13 N=16				52					
27A	24/24 100%	SS	4-7 11-14 N=18				54					
28A	24/24 100%	SS	6-12 9-18 N=21				56	Light yellowish brown (10YR6/5), moist, very fine- to medium-grained SAND, with some silt, little small to medium gravel.				
28B							56	Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, little small gravel, trace wood fragments.				
29A	24/24 100%	SS	6-10 11-11 N=21				58	Light yellowish brown (10YR6/5), wet, medium dense, very fine- to coarse-grained SAND, little small gravel, few silt.				
30A	24/24 100%	SS	4-5 8-9 N=13				60	Dark gray (10YR4/1) with 5% dark yellowish brown (10YR3/6) mottles, moist, stiff, lean CLAY, with some silt.				

NOTE(S): G282D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/18/2021
Finish: 1/18/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G283
Well ID: G283
Surface Elev: 608.30 ft. MSL
Completion: 18.00 ft. BGS
Station: 874,113.00N
 2,516,503.00E

WEATHER: Overcast, cold (30s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ = 14.00 - During Drilling	▽ =	▽ =
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	19/24 79%	SS	1-2 4-3 N=6								608		
1B								Very dark grayish brown (10YR3/2), moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace roots.					
2								Yellowish brown (10YR5/4), moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, few small gravel.					
2A	23/24 96%	SS	8-15 24-31 N=39					Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, little small gravel, trace clay.			606		
3A	20/23 87%	SS	5-27 35-50/5" N=62					Yellowish brown (10YR5/6) with 30% gray (10YR6/1) mottles and frequent yellowish red (5YR4/6) oxidation along fracture faces, moist, hard SILT, with some very fine- to fine-grained sand, little small gravel, trace clay.			604		
4A	18/18 100%	SS	15-31 50/5.5"					Gray (10YR5/1), moist, hard, SILT, with some very fine- to fine-grained sand, little small gravel, trace clay.			602		
5A	19/22 86%	SS	9-29 38-50/4" N=67					Gray (10YR5/1), moist, hard, SILT, with some to little very fine- to fine-grained sand, little small gravel, trace clay.			600		
6A	21/24 88%	SS	12-24 25-30 N=49					Gray (10YR5/1), moist, hard, SILT, with some to little very fine- to fine-grained sand, little to few small gravel, trace clay.			598		
7A	24/24 100%	SS	14-22 32-38 N=54					Gray (10YR5/1), moist, hard, SILT, with some to little very fine- to fine-grained sand, little to few small gravel, trace clay.			596		
8A	22/24 92%	SS	6-17 12-12 N=29					Dark gray (10YR4/1), wet, medium dense, very fine- to medium-grained SAND, with some silt, few small gravel, occasional thin clayey seams.			594		0.5" very fine-grained SAND seam at 13.6 ft.
9A											592		
9B	24/24 100%	SS	0-8 16-19 N=24					Gray (10YR5/1), moist, very stiff, SILT, with some to little very fine-grained sand.					
								18	End of boring = 18.0 feet				

NOTE(S): G283 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/20/2021
 Finish: 1/20/2021
WEATHER: Clear, cool (40s)

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G284
Well ID: G284
Surface Elev: 615.33 ft. MSL
Completion: 14.00 ft. BGS
Station: 874,423.60N
 2,516,922.90E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	SS	0-2 2-5 N=4					0	Very dark gray (10YR3/1), moist, very stiff, lean CLAY, some silt, few roots.			
2A	22/24 92%	SS	1-2 3-4 N=5					2	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, trace very fine-grained sand.		614	
3A	20/24 83%	SS	0-3 4-4 N=7					4	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand.		612	
4A	22/24 92%	SS	2-3 4-6 N=7					6	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) moist, very stiff, lean CLAY, with some very fine-grained sand and silt, few small gravel.		610	
5A	24/24 100%	SS	2-3 4-4 N=7					8	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) moist, very stiff, lean CLAY, with some very fine-grained sand and silt, few small gravel.		608	
5B								10	Yellowish brown (10YR5/4), moist, loose, SILT, with some very fine- to fine-grained sand, few clay and small gravel.		606	
6A	20/24 83%	SS	1-2 3-3 N=5					12	Yellowish brown (10YR5/4) with 20% yellowish brown (10YR5/6) mottles, moist, loose, SILT, with some very fine- to fine-grained sand, few clay and small gravel.		604	
7A	23/24 96%	SS	7-22 27-48 N=49					14	Light yellowish brown (10YR6/4) with 10% yellowish brown (10YR5/6) mottles, moist, hard, SILT, trace very fine-grained sand.		602	

End of boring = 14.0 feet

NOTE(S): G284 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/19/2021
 Finish: 1/19/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G285
Well ID: G285
Surface Elev: 610.54 ft. MSL
Completion: 26.00 ft. BGS
Station: 874,795.00N
 2,516,680.40E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
							Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ = Dry During Drilling	▽ =	▽ =
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	20/24 83%	SS	0-2 3-3 N=5				0	Yellowish brown (10YR5/4), moist, stiff, lean CLAY, with some silt, few fine-grained sand.		610		
2A	18/24 75%	SS	1-3 4-6 N=7				2	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, few very fine-grained sand, trace small gravel.		608		
3A	22/24 92%	SS	2-5 8-9 N=13				4	Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, very stiff, lean CLAY, with some silt, few very fine-grained sand, trace small gravel.		606		
4A	22/24 92%	SS	3-8 12-15 N=20				6	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, hard, lean CLAY, with some silt, few very fine-grained sand and small gravel.		604		
5A	21/24 88%	SS	4-11 16-18 N=27				8	Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, hard, lean CLAY, with some silt, few very fine-grained sand and small gravel.		602		
6A	23/24 96%	SS	3-8 13-17 N=21				10			600		
7A	23/24 96%	SS	4-9 12-17 N=21				12	Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/6) mottles, hard, lean CLAY, with some silt, few very fine-grained sand and small gravel.		598		
8A	23/24 96%	SS	5-14 21-33 N=35				14	Grayish brown (10YR5/2) with 15% yellowish brown (10YR5/6) mottles and 15% dark reddish brown (5YR3/4) oxidation along vertical fracture, hard, lean CLAY, with some silt, few very fine-grained sand and small gravel.		596		
9A	23/24 96%	SS	5-7 10-15 N=17				16			594		
10A	4/24 17%	SS	4-6 12-14 N=18				18	Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine-grained sand and small gravel.		592	Sampler pushing gravel in Run 10.	
							20					

NOTE(S): G285 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler

BOREHOLE ID: G285
Well ID: G285
Surface Elev: 610.54 ft. MSL
Completion: 26.00 ft. BGS
Station: 874,795.00N
 2,516,680.40E

DATES: Start: 1/19/2021
 Finish: 1/19/2021

FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

WEATHER: Clear, cold (20s)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	24/24 100%	SS	4-7 9-11 N=16									590	
12A	3/24 13%	SS	4-7 8-13 N=15						22	Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine-grained sand and small gravel. [Continued from previous page]		588	Sampler pushing gravel in Run 12.
13A	23/24 96%	SS	3-5 7-11 N=12					24			586		
								26	End of boring = 26.0 feet				

NOTE(S): G285 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/19/2021
 Finish: 1/19/2021
WEATHER: Clear, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G286
Well ID: G286
Surface Elev: 609.97 ft. MSL
Completion: 10.00 ft. BGS
Station: 875,072.20N
 2,516,561.80E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	24/24 100%	SS	0-0 2-2 N=2				0	Dark grayish brown (10YR4/2), moist, medium stiff, SILT, with some clay, trace very fine-grained sand and small gravel.			
2A							2	Brown (10YR5/3), moist, stiff, SILT, with some clay, few very fine- to fine-grained sand, trace small gravel.		608	
2B	19/24 79%	SS	0-2 5-7 N=7				4	Grayish brown (10YR5/2), moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		606	
3A	19/24 79%	SS	4-7 5-4 N=12				6	Dark grayish brown (10YR4/2) with 20% grayish brown (10YR5/2) mottles, moist, very stiff, lean CLAY, with some very fine- to medium-grained sand, little silt, few small gravel.		604	
4A	20/24 83%	SS	2-3 3-4 N=6				8	Gray (10YR5/1) with 10% dark yellowish brown (10YR3/6) mottles, wet, loose, SAND, with some silt and small to medium gravel, trace clay.		602	
4B											
5A	20/24 83%	SS	2-5 7-15 N=12				10	Gray (10YR5/1), moist, hard, SILT, with some very fine- to fine-grained sand, few small gravel.		600	

End of boring = 10.0 feet

NOTE(S): G286 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler

BOREHOLE ID: G287
Well ID: G287
Surface Elev: 614.34 ft. MSL
Completion: 11.80 ft. BGS
Station: 875,442.80N
 2,516,415.50E

DATES: Start: 1/18/2021
 Finish: 1/18/2021

FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

WEATHER: Overcast, cold (30s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	12/24 50%	SS	0-1 2-3 N=3					0	Gray (10YR5/1), moist, stiff, lean CLAY, with some silt, trace roots.		614	
2A	20/24 83%	SS	1-2 4-6 N=6					2	Gray (10YR6/1), moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.		612	
3A	20/24 83%	SS	3-4 5-9 N=9					4	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small to medium gravel.		610	
4A	23/24 96%	SS	2-3 3-4 N=6					6			608	
4B								7				
5A	20/24 83%	SS	2-3 7-8 N=10					8	Yellowish brown (10YR5/6) with 20% gray (10YR5/1), wet, loose, SAND, with some silt, few small gravel, trace clay.		606	
5B								9				
6A	17/22 77%	SS	3-7 15-50/4" N=22					10	Gray (10YR6/1) with 30% yellowish brown (10YR5/6), moist, stiff, SILT, with little very fine- to fine-grained sand, few clay and small gravel.		604	
								11.8	Gray (10YR6/1) with 40% yellowish brown (10YR5/6), moist, stiff, SILT, with some very fine- to fine-grained sand, few clay and little small gravel.			

End of boring = 11.8 feet

NOTE(S): G287 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/21/2021
 Finish: 1/21/2021
WEATHER: Clear, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G288
Well ID: G288
Surface Elev: 617.08 ft. MSL
Completion: 14.00 ft. BGS
Station: 875,279.60N
 2,517,071.40E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 11.20 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	18/24 75%	SS	1-2 2-3 N=4						1	Brown (10YR4/3), moist, medium stiff, SILT, with some clay, trace roots.		616	
2A	20/24 83%	SS	1-1 2-3 N=3						2			614	
2B									4	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace small gravel.			
3A	20/24 83%	SS	2-3 4-4 N=7						6	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.		612	
4A	24/24 100%	SS	1-2 4-4 N=6						8	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel.		610	
5A	24/24 100%	SS	0-2 2-3 N=4						10	Light yellowish brown (10YR6/4), moist, stiff, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel.		606	
6A									12	Gray (10YR6/1), wet, medium stiff, SILT, with some very fine- to fine-grained sand, few small to medium gravel, occasional thin sand seams.		604	
6B	21/24 88%	SS	1-2 3-5 N=5						14	Gray (10YR6/1) with 25% dark yellowish brown (10YR4/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, few clay, trace small gravel.			
7A	22/24 92%	SS	5-12 19-19 N=31										

End of boring = 14.0 feet

NOTE(S): G288 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/26/2021
 Finish: 1/27/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G289
Well ID: n/a
Surface Elev: ft. MSL
Completion: 60.0 ft. BGS
Station: N
 E

WEATHER: Overcast, cold (30s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 18.60 - During Drilling ▽ = 8.00 - 1/27/2021 8am ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	SS	2-2 4-4 N=6							0	Brown (10YR4/3) with 10% gray (10YR6/1) and 10% yellowish brown (10YR5/6) mottles, moist, stiff, SILT, with some clay, trace very fine- to fine-grained sand.			
2A	19/24 79%	SS	3-5 6-8 N=11							2				
3A	19/24 79%	SS	3-4 4-10 N=8							4	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.			
4A	21/24 88%	SS	2-2 4-6 N=6							6	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.			
5A	18/24 75%	SH								8	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/6) and 5% very pale brown (10YR7/3) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.			
6A	22/24 92%	SS	3-4 4-6 N=8							10	Dark grayish brown (10YR4/3), moist, stiff, lean CLAY, with some silt.			
6B										12	Gray (10YR6/1) with 20% dark grayish brown (10YR4/2) and 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.			
7A	19/24 79%	SS	3-4 6-6 N=10							14	Gray (10YR6/1) with 30% dark yellowish brown (10YR4/6) and 20% dark grayish brown (10YR4/2) mottles, moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.			
8A	22/24 92%	SS	3-4 5-6 N=9							16	Very dark gray (10YR3/1), moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.			
9A	24/24 100%	SS	2-3 4-4 N=7							18	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, medium stiff, lean CLAY, with some silt and very fine- to fine-grained sand, few small gravel.			
10A	24/24 100%	SS	2-2 1-2 N=3							20	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist to wet, SILT, with some very fine- to fine-grained sand and clay, few small gravel.			
10B										20	Strong brown (7.5YR5/8), moist to wet, SILT, with some very			

NOTE(S):

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/26/2021
 Finish: 1/27/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G289
Well ID: n/a
Surface Elev: ft. MSL
Completion: 60.0 ft. BGS
Station: N
 E

WEATHER: Overcast, cold (30s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 18.60 - During Drilling ▽ = 8.00 - 1/27/2021 8am ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
11A	22/24 92%	SS	2-2 4-7 N=6						22	fine- to fine-grained sand and clay, few small gravel. Strong brown (7.5YR5/8) with 20% gray (10YR6/1) mottles, moist to wet, SILT, with some very fine- to fine-grained sand and clay, few small gravel.			1" wet, fine- to coarse-grained SAND, with little small gravel.
12A	21/24 88%	SS	6-20 17-26 N=37						24	Gray (10YR6/1), moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel. Gray (10YR6/1), wet, SILT, with some sand, little small to medium gravel. Gray (10YR6/1), moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel.			
13B	23/24 96%	SS	4-23 32-41 N=55						26	Gray (10YR6/1), wet, loose, very fine- to medium-grained SAND, with some silt, little small to medium gravel.			
14A	22/24 92%	SS	23-25 24-27 N=49						28				Trace large gravel from 27 to 28 ft.
15A	23/23 100%	SS	14-22 32-50/5" N=54						30	Gray (10YR6/1), moist, hard, SILT, with some very fine- to fine-grained sand, little clay, few small gravel.			
16A	14/17 82%	SS	4-6 50/5"						32				Trace large gravel from 31 to 31.5 ft.
17A	11/11 100%	SS	27-50/5"						34				
18A	19/24 79%	SS	20-17 15-19 N=32						36				
19A	24/24 100%	SS	7-14 16-16 N=30						38	Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine- to fine- grained sand, few small gravel.			
20A	24/24 100%	SS	7-12 15-18 N=27						40				

NOTE(S):

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/26/2021
Finish: 1/27/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G289
Well ID: n/a
Surface Elev: ft. MSL
Completion: 60.0 ft. BGS
Station: N
 E

WEATHER: Overcast, cold (30s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 18.60 - During Drilling ▽ = 8.00 - 1/27/2021 8am ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	24/24 100%	SH											
22A	24/24 100%	SS	5-13 20-22 N=33						42				Trace wood fragments below 42 ft.
23A	22/24 92%	SS	3-7 11-14 N=18						44				
24A	24/24 100%	SS	4-9 10-14 N=19				Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, few very fine- to fine- grained sand, few small gravel. [Continued from previous page]		46				
25A	23/24 96%	SS	2-5 7-10 N=12						48				
26A	24/24 100%	SS	4-10 12-18 N=22						50				
27A	24/24 100%	SS	5-9 11-15 N=20						52				
28A	24/24 100%	SS	3-5 7-8 N=12				Gray (10YR5/1) with 15% dark brown (10YR3/6) mottles, moist, stiff, lean CLAY, with some silt.		54				
29A	24/24 100%	SS	3-5 9-9 N=14						56				
30A	24/24 100%	SS	0-4 8-9 N=12				Gray (10YR5/1) with 20% dark brown (10YR3/6) and 10% dark gray (10YR4/1) mottles, moist, stiff, lean CLAY, with some silt.		58				Few fine- to medium-grained sand below 59 ft.
									60				

NOTE(S):

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/26/2021
Finish: 1/27/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G289
Well ID: n/a
Surface Elev: ft. MSL
Completion: 60.0 ft. BGS
Station: N
 E

WEATHER: Overcast, cold (30s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL	▼ = 18.60 - During Drilling	▼ = 8.00 - 1/27/2021 8am	▼ =
						Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks

TOPOGRAPHIC MAP INFORMATION:
 Quadrangle: Coffeen, IL
 Township: East Fork Township
 Section 11, Tier 7N; Range 3W

WATER LEVEL INFORMATION:
 ▼ = 18.60 - During Drilling
 ▼ = 8.00 - 1/27/2021 8am
 ▼ =

End of boring = 60.0 feet

NOTE(S):

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/9/2021
 Finish: 2/9/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G307D
Well ID: G307D
Surface Elev: 622.51 ft. MSL
Completion: 60.00 ft. BGS
Station: 871,397.20N
 2,515,560.30E

WEATHER: Overcast, very cold (10s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ = 12.80 - During Drilling	▽ =	▽ =
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	18/24 75%	SS	3-2 1-2 N=3										
2A	16/24 67%	SS	2-3 5-6 N=8										
3A	24/24 100%	SS	2-3 4-5 N=7										
4A	24/24 100%	SS	3-4 6-7 N=10										
5A	24/24 100%	SH											
6A	24/24 100%	SS	1-2 4-4 N=6										
7A	20/24 83%	SS	4-7 3-8 N=10										
7B													
8A	21/24 88%	SS	0-6 10-13 N=16										
9A	11/11 100%	SS	29-50/5"										
10A	7/9 78%	SS	45-50/3"										

NOTE(S): G307D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler

BOREHOLE ID: G307D
Well ID: G307D
Surface Elev: 622.51 ft. MSL
Completion: 60.00 ft. BGS
Station: 871,397.20N
 2,515,560.30E

DATES: Start: 2/9/2021
 Finish: 2/9/2021

FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

WEATHER: Overcast, very cold (10s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	8/8 100%	SS	35-50/2"						▽ = 12.80 - During Drilling ▽ = ▽ =		602	Little sand below 20 ft.
12A	8/9 89%	SS	30-50/3"								600	
13A	17/17 100%	SS	23-39 50/5"								598	
14A	9/9 100%	SS	38-50/3"								596	
15A	23/23 100%	SS	31-44 34-50/5" N=78				Dark gray (10YR4/1) with 10% yellowish brown (10YR5/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, few clay and small gravel, trace medium gravel. [Continued from previous page]				594	1" dark gray (10YR4/1), fine- to medium-grained sand, with some silt, little small gravel.
16A	15/15 100%	SS	3-44 50/3"								592	
17A	10/10 100%	SS	34-50/4"								590	
18A	11/11 100%	SS	31-50/5"								588	
19A	17/17 100%	SS	28-44 50/5"								586	
20A	24/24 100%	SS	14-19 13-24 N=32				Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, little to few very fine- to fine-grained sand, few small gravel.				584	

NOTE(S): G307D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/9/2021
 Finish: 2/9/2021
WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G307D
Well ID: G307D
Surface Elev: 622.51 ft. MSL
Completion: 60.00 ft. BGS
Station: 871,397.20N
 2,515,560.30E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 12.80 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	24/24 100%	SS	7-8 12-15 N=20									582	Trace medium gravel below 40 ft.
22A	20/24 83%	SH							42			580	
23A	24/24 100%	SS	6-8 10-12 N=18						44			578	
24A	24/24 100%	SS	5-6 10-12 N=16				Dark gray (10YR4/1), moist, hard, lean CLAY, with some silt, little to few very fine- to fine-grained sand, few small gravel. [Continued from previous page]		46			576	
25A	24/24 100%	SS	3-6 11-11 N=17						48			574	Trace wood fragments below 48.5 ft.
26A	24/24 100%	SS	3-7 9-10 N=16						50			572	
27A	16/24 67%	SS	4-7 11-9 N=18						52			570	Gravel plugged shoe in Run 27. Large lignite fragment at 52.4 ft.
28A	24/24 100%	SS	3-6 10-9 N=16				Greenish gray (GLEYS/6) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace small gravel.		54			568	
29A	24/24 100%	SS	4-6 8-12 N=14				Yellowish brown (10YR5/6) with 30% greenish gray (GLEYS/1) and 5% very dark grayish brown (10YR3/2) mottles, moist, stiff, lean CLAY, with some silt, trace small gravel.		56			566	
30A	24/24 100%	SS	2-5 8-10 N=13				Yellowish brown (10YR5/6) with 20% greenish gray (GLEYS/1) and 5% very dark grayish brown (10YR3/2) mottles, moist, stiff, lean CLAY, with some silt, trace small gravel.		58			564	Large wood fragment
									60				

NOTE(S): G307D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/9/2021
Finish: 2/9/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G307D
Well ID: G307D
Surface Elev: 622.51 ft. MSL
Completion: 60.00 ft. BGS
Station: 871,397.20N
 2,515,560.30E

WEATHER: Overcast, very cold (10s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W		▽ = 12.80 - During Drilling ▽ = ▽ =		
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks

End of boring = 60.0 feet at 59.8 ft.

NOTE(S): G307D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/13/2021
Finish: 1/13/2021
WEATHER: Clear, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G308
Well ID: G308
Surface Elev: 621.59 ft. MSL
Completion: 15.80 ft. BGS
Station: 871,454.70N
 2,515,101.40E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 13.00 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	16/24 67%	SS	2-2 2-3 N=4						0	Grayish brown (10YR5/2), moist, stiff, lean CLAY, with some silt, trace roots.		620	
2A	19/24 79%	SS	0-2 3-5 N=5						2	Brown (10YR5/3) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace roots.		618	
3A	17/24 71%	SS	2-4 5-6 N=9						4			616	
4A	23/24 96%	SS	1-4 5-6 N=9						6	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.		614	
5A	22/24 92%	SS	2-4 6-7 N=10						8	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, trace very fine-grained sand and small to medium gravel.		612	
6A	21/24 88%	SS	2-3 4-5 N=7						10	Gray (10YR5/1) with 40% yellowish brown (10YR5/6) mottles, moist, stiff to very stiff, lean CLAY, with some silt, little to trace very fine-grained sand, trace small to medium gravel.		610	
7A	24/24 100%	SS	2-3 4-6 N=7						12			608	
8A	19/22 86%	SS	4-12 33-50/4" N=45						14	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, wet, stiff, lean CLAY, with some very fine- to fine-grained sand, little silt, trace small gravel.		606	
End of boring = 15.8 feet													

NOTE(S): G308 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/12/2021
 Finish: 1/12/2021
WEATHER: Clear, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G309
Well ID: G309
Surface Elev: 622.77 ft. MSL
Completion: 18.00 ft. BGS
Station: 871,865.80N
 2,515,067.10E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 16.00 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	13/24 54%	SS	2-4 3-3 N=7						0	Brown (10YR4/3), moist, very stiff, lean CLAY, with some silt, few to little sand, trace gravel and ash. [FILL]		622	
2A	8/24 33%	SS	3-2 2-1 N=4						2	Black (10YR2/1), moist, very loose, ASH. [FILL]		620	
3A	17/24 71%	SS	3-2 2-4 N=4						4	Yellowish brown (10YR5/6) with 30% gray (10YR5/1) mottles, moist, stiff, lean CLAY, with some silt.		618	
4A	23/24 96%	SS	2-3 3-3 N=6						6	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.		616	
5A	23/24 96%	SS	2-3 4-5 N=7						8	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.		614	
6A	22/24 92%	SS	1-2 3-5 N=5						10	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.		612	
7A	21/24 88%	SS	2-3 6-5 N=9						12	Yellowish brown (10YR5/6) with 20% gray (10YR5/1) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.		610	
8A	13/24 54%	SS	2-2 3-4 N=5						14	Light yellowish brown (10YR6/4) with 10% yellowish brown (10YR5/6) mottles, moist, medium stiff, lean CLAY, with some fine-grained sand, little small gravel, trace medium to large gravel.		608	
9A	8/24 33%	SS	5-12 24-22 N=36						16	Pale brown (10YR6/3), wet, hard, SILT, little very fine-grained sand, few clay and small to medium gravel.		606	
									18	Brownish yellow (10YR6/6), wet, loose, very fine- to fine-grained SAND, trace clay and small to medium gravel.			
									18	Pale brown (10YR6/3), wet, hard, SILT, little very fine-grained sand, few clay and small to medium gravel.			

Gravel plugged shoe in Run 8.

Sampler shoe lost in hole at 18 ft.

End of boring = 18.0 feet

NOTE(S): G309 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/15/2021
Finish: 1/15/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G310
Well ID: G310
Surface Elev: 619.89 ft. MSL
Completion: 15.90 ft. BGS
Station: 872,239.40N
 2,515,159.40E

WEATHER: Clear, light snow, cold (30s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value	RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	21/24 88%	SS	2-7 4-6 N=11							Dark gray (10YR4/1), moist, very stiff, lean CLAY, with some silt, trace roots.			
1B									2	Gray (10YR6/1) with 15% yellowish brown (10YR5/6) mottles, moist, very stiff, SILT, few clay, trace roots.		618	
2A	18/24 75%	SS	2-4 4-6 N=8						4	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt.		616	
3A	23/24 96%	SS	2-3 4-5 N=7						6	Gray (10YR6/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt.		614	
4A	24/24 100%	SS	2-3 5-6 N=8						8	Dark gray (10YR4/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand and small gravel.		612	
5A	24/24 100%	SS	2-3 4-5 N=7						10	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, few small gravel, trace very fine- to fine-grained sand.		610	
6A	24/24 100%	SS	2-3 4-4 N=7						12			608	
7A	24/24 100%	SS	0-1 2-2 N=3						14	Gray (10YR5/1), moist, medium stiff, SILT, with some clay, little very fine- to fine-grained sand, few small gravel.		606	
8A	17/23 74%	SS	8-16 26-50/5" N=42							Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, wet, hard, SILT, with some very fine-grained sand, few small gravel, trace clay.			
										Yellowish brown (10YR5/6), wet, SAND, some silt, few gravel.			
										Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, wet, hard, SILT, with some very fine-grained sand, few small gravel, trace clay.			
										Yellowish brown (10YR5/6), wet, SAND, some silt, few gravel seam from 15.3 to 15.5 ft.			
										Yellowish brown (10YR5/6) with 10% gray (10YR5/1) mottles, wet, hard, SILT, with some very fine-grained sand, few small gravel, trace clay.		604	

End of boring = 15.9 feet

NOTE(S): G310 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/5/2021
 Finish: 2/5/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model:
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G311
Well ID: G311
Surface Elev: 618.32 ft. MSL
Completion: 14.40 ft. BGS
Station: 872,238.70N
 2,515,881.80E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:				
							Quadrangle: Coffeen, IL Township: East Fork Section 11, Tier 7N; Range 3W				
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
							0			618	
							2			616	
							4			614	
							6			612	
							8			610	
							10			608	
							12			606	
							14			604	

NOTE(S): G311 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/5/2021
 Finish: 2/5/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 3.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G311D
Well ID: G311D
Surface Elev: 618.39 ft. MSL
Completion: 60.00 ft. BGS
Station: 872,238.70N
 2,515,881.80E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 11.20 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	13/24 54%	SS	0-4 4-4 N=8							0	Yellowish brown (10YR5/6), moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		618	
2A	24/24 100%	SS	3-4 5-6 N=9							2	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) and 5% very dark grayish brown (10YR3/2) mottles, moist, stiff, lean CLAY, with some silt, trace very fine- to fine-grained sand, and small gravel.		616	
3A	24/24 100%	SS	2-4 5-7 N=9							4	Very dark grayish brown (10YR3/2) with 10% gray (10YR5/1) mottles, moist, stiff, lean CLAY, with some silt, trace very fine- to fine-grained sand, and small gravel.		614	
4A	24/24 100%	SH								6			612	
5A	18/24 75%	SS	1-2 4-5 N=6							8	Gray (10YR5/1) with 15% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, few small gravel.		610	
6A	24/24 100%	SS	1-3 3-4 N=6							10			608	
7A	24/24 100%	SS	2-6 12-13 N=18							12	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist to wet, medium stiff, lean CLAY, with some very fine- to fine-grained sand, little silt, few small gravel. Yellowish brown (10YR5/6), moist to wet, medium stiff, lean CLAY, with some very fine- to fine-grained sand, little silt, few small gravel.		606	
8A	24/24 100%	SS	2-6 12-13 N=18							14	Gray (10YR5/1), moist to wet, medium stiff, SILT, with some very fine- to fine-grained sand, few clay, trace small gravel.		604	1" wet, SAND at 13.5 ft.
9A	24/24 100%	SS	6-15 18-22 N=33							16	Yellowish brown (10YR5/6), moist, hard, SILT, with some clay, some to little sand, few small gravel.		602	Vertical fracture with very fine- to fine-grained sand from 16.9 to 18 ft.
10A	24/24 100%	SS	6-13 16-22 N=29							18	Dark grayish brown (10YR4/2) with frequent yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		600	
										20				

NOTE(S): G311D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/5/2021
 Finish: 2/5/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 3.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G311D
Well ID: G311D
Surface Elev: 618.39 ft. MSL
Completion: 60.00 ft. BGS
Station: 872,238.70N
 2,515,881.80E

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	24/24 100%	SS	5-15 20-26 N=35									
12A	12/24 50%	SS	2-3 14-17 N=17									No oxidation below 22 ft.
13A	24/24 100%	SS	6-11 14-20 N=25									Trace medium gravel below 24 ft.
14A	24/24 100%	SS	4-8 11-16 N=19									
15A	24/24 100%	SH										
16A	24/24 100%	SS	0-3 5-8 N=8					Dark gray (10YR4/1) with frequent yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.				
17A	24/24 100%	SS	2-4 6-8 N=10									
18A	24/24 100%	SS	2-5 7-7 N=12									Gravel plugged shoe in Run 18.
19A	13/24 54%	SS	2-7 8-11 N=15									Trace large gravel from 35 to 36 ft.
20A	24/24 100%	SS	2-6 10-8 N=16									Trace lignite and wood fragments below 36 ft.

NOTE(S): G311D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/5/2021
 Finish: 2/5/2021
WEATHER: Clear, cold (20s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 3.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G311D
Well ID: G311D
Surface Elev: 618.39 ft. MSL
Completion: 60.00 ft. BGS
Station: 872,238.70N
 2,515,881.80E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
21A	16/24 67%	SS	3-3 6-8 N=9					42	Dark gray (10YR4/1) with frequent yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel. [Continued from previous page]		578	
22B	24/24 100%	SS	3-4 7-8 N=11					44	Greenish gray (GLE Y15/1) with 20% dark reddish brown (10YR3/2) mottles, moist, medium stiff, lean CLAY, with some silt.		576	
23A	24/24 100%	SS	1-3 5-7 N=8					46			574	
24A	20/24 83%	SS	3-4 10-8 N=14					48	Dark grayish brown (10YR4/1) with frequent yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small to large gravel.		572	
25A	24/24 100%	SS	3-5 8-13 N=13					50			568	
26A	24/24 100%	SS	2-5 10-12 N=15					52			566	
27A	24/24 100%	SS	2-6 10-14 N=16					54			564	Trace small gravel below 54.3 ft.
28A	24/24 100%	SS	4-7 7-11 N=14					56	Dark gray (10YR4/1) with 20% greenish gray (GLE Y15/1) and 5% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.		562	
29A	24/24 100%	SS	2-5 9-11 N=14					58	Greenish gray (GLE Y16/1) with 40% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.		560	0.5" small to medium GRAVEL.
30A	24/24 100%	SS	3-7 10-13 N=17					60	Yellowish brown (10YR5/6) with 30% greenish gray (GLE Y16/1) mottles, moist, very stiff, lean CLAY, with some silt, trace very fine-grained sand and small gravel.			

NOTE(S): G311D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 3.25" HSA w/SS sampler

BOREHOLE ID: G311D
Well ID: G311D
Surface Elev: 618.39 ft. MSL
Completion: 60.00 ft. BGS
Station: 872,238.70N
 2,515,881.80E

DATES: Start: 2/5/2021
 Finish: 2/5/2021

FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

WEATHER: Clear, cold (20s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ = 11.20 - During Drilling	▽ =	▽ =
						Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks		

End of boring = 60.0 feet

NOTE(S): G311D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/4/2021
 Finish: 2/4/2021
WEATHER: Rain, cold (30s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G312
Well ID: G312
Surface Elev: 616.92 ft. MSL
Completion: 15.25 ft. BGS
Station: 872,260.90N
 2,516,557.40E

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 12.70 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	14/24 58%	SS	0-2 3-2 N=5							Brown (10YR5/3) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.		616	
2A	17/24 71%	SS	2-1 2-2 N=3							Yellowish brown (10YR5/6), moist, stiff to very stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.		614	
3A	18/24 75%	SS	1-3 5-6 N=8							Yellowish brown (10YR5/6) with 10% very dark grayish brown (10YR3/2) mottles, moist, stiff to very stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, trace small gravel.		612	
4A	17/24 71%	SS	2-3 3-4 N=6							Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		610	
5A	20/24 83%	SS	2-4 5-7 N=9							Gray (10YR5/1) with 20% yellowish brown (10YR5/6) and 5% very dark grayish brown (10YR3/2) mottles, moist, very stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		608	
6A	20/24 83%	SS	1-3 4-4 N=7							Gray (10YR5/1), moist, soft SILT, some clay, trace roots. Gray (10YR5/1) with 20% yellowish brown (10YR5/6) and 5% very dark grayish brown (10YR3/2) mottles, moist, very stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		606	
7A	24/24 100%	SS	0-3 2-5 N=5							Yellowish brown (10YR5/6) with 15% gray (10YR5/1) mottles, moist, very stiff, lean CLAY, with some silt and very fine- to fine-grained sand.		604	
7B										Gray (10YR6/1) with 15% yellowish brown (10YR5/6), moist, very stiff, lean CLAY, with some silt and very fine- to fine-grained sand.			
8A	15/15 100%	SS	4-17 50/3"							Gray (10YR5/1), wet, loose, SILT, with some very fine- to fine-grained sand, few clay, trace small gravel.		602	
8B										Yellowish brown (10YR5/6) with 5% very dark grayish brown (10YR3/2), wet, loose, SILT, with some very fine- to fine-grained sand, few clay and small gravel.			

End of boring = 15.25 feet

NOTE(S): G312 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/14/2021
Finish: 1/14/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G313
Well ID: G313
Surface Elev: 611.51 ft. MSL
Completion: 12.00 ft. BGS
Station: 871,976.80N
 2,516,803.70E

WEATHER: Overcast, cool (40s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 9.80 - During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24	SS	0-2							Dark yellowish brown (10YR4/4), wet, stiff, lean CLAY, with some silt, few roots, trace small gravel.			
1B	71%		3-3 N=5							Gray (10YR6/1) with 15% yellowish brown (10YR5/6) mottles, moist, very stiff, SILT, with little clay, few roots.		610	
2A	13/24	SS	0-2 2-3 N=4							Dark gray (10YR4/1) with 5% black (10YR2/1) mottles, moist, stiff, lean CLAY, with some silt.		608	
3A	19/24	SS	2-3 5-5 N=8							Dark gray (10YR4/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine- to fine-grained sand and small gravel.		606	
4A	22/24	SS	2-3 4-5 N=7							Very dark gray (10YR3/1), moist, stiff, lean CLAY, with some silt, little very fine- to fine-grained sand and small gravel.		604	
5A	22/24	SS	2-5 9-12 N=14							Yellowish brown (10YR5/6) with 10% very dark gray (10YR3/1) mottles, moist, stiff SILT, with some very fine- to fine-grained sand and clay, few small gravel.		602	
6A	21/24	SS	9-12 21-22 N=33							Yellowish brown (10YR5/4), wet, medium dense, very fine- to medium-grained SAND, with some silt, few small gravel.			
6B	88%									Dark grayish brown (10YR4/2) with 10% yellowish brown (10YR5/6) mottles, moist, hard, SILT, with some very fine- to fine-grained sand, few clay and small gravel, trace medium gravel.		600	
End of boring = 12.0 feet													

NOTE(S): G313 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A

CONTRACTOR: Holcomb Foundation Engineering Co.
Rig mfg/model:
Drilling Method: 4.25" HSA w/SS sampler

BOREHOLE ID: G314
Well ID: G314
Surface Elev: 611.11 ft. MSL
Completion: 20.05 ft. BGS
Station: 871,630.20N
 2,516,852.10E

DATES: **Start:** 2/26/2021
Finish: 2/26/2021

FIELD STAFF: **Driller:** Steve
Helper: Jeff

WEATHER: Clear, mild (40s)

Eng/Geo: C. Colin Winter

SAMPLE			TESTING				Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type					

NOTE(S): G314 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/10/2021
 Finish: 2/12/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G314D
Well ID: G314D
Surface Elev: 610.87 ft. MSL
Completion: 100.30 ft. BGS
Station: 871,642.00N
 2,516,853.90E

WEATHER: Overcast, very cold (10s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	4/24 17%	SS	13-7 4-5 N=11					0	Moist, dense, small to large GRAVEL. [FILL]		610	
	1/24 4%	SS	3-4 5-5 N=9					2	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, few very fine- to fine-grained sand, trace small gravel.		608	
3A	24/24 100%	SS	1-3 4-5 N=7					4				606
4A	24/24 100%	SS	2-5 15-17 N=20					6	Yellowish brown (10YR5/6) with occasional yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel.		604	
5A	24/24 100%	SS	4-10 12-16 N=22					8				602
6A	24/24 100%	SS	4-5 14-18 N=19					10	Yellowish brown (10YR5/6) with 10% very dark grayish brown (10YR3/2) mottles and occasional yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel.		600	
7A	24/24 100%	SS	3-11 14-18 N=25					12				598
8A	24/24 100%	SS	3-9 13-18 N=22					14	Yellowish brown (10YR5/6) with 10% very dark grayish brown (10YR3/2) and 10% dark yellowish brown (10YR4/6) mottles and occasional yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel.		596	
9A	24/24 100%	SS	1-7 9-13 N=16					16				594
								18	Dark grayish brown (10YR4/2) with 15% yellowish brown (10YR5/6) mottles and occasional yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel.		592	
								20				

NOTE(S): G314D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/10/2021
 Finish: 2/12/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G314D
Well ID: G314D
Surface Elev: 610.87 ft. MSL
Completion: 100.30 ft. BGS
Station: 871,642.00N
 2,516,853.90E

WEATHER: Overcast, very cold (10s)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
10A	24/24 100%	SS	1-5 10-13 N=15						Dark grayish brown (10YR4/2) with 15% yellowish brown (10YR5/6) and 5% dark yellowish brown (10YR4/6) mottles and occasional yellowish red (5YR4/6) oxidation along fractures, moist, hard, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel. <i>[Continued from previous page]</i>		590	1" wet, SAND, with some silt at 20 ft.	
11A	24/24 100%	SS	1-6 9-12 N=15					22				588	Frequent oxidation below 21.7 ft.
12A	24/24 100%	SS	4-7 10-13 N=17					24				586	
13A	24/24 100%	SS	3-7 10-13 N=17					26				584	
14A	24/24 100%	SS	4-6 10-10 N=16					28				582	1 ft vertical fracture with yellowish red (5YR4/6) oxidation at 28.3 ft.
15A	24/24 100%	SS	3-9 10-12 N=19					30				580	
16A	24/24 100%	SS	2-3 7-8 N=10					32				578	
17A	24/24 100%	SS	2-4 7-9 N=11					34				576	Trace roots below 34 ft.
18A	21/24 88%	SH						36		Gray (GLE15/), moist, stiff, lean CLAY, with some silt.		574	
								38		Dark grayish brown (10YR4/2), moist, hard, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel and roots.		572	
								40					

NOTE(S): G314D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/10/2021
 Finish: 2/12/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G314D
Well ID: G314D
Surface Elev: 610.87 ft. MSL
Completion: 100.30 ft. BGS
Station: 871,642.00N
 2,516,853.90E

WEATHER: Overcast, very cold (10s)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
								Quadrangle: Coffeen, IL	Township: East Fork Township	Section 11, Tier 7N; Range 3W	▽ =	During Drilling	▽ =
								Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
19A	24/24 100%	SS	7-12 N=12					42			570	Trace medium gravel below 41.3 ft.	
20A	24/24 100%	SS	4-8 10-14 N=18					44	Dark grayish brown (10YR4/2), moist, hard, lean CLAY, with some silt and very fine- to fine-grained sand, trace small gravel and roots. <i>[Continued from previous page]</i>		568		
21A	24/24 100%	SS	3-9 13-16 N=22					46	Gray (10YR5/1), wet, dense, SAND, with some silt, few small gravel.		566		
22A	24/24 100%	SS	12-31 14-11 N=45					48	Gray (GLEYS/15) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand.		564	1" SILT, with some sand at 48.2 ft.	
22B								50			562		
23A	24/24 100%	SS	3-4 6-9 N=10					52	Yellowish brown (10YR5/6) with 20% gray (GLEYS/15) mottles, moist, stiff, lean CLAY, with some silt, trace very fine-grained sand.		560		
24A	24/24 100%	SS	5-11 14-17 N=25					54	Yellowish brown (10YR5/6), moist, hard, lean CLAY, with some silt, little very fine- to fine-grained sand and small gravel.		558		
25A	24/24 100%	SS	4-6 11-15 N=17					56	Yellowish brown (10YR5/6) with 20% gray (GLEYS/15) mottles, moist, hard, lean CLAY, with some silt, little very fine- to fine-grained sand and small gravel.		556		
26A	20/24 83%	SS	4-5 18-30 N=23					58	Yellowish brown (10YR5/6) with 20% gray (GLEYS/15) mottles, moist, hard, lean CLAY, with some silt, little very fine- to fine-grained sand and small gravel.		554		
27A	20/24 83%	SS	9-22 33-33 N=55					60	Yellowish brown (10YR5/6) with 10% gray (GLEYS/15) and 5% dark yellowish brown (10YR4/6) mottles, moist, hard, lean CLAY, with some silt, little very fine- to fine-grained sand and small gravel.		552		
28A	24/24 100%	SS	7-19 29-43										

NOTE(S): G314D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/10/2021
 Finish: 2/12/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G314D
Well ID: G314D
Surface Elev: 610.87 ft. MSL
Completion: 100.30 ft. BGS
Station: 871,642.00N
 2,516,853.90E

WEATHER: Overcast, very cold (10s)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
			N=48									
29A	20/24 83%	SS	13-20 30-38 N=50					62			550	
30A	24/24 100%	SS	8-15 30-34 N=45					64	Yellowish brown (10YR5/6) with 10% gray (GLE15/) and 5% dark yellowish brown (10YR4/6) mottles, moist, hard, lean CLAY, with some silt, little very fine- to fine-grained sand and small gravel. <i>[Continued from previous page]</i>		548	
31A	24/24 100%	SS	5-17 28-29 N=45					66			546	
32A	24/24 100%	SS	7-18 29-43 N=47					68	Yellowish brown (10YR5/6) with 10% gray (GLE15/), 5% dark yellowish brown (10YR4/6) and 5% very dark grayish brown (10YR3/2) mottles, moist, hard, lean CLAY, with some silt, little very fine- to fine-grained sand and small gravel.		544	
33A	24/24 100%	SS	5-14 32-26 N=46					70	Dark grayish brown (10YR4/2) with 5% gray (GLE15/), 5% yellowish brown (10YR5/6), and 5% light gray (10YR7/1) mottles, moist, hard, lean CLAY, with some silt, little sand and gravel.		542	
34A	24/24 100%	SS	8-17 26-38 N=43					72	Yellowish brown (10YR5/6) with 5% gray (GLE15/), 5% dark grayish brown (10YR4/2), and 5% light gray (10YR7/1) mottles.		540	
35A	24/24 100%	SS	5-12 24-24 N=36					74	Dark grayish brown (10YR4/2) with 5% gray (GLE15/), 5% yellowish brown (10YR5/6), and 5% light gray (10YR7/1) mottles.		538	
36A	24/24 100%	SS	14-24 20-33 N=44					76	Dark grayish brown (10YR4/2) with 5% gray (GLE15/), 5% yellowish brown (10YR5/6), and 5% light gray (10YR7/1) mottles.		536	
37A	24/24 100%	SS	6-14 17-31 N=31					78	Dark grayish brown (10YR5/2) with 20% reddish brown (5YR4/4) and 5% light gray (10YR7/1) mottles, moist, hard, lean CLAY, with some silt, trace very fine- to fine-grained sand and small gravel.		534	
								80	Dark gray (10YR4/1) with 5% light gray (10YR7/1) mottles, moist, hard, lean CLAY, with some silt, trace very fine- to fine-grained sand and small gravel.		532	

NOTE(S): G314D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/10/2021
 Finish: 2/12/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G314D
Well ID: G314D
Surface Elev: 610.87 ft. MSL
Completion: 100.30 ft. BGS
Station: 871,642.00N
 2,516,853.90E

WEATHER: Overcast, very cold (10s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
38A	24/24 100%	ss	7-17 21-29 N=38					82	Dark gray (10YR4/1) with 5% light gray (10YR7/1) mottles, moist, hard, lean CLAY, with some silt, trace very fine- to fine-grained sand and small gravel. [Continued from previous page]		530	Trace roots at 82 ft.
39A	24/24 100%	ss	8-24 26-27 N=50					84	Very dark gray (10YR3/1), moist, hard, lean CLAY, with some silt.		528	
40A	24/24 100%	ss	5-9 10-13 N=19					86	Dark gray (10YR4/1) with 5% black (10YR2/1) mottles, moist, hard, lean CLAY, with some silt.		526	
41A	24/24 100%	ss	6-10 25-33 N=35					88	Dark greenish gray (GLE Y14/1) with 5% yellowish brown (10YR5/6) mottles, moist, hard, lean CLAY, with some silt.		524	Few very fine- to fine-grained sand below 88 ft.
42A	11/11 100%	ss	3-50/5"					90			522	
43A	5/5 100%	ss	50/5"					92	Light reddish brown (2.5YR6/3) with 10% gray (GLE Y15/), dry, hard, SILT, with few clay and very fine-grained sand.		520	
44A	5/5 100%	ss	50/5"					94			518	
45A	8/8 100%	ss	49-50/2"					96			516	
46A	11/11 100%	ss	25-50/5"					98	Light reddish brown (2.5YR6/3) with 10% gray (GLE Y15/) and 5% dark yellowish brown (10YR4/6) mottles, dry, hard, SILT, with few clay and very fine-grained sand.		514	
								100			512	

NOTE(S): G314D installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/13/2021
Finish: 1/13/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G315
Well ID: G315
Surface Elev: 620.94 ft. MSL
Completion: 15.90 ft. BGS
Station: 871,385.00N
 2,516,086.60E

WEATHER: Overcast, cool (40s)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
1A	15/24 63%	SS	0-2 3-2 N=5				0	Light gray (10YR7/1) with 20% brown (10YR4/3) mottles, moist, stiff, lean CLAY, with some silt, few roots.		620		
2A							2					
	9/24 38%	SS	1-1 2-2 N=3				4	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, lean CLAY, with some silt.		618		
3A	22/24 92%	SS	2-3 3-5 N=6				6			616		
4A	24/24 100%	SS	1-2 2-4 N=4				8	Gray (10YR5/1) with 20% yellowish brown (10YR5/6) mottles, lean CLAY, with some silt, few to trace small gravel.		614		
5A	23/24 96%	SS	2-2 3-4 N=5				10	Gray (10YR5/1) with 30% yellowish brown (10YR5/6) mottles, lean CLAY, with some silt, few to trace small gravel.		612		
6A	19/24 79%	SS	0-2 4-5 N=6				12	Gray (10YR5/1) with 30% yellowish brown (10YR6/1) mottles, lean CLAY, with some silt, few to trace small gravel.		610	Gray (10YR6/1), SILT seam.	
7A							14					
7B	24/24 100%	SS	1-2 3-3 N=5				14	Gray (10YR6/1) with 40% yellowish brown (10YR5/6) mottles, wet, medium stiff, lean CLAY, with some very fine- to fine-grained sand, little silt, few small to medium gravel.		608		
8A	21/23 91%	SS	8-16 32-50/5" N=48				15.9	Yellowish brown (10YR5/6), wet, very stiff, SILT, with some very fine- to fine-grained sand, little small to medium gravel, few clay.		606	Gravel plugged shoe in Run 8.	

End of boring = 15.9 feet

NOTE(S): G315 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/14/2021
Finish: 1/14/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G316
Well ID: G316
Surface Elev: 599.64 ft. MSL
Completion: 15.75 ft. BGS
Station: 871,643.10N
 2,517,211.60E

WEATHER: Overcast, cold (30s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 8.60 - During Drilling ▽ = 13.80 - During Drilling ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	15/24 63%	SS	10-9 5-4 N=14							Brown (10YR5/3), moist, medium dense, small to large GRAVEL, with some clay and fine to coarse grained sand. [FILL]			
2A	10/24 42%	SS	1-3 4-6 N=7						2	Gray (10YR5/1), moist, hard, lean CLAY, with some silt, few small gravel and very fine- to fine-grained sand.		598	
3A	14/24 58%	SS	2-2 3-3 N=5						4	Gray (10YR5/1) with 10% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, few small gravel and very fine- to fine-grained sand.		596	
4A	10/24 42%	SS	0-1 1-2 N=2						6	Gray (10YR5/1) with 30% black (10YR2/1) mottles, moist, stiff, lean CLAY, with some silt, few very fine-grained sand, organics and wood fragments.		594	
5A									8			592	
5B	22/24 92%	SS	1-1 1-2 N=2						10	Very dark gray (10YR3/1), wet, medium stiff to stiff, SILT, with some clay and very fine- to fine-grained sand.		590	
6A	20/24 83%	SS	1-2 4-7 N=6						12	Brown (10YR4/3), moist, very stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, few small gravel.		588	
7A	22/24 92%	SS	0-2 3-3 N=5						14	Dark gray (10YR4/1) with 20% yellowish brown (10YR5/6) mottles, moist, medium stiff to stiff, lean CLAY, with some silt, little very fine- to fine-grained sand, few small gravel.		586	
8A	21/17 124%	SS	2-2 26-50/3" N=28							Brown (10YR4/3), wet, loose, very fine- to fine-grained SAND, with some silt, trace clay.			
8B										Yellowish brown (10YR5/6), moist, hard, SILT, with some very fine- to fine-grained sand, trace small gravel.		584	

End of boring = 15.75 feet

NOTE(S): G316 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/13/2021
 Finish: 1/14/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: G317
Well ID: G317
Surface Elev: 638.85 ft. MSL
Completion: 36.00 ft. BGS
Station: 871,234.20N
 2,517,087.40E

WEATHER: Overcast, cold (30s to 40s)

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:				
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf)	Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	17/24 71%	SS	2-2 5-3 N=7					0			638	
2A	12/24 50%	SS	3-5 4-4 N=9					2	Light yellowish brown (10YR6/4) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, little fine-grained sand and small to medium gravel, trace to no roots.		636	
3A	15/24 63%	SS	1-2 3-4 N=5					4			634	
4A	11/24 46%	SS	3-5 5-4 N=10					6	Gray (10YR6/1) with 20% yellowish brown (10YR5/6) mottles, moist, stiff, lean CLAY, with some silt, few fine-grained sand and small gravel.		632	
5A	12/24 50%	SS	3-4 4-4 N=8					8	Brownish yellow (10YR6/8) with 20% gray (10YR6/1) mottles, moist, stiff, SILT, with little fine-grained sand and small gravel.		630	
6A	4/24 17%	SS	2-2 3-4 N=5					10			628	Wood fragment plugged shoe in Run 6.
7A	14/24 58%	SS	1-3 4-3 N=7					12	Gray (10YR5/1) with 30% very dark gray (20YR3/1) mottles, moist, very stiff, lean CLAY, with some silt, trace lignite fragments and organics.		626	Gravel plugged shoe in Run 7.
7B								14	Gray (10YR5/1) with 10% yellowish brown (10YR5/4) mottles, moist, stiff, SILT, with some fine-grained sand, little small gravel.		624	
8A	20/24 83%	SS	2-2 3-4 N=5					16	Gray (10YR5/1) with 10% yellowish brown (10YR5/4) and 10% very dark gray (10YR3/1) mottles, moist, very stiff, lean CLAY, with some silt, few fine-grained sand and small gravel.		622	
9A	17/24 71%	SS	1-2 3-4 N=5					18	Gray (10YR5/1) with 10% very dark gray (10YR3/1) mottles, moist, medium stiff, lean CLAY, with some silt, little small gravel, trace lignite fragments and organics.		620	Thin wood fragment seam at 19.4 ft.
10A	16/24 67%	SS	0-2 5-5 N=7					20				

NOTE(S): G317 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 1/13/2021
 Finish: 1/14/2021

CONTRACTOR: Roberts
Rig mfg/model: GeoProbe 8040DT
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
Eng/Geo: C. Colin Winter

BOREHOLE ID: G317
Well ID: G317
Surface Elev: 638.85 ft. MSL
Completion: 36.00 ft. BGS
Station: 871,234.20N
 2,517,087.40E

WEATHER: Overcast, cold (30s to 40s)

SAMPLE			TESTING				TOPOGRAPHIC MAP INFORMATION:			WATER LEVEL INFORMATION:		
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks	
11A	16/24 67%	SS	3-5 6-11 N=11				18	Gray (10YR5/1) with 10% very dark gray (10YR3/1) mottles, moist, medium stiff, lean CLAY, with some silt, little small gravel, trace lignite fragments and organics. [Continued from previous page] Gray (10YR5/1) moist, SILT, with some clay.		618		
12A	21/24 88%	SS	4-4 7-9 N=11				22			616		
13A	18/24 75%	SS	3-5 7-11 N=12				24	Gray (10YR5/1) with 10% very dark gray (10YR3/1) mottles, moist, medium stiff to very stiff, lean CLAY, with some silt, little small gravel, few wood fragments and organics.		614		
14A	20/24 83%	SS	2-5 7-9 N=12				26	Yellowish brown (10YR5/6) with 20% gray (10YR6/1) mottles, moist, very stiff, lean CLAY, with some silt, few to no fine-grained sand, trace to no small gravel.		612		
15A	19/24 79%	SS	2-4 6-8 N=10				28			610		
16A	24/24 100%	SS	1-5 7-9 N=12				30	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) mottles, moist, very stiff, lean CLAY, with some silt, few to no fine-grained sand, trace to no small gravel.		608		
17A	24/24 100%	SS	2-4 6-7 N=10				32			606		
18A	24/24 100%	SS	0-1 14-34 N=15				34	Gray (10YR6/1) with 30% yellowish brown (10YR5/6) and 10% black (10YR2/1) mottles, moist, very stiff, lean CLAY, with some silt, few to no fine-grained sand, trace to no small gravel.		604		
18B	24/24 100%	SS					34	Yellowish brown (10YR5/4) with 10% gray (10YR6/1) mottles, wet, loose, very fine- to fine-grained SAND, with some clay, little silt, few small gravel.		604	Sampler full in Run 18.	
							36	Yellowish brown (10YR5/6) with 10% gray (10YR6/1) mottles, moist, very stiff, SILT, with some very fine- to fine-grained sand, little small to medium gravel, trace clay.				

End of boring = 36.0 feet

NOTE(S): G317 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/8/2021
 Finish: 2/8/2021
WEATHER: Overcast, very cold (10s)

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: XPW01
Well ID: XPW01
Surface Elev: 631.85 ft. MSL
Completion: 14.00 ft. BGS
Station: 871,638.70N
 2,515,366.30E

SAMPLE			TESTING			TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:					
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 5.00 - Approx. During Drilling ▽ = ▽ =	Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	SS	11-20 21-13 N=41						2	Black (10YR2/1), moist, BOTTOM ASH, fine- to coarse-grained, cohesionless. [FILL]		630	
2A	24/24 100%	SS	3-6 8-6 N=14						4	Black (10YR2/1), moist, BOTTOM ASH, very fine- to fine-grained, cohesive. [FILL]		628	
3A	0/24 0%	SH							6			626	
4A	24/24 100%	SS	1-5 7-8 N=12						8			624	
5A	19/24 79%	SS	1-1 1-6 N=2						10	Black (10YR2/1), wet, BOTTOM ASH, fine- to coarse-grained. [FILL]		622	
6A	15/24 63%	SS	4-5 4-4 N=9						12			620	
7A	18/24 75%	SS	1-3 5-6 N=8						14	Gray (10YR5/1) with 20% yellowish brown (10YR5/6), moist, stiff, lean CLAY, some silt, trace very fine grained sand.		618	

End of boring = 14.0 feet

NOTE(S): XPW01 installed in borehole.

FIELD BORING LOG



CLIENT: Illinois Power Generating Co.
Site: Coffeen Part 845 Groundwater
Location: Coffeen, Illinois
Project: 20E0111A
DATES: Start: 2/8/2021
 Finish: 2/8/2021

CONTRACTOR: Roberts
Rig mfg/model: CME-75 Track Rig
Drilling Method: 4.25" HSA w/SS sampler
FIELD STAFF: Driller: Matt
 Helper: Corey
 Eng/Geo: C. Colin Winter

BOREHOLE ID: XPW02
Well ID: XPW02
Surface Elev: 636.64 ft. MSL
Completion: 18.00 ft. BGS
Station: 871,987.10N
 2,515,627.30E

WEATHER: Overcast, very cold (10s)

SAMPLE		TESTING				TOPOGRAPHIC MAP INFORMATION:		WATER LEVEL INFORMATION:			
Number	Recov / Total (in) % Recovery	Type	Blows / 6 in N - Value RQD	Water Content (%)	Dry Density (lb/ft ³)	Qu (tsf) Qp (tsf) Failure Type	Quadrangle: Coffeen, IL Township: East Fork Township Section 11, Tier 7N; Range 3W	▽ = 6.00 - During Drilling ▽ = ▽ =			
							Depth ft. BGS	Lithologic Description	Borehole Detail	Elevation ft. MSL	Remarks
1A	19/24 79%	SS	5-5 10-15 N=15							636	
2A	21/24 88%	SS	5-14 17-18 N=31				Black (10YR2/1), moist, BOTTOM ASH, fine- to coarse-grained, cohesionless. [FILL]			634	
3A	24/24 100%	SS	4-13 15-14 N=28							632	
	24/24 100%	SS	4-9 13-15 N=22							630	
5A	22/24 92%	SS	3-4 7-12 N=11							628	
6A	23/24 96%	SS	1-9 14-14 N=23				Black (10YR2/1), wet, BOTTOM ASH, fine- to coarse-grained, cohesionless. [FILL]			626	
7A	24/24 100%	SS	5-7 7-7 N=14							624	
8A	21/24 88%	SS	4-4 5-4 N=9							622	
9A	24/24 100%	SS	0-1 1-2 N=2							620	
9B							Dark gray (10YR4/1), moist, very soft, SILT, some clay.				
							18	End of boring = 18.0 feet			

NOTE(S): XPW02 installed in borehole.

WELL CONSTRUCTION LOGS



Site #: _____ County: Montgomery Well #: G45D
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: G405D
State _____
Plane Coordinate: X 2,515,322.2 Y 873,998.0 (or) Latitude: 39° 3' 51.657" Longitude: -89° 23' 46.612"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Bulldog Drilling, Inc. Driller: J. Dittmaier
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 8/16/2016 Date Finished: 8/17/2016
Report Form Completed By: Suzanna L. Keim Date: 8/24/2016

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (624.16, -3.22), Top of Riser Pipe (623.81, -2.87), Ground Surface (620.94, 0.00), Top of Annular Sealant (618.94, 2.00), Static Water Level (After Completion), Top of Seal (590.59, 30.35), Top of Sand Pack (589.62, 31.32), Top of Screen (589.06, 31.88), Bottom of Screen (579.42, 41.52), Bottom of Well (579.02, 41.92), Bottom of Borehole (578.94, 42.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (34.75 feet), Bottom of Screen to End Cap (0.40 feet), Screen Length (1st slot to last slot) (9.64 feet), Total Length of Casing (44.79 feet), Screen Slot Size ** (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G46D
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: G406D
State _____
Plane Coordinate: X 2,514,697.8 Y 872,519.7 (or) Latitude: 39° 3' 37.098" Longitude: -89° 23' 54.687"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Bulldog Drilling, Inc. Driller: J. Dittmaier
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 8/19/2016 Date Finished: 8/19/2016
Report Form Completed By: Suzanna L. Keim Date: 8/24/2016

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (625.53, -3.62), Top of Riser Pipe (625.24, -3.33), Ground Surface (621.91, 0.00), Top of Annular Sealant (619.91, 2.00), Static Water Level (After Completion), Top of Seal (582.34, 39.57), Top of Sand Pack (581.21, 40.70), Top of Screen (580.30, 41.61), Bottom of Screen (570.65, 51.26), Bottom of Well (570.26, 51.65), Bottom of Borehole (569.91, 52.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (44.94 feet), Bottom of Screen to End Cap (0.39 feet), Screen Length (1st slot to last slot) (9.65 feet), Total Length of Casing (54.98 feet), Screen Slot Size ** (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G101

Site Name: CCB Management Facility Borehole #: G101

State _____
Plane Coordinate: X 2,514,214.3 Y 876,551.8 (or) Latitude: 39° 4' 17.000" Longitude: -89° 24' 0.400"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

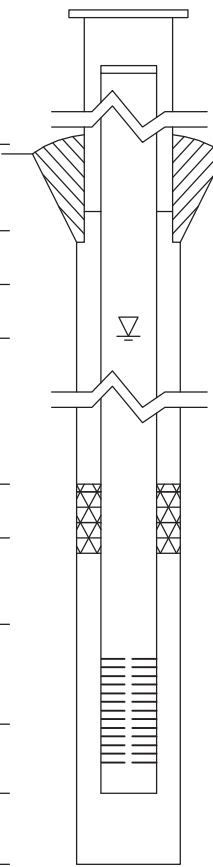
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna Simpson Date Started: 2/2/2010 Date Finished: 2/2/2010

Report Form Completed By: Suzanna Simpson Date: 2/4/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>627.89</u>	<u>-2.62</u>	Top of Protective Casing
	<u>627.60</u>	<u>-2.33</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>625.27</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>622.27</u>	<u>3.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>617.96</u>	<u>7.31</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>614.27</u>	<u>11.00</u>	Top of Seal
Setting Time: <u>20 min</u>	<u>612.14</u>	<u>13.13</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>609.59</u>	<u>15.68</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)	<u>604.95</u>	<u>20.32</u>	Bottom of Screen
Installation Method: <u>Gravity</u>	<u>604.38</u>	<u>20.89</u>	Bottom of Well
Type of Backfill Material: <u>Quartz sand</u> (if applicable)			
Installation Method: <u>Gravity</u>	<u>603.35</u>	<u>21.92</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	18.01
Bottom of Screen to End Cap	(feet)	0.57
Screen Length (1st slot to last slot)	(feet)	4.64
Total Length of Casing	(feet)	23.22
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G102 (MW3S)

Site Name: CCB Management Facility Borehole #: SB-03a

State _____
Plane Coordinate: X 2,514,531.5 Y 876,554.8 (or) Latitude: _____ Longitude: _____

Surveyed By: Darren E. Forgy IL Registration #: 035-003637

Drilling Contractor: Testing Service Corp. Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

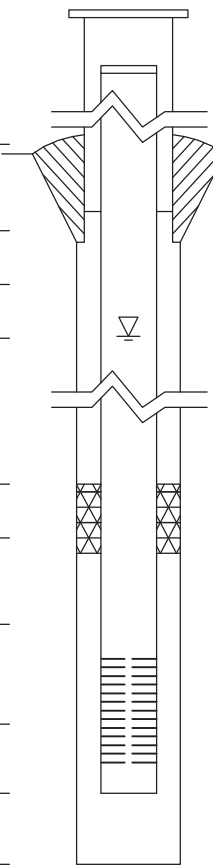
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water

Logged By: Testing Services Corp. Date Started: 4/28/2006 Date Finished: 4/28/2006

Report Form Completed By: Rhonald W. Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>629.45</u>	<u>-3.75</u>	Top of Protective Casing
	<u>628.96</u>	<u>-3.26</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>625.70</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>Bentonite chips</u>	<u>623.70</u>	<u>2.00</u>	Top of Annular Sealant
Installation Method: <u>Gravity</u>			
Setting Time: <u>+24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="radio"/> Granular <input type="radio"/> Pellet <input type="radio"/> Slurry (choose one)	<u>618.67</u>	<u>7.03</u>	Static Water Level (After Completion) 6/1/2006
Installation Method: <u>Gravity</u>	<u>623.70</u>	<u>2.00</u>	Top of Seal
Setting Time: <u>+24 hr.</u>	<u>616.20</u>	<u>9.50</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>#5</u> (sieve size)	<u>613.68</u>	<u>12.02</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>608.92</u>	<u>16.78</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>608.55</u>	<u>17.15</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>608.55</u>	<u>17.15</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	15.28
Bottom of Screen to End Cap	(feet)	0.37
Screen Length (1st slot to last slot)	(feet)	4.76
Total Length of Casing	(feet)	20.41
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="radio"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="radio"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="radio"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G103

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G103

State- Plant
Plane Coordinate: X 2,514,501.2 Y 876,199.5 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

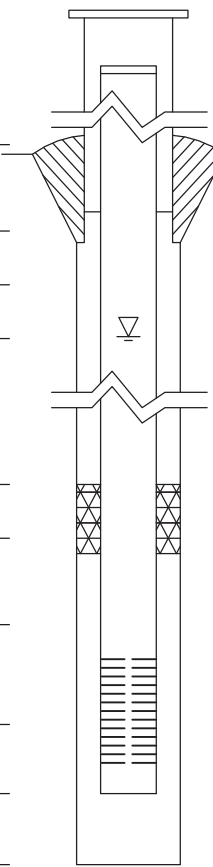
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/15/2010 Date Finished: 2/15/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>634.07</u>	<u>-3.08</u>	Top of Protective Casing
	<u>633.80</u>	<u>-2.81</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.99</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>624.93</u>	<u>6.06</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>614.00</u>	<u>16.99</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>618.51</u>	<u>12.48</u>	Top of Seal
Setting Time: <u>8 min</u>	<u>617.35</u>	<u>13.64</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>615.11</u>	<u>15.88</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>610.32</u>	<u>20.67</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>609.90</u>	<u>21.09</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>609.90</u>	<u>21.09</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	18.69
Bottom of Screen to End Cap	(feet)	0.42
Screen Length (1st slot to last slot)	(feet)	4.79
Total Length of Casing	(feet)	23.90
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:



Site #: _____ County: Montgomery Well #: G104

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G104

State- Plant
Plane Coordinate: X 2,514,505.0 Y 875,849.3 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

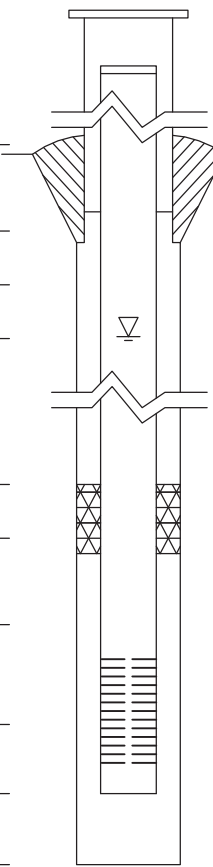
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/15/2010 Date Finished: 2/15/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.29</u>	<u>-3.17</u>	Top of Protective Casing
	<u>632.94</u>	<u>-2.82</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.12</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>624.92</u>	<u>5.20</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>614.72</u>	<u>15.40</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>619.17</u>	<u>10.95</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>617.42</u>	<u>12.70</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>615.21</u>	<u>14.91</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)	<u>610.51</u>	<u>19.61</u>	Bottom of Screen
Installation Method: <u>Gravity</u>	<u>610.04</u>	<u>20.08</u>	Bottom of Well
Type of Backfill Material: <u>Quartz sand</u> (if applicable)			
Installation Method: <u>Gravity</u>	<u>607.92</u>	<u>22.20</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS

(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	17.73
Bottom of Screen to End Cap	(feet)	0.47
Screen Length (1st slot to last slot)	(feet)	4.70
Total Length of Casing	(feet)	22.90
Screen Slot Size **	(inches)	0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
DIVISION OF ENVIRONMENTAL HEALTH
525 W. JEFFERSON ST.
SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)** **Range** 3 **(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with _____ **from** _____ **to** _____ **ft.**

Kind of plug _____ **from** _____ **to** _____ **ft.**

Filled with _____ **from** _____ **to** _____ **ft.**

Kind of plug _____ **from** _____ **to** _____ **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: G105

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G105

State- Plant
Plane Coordinate: X 2,514,509.2 Y 875,499.7 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

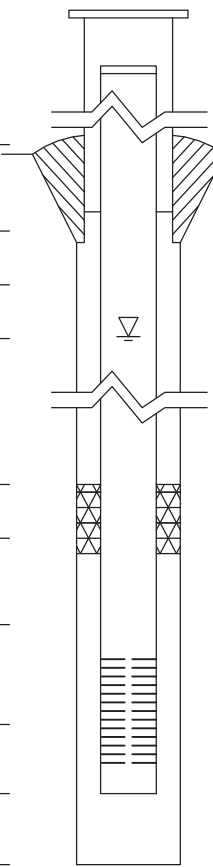
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/16/2010 Date Finished: 2/16/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>632.40</u>	<u>-3.14</u>	Top of Protective Casing
	<u>632.08</u>	<u>-2.82</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>629.26</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>623.80</u>	<u>5.46</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>613.18</u>	<u>16.08</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>616.55</u>	<u>12.71</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>615.50</u>	<u>13.76</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>613.15</u>	<u>16.11</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>608.36</u>	<u>20.90</u>	Bottom of Screen
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>607.89</u>	<u>21.37</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>606.80</u>	<u>22.46</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	18.93
Bottom of Screen to End Cap	(feet)	0.47
Screen Length (1st slot to last slot)	(feet)	4.79
Total Length of Casing	(feet)	24.19
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G106
Site Name: CCB Management Facility Borehole #: G106
State _____
Plane Coordinate: X 2,514,512.8 Y 875,149.8 (or) Latitude: 39° 4' 3.100" Longitude: -89° 23' 56.800"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: T. List
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna Simpson Date Started: 2/16/2010 Date Finished: 2/16/2010
Report Form Completed By: Suzanna Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Includes Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.13 feet), Bottom of Screen to End Cap (0.48 feet), Screen Length (4.69 feet), Total Length of Casing (22.30 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table for well construction materials with columns for material type (SS304, SS316, PTFE, PVC, OTHER) and selection for Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.



Site #: _____ County: Montgomery Well #: G107

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G107

State- Plant
Plane Coordinate: X 2,514,358.3 Y 874,994.3 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

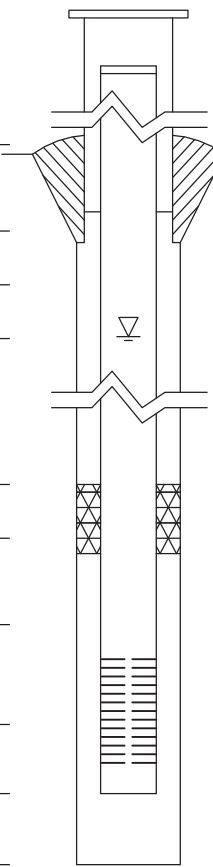
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/17/2010 Date Finished: 2/17/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>630.60</u>	<u>-2.81</u>	Top of Protective Casing
	<u>630.23</u>	<u>-2.44</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>627.79</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>624.08</u>	<u>3.71</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>617.39</u>	<u>10.40</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>617.41</u>	<u>10.38</u>	Top of Seal
Setting Time: <u>8 min</u>	<u>616.24</u>	<u>11.55</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>613.92</u>	<u>13.87</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>609.29</u>	<u>18.50</u>	Bottom of Screen
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>608.82</u>	<u>18.97</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>607.08</u>	<u>20.71</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	16.31
Bottom of Screen to End Cap	(feet)	0.47
Screen Length (1st slot to last slot)	(feet)	4.63
Total Length of Casing	(feet)	21.41
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G108

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G108

State- Plant
Plane Coordinate: X 2,514,248.3 Y 874,948.8 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

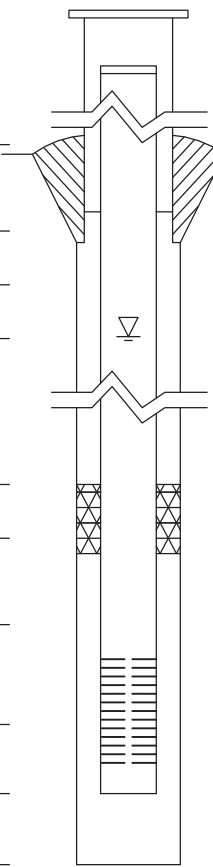
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Diane M. Lamb Date Started: 2/12/2010 Date Finished: 2/12/2010

Report Form Completed By: Diane M. Lamb Date: 2/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>630.52</u>	<u>-3.02</u>	Top of Protective Casing
	<u>630.22</u>	<u>-2.72</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>627.50</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>622.50</u>	<u>5.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>618.57</u>	<u>8.93</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Granular</u>	<u>614.00</u>	<u>13.50</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>612.80</u>	<u>14.70</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>610.68</u>	<u>16.82</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>606.00</u>	<u>21.50</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>605.50</u>	<u>22.00</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>605.50</u>	<u>22.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	19.54
Bottom of Screen to End Cap	(feet)	0.50
Screen Length (1st slot to last slot)	(feet)	4.68
Total Length of Casing	(feet)	24.72
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G109

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G109

State- Plant
Plane Coordinate: X 2,514,137.8 Y 874,970.1 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

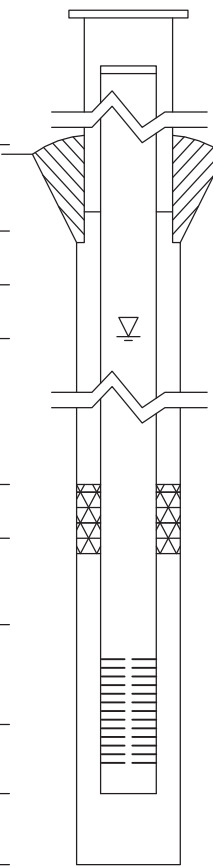
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Diane M. Lamb Date Started: 2/11/2010 Date Finished: 2/11/2010

Report Form Completed By: Diane M. Lamb Date: 2/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>630.08</u>	<u>-2.88</u>	Top of Protective Casing
	<u>629.76</u>	<u>-2.56</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>627.20</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>621.70</u>	<u>5.50</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>618.35</u>	<u>8.85</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Granular</u>	<u>614.90</u>	<u>12.30</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>614.60</u>	<u>12.60</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>611.81</u>	<u>15.39</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>607.27</u>	<u>19.93</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>606.70</u>	<u>20.50</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.70</u>	<u>20.50</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	17.95
Bottom of Screen to End Cap	(feet)	0.57
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	23.06
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G110

Site Name: CCB Management Facility Borehole #: G110

State _____
Plane Coordinate: X 2,514,057.7 Y 875,015.4 (or) Latitude: 39° 4' 1.800" Longitude: -89° 24' 2.500"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

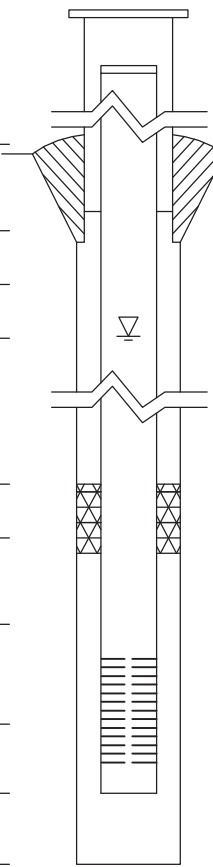
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: _____ Date Started: 2/11/2010 Date Finished: 2/11/2010

Report Form Completed By: _____ Date: 2/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>629.96</u>	<u>-2.94</u>	Top of Protective Casing
	<u>629.65</u>	<u>-2.63</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>627.02</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>621.86</u>	<u>5.16</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>617.52</u>	<u>9.50</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Granular</u>	<u>615.46</u>	<u>11.56</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>614.06</u>	<u>12.96</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>611.97</u>	<u>15.05</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>607.43</u>	<u>19.59</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>606.86</u>	<u>20.16</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.86</u>	<u>20.16</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	17.68
Bottom of Screen to End Cap	(feet)	0.57
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	22.79
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G111

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G111

State- Plant
Plane Coordinate: X 2,513,981.7 Y 875,058.7 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

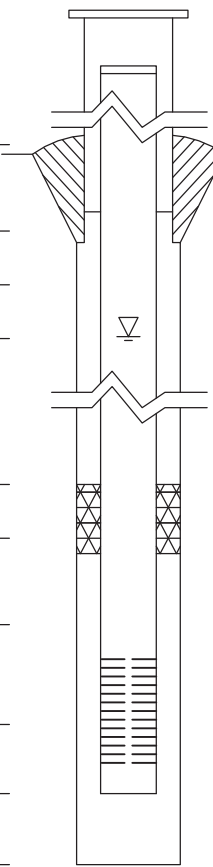
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Diane M. Lamb Date Started: 2/10/2010 Date Finished: 2/11/2010

Report Form Completed By: Diane M. Lamb Date: 2/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>630.19</u>	<u>-2.95</u>	Top of Protective Casing
	<u>629.90</u>	<u>-2.66</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>627.24</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>622.52</u>	<u>4.72</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>616.74</u>	<u>10.50</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Granular</u>	<u>616.41</u>	<u>10.83</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>614.52</u>	<u>12.72</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>612.63</u>	<u>14.61</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>608.09</u>	<u>19.15</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>607.52</u>	<u>19.72</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>607.52</u>	<u>19.72</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	17.27
Bottom of Screen to End Cap	(feet)	0.57
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	22.38
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G119

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G119

State- Plant
Plane Coordinate: X 2,513,907.7 Y 875,675.0 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

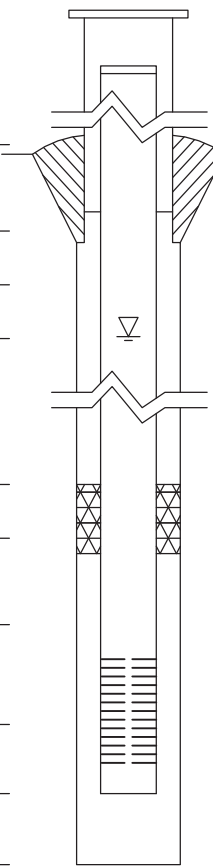
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/9/2010 Date Finished: 2/9/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>631.85</u>	<u>-3.00</u>	Top of Protective Casing
	<u>631.55</u>	<u>-2.70</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>628.85</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>623.57</u>	<u>5.28</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>617.59</u>	<u>11.26</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>614.87</u>	<u>13.98</u>	Top of Seal
Setting Time: <u>15 min</u>	<u>613.57</u>	<u>15.28</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>611.56</u>	<u>17.29</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>607.02</u>	<u>21.83</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>606.47</u>	<u>22.38</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.47</u>	<u>22.38</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

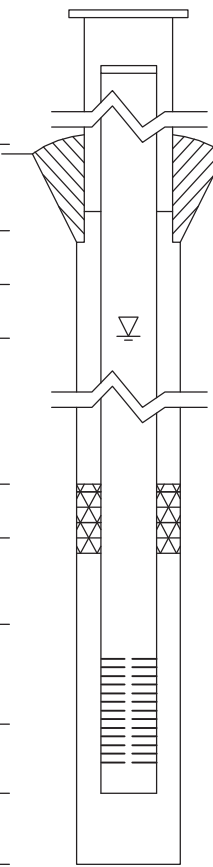
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	19.99
Bottom of Screen to End Cap	(feet)	0.55
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	25.08
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G120
Site Name: CCB Management Facility Borehole #: G120
State _____
Plane Coordinate: X 2,513,905.8 Y 875,854.4 (or) Latitude: 39° 4' 10.100" Longitude: -89° 24' 4.400"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: T. List
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna Simpson Date Started: 2/8/2010 Date Finished: 2/8/2010
Report Form Completed By: Suzanna Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes data for Top of Protective Casing, Top of Riser Pipe, Ground Surface, Top of Annular Sealant, Static Water Level, Top of Seal, Top of Sand Pack, Top of Screen, Bottom of Screen, Bottom of Well, and Bottom of Borehole.



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Includes Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.67 feet), Bottom of Screen to End Cap (0.59 feet), Screen Length (4.52 feet), Total Length of Casing (22.78 feet), and Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.



Site #: _____ County: Montgomery Well #: G121

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G121

State- Plant
Plane Coordinate: X 2,513,904.4 Y 875,964.6 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

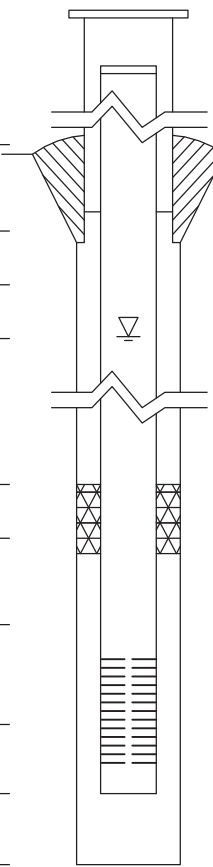
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/4/2010 Date Finished: 2/4/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.14</u>	<u>-3.57</u>	Top of Protective Casing
	<u>632.83</u>	<u>-3.26</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>629.57</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>624.41</u>	<u>5.16</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>615.13</u>	<u>14.44</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>616.81</u>	<u>12.76</u>	Top of Seal
Setting Time: <u>7 min</u>	<u>615.49</u>	<u>14.08</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>612.78</u>	<u>16.79</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>608.10</u>	<u>21.47</u>	Bottom of Screen
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>607.62</u>	<u>21.95</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>605.41</u>	<u>24.16</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	20.05
Bottom of Screen to End Cap	(feet)	0.48
Screen Length (1st slot to last slot)	(feet)	4.68
Total Length of Casing	(feet)	25.21
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:



Site #: _____ County: Montgomery Well #: G122

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G122

State- Plant
Plane Coordinate: X 2,513,902.8 Y 876,080.1 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

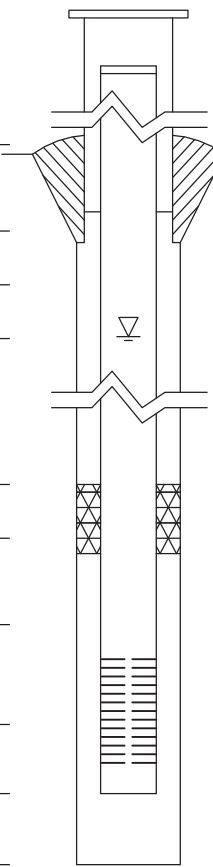
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Diane M. Lamb Date Started: 2/4/2010 Date Finished: 2/4/2010

Report Form Completed By: Diane M. Lamb Date: 2/9/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>632.98</u>	<u>-3.12</u>	Top of Protective Casing
	<u>632.69</u>	<u>-2.83</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>629.86</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>625.01</u>	<u>4.85</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>617.02</u>	<u>12.84</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>617.01</u>	<u>12.85</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>615.41</u>	<u>14.45</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>613.35</u>	<u>16.51</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>608.81</u>	<u>21.05</u>	Bottom of Screen
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>608.20</u>	<u>21.66</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>608.01</u>	<u>21.85</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	19.34
Bottom of Screen to End Cap	(feet)	0.61
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	24.49
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G123

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G123

State- Plant
Plane Coordinate: X 2,513,901.5 Y 876,189.6 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

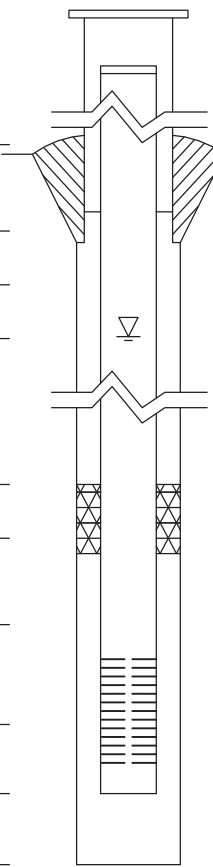
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Diane M. Lamb Date Started: 2/3/2010 Date Finished: 2/4/2010

Report Form Completed By: Diane M. Lamb Date: 2/9/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.29</u>	<u>-3.16</u>	Top of Protective Casing
	<u>632.96</u>	<u>-2.83</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.13</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>625.06</u>	<u>5.07</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>614.15</u>	<u>15.98</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>612.31</u>	<u>17.82</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>611.14</u>	<u>18.99</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>609.19</u>	<u>20.94</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>604.67</u>	<u>25.46</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>604.06</u>	<u>26.07</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>604.06</u>	<u>26.07</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	23.77
Bottom of Screen to End Cap	(feet)	0.61
Screen Length (1st slot to last slot)	(feet)	4.52
Total Length of Casing	(feet)	28.90
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G124

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G124

State- Plant
Plane Coordinate: X 2,513,900.3 Y 876,304.9 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

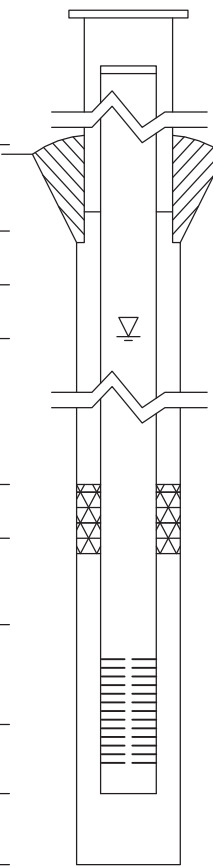
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/3/2010 Date Finished: 2/3/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/5/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.70</u>	<u>-3.28</u>	Top of Protective Casing
	<u>633.39</u>	<u>-2.97</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.42</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>625.45</u>	<u>4.97</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>619.43</u>	<u>10.99</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>	<u>618.34</u>	<u>12.08</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>616.50</u>	<u>13.92</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>614.44</u>	<u>15.98</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>609.91</u>	<u>20.51</u>	Bottom of Screen
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>609.36</u>	<u>21.06</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>608.45</u>	<u>21.97</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	18.95
Bottom of Screen to End Cap	(feet)	0.55
Screen Length (1st slot to last slot)	(feet)	4.53
Total Length of Casing	(feet)	24.03
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:



Site #: _____ County: Montgomery Well #: G125

Site Name: CCB Management Facility Borehole #: G125

State _____
Plane Coordinate: X 2,513,899.1 Y 876,409.5 (or) Latitude: 39° 4' 15.600" Longitude: -89° 24' 4.400"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

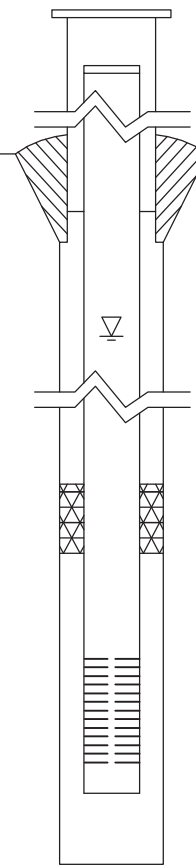
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna Simpson Date Started: 2/2/2010 Date Finished: 2/3/2010

Report Form Completed By: Suzanna Simpson Date: 2/5/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.82</u>	<u>-3.14</u>	Top of Protective Casing
	<u>633.51</u>	<u>-2.83</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.68</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>625.77</u>	<u>4.91</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>622.10</u>	<u>8.58</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Gravity</u>			
Setting Time: <u>10 min</u>			
Type of Sand Pack: <u>Quartz sand</u>	<u>617.15</u>	<u>13.53</u>	Top of Seal
Grain Size: <u>10/20</u> (sieve size)	<u>615.95</u>	<u>14.73</u>	Top of Sand Pack
Installation Method: <u>Gravity</u>			
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>613.65</u>	<u>17.03</u>	Top of Screen
Installation Method: <u>n/a</u>	<u>609.12</u>	<u>21.56</u>	Bottom of Screen
	<u>608.64</u>	<u>22.04</u>	Bottom of Well
	<u>608.64</u>	<u>22.04</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	19.86
Bottom of Screen to End Cap	(feet)	0.58
Screen Length (1st slot to last slot)	(feet)	4.53
Total Length of Casing	(feet)	24.97
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G126

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G126

State- Plant
Plane Coordinate: X 2,513,895.4 Y 875,062.4 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

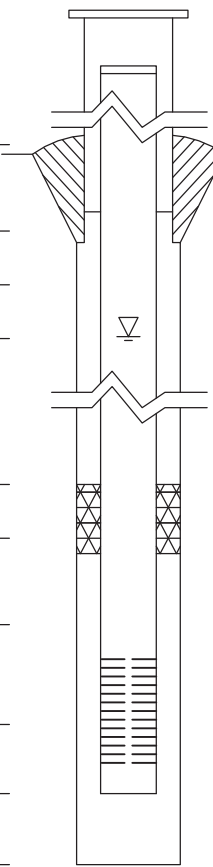
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Diane M. Lamb Date Started: 2/10/2010 Date Finished: 2/10/2010

Report Form Completed By: Diane M. Lamb Date: 2/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>625.69</u>	<u>-2.73</u>	Top of Protective Casing
	<u>625.39</u>	<u>-2.43</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>622.96</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>619.96</u>	<u>3.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>614.14</u>	<u>8.82</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Granular</u>	<u>613.96</u>	<u>9.00</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>612.86</u>	<u>10.10</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>610.07</u>	<u>12.89</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>605.53</u>	<u>17.43</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>604.96</u>	<u>18.00</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>604.96</u>	<u>18.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	15.32
Bottom of Screen to End Cap	(feet)	0.57
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	20.43
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G151

Site Name: CCB Management Facility Borehole #: G151

State _____
Plane Coordinate: X 2,513,805.9 Y 875,023.7 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Testing Service Corp. Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Ryne M. Fiorito Date Started: 12/19/2011 Date Finished: 12/19/2011

Report Form Completed By: Rhonald W. Hasenyager Date: 12/27/2011

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>626.24</u>	<u>-3.42</u>	Top of Protective Casing
	<u>625.93</u>	<u>-3.11</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>622.82</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>620.49</u>	<u>2.33</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>615.03</u>	<u>7.79</u>	Static Water Level (After Completion) 12/21/2011
Installation Method: <u>Gravity</u>	<u>611.82</u>	<u>11.00</u>	Top of Seal
Setting Time: <u>24 min</u>	<u>609.07</u>	<u>13.75</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>607.48</u>	<u>15.34</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)	<u>602.98</u>	<u>19.84</u>	Bottom of Screen
Installation Method: <u>Gravity</u>	<u>602.36</u>	<u>20.46</u>	Bottom of Well
Type of Backfill Material: <u>n/a</u> (if applicable)			
Installation Method: <u>n/a</u>	<u>602.36</u>	<u>20.46</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	18.45
Bottom of Screen to End Cap	(feet)	0.62
Screen Length (1st slot to last slot)	(feet)	4.50
Total Length of Casing	(feet)	23.57
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G152
Site Name: CCB Management Facility Borehole #: G152
State _____
Plane Coordinate: X 2,513,894.5 Y 874,687.5 (or) Latitude: _____ Longitude: _____
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Testing Service Corp. Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Ryne M. Fiorito Date Started: 12/20/2011 Date Finished: 12/20/2011
Report Form Completed By: Rhonald W. Hasenyager Date: 12/27/2011

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Key data points include: Top of Protective Casing (626.67 MSL, -3.61 BGS), Top of Riser Pipe (626.52 MSL, -3.46 BGS), Ground Surface (623.06 MSL, 0.00 BGS), Top of Annular Sealant (621.06 MSL, 2.00 BGS), Static Water Level (606.06 MSL, 17.00 BGS), Top of Seal (621.06 MSL, 2.00 BGS), Top of Sand Pack (611.56 MSL, 11.50 BGS), Top of Screen (609.47 MSL, 13.59 BGS), Bottom of Screen (604.97 MSL, 18.09 BGS), Bottom of Well (604.49 MSL, 18.57 BGS), Bottom of Borehole (604.49 MSL, 18.57 BGS).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.05 feet), Bottom of Screen to End Cap (0.48 feet), Screen Length (4.50 feet), Total Length of Casing (22.03 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.



Site #: _____ County: Montgomery Well #: G153

Site Name: CCB Management Facility Borehole #: G153

State _____
Plane Coordinate: X 2,513,532.7 Y 874,532.7 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Testing Service Corp. Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

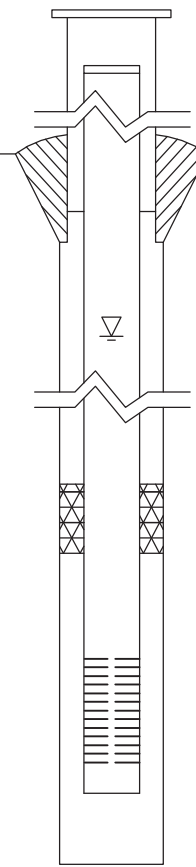
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Ryne M. Fiorito Date Started: 12/15/2011 Date Finished: 12/15/2011

Report Form Completed By: Rhonald W. Hasenyager Date: 12/27/2011

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>626.59</u>	<u>-3.29</u>	Top of Protective Casing
	<u>626.35</u>	<u>-3.05</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>623.30</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>622.63</u>	<u>0.67</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>605.75</u>	<u>17.55</u>	Static Water Level (After Completion) 12/21/2011
Installation Method: <u>Gravity</u>	<u>610.30</u>	<u>13.00</u>	Top of Seal
Setting Time: <u>>24 hr.</u>	<u>608.30</u>	<u>15.00</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>607.40</u>	<u>15.90</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>602.96</u>	<u>20.34</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>602.54</u>	<u>20.76</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>602.54</u>	<u>20.76</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	18.95
Bottom of Screen to End Cap	(feet)	0.42
Screen Length (1st slot to last slot)	(feet)	4.44
Total Length of Casing	(feet)	23.81
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G154
Site Name: CCB Management Facility Borehole #: G154
State _____
Plane Coordinate: X 2,513,243.1 Y 874,978.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Testing Service Corp. Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Ryne M. Fiorito Date Started: 12/16/2011 Date Finished: 12/16/2011
Report Form Completed By: Rhonald W. Hasenyager Date: 12/27/2011

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

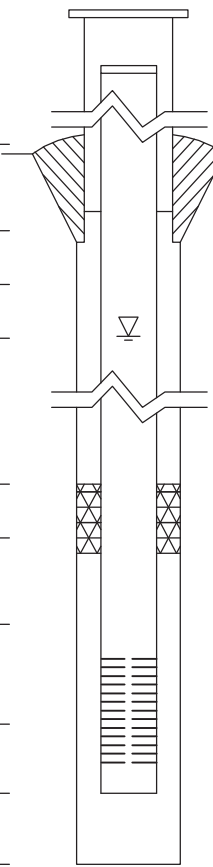
Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: G155
Site Name: CCB Management Facility Borehole #: G155
State _____
Plane Coordinate: X 2,513,501.8 Y 875,127.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Testing Service Corp. Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Ryne M. Fiorito Date Started: 12/19/2011 Date Finished: 12/19/2011
Report Form Completed By: Rhonald W. Hasenyager Date: 12/27/2011

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes data for Top of Protective Casing, Top of Riser Pipe, Ground Surface, Top of Annular Sealant, Static Water Level, Top of Seal, Top of Sand Pack, Top of Screen, Bottom of Screen, Bottom of Well, and Bottom of Borehole.



Type of Surface Seal: Concrete
Type of Annular Sealant: High-solids bentonite
Installation Method: Tremie
Setting Time: >24 hr.
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: 27 min
Type of Sand Pack: Quartz sand
Grain Size: 10/20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Includes Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (18.28 feet), Bottom of Screen to End Cap (0.43 feet), Screen Length (4.49 feet), Total Length of Casing (23.20 feet), and Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.



Site #: _____ County: Montgomery Well #: G200

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G200

State- Plant
Plane Coordinate: X 877,930.6 Y 2,515,650.0 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Testing Service Corporation Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246

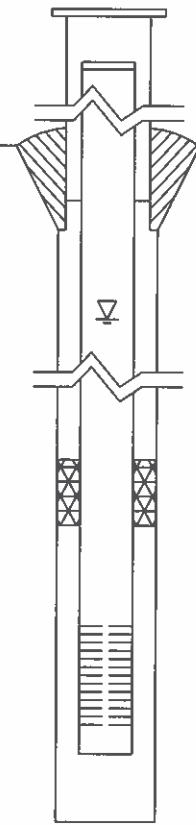
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____

Logged By: Suzanna L Simpson Date Started: 2/25/2008 Date Finished: 2/25/2008

Report Form Completed By: Suzanna L Simpson Date: 2/29/2008

ANNULAR SPACE DETAILS

Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
<u>626.54</u>	<u>-2.34</u>	Top of Protective Casing
<u>625.94</u>	<u>-1.74</u>	Top of Riser Pipe
<u>624.20</u>	<u>0.00</u>	Ground Surface
<u>620.70</u>	<u>3.50</u>	Top of Annular Sealant
<u>621.45</u>	<u>2.75</u>	Static Water Level (After Completion) 3/12/2008
<u>620.70</u>	<u>3.50</u>	Top of Seal
<u>614.20</u>	<u>10.00</u>	Top of Sand Pack
<u>612.01</u>	<u>12.19</u>	Top of Screen
<u>607.22</u>	<u>16.98</u>	Bottom of Screen
<u>606.84</u>	<u>17.36</u>	Bottom of Well
<u>606.20</u>	<u>18.00</u>	Bottom of Borehole



Type of Surface Seal: Concrete

Type of Annular Sealant: Bentonite chips

Installation Method: Gravity

Setting Time: >24 hr.

Type of Bentonite Seal -- Granular Pellet Slurry
(choose one)

Installation Method: Gravity

Setting Time: >24 hr.

Type of Sand Pack: Quartz sand

Grain Size: 10/20 (sieve size)

Installation Method: Gravity

Type of Backfill Material: Formation Sand
(if applicable)

Installation Method: Slough

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	13.93
Bottom of Screen to End Cap	(feet)	0.38
Screen Length (1st slot to last slot)	(feet)	4.79
Total Length of Casing	(feet)	19.10
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G201

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G201

State- Plant
Plane Coordinate: X 877,924.9 Y 2,514,849.5 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Testing Service Corporation Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246

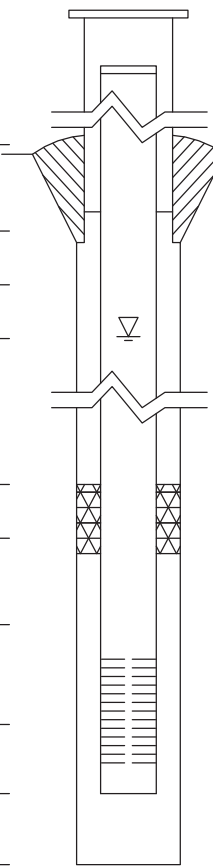
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____

Logged By: Suzanna L Simpson Date Started: 2/25/2008 Date Finished: 2/25/2008

Report Form Completed By: Suzanna L Simpson Date: 2/29/2008

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>627.66</u>	<u>-3.76</u>	Top of Protective Casing
	<u>627.12</u>	<u>-3.22</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>623.90</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>Bentonite chips</u>	<u>620.60</u>	<u>3.30</u>	Top of Annular Sealant
Installation Method: <u>Gravity</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>621.73</u>	<u>2.17</u>	Static Water Level (After Completion) 3/12/2008
Installation Method: <u>Gravity</u>	<u>620.60</u>	<u>3.30</u>	Top of Seal
Setting Time: <u>>24 hr.</u>	<u>611.80</u>	<u>12.10</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>610.89</u>	<u>13.01</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>606.10</u>	<u>17.80</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>605.75</u>	<u>18.15</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>605.75</u>	<u>18.15</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	16.23
Bottom of Screen to End Cap	(feet)	0.35
Screen Length (1st slot to last slot)	(feet)	4.79
Total Length of Casing	(feet)	21.37
Screen Slot Size **	(inches)	0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
DIVISION OF ENVIRONMENTAL HEALTH
525 W. JEFFERSON ST.
SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)**~~(S)~~ **Range** 3 ~~(E)~~**(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: G205

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G205

State- Plant
Plane Coordinate: X 875,550.2 Y 2,515,914.9 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Testing Service Corporation Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246

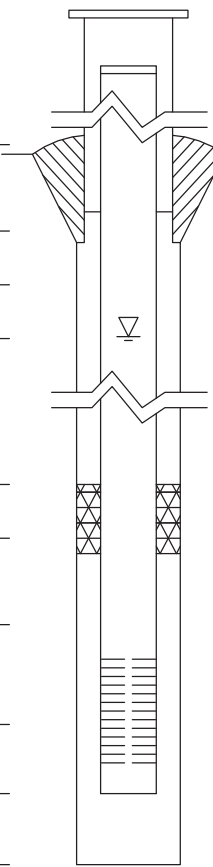
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____

Logged By: Suzanna L Simpson Date Started: 2/21/2008 Date Finished: 2/21/2008

Report Form Completed By: Suzanna L Simpson Date: 2/29/2008

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>624.87</u>	<u>-2.72</u>	Top of Protective Casing
	<u>624.45</u>	<u>-2.30</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>622.15</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>Bentonite chips</u>	<u>619.95</u>	<u>2.20</u>	Top of Annular Sealant
Installation Method: <u>Gravity</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>617.09</u>	<u>5.06</u>	Static Water Level (After Completion) 3/12/2008
Installation Method: <u>Gravity</u>	<u>619.95</u>	<u>2.20</u>	Top of Seal
Setting Time: <u>>24 hr.</u>	<u>613.35</u>	<u>8.80</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>612.11</u>	<u>10.04</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>607.62</u>	<u>14.53</u>	Bottom of Screen
Type of Backfill Material: <u>Formation Sand</u> (if applicable)	<u>607.08</u>	<u>15.07</u>	Bottom of Well
Installation Method: <u>Slough</u>	<u>606.15</u>	<u>16.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	12.34
Bottom of Screen to End Cap	(feet)	0.54
Screen Length (1st slot to last slot)	(feet)	4.49
Total Length of Casing	(feet)	17.37
Screen Slot Size **	(inches)	0.010

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
 DIVISION OF ENVIRONMENTAL HEALTH
 525 W. JEFFERSON ST.
 SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
 LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** Coffeen Power Station - GMF (G205)

2. **Well Location** 134 CIPS Lane Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)** **Range** 3 **(W)** **Section** 11
SW **Quarter of the** NE **Quarter of the** NW **Quarter**

3. **Year Drilled** 2008

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 16 **Diameter (inches)** 8

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING** *(riser pipe and screen pulled and annulus was grouted)*

Filled with Bentonite grout **from** 0 **to** 16.0 **ft.**
(cement or other materials)

Kind of plug _____ **from** _____ **to** _____ **ft.**

Filled with _____ **from** _____ **to** _____ **ft.**

Kind of plug _____ **from** _____ **to** _____ **ft.**

Filled with _____ **from** _____ **to** _____ **ft.**

Kind of plug _____ **from** _____ **to** _____ **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** March **Day** 20 **Year** 2017

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: G206

Site Name: CCB Management Facility Borehole #: G206

State _____
Plane Coordinate: X 2,514,669.2 Y 875,103.9 (or) Latitude: 39° 4' 2.600" Longitude: -89° 23' 54.800"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

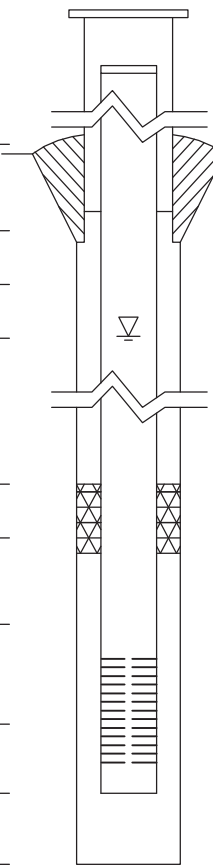
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna Simpson Date Started: 10/14/2010 Date Finished: 10/14/2010

Report Form Completed By: Suzanna Simpson Date: 10/15/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.07</u>	<u>-2.53</u>	Top of Protective Casing
	<u>632.82</u>	<u>-2.28</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.54</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>627.84</u>	<u>2.70</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>611.96</u>	<u>18.58</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>	<u>616.24</u>	<u>14.30</u>	Top of Seal
Setting Time: <u>15 min</u>	<u>615.04</u>	<u>15.50</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>613.03</u>	<u>17.51</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>608.62</u>	<u>21.92</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>608.12</u>	<u>22.42</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.54</u>	<u>24.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	19.79
Bottom of Screen to End Cap	(feet)	0.50
Screen Length (1st slot to last slot)	(feet)	4.41
Total Length of Casing	(feet)	24.70
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G207

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G207

State _____
Plane Coordinate: X 2,514,837.9 Y 875,166.4 (or) Latitude: 39° 4' 3.2" Longitude: -89° 23' 52.6"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

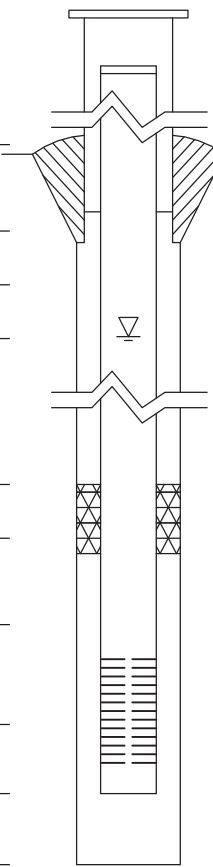
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 10/8/2010 Date Finished: 10/8/2010

Report Form Completed By: Suzanna L. Simpson Date: 10/8/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.37</u>	<u>-2.76</u>	Top of Protective Casing
	<u>633.21</u>	<u>-2.60</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.61</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>628.61</u>	<u>2.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>612.86</u>	<u>17.75</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>	<u>614.76</u>	<u>15.85</u>	Top of Seal
Setting Time: <u>15 min</u>	<u>613.63</u>	<u>16.98</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>612.37</u>	<u>18.24</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>607.84</u>	<u>22.77</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>607.31</u>	<u>23.30</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.61</u>	<u>24.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	20.84
Bottom of Screen to End Cap	(feet)	0.53
Screen Length (1st slot to last slot)	(feet)	4.53
Total Length of Casing	(feet)	25.90
Screen Slot Size **	(inches)	0.010



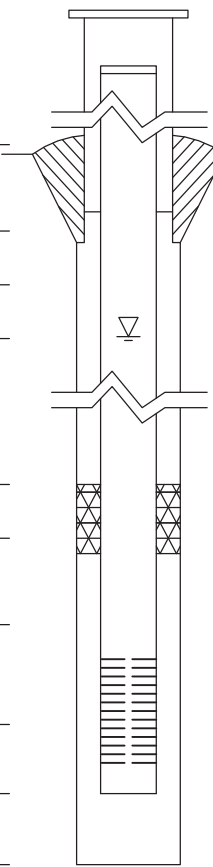
Site #: _____ County: Montgomery Well #: G208
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G208
State _____
Plane Coordinate: X 2,514,993.6 Y 875,231.5 (or) Latitude: 39° 4' 3.9" Longitude: -89° 23' 50.6"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna L. Simpson Date Started: 10/7/2010 Date Finished: 10/7/2010
Report Form Completed By: Suzanna L. Simpson Date: 10/8/2010

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.)

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.43</u>	<u>-2.86</u>	Top of Protective Casing
	<u>633.16</u>	<u>-2.59</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.57</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>627.77</u>	<u>2.80</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>614.76</u>	<u>15.81</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>			
Setting Time: <u>9 min</u>			
Type of Sand Pack: <u>Quartz sand</u>	<u>616.07</u>	<u>14.50</u>	Top of Seal
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>			
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>613.90</u>	<u>16.67</u>	Top of Sand Pack
Installation Method: <u>n/a</u>			
	<u>613.04</u>	<u>17.53</u>	Top of Screen
	<u>608.51</u>	<u>22.06</u>	Bottom of Screen
	<u>607.97</u>	<u>22.60</u>	Bottom of Well
	<u>606.57</u>	<u>24.00</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum



WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	20.12
Bottom of Screen to End Cap	(feet)	0.54
Screen Length (1st slot to last slot)	(feet)	4.53
Total Length of Casing	(feet)	25.19
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G209
Site Name: CCB Management Facility Borehole #: G209
State _____
Plane Coordinate: X 2,515,149.6 Y 875,298.2 (or) Latitude: 39° 4' 4.500" Longitude: -89° 23' 48.700"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna Simpson Date Started: 10/7/2010 Date Finished: 10/7/2010
Report Form Completed By: Suzanna Simpson Date: 10/8/2010

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

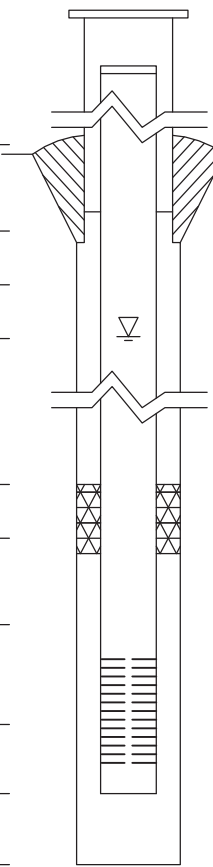
Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: G210
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G210
State _____
Plane Coordinate: X 2,515,299.0 Y 875,359.7 (or) Latitude: 39° 4' 5.1" Longitude: -89° 23' 46.8"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna L. Simpson Date Started: 10/6/2010 Date Finished: 10/6/2010
Report Form Completed By: Suzanna L. Simpson Date: 10/8/2010

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.)



WELL CONSTRUCTION MATERIALS

Table for well construction materials with columns for material type and selection options.

CASING MEASUREMENTS

Table for casing measurements with columns for measurement type and value.



Site #: _____ County: Montgomery Well #: G211

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G211

State _____
Plane Coordinate: X 2,515,449.1 Y 875,424.5 (or) Latitude: 39° 4' 5.7" Longitude: -89° 23' 44.9"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

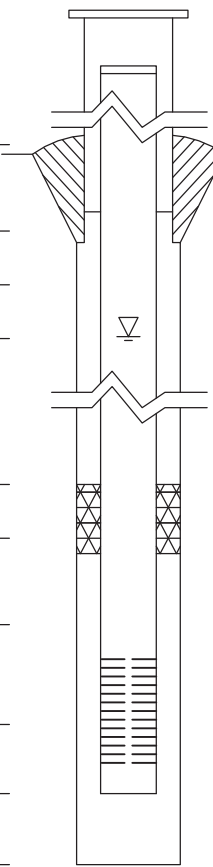
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 10/11/2010 Date Finished: 10/11/2010

Report Form Completed By: Suzanna L. Simpson Date: 10/15/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>632.83</u>	<u>-2.52</u>	Top of Protective Casing
	<u>632.64</u>	<u>-2.33</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.31</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>627.31</u>	<u>3.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>616.17</u>	<u>14.14</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>	<u>616.01</u>	<u>14.30</u>	Top of Seal
Setting Time: <u>15 min</u>	<u>614.91</u>	<u>15.40</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>612.97</u>	<u>17.34</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>608.43</u>	<u>21.88</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>607.90</u>	<u>22.41</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.31</u>	<u>24.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

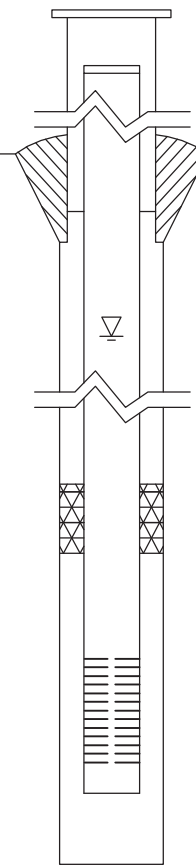
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	19.67
Bottom of Screen to End Cap	(feet)	0.53
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	24.74
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G212
Site Name: CCB Management Facility Borehole #: G212
State _____
Plane Coordinate: X 2,515,583.0 Y 875,486.5 (or) Latitude: 39° 4' 6.300" Longitude: -89° 23' 43.100"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna Simpson Date Started: 10/11/2010 Date Finished: 10/11/2010
Report Form Completed By: Suzanna Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.12</u>	<u>-2.53</u>	Top of Protective Casing
	<u>632.89</u>	<u>-2.30</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.59</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>627.59</u>	<u>3.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>616.10</u>	<u>14.49</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>	<u>616.89</u>	<u>13.70</u>	Top of Seal
Setting Time: <u>17 min</u>	<u>615.79</u>	<u>14.80</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>613.85</u>	<u>16.74</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>609.30</u>	<u>21.29</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>608.78</u>	<u>21.81</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.59</u>	<u>24.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	19.04
Bottom of Screen to End Cap	(feet)	0.52
Screen Length (1st slot to last slot)	(feet)	4.55
Total Length of Casing	(feet)	24.11
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G213
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G213
State _____
Plane Coordinate: X 2,515,723.5 Y 875,544.4 (or) Latitude: 39° 4' 6.9" Longitude: -89° 23' 41.4"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna L. Simpson Date Started: 10/12/2010 Date Finished: 10/12/2010
Report Form Completed By: Suzanna L. Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.) descriptions. Includes a central diagram of a well casing and screen assembly. Descriptions include Top of Protective Casing, Top of Riser Pipe, Ground Surface, Top of Annular Sealant, Static Water Level, Top of Seal, Top of Sand Pack, Top of Screen, Bottom of Screen, Bottom of Well, and Bottom of Borehole.

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 2 columns: Material Type and Material Options. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: G214

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G214

State _____
Plane Coordinate: X 2,515,960.8 Y 875,668.0 (or) Latitude: 39° 4' 8.1" Longitude: -89° 23' 38.3"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

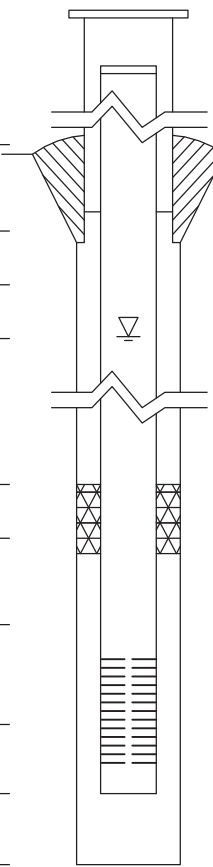
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 10/14/2010 Date Finished: 10/14/2010

Report Form Completed By: Suzanna L. Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.08</u>	<u>-2.69</u>	Top of Protective Casing
	<u>632.85</u>	<u>-2.46</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.39</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>626.99</u>	<u>3.40</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>609.48</u>	<u>20.91</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>	<u>615.39</u>	<u>15.00</u>	Top of Seal
Setting Time: <u>14 min</u>	<u>614.34</u>	<u>16.05</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>612.64</u>	<u>17.75</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>608.25</u>	<u>22.14</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>607.74</u>	<u>22.65</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>606.39</u>	<u>24.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

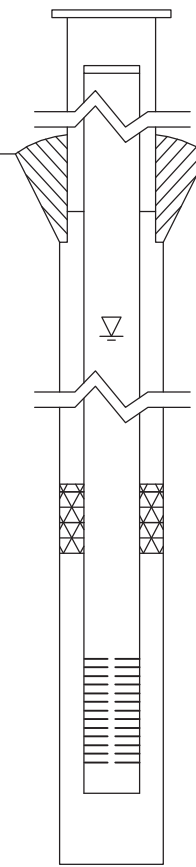
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	20.21
Bottom of Screen to End Cap	(feet)	0.51
Screen Length (1st slot to last slot)	(feet)	4.39
Total Length of Casing	(feet)	25.11
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G215
Site Name: CCB Management Facility Borehole #: G215
State _____
Plane Coordinate: X 2,515,971.6 Y 875,810.2 (or) Latitude: 39° 4' 9.500" Longitude: -89° 23' 38.200"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Suzanna Simpson Date Started: 10/13/2010 Date Finished: 10/13/2010
Report Form Completed By: Suzanna Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes data for Top of Protective Casing, Top of Riser Pipe, Ground Surface, Top of Annular Sealant, Static Water Level, Top of Seal, Top of Sand Pack, Top of Screen, Bottom of Screen, Bottom of Well, and Bottom of Borehole.



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: G216

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G216

State _____
Plane Coordinate: X 2,515,968.5 Y 875,976.1 (or) Latitude: 39° 4' 11.2" Longitude: -89° 23' 38.2"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

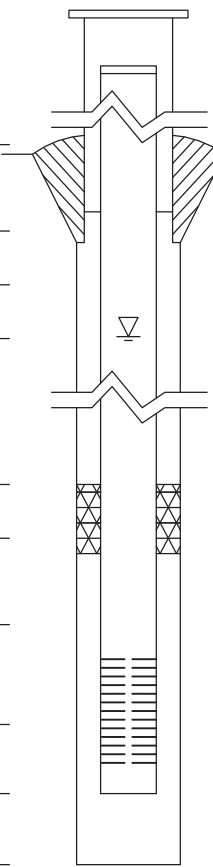
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 10/13/2010 Date Finished: 10/13/2010

Report Form Completed By: Suzanna L. Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.02</u>	<u>-2.74</u>	Top of Protective Casing
	<u>632.76</u>	<u>-2.48</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.28</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>627.78</u>	<u>2.50</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>607.52</u>	<u>22.76</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>			
Setting Time: <u>15 min</u>			
Type of Sand Pack: <u>Quartz sand</u>	<u>613.28</u>	<u>17.00</u>	Top of Seal
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>612.08</u>	<u>18.20</u>	Top of Sand Pack
Type of Backfill Material: <u>n/a</u> (if applicable)			
Installation Method: <u>n/a</u>			
	<u>610.24</u>	<u>20.04</u>	Top of Screen
	<u>605.86</u>	<u>24.42</u>	Bottom of Screen
	<u>605.35</u>	<u>24.93</u>	Bottom of Well
	<u>604.28</u>	<u>26.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	22.52
Bottom of Screen to End Cap	(feet)	0.51
Screen Length (1st slot to last slot)	(feet)	4.38
Total Length of Casing	(feet)	27.41
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G217

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G217

State _____
Plane Coordinate: X 2,515,963.0 Y 876,185.6 (or) Latitude: 39° 4' 13.2" Longitude: -89° 23' 38.3"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

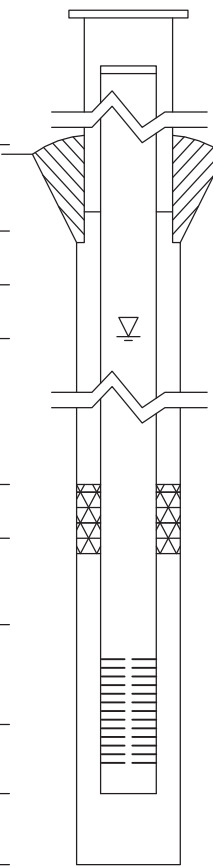
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 10/12/2010 Date Finished: 10/12/2010

Report Form Completed By: Suzanna L. Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.34</u>	<u>-2.67</u>	Top of Protective Casing
	<u>633.10</u>	<u>-2.43</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.67</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>628.27</u>	<u>2.40</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>609.28</u>	<u>21.39</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>	<u>612.82</u>	<u>17.85</u>	Top of Seal
Setting Time: <u>11 min</u>	<u>611.82</u>	<u>18.85</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>610.18</u>	<u>20.49</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>605.79</u>	<u>24.88</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>605.29</u>	<u>25.38</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>604.67</u>	<u>26.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

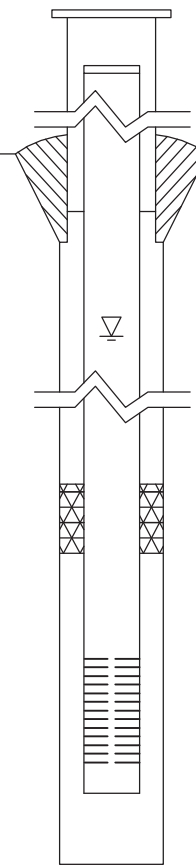
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	22.92
Bottom of Screen to End Cap	(feet)	0.50
Screen Length (1st slot to last slot)	(feet)	4.39
Total Length of Casing	(feet)	27.81
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G218
 Site Name: CCB Management Facility Borehole #: G218
 State _____
 Plane Coordinate: X 2,515,962.2 Y 876,380.9 (or) Latitude: 39° 4' 15.200" Longitude: -89° 23' 38.200"
 Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
 Drilling Contractor: Layne-Western Co Driller: D. Mahurin
 Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
 Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
 Logged By: Suzanna Simpson Date Started: 10/12/2010 Date Finished: 10/12/2010
 Report Form Completed By: Suzanna Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>633.34</u>	<u>-2.70</u>	Top of Protective Casing
	<u>633.11</u>	<u>-2.47</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>630.64</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>627.14</u>	<u>3.50</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>	<u>609.89</u>	<u>20.75</u>	Static Water Level (After Completion) 11/15/2010
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)			
Installation Method: <u>Gravity</u>	<u>613.14</u>	<u>17.50</u>	Top of Seal
Setting Time: <u>17 min</u>	<u>612.14</u>	<u>18.50</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>610.31</u>	<u>20.33</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>605.87</u>	<u>24.77</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>605.37</u>	<u>25.27</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>604.64</u>	<u>26.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	22.80
Bottom of Screen to End Cap	(feet)	0.50
Screen Length (1st slot to last slot)	(feet)	4.44
Total Length of Casing	(feet)	27.74
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: G270
Site Name: CCB Management Facility Borehole #: G270
State _____
Plane Coordinate: X 2,514,996.8 Y 874,801.9 (or) Latitude: 39° 3' 59.600" Longitude: -89° 23' 50.700"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Testing Service Corp. Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____
Logged By: Suzanna Simpson Date Started: 2/26/2008 Date Finished: 2/26/2008
Report Form Completed By: Suzanna Simpson Date: 2/29/2008

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (626.41, -3.49), Top of Riser Pipe (625.97, -3.05), Ground Surface (622.92, 0.00), Top of Annular Sealant (619.92, 3.00), Static Water Level (617.30, 5.62), Top of Seal (619.92, 3.00), Top of Sand Pack (610.92, 12.00), Top of Screen (609.79, 13.13), Bottom of Screen (605.00, 17.92), Bottom of Well (604.65, 18.27), Bottom of Borehole (604.65, 18.27).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (16.18 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (4.79 feet), Total Length of Casing (21.32 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 2 columns: Material Type, Material Options. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.



Site #: _____ County: Montgomery Well #: G271
Site Name: CCB Management Facility Borehole #: G271
State _____
Plane Coordinate: X 2,515,517.1 Y 874,239.4 (or) Latitude: 39° 3' 54.000" Longitude: -89° 23' 44.100"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: G. Mills
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____
Logged By: Rhonald W. Hasenyager Date Started: 9/9/2009 Date Finished: 9/10/2009
Report Form Completed By: Suzanna Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (625.88, -2.99), Top of Riser Pipe (625.57, -2.68), Ground Surface (622.89, 0.00), Top of Annular Sealant (619.89, 3.00), Static Water Level (610.39, 12.50), Top of Seal (616.16, 6.73), Top of Sand Pack (613.87, 9.02), Top of Screen (612.93, 9.96), Bottom of Screen (608.58, 14.31), Bottom of Well (608.10, 14.79), Bottom of Borehole (606.89, 16.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (12.64 feet), Bottom of Screen to End Cap (0.48 feet), Screen Length (4.35 feet), Total Length of Casing (17.47 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.



Site #: _____ County: Montgomery Well #: G272

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G272

State- Plant
Plane Coordinate: X 2,515,745.0 Y 874,234.8 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: G. Mills

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): _____

Logged By: Rhonald W. Hasenyager Date Started: 9/10/2009 Date Finished: 9/10/2009

Report Form Completed By: Suzanna L. Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete

Type of Annular Sealant: High-solids bentonite

Installation Method: Tremie

Setting Time: >24 hr.

Type of Bentonite Seal -- Granular Pellet Slurry (choose one)

Installation Method: Gravity

Setting Time: 10 min

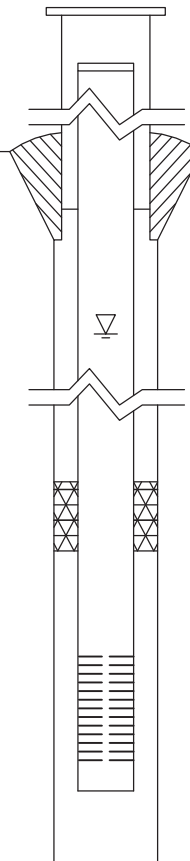
Type of Sand Pack: Quartz sand

Grain Size: 10/20 (sieve size)

Installation Method: Gravity

Type of Backfill Material: n/a (if applicable)

Installation Method: n/a



<u>624.11</u>	<u>-3.39</u>	Top of Protective Casing
<u>623.81</u>	<u>-3.09</u>	Top of Riser Pipe
<u>620.72</u>	<u>0.00</u>	Ground Surface
<u>617.72</u>	<u>3.00</u>	Top of Annular Sealant
<u>611.23</u>	<u>9.49</u>	Static Water Level (After Completion) 9/21/2009
<u>614.55</u>	<u>6.17</u>	Top of Seal
<u>612.74</u>	<u>7.98</u>	Top of Sand Pack
<u>611.61</u>	<u>9.11</u>	Top of Screen
<u>606.74</u>	<u>13.98</u>	Bottom of Screen
<u>606.40</u>	<u>14.32</u>	Bottom of Well
<u>606.40</u>	<u>14.32</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

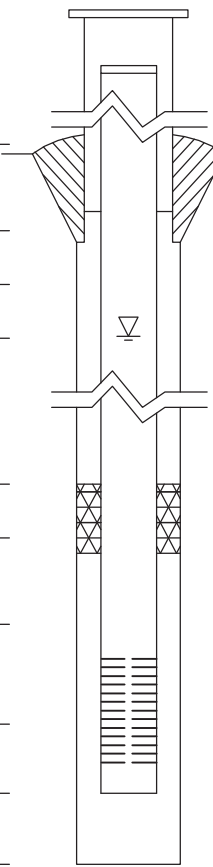
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	12.20
Bottom of Screen to End Cap	(feet)	0.34
Screen Length (1st slot to last slot)	(feet)	4.87
Total Length of Casing	(feet)	17.41
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G273
Site Name: CCB Management Facility Borehole #: G273
State _____
Plane Coordinate: X 2,515,975.5 Y 874,235.2 (or) Latitude: 39° 3' 53.900" Longitude: -89° 23' 38.300"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: G. Mills
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____
Logged By: Rhonald W. Hasenyager Date Started: 9/10/2009 Date Finished: 9/10/2009
Report Form Completed By: Suzanna Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>623.33</u>	<u>-3.16</u>	Top of Protective Casing
	<u>623.02</u>	<u>-2.85</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>620.17</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>617.17</u>	<u>3.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>	<u>610.28</u>	<u>9.89</u>	Static Water Level (After Completion) 9/21/2009
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)			
Installation Method: <u>Gravity</u>	<u>614.07</u>	<u>6.10</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>612.45</u>	<u>7.72</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>611.09</u>	<u>9.08</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>605.61</u>	<u>14.56</u>	Bottom of Screen
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>605.07</u>	<u>15.10</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>604.17</u>	<u>16.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: _____
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: _____
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: _____

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	11.93
Bottom of Screen to End Cap	(feet)	0.54
Screen Length (1st slot to last slot)	(feet)	5.48
Total Length of Casing	(feet)	17.95
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G274
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G274
State- Plant
Plane Coordinate: X 2,516,195.6 Y 874,239.2 (or) Latitude: _____ Longitude: _____
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: G. Mills
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____
Logged By: Rhonald W. Hasenyager Date Started: 9/16/2009 Date Finished: 9/16/2009
Report Form Completed By: Suzanna L. Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete
Type of Annular Sealant: High-solids bentonite
Installation Method: Tremie
Setting Time: >24 hr.
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: 15 min
Type of Sand Pack: Quartz sand
Grain Size: 10/20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

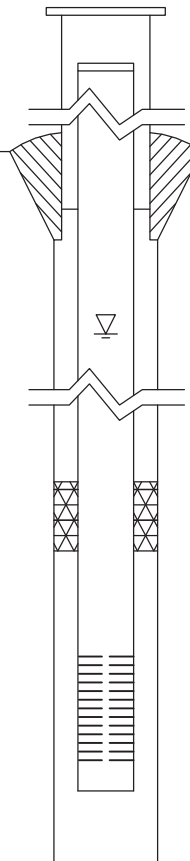


Table with 3 columns: Elevation (MSL)*, Depth (BGS), and Description. Rows include: Top of Protective Casing (624.32, -2.65), Top of Riser Pipe (624.04, -2.37), Ground Surface (621.67, 0.00), Top of Annular Sealant (618.67, 3.00), Static Water Level (608.55, 13.12), Top of Seal (611.93, 9.74), Top of Sand Pack (610.15, 11.52), Top of Screen (608.77, 12.90), Bottom of Screen (604.00, 17.67), Bottom of Well (603.61, 18.06), Bottom of Borehole (603.61, 18.06).

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 2 columns: Material Type and Material Options. Rows include: Protective Casing (SS304, SS316, PTFE, PVC, OTHER: Steel), Riser Pipe Above W.T. (SS304, SS316, PTFE, PVC, OTHER:), Riser Pipe Below W.T. (SS304, SS316, PTFE, PVC, OTHER:), Screen (SS304, SS316, PTFE, PVC, OTHER:).

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (15.27 feet), Bottom of Screen to End Cap (0.39 feet), Screen Length (4.77 feet), Total Length of Casing (20.43 feet), Screen Slot Size (0.010 inches).



Site #: _____ County: Montgomery Well #: G275
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G275
State- Plant
Plane Coordinate: X 2,516,375.9 Y 874,298.9 (or) Latitude: _____ Longitude: _____
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: G. Mills
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____
Logged By: Rhonald W. Hasenyager Date Started: 9/16/2009 Date Finished: 9/16/2009
Report Form Completed By: Suzanna L. Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete
Type of Annular Sealant: High-solids bentonite
Installation Method: Tremie
Setting Time: >24 hr.
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: 15 min
Type of Sand Pack: Quartz sand
Grain Size: 10/20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

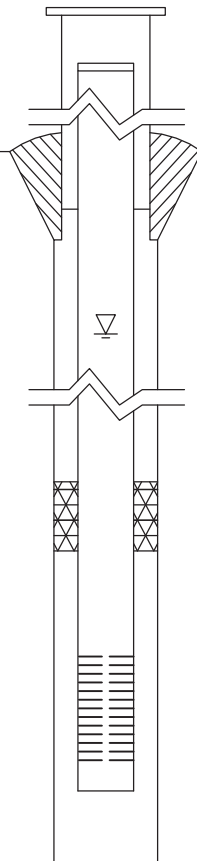


Table with 3 columns: Elevation (MSL)*, Depth (BGS), and Description. Rows include: Top of Protective Casing (618.53, -2.39), Top of Riser Pipe (618.26, -2.12), Ground Surface (616.14, 0.00), Top of Annular Sealant (613.14, 3.00), Static Water Level (605.89, 10.25), Top of Seal (610.42, 5.72), Top of Sand Pack (609.16, 6.98), Top of Screen (607.92, 8.22), Bottom of Screen (603.52, 12.62), Bottom of Well (602.95, 13.19), Bottom of Borehole (602.95, 13.19).

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 2 columns: Material Type and Material Options. Rows include: Protective Casing (SS304, SS316, PTFE, PVC, OTHER: Steel), Riser Pipe Above W.T. (SS304, SS316, PTFE, PVC, OTHER:), Riser Pipe Below W.T. (SS304, SS316, PTFE, PVC, OTHER:), Screen (SS304, SS316, PTFE, PVC, OTHER:).

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (10.34 feet), Bottom of Screen to End Cap (0.52 feet), Screen Length (4.45 feet), Total Length of Casing (15.31 feet), Screen Slot Size (0.010 inches).



Site #: _____ County: Montgomery Well #: G276

Site Name: CCB Management Facility Borehole #: G276

State _____
Plane Coordinate: X 2,516,358.8 Y 874,438.6 (or) Latitude: 39° 3' 55.900" Longitude: -89° 23' 33.400"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: G. Mills

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

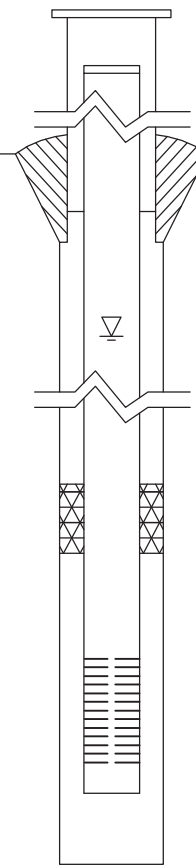
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____

Logged By: Rhonald W. Hasenyager Date Started: 9/16/2009 Date Finished: 9/16/2009

Report Form Completed By: Suzanna Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>632.40</u>	<u>-3.26</u>	Top of Protective Casing
	<u>632.00</u>	<u>-2.86</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>629.14</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>626.14</u>	<u>3.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>603.59</u>	<u>25.55</u>	Static Water Level (After Completion) 9/21/2009
Installation Method: <u>Gravity</u>	<u>610.06</u>	<u>19.08</u>	Top of Seal
Setting Time: <u>15 min</u>	<u>608.11</u>	<u>21.03</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>	<u>606.73</u>	<u>22.41</u>	Top of Screen
Grain Size: <u>10/20</u> (sieve size)	<u>601.92</u>	<u>27.22</u>	Bottom of Screen
Installation Method: <u>Gravity</u>	<u>601.49</u>	<u>27.65</u>	Bottom of Well
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>601.14</u>	<u>28.00</u>	Bottom of Borehole
Installation Method: <u>Gravity</u>			



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	25.27
Bottom of Screen to End Cap	(feet)	0.43
Screen Length (1st slot to last slot)	(feet)	4.81
Total Length of Casing	(feet)	30.51
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G277

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G277

State- Plant
Plane Coordinate: X 2,516,370.5 Y 874,581.8 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: G. Mills

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): _____

Logged By: Rhonald W. Hasenyager Date Started: 9/14/2009 Date Finished: 9/14/2009

Report Form Completed By: Suzanna L. Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete

Type of Annular Sealant: High-solids bentonite

Installation Method: Tremie

Setting Time: >24 hr.

Type of Bentonite Seal -- Granular Pellet Slurry (choose one)

Installation Method: Gravity

Setting Time: 18 min

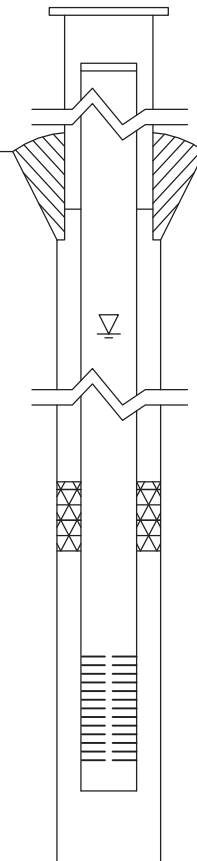
Type of Sand Pack: Quartz sand

Grain Size: 10/20 (sieve size)

Installation Method: Gravity

Type of Backfill Material: Quartz sand (if applicable)

Installation Method: Gravity



<u>623.35</u>	<u>-2.56</u>	Top of Protective Casing
<u>623.08</u>	<u>-2.29</u>	Top of Riser Pipe
<u>620.79</u>	<u>0.00</u>	Ground Surface
<u>617.79</u>	<u>3.00</u>	Top of Annular Sealant
<u>602.56</u>	<u>18.23</u>	Static Water Level (After Completion) 9/21/2009
<u>608.00</u>	<u>12.79</u>	Top of Seal
<u>607.00</u>	<u>13.79</u>	Top of Sand Pack
<u>606.50</u>	<u>14.29</u>	Top of Screen
<u>602.02</u>	<u>18.77</u>	Bottom of Screen
<u>601.55</u>	<u>19.24</u>	Bottom of Well
<u>600.79</u>	<u>20.00</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	16.58
Bottom of Screen to End Cap	(feet)	0.47
Screen Length (1st slot to last slot)	(feet)	4.48
Total Length of Casing	(feet)	21.53
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G278

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: G278

State- Plant
Plane Coordinate: X 2,516,200.7 Y 874,875.4 (or) Latitude: _____ Longitude: _____

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: G. Mills

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): _____

Logged By: Rhonald W. Hasenyager Date Started: 9/11/2009 Date Finished: 9/11/2009

Report Form Completed By: Suzanna L. Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete

Type of Annular Sealant: High-solids bentonite

Installation Method: Tremie

Setting Time: >24 hr.

Type of Bentonite Seal -- Granular Pellet Slurry (choose one)

Installation Method: Gravity

Setting Time: 22 min

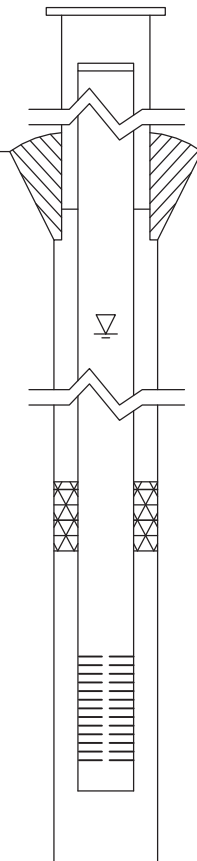
Type of Sand Pack: Quartz sand

Grain Size: 10/20 (sieve size)

Installation Method: Gravity

Type of Backfill Material: n/a (if applicable)

Installation Method: n/a



Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
<u>631.49</u>	<u>-2.64</u>	Top of Protective Casing
<u>631.17</u>	<u>-2.32</u>	Top of Riser Pipe
<u>628.85</u>	<u>0.00</u>	Ground Surface
<u>625.85</u>	<u>3.00</u>	Top of Annular Sealant
<u>604.87</u>	<u>23.98</u>	Static Water Level (After Completion) 9/21/2009
<u>613.74</u>	<u>15.11</u>	Top of Seal
<u>611.90</u>	<u>16.95</u>	Top of Sand Pack
<u>609.92</u>	<u>18.93</u>	Top of Screen
<u>605.15</u>	<u>23.70</u>	Bottom of Screen
<u>604.79</u>	<u>24.06</u>	Bottom of Well
<u>604.79</u>	<u>24.06</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	21.25
Bottom of Screen to End Cap	(feet)	0.36
Screen Length (1st slot to last slot)	(feet)	4.77
Total Length of Casing	(feet)	26.38
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G279
Site Name: CCB Management Facility Borehole #: G279
State _____
Plane Coordinate: X 2,516,245.6 Y 875,028.1 (or) Latitude: 39° 4' 1.800" Longitude: -89° 23' 34.800"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: G. Mills
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____
Logged By: Rhonald W. Hasenyager Date Started: 9/10/2009 Date Finished: 9/10/2009
Report Form Completed By: Suzanna Simpson Date: 10/7/2009

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.) descriptions. Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (632.33, -3.14), Top of Riser Pipe (632.04, -2.85), Ground Surface (629.19, 0.00), Top of Annular Sealant (626.19, 3.00), Static Water Level (601.66, 27.53), Top of Seal (610.45, 18.74), Top of Sand Pack (608.77, 20.42), Top of Screen (606.79, 22.40), Bottom of Screen (602.40, 26.79), Bottom of Well (604.51, 24.68), Bottom of Borehole (601.19, 28.00). A red box highlights the value 27.3.

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement and Value. Measurements include Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (25.25 feet), Bottom of Screen to End Cap (0.53 feet), Screen Length (4.39 feet), Total Length of Casing (30.17 feet), and Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 2 columns: Material Area and Material Options. Areas include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen. Options include SS304, SS316, PTFE, PVC, and OTHER.



Site #: _____ County: Montgomery Well #: G280
Site Name: CCB Management Facility Borehole #: G280
State _____
Plane Coordinate: X 2,515,679.5 Y 875,045.1 (or) Latitude: 39° 4' 2.000" Longitude: -89° 23' 42.000"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Testing Service Corp. Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): _____
Logged By: Suzanna Simpson Date Started: 2/26/2008 Date Finished: 2/26/2008
Report Form Completed By: Suzanna Simpson Date: 2/29/2008

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (625.79, -2.84), Top of Riser Pipe (625.30, -2.35), Ground Surface (622.95, 0.00), Top of Annular Sealant (620.85, 2.10), Static Water Level (618.61, 4.34), Top of Seal (620.85, 2.10), Top of Sand Pack (611.75, 11.20), Top of Screen (610.16, 12.79), Bottom of Screen (605.32, 17.63), Bottom of Well (604.97, 17.98), Bottom of Borehole (604.97, 17.98).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (15.14 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (4.84 feet), Total Length of Casing (20.33 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

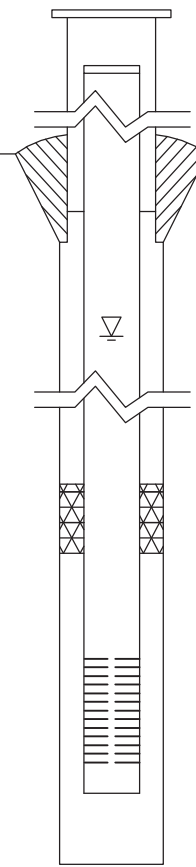
Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.



Site #: _____ County: Montgomery Well #: G281
Site Name: Natural Resource Technology, Inc. Coffeen Energy Center Borehole #: G281
State _____
Plane Coordinate: X 874,375.4 Y 2,514,455.5 (or) Latitude: _____ Longitude: _____
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Geotechnical Engineering, LLC Driller: D. Crump
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 9/8/2015 Date Finished: 9/8/2015
Report Form Completed By: Suzanna L. Keim Date: 10/6/2015

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.)



Type of Surface Seal: Concrete
Type of Annular Sealant: High-solids bentonite
Installation Method: Tremie
Setting Time: >24 hours
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: 25 minutes
Type of Sand Pack: Quartz Sand
Grain Size: 10-20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: _____

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER: Steel



Site #: _____ County: Montgomery Well #: G301

Site Name: Natural Resource Technology, Inc. Coffeen Energy Center Borehole #: G301

State _____
Plane Coordinate: X 872,234.8 Y 2,515,583.0 (or) Latitude: _____ Longitude: _____

Surveyed By: Gary C. Rogers IL Registration #: 035-002957

Drilling Contractor: Ramsey Geotechnical Engineering, LLC Driller: D. Crump

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

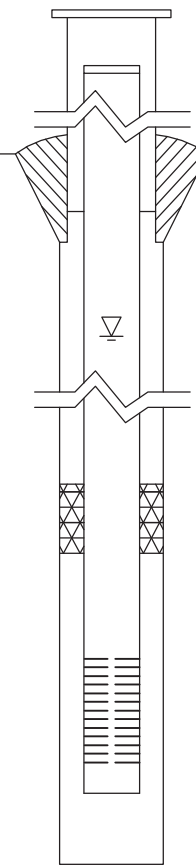
Drilling Method: Hollow stem auger Drilling Fluid (Type): none

Logged By: Kristen L. Theesfeld Date Started: 9/4/2015 Date Finished: 9/4/2015

Report Form Completed By: Suzanna L. Keim Date: 10/6/2015

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>622.98</u>	<u>-2.71</u>	Top of Protective Casing
	<u>622.65</u>	<u>-2.38</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>620.27</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>Bentonite Chips</u>	<u>618.20</u>	<u>2.07</u>	Top of Annular Sealant
Installation Method: <u>Gravity</u>			Static Water Level (After Completion)
Setting Time: <u>>24 hours</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>n/a</u>	<u>n/a</u>	Top of Seal
Installation Method: <u>Gravity</u>	<u>612.75</u>	<u>7.52</u>	Top of Sand Pack
Setting Time: <u>25 minutes</u>			
Type of Sand Pack: <u>Quartz Sand</u>	<u>608.96</u>	<u>11.31</u>	Top of Screen
Grain Size: <u>10-20</u> (sieve size)	<u>604.31</u>	<u>15.96</u>	Bottom of Screen
Installation Method: <u>Gravity</u>	<u>604.06</u>	<u>16.21</u>	Bottom of Well
Type of Backfill Material: <u>n/a</u> (if applicable)			
Installation Method: _____	<u>604.06</u>	<u>16.21</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	13.56
Bottom of Screen to End Cap	(feet)	0.38
Screen Length (1st slot to last slot)	(feet)	4.65
Total Length of Casing	(feet)	18.59
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G302
Site Name: Natural Resource Technology, Inc. Coffeen Energy Center Borehole #: G302
State _____
Plane Coordinate: X 872,253.0 Y 2,516,214.2 (or) Latitude: _____ Longitude: _____
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Geotechnical Engineering, LLC Driller: D. Crump
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 9/3/2015 Date Finished: 9/4/2015
Report Form Completed By: Suzanna L. Keim Date: 10/7/2015

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER: Steel. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.

Surface Elevation: 619.10

Completion Date: 8/26/10

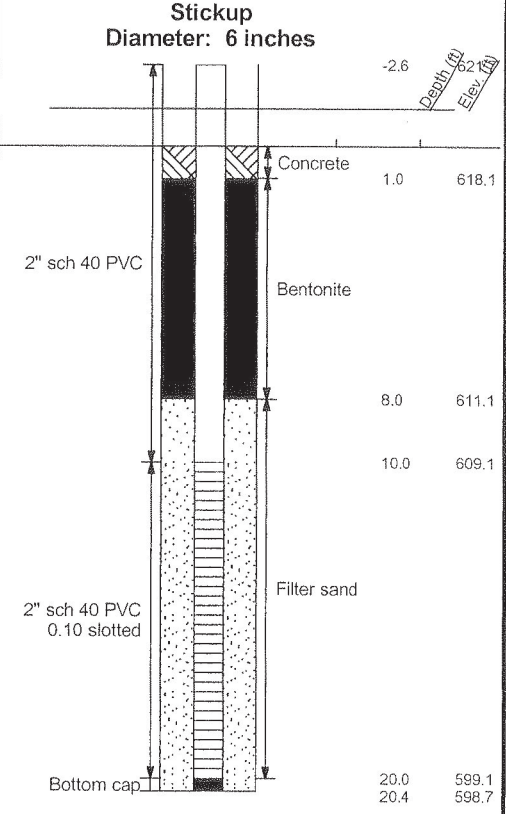
Northing: 871382.45

Easting: 2516641.06

Datum msl

WELL DIAGRAM

DEPTH IN FEET	DESCRIPTION OF MATERIAL	GRAPHIC LOG	DRY UNIT WEIGHT (pcf) SPT BLOW COUNTS CORE RECOVERY/RQD	SAMPLES
	FILL: brown, silty clay			
			1-2-3	SS1
	Medium stiff, grayish-brown, silty CLAY with lignite - CL			
5			1-3-3	SS2
			1-3-4	SS3
	Soft, brown, silty CLAY - CL			
10	Stiff to hard, brown to grayish-brown, silty CLAY - CL (TILL)			
			2-3-3	SS4
			27-44 -50/1"	SS5
15				
			44-50/2"	SS6
20	Boring terminated at 20 feet.			
25				
30				
35				



NOTE: STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARIES BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. GRAPHIC LOG FOR ILLUSTRATION PURPOSES ONLY.

GROUNDWATER DATA

ENCOUNTERED AT 9 FEET ∇

DRILLING DATA

___ AUGER 4 1/4" HOLLOW STEM
 WASHBORING FROM ___ FEET
MVU DRILLER SWG LOGGER
CME 55TRK DRILL RIG
 HAMMER TYPE Auto

REMARKS:

Drawn by: KSA Checked by: SK App'vd. by: KBS
 Date: 9/10/10 Date: 1-4-11 Date: 1/4/11



Ameren-Coffeen Ash Pond Evaluation

Renamed G303
 LOG OF BORING: APW-3

Project No. J017150.01

Surface Elevation: 623.46

Completion Date: 8/26/10

Northing: 871397.48

Easting: 2515520.23

Datum msl

WELL DIAGRAM

DEPTH IN FEET

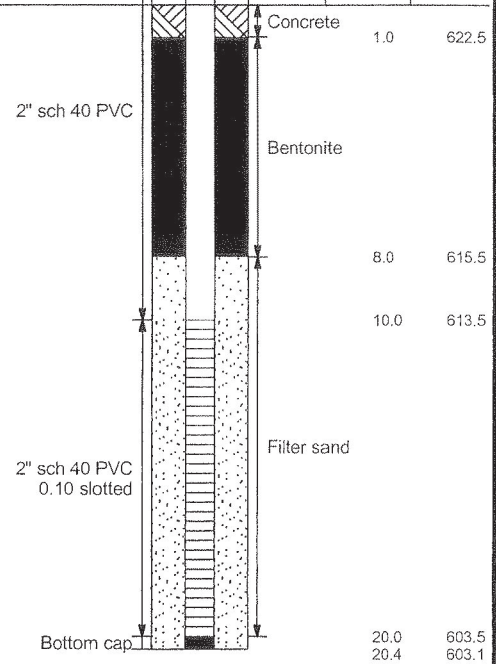
DESCRIPTION OF MATERIAL

GRAPHIC LOG

DRY UNIT WEIGHT (pcf)
SPT BLOW COUNTS
CORE RECOVERY/RQD

SAMPLES

Stickup
Diameter: 6 inches



Medium stiff, grayish-brown, silty CLAY - CL

0-1-1 SS1

1-2-5 SS2

1-3-4 SS3

1-2-3 SS4

Medium to fine SAND - SP
Hard, gray, silty CLAY - CL (TILL)

1-2-1 SS5

14-47 SS6

Boring terminated at 20 feet.

NOTE: STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARIES BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. GRAPHIC LOG FOR ILLUSTRATION PURPOSES ONLY.

LOG OF BORING 2002 WL J017150.01 - COFFEEN.GPJ GTINC 0638301.GPJ 12/13/10

GROUNDWATER DATA

ENCOUNTERED AT 14 FEET ∇

DRILLING DATA

 AUGER 4 1/4" HOLLOW STEM
WASHBORING FROM FEET
MVU DRILLER SWG LOGGER
CME 55TRK DRILL RIG
HAMMER TYPE Auto

REMARKS:

Drawn by: KSA Checked by: DK App'vd. by: RS
Date: 9/10/10 Date: 1-4-11 Date: 1/7/11



Ameren-Coffeen Ash Pond Evaluation

Renamed G304
LOG OF BORING: APW-4

Project No. J017150.01

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
DIVISION OF ENVIRONMENTAL HEALTH
525 W. JEFFERSON ST.
SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)** **Range** 3 **(E)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with _____ **from** _____ **to** _____ **ft.**

Kind of plug _____ **from** _____ **to** _____ **ft.**

Filled with _____ **from** _____ **to** _____ **ft.**

Kind of plug _____ **from** _____ **to** _____ **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: G305
Site Name: Coffeen Power Station Borehole #: G305
State _____
Plane Coordinate: X 871,156.3 Y 2,515,199.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Geotechnical Engineering LLC Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Suzanna L. Keim Date Started: 5/3/2016 Date Finished: 5/3/2016
Report Form Completed By: Suzanna L. Keim Date: 5/19/2016

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (625.88, -3.34), Top of Riser Pipe (625.55, -3.01), Ground Surface (622.54, 0.00), Top of Annular Sealant (621.54, 1.00), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (611.04, 11.50), Top of Screen (609.10, 13.44), Bottom of Screen (604.27, 18.27), Bottom of Well (604.09, 18.45), Bottom of Borehole (604.09, 18.45).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (16.45 feet), Bottom of Screen to End Cap (0.18 feet), Screen Length (4.83 feet), Total Length of Casing (21.46 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.



Site #: _____ County: Montgomery Well #: G306
Site Name: Coffeen Power Station Borehole #: G306
State _____
Plane Coordinate: X 871,141.0 Y 2,516,120.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Geotechnical Engineering LLC Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Suzanna L. Keim Date Started: 5/3/2016 Date Finished: 5/3/2016
Report Form Completed By: Suzanna L. Keim Date: 5/19/2016

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (626.12, -3.28), Top of Riser Pipe (625.72, -2.88), Ground Surface (622.84, 0.00), Top of Annular Sealant (621.84, 1.00), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (611.24, 11.60), Top of Screen (609.77, 13.07), Bottom of Screen (605.16, 17.68), Bottom of Well (604.98, 17.86), Bottom of Borehole (604.84, 18.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (15.95 feet), Bottom of Screen to End Cap (0.18 feet), Screen Length (1st slot to last slot) (4.61 feet), Total Length of Casing (20.74 feet), Screen Slot Size ** (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.



Site #: _____ County: Montgomery Well #: G307

Site Name: Coffeen Power Station Ash Pond 1 Borehole #: G307

State _____
Plane Coordinate: X 2,515,553.3 Y 871,398.6 (or) Latitude: _____ Longitude: _____

Surveyed By: _____ IL Registration #: _____

Drilling Contractor: Bulldog Drilling, Inc. Driller: J. Gates

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

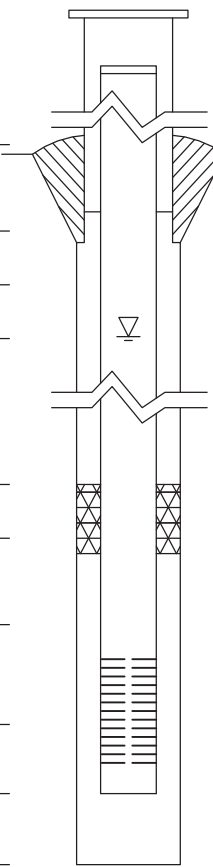
Drilling Method: Hollow Stem Auger Drilling Fluid (Type): none

Logged By: Rhonald W. Hasenyager Date Started: 7/26/2016 Date Finished: 7/27/2016

Report Form Completed By: Rhonald W. Hasenyager Date: 7/28/2016

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>624.72</u>	<u>-2.64</u>	Top of Protective Casing
	<u>624.47</u>	<u>-2.39</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>622.08</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>Bentonite Chips</u>	<u>620.08</u>	<u>2.00</u>	Top of Annular Sealant
Installation Method: <u>Gravity</u>			
Setting Time: <u>18 hrs.</u>			
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)			
Installation Method: _____	<u>n/a</u>	<u>n/a</u>	Top of Seal
Setting Time: _____			
Type of Sand Pack: <u>Quartz sand</u>	<u>610.10</u>	<u>11.98</u>	Top of Sand Pack
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>609.12</u>	<u>12.96</u>	Top of Screen
Type of Backfill Material: <u>none</u> (if applicable)	<u>604.28</u>	<u>17.80</u>	Bottom of Screen
Installation Method: <u>n/a</u>	<u>603.86</u>	<u>18.22</u>	Bottom of Well
	<u>603.86</u>	<u>18.22</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS

(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	15.37
Bottom of Screen to End Cap	(feet)	0.40
Screen Length (1st slot to last slot)	(feet)	4.84
Total Length of Casing	(feet)	20.61
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G401
Site Name: Natural Resource Technology, Inc. Coffeen Energy Center Borehole #: G401
State _____
Plane Coordinate: X 872,510.6 Y 2,515,614.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Geotechnical Engineering, LLC Driller: D. Crump
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Rhonald W. Hasenyager Date Started: 9/14/2015 Date Finished: 9/14/2015
Report Form Completed By: Suzanna L. Keim Date: 10/7/2015

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (625.84, -2.81), Top of Riser Pipe (625.57, -2.54), Ground Surface (623.03, 0.00), Top of Annular Sealant (621.33, 1.70), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (610.12, 12.91), Top of Screen (608.67, 14.36), Bottom of Screen (604.24, 18.79), Bottom of Well (603.74, 19.29), Bottom of Borehole (603.73, 19.30).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (16.70 feet), Bottom of Screen to End Cap (0.50 feet), Screen Length (4.63 feet), Total Length of Casing (21.83 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.

Surface Elevation: 610.56

Completion Date: 8/27/10

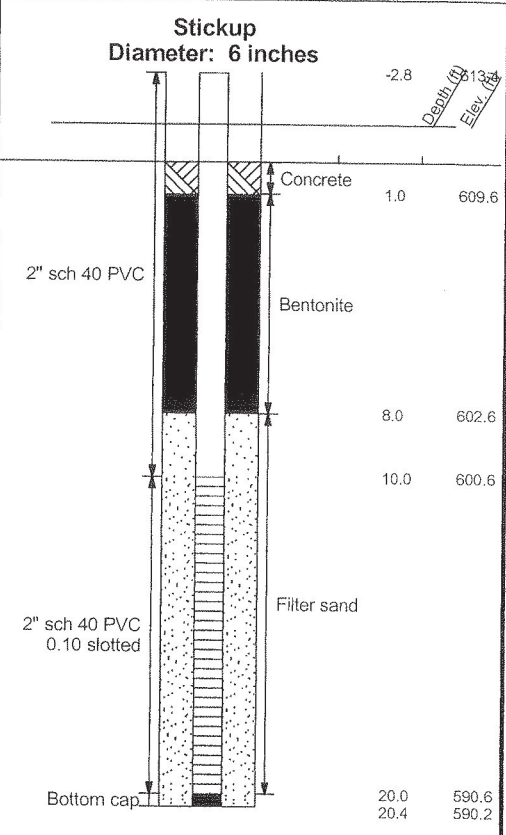
Datum msl

Northing: 872502.26

Easting: 2516632.59

WELL DIAGRAM

DEPTH IN FEET	DESCRIPTION OF MATERIAL	GRAPHIC LOG	DRY UNIT WEIGHT (pcf) SPT BLOW COUNTS CORE RECOVERY/RQD	SAMPLES
	Medium stiff, brown, silty CLAY - CL			
			6-3-3	SS1
			1-2-4	SS2
5	Medium stiff, gray, silty CLAY, trace sand - CL			
			1-2-5	SS3
10	Hard to stiff, brown to gray, silty CLAY, trace sand seams - CL (TILL)			
			9-22-28	SS4
15			1-6-12	SS5
20	Boring terminated at 20 feet.		3-4-9	SS6



NOTE: STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARIES BETWEEN SOIL TYPES AND THE TRANSITION MAY BE GRADUAL. GRAPHIC LOG FOR ILLUSTRATION PURPOSES ONLY.

LOG OF BORING, 2002.WL, J017150.01 - COFFEEN.GPJ, STINC 0638301.GPJ, 12/13/10

GROUNDWATER DATA

DRILLING DATA

ENCOUNTERED AT 7 FEET ∇

 AUGER 4 1/4" HOLLOW STEM
WASHBORING FROM FEET
MVU DRILLER SWG LOGGER
CME 55TRK DRILL RIG
HAMMER TYPE Auto

REMARKS:

Drawn by: KSA	Checked by: <u>DTK</u>	App'vd. by: <u>KR8</u>
Date: 9/10/10	Date: <u>1-4-11</u>	Date: <u>1/4/11</u>



Ameren-Coffeen Ash Pond Evaluation

Renamed G402
LOG OF BORING: APW-2

Project No. J017150.01



Site #: _____ County: Montgomery Well #: G403
Site Name: Natural Resource Technology, Inc. Coffeen Energy Center Borehole #: G403
State _____
Plane Coordinate: X 873,561.3 Y 2,514,616.6 (or) Latitude: _____ Longitude: _____
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Geotechnical Engineering, LLC Driller: D. Crump
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 9/11/2015 Date Finished: 9/11/2015
Report Form Completed By: Suzanna L. Keim Date: 10/7/2015

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (626.72, -2.91), Top of Riser Pipe (626.47, -2.66), Ground Surface (623.81, 0.00), Top of Annular Sealant (621.81, 2.00), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (612.64, 11.17), Top of Screen (610.70, 13.11), Bottom of Screen (606.03, 17.78), Bottom of Well (605.66, 18.15), Bottom of Borehole (605.66, 18.15).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (15.77 feet), Bottom of Screen to End Cap (0.37 feet), Screen Length (4.67 feet), Total Length of Casing (20.81 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.



Site #: _____ County: Montgomery Well #: G404
Site Name: Coffeen Energy Center Borehole #: SB22
State _____
Plane Coordinate: X 2,516,397.9 Y 873,999.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: A. Rachford
Consulting Firm: Hanson Professional Services Inc. Geologist: _____, LPG # _____
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Rhonald W. Hasenyager Date Started: 5/1/2007 Date Finished: 5/1/2007
Report Form Completed By: Rhonald W. Hasenyager Date: 5/2/2007

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (616.02, -2.92), Top of Riser Pipe (615.77, -2.67), Ground Surface (613.10, 0.00), Top of Annular Sealant (613.10, 0.00), Static Water Level (611.03, 2.07), Top of Seal (n/a, n/a), Top of Sand Pack (608.05, 5.05), Top of Screen (606.68, 6.42), Bottom of Screen (601.93, 11.17), Bottom of Well (601.48, 11.62), Bottom of Borehole (601.10, 12.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (9.09 feet), Bottom of Screen to End Cap (0.45 feet), Screen Length (4.75 feet), Total Length of Casing (14.29 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G405
Site Name: Coffeen Energy Center Borehole #: SB21
State _____
Plane Coordinate: X 2,515,335.7 Y 873,996.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: A. Rachford
Consulting Firm: Hanson Professional Services Inc. Geologist: , LPG #
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Rhonald W. Hasenyager Date Started: 5/1/2007 Date Finished: 5/1/2007
Report Form Completed By: Rhonald W. Hasenyager Date: 5/2/2007

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Key data points include: Top of Protective Casing (624.04 MSL, -3.14 BGS), Top of Riser Pipe (623.78 MSL, -2.88 BGS), Ground Surface (620.90 MSL, 0.00 BGS), Top of Annular Sealant (620.90 MSL, 0.00 BGS), Static Water Level (619.67 MSL, 1.23 BGS), Top of Seal (n/a MSL, n/a BGS), Top of Sand Pack (613.19 MSL, 7.71 BGS), Top of Screen (611.89 MSL, 9.01 BGS), Bottom of Screen (607.14 MSL, 13.76 BGS), Bottom of Well (606.69 MSL, 14.21 BGS), Bottom of Borehole (606.69 MSL, 14.21 BGS).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (11.89 feet), Bottom of Screen to End Cap (0.45 feet), Screen Length (4.75 feet), Total Length of Casing (17.09 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G406
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: G406
State _____
Plane Coordinate: X 2,514,702.4 Y 872,521.3 (or) Latitude: 39° 3' 37.114" Longitude: -89° 23' 54.628"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Bulldog Drilling, Inc. Driller: J. Dittmaier
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 8/19/2016 Date Finished: 8/19/2016
Report Form Completed By: Suzanna L. Keim Date: 8/24/2016

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (625.70, -3.84), Top of Riser Pipe (625.36, -3.50), Ground Surface (621.86, 0.00), Top of Annular Sealant (619.86, 2.00), Static Water Level (After Completion), Top of Seal (610.74, 11.12), Top of Sand Pack (609.65, 12.21), Top of Screen (608.30, 13.56), Bottom of Screen (603.49, 18.37), Bottom of Well (603.11, 18.75), Bottom of Borehole (603.11, 18.75).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.06 feet), Bottom of Screen to End Cap (0.38 feet), Screen Length (4.81 feet), Total Length of Casing (22.25 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G407
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: G407
State _____
Plane Coordinate: X 2,513,705.9 Y 872,973.4 (or) Latitude: 39° 3' 41.665" Longitude: -89° 24' 7.213"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Bulldog Drilling, Inc. Driller: J. Dittmaier
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 8/16/2016 Date Finished: 8/16/2016
Report Form Completed By: Suzanna L. Keim Date: 8/24/2016

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (621.70, -3.35), Top of Riser Pipe (621.32, -2.97), Ground Surface (618.35, 0.00), Top of Annular Sealant (616.35, 2.00), Static Water Level (After Completion), Top of Seal (607.50, 10.85), Top of Sand Pack (605.50, 12.85), Top of Screen (604.57, 13.78), Bottom of Screen (599.74, 18.61), Bottom of Well (599.31, 19.04), Bottom of Borehole (598.35, 20.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (16.75 feet), Bottom of Screen to End Cap (0.43 feet), Screen Length (4.83 feet), Total Length of Casing (22.01 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G410
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: G410
State _____
Plan Coordinate: X 2,513,206.3 Y 872,968.5 (or) Latitude: 39° 3' 41.658" Longitude: -89° 24' 13.546"
Surveyed By: Matthew H. Schrader IL Registration #: 035-003487
Drilling Contractor: Bulldog Drilling, Inc. Driller: C. Dutton
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Rhonald W. Hasenyager Date Started: 2/23/2018 Date Finished: 2/23/2018
Report Form Completed By: Suzanna L. Keim Date: 2/26/2018

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete
Type of Annular Sealant: _____
Installation Method: _____
Setting Time: _____
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: >24 hours
Type of Sand Pack: Filter Sand
Grain Size: 20/40 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

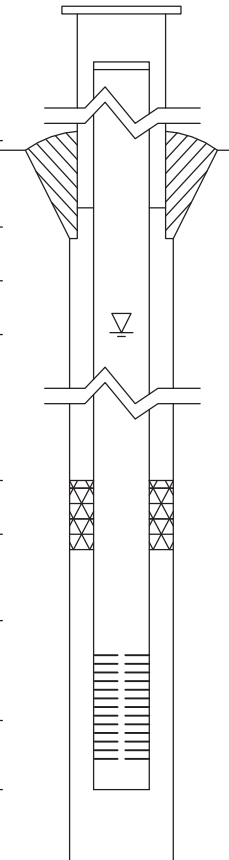


Table with 3 columns: Elevation (MSL)*, Depth (BGS), and Description. Rows include: Top of Protective Casing (620.18, -2.97), Top of Riser Pipe (619.79, -2.58), Ground Surface (617.21, 0.00), Top of Annular Sealant (n/a), Static Water Level (After Completion) 3/1/2018 (612.39, 4.82), Top of Seal (616.71, 0.50), Top of Sand Pack (609.81, 7.40), Top of Screen (608.32, 8.89), Bottom of Screen (603.53, 13.68), Bottom of Well (603.12, 14.09), Bottom of Borehole (603.12, 14.09).

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).

CASING MEASUREMENTS

Table with 2 columns: Measurement and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (11.47 feet), Bottom of Screen to End Cap (0.41 feet), Screen Length (1st slot to last slot) (4.79 feet), Total Length of Casing (16.67 feet), Screen Slot Size (0.010 inches).



Site #: _____ County: Montgomery Well #: G411
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: G411
State _____
Plan Coordinate: X 2,513,122.4 Y 873,844.8 (or) Latitude: 39° 3' 50.326" Longitude: -89° 24' 14.517"
Surveyed By: Matthew H. Schrader IL Registration #: 035-003487
Drilling Contractor: Bulldog Drilling, Inc. Driller: C. Dutton
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Rhonald W. Hasenyager Date Started: 2/22/2018 Date Finished: 2/22/2018
Report Form Completed By: Suzanna L. Keim Date: 2/26/2018

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete
Type of Annular Sealant: _____
Installation Method: _____
Setting Time: _____
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: >24 hours
Type of Sand Pack: Filter Sand
Grain Size: 20/40 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

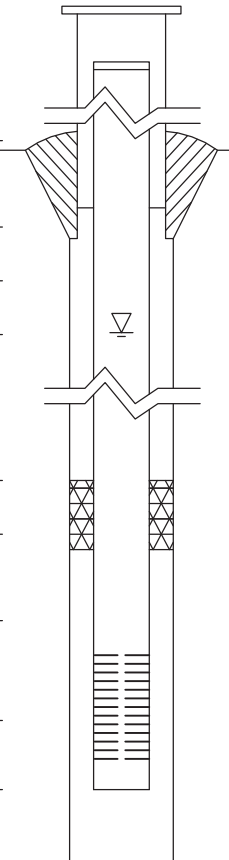


Table with 3 columns: Elevation (MSL)*, Depth (BGS), and Description. Rows include: Top of Protective Casing (623.60, -3.11), Top of Riser Pipe (623.25, -2.76), Ground Surface (620.49, 0.00), Top of Annular Sealant (n/a), Static Water Level (617.84, 2.65), Top of Seal (619.99, 0.50), Top of Sand Pack (610.42, 10.07), Top of Screen (609.28, 11.21), Bottom of Screen (604.42, 16.07), Bottom of Well (604.02, 16.47), Bottom of Borehole (604.02, 16.47).

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (13.97 feet), Bottom of Screen to End Cap (0.40 feet), Screen Length (4.86 feet), Total Length of Casing (19.23 feet), Screen Slot Size (0.010 inches).



Site #: _____ County: Montgomery Well #: MW1D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-01
State _____
Plane Coordinate: X 874,972.6 Y 2,513,478.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 5/3/2006 Date Finished: 5/3/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
 DIVISION OF ENVIRONMENTAL HEALTH
 525 W. JEFFERSON ST.
 SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
 LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)**~~(S)~~ **Range** 3 ~~(E)~~**(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: MW2D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-02b
State _____
Plane Coordinate: X 876,414.0 Y 2,513,209.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 5/5/2006 Date Finished: 5/5/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (25.00 feet), Bottom of Screen to End Cap (0.39 feet), Screen Length (4.80 feet), Total Length of Casing (30.19 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS

Table for well construction materials with columns for material type (Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen) and material options (SS304, SS316, PTFE, PVC, OTHER: Steel).

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
DIVISION OF ENVIRONMENTAL HEALTH
525 W. JEFFERSON ST.
SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

**RETURN ALL COPIES TO IDPH OR
LOCAL HEALTH DEPARTMENT**

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)**~~(S)~~ **Range** 3 ~~(E)~~**(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

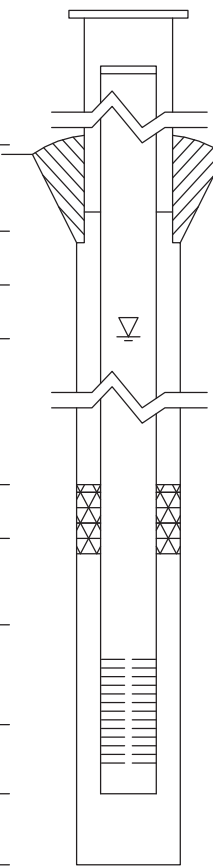
This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: MW2S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-02a
State _____
Plane Coordinate: X 876,408.9 Y 2,513,210.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 5/5/2006 Date Finished: 5/5/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.)



WELL CONSTRUCTION MATERIALS

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
DIVISION OF ENVIRONMENTAL HEALTH
525 W. JEFFERSON ST.
SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)**~~(S)~~ **Range** 3 ~~(E)~~**(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

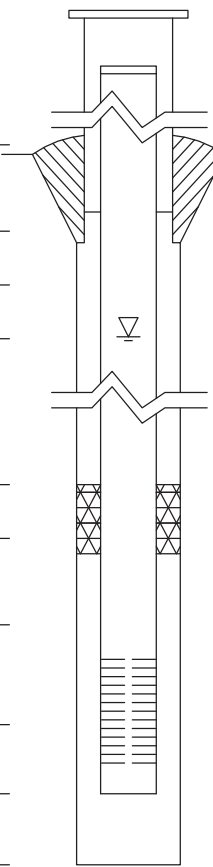
This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: MW3D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-03
State _____
Plane Coordinate: X 876,554.5 Y 2,514,535.3 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 4/27/2006 Date Finished: 4/27/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.)



WELL CONSTRUCTION MATERIALS

Table for well construction materials with columns for material type and selection options.

CASING MEASUREMENTS

Table for casing measurements with columns for measurement type and value.



Site #: _____ County: Montgomery Well #: MW4S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-04a
State _____
Plane Coordinate: X 877,999.7 Y 2,514,450.6 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 5/11/2006 Date Finished: 5/11/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.) values. Includes a central diagram of a well casing and screen assembly.

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Lists measurements such as Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, etc.

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

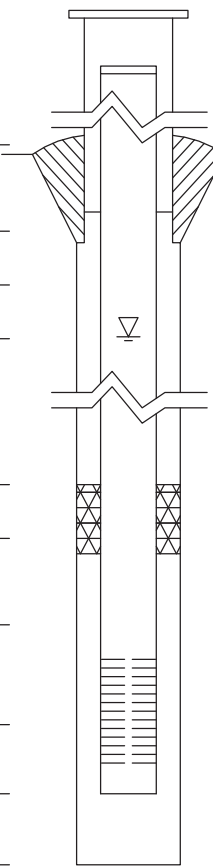
Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Lists materials for Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.



Site #: _____ County: Montgomery Well #: MW5D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-05
State _____
Plane Coordinate: X 878,174.8 Y 2,513,290.3 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 5/12/2006 Date Finished: 5/17/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.)



Type of Surface Seal: Concrete
Type of Annular Sealant: Bentonite grout
Installation Method: Tremie
Setting Time: +24 hr.
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: 18 min.
Type of Sand Pack: Quartz sand
Grain Size: #5 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

WELL CONSTRUCTION MATERIALS

Table with 6 columns: Material type, SS304, SS316, PTFE, PVC, OTHER: (Steel)

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value



Site #: _____ County: Montgomery Well #: MW55
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-05a
State _____
Plane Coordinate: X 878,175.6 Y 2,513,285.5 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 5/17/2006 Date Finished: 5/17/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths. Includes data for Protective Casing, Riser Pipe, Ground Surface, Annular Sealant, Static Water Level, Seal, Sand Pack, Screen, and Bottom of Well/Borehole.

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (15.69 feet), Bottom of Screen to End Cap (0.40 feet), Screen Length (4.75 feet), Total Length of Casing (20.84 feet), and Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table for material selection with columns for Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen. Materials listed include SS304, SS316, PTFE, PVC, and OTHER (Steel, PVC).



Site #: _____ County: Montgomery Well #: MW6S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-06a
State _____
Plane Coordinate: X 879,021.2 Y 2,513,189.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Testing Services Corp. Date Started: 5/4/2006 Date Finished: 5/4/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (626.67, -3.57), Top of Riser Pipe (626.21, -3.11), Ground Surface (623.10, 0.00), Top of Annular Sealant (620.10, 3.00), Static Water Level (616.89, 6.21), Top of Seal (620.10, 3.00), Top of Sand Pack (613.34, 9.76), Top of Screen (612.06, 11.04), Bottom of Screen (607.48, 15.62), Bottom of Well (607.02, 16.08), Bottom of Borehole (607.02, 16.08).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (7.3 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (14.15 feet), Bottom of Screen to End Cap (0.46 feet), Screen Length (4.58 feet), Total Length of Casing (19.19 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW7S

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-07a

State _____
Plane Coordinate: X 879,181.1 Y 2,514,397.5 (or) Latitude: _____° _____' _____" Longitude: _____° _____' _____"

Surveyed By: Darren E. Forgy IL Registration #: 035-003637

Drilling Contractor: Reynolds Drilling Corp. Driller: P. McIntire

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246

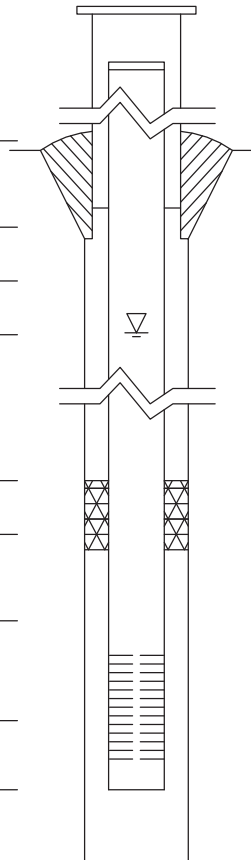
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water

Logged By: Rhonald W Hasenyager Date Started: 5/9/2006 Date Finished: 5/9/2006

Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
<u>627.71</u>	<u>-3.21</u>	Top of Protective Casing
<u>627.56</u>	<u>-3.06</u>	Top of Riser Pipe
<u>624.50</u>	<u>0.00</u>	Ground Surface
<u>621.70</u>	<u>2.80</u>	Top of Annular Sealant
<u>619.60</u>	<u>4.90</u>	Static Water Level (After Completion) 6/1/2006
<u>621.70</u>	<u>2.80</u>	Top of Seal
<u>616.23</u>	<u>8.27</u>	Top of Sand Pack
<u>614.59</u>	<u>9.91</u>	Top of Screen
<u>610.71</u>	<u>13.79</u>	Bottom of Screen
<u>610.11</u>	<u>14.39</u>	Bottom of Well
<u>610.11</u>	<u>14.39</u>	Bottom of Borehole



Type of Surface Seal: Concrete

Type of Annular Sealant: Bentonite chips

Installation Method: Gravity

Setting Time: +24 hr.

Type of Bentonite Seal -- Granular Pellet Slurry
(choose one)

Installation Method: Gravity

Setting Time: +24 hr.

Type of Sand Pack: Quartz sand

Grain Size: #JC50FS (sieve size)

Installation Method: Gravity

Type of Backfill Material: n/a
(if applicable)

Installation Method: n/a

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	12.37
Bottom of Screen to End Cap	(feet)	0.60
Screen Length (1st slot to last slot)	(feet)	4.48
Total Length of Casing	(feet)	17.45
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

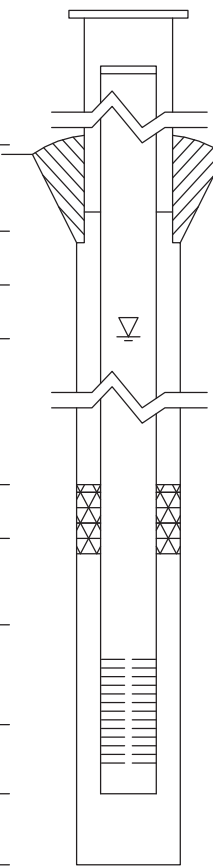
Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:



Site #: _____ County: Montgomery Well #: MW8S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-08a
State _____
Plane Coordinate: X 879,776.6 Y 2,514,478.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/10/2006 Date Finished: 5/10/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.)



WELL CONSTRUCTION MATERIALS

Table for well construction materials with columns for material type and selection options.

CASING MEASUREMENTS

Table for casing measurements with columns for measurement type and value.



Site #: _____ County: Montgomery Well #: MW9D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-09
State _____
Plane Coordinate: X 879,679.7 Y 2,515,666.3 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/3/2006 Date Finished: 5/3/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (627.84, -3.24), Top of Riser Pipe (627.52, -2.92), Ground Surface (624.60, 0.00), Top of Annular Sealant (621.70, 2.90), Static Water Level (572.14, 52.46), Top of Seal (582.60, 42.00), Top of Sand Pack (580.80, 43.80), Top of Screen (578.79, 45.81), Bottom of Screen (574.03, 50.57), Bottom of Well (573.60, 51.00), Bottom of Borehole (570.60, 54.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (52.25 feet), Bottom of Screen to End Cap (0.43 feet), Screen Length (4.76 feet), Total Length of Casing (57.44 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

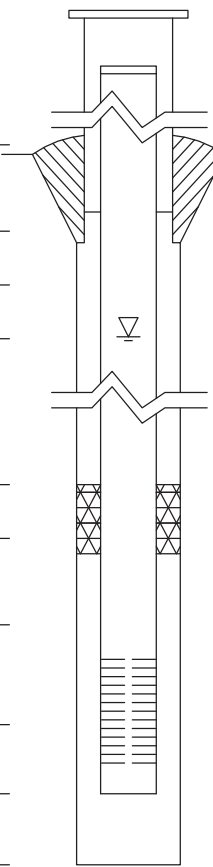
Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW9S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-09a
State _____
Plane Coordinate: X 879,684.9 Y 2,515,666.2 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/3/2006 Date Finished: 5/3/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Rows include: Top of Protective Casing (627.84, -3.24), Top of Riser Pipe (627.51, -2.91), Ground Surface (624.60, 0.00), Top of Annular Sealant (621.35, 3.25), Static Water Level (619.37, 5.23), Top of Seal (621.35, 3.25), Top of Sand Pack (615.49, 9.11), Top of Screen (613.39, 11.21), Bottom of Screen (608.98, 15.62), Bottom of Well (608.40, 16.20), Bottom of Borehole (608.40, 16.20).



Type of Surface Seal: Concrete
Type of Annular Sealant: Bentonite chips
Installation Method: Gravity
Setting Time: +24 hr.
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: +24 hr.
Type of Sand Pack: Quartz sand
Grain Size: #JC50FS (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (14.12 feet), Bottom of Screen to End Cap (0.58 feet), Screen Length (4.41 feet), Total Length of Casing (19.11 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW10D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-10
State _____
Plane Coordinate: X 878,245.1 Y 2,515,914.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/1/2006 Date Finished: 5/1/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

WELL CONSTRUCTION MATERIALS

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: MW10S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-10a
State _____
Plane Coordinate: X 878,250.5 Y 2,515,914.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/2/2006 Date Finished: 5/2/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (624.55, -3.35), Top of Riser Pipe (624.24, -3.04), Ground Surface (621.20, 0.00), Top of Annular Sealant (618.83, 2.37), Static Water Level (616.29, 4.91), Top of Seal (618.83, 2.37), Top of Sand Pack (611.90, 9.30), Top of Screen (609.92, 11.28), Bottom of Screen (605.44, 15.76), Bottom of Well (604.90, 16.30), Bottom of Borehole (604.90, 16.30).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (14.32 feet), Bottom of Screen to End Cap (0.54 feet), Screen Length (4.48 feet), Total Length of Casing (19.34 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW11D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-11
State _____
Plane Coordinate: X 876,749.6 Y 2,515,976.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 4/27/2006 Date Finished: 4/28/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (625.70, -3.70), Top of Riser Pipe (625.36, -3.36), Ground Surface (622.00, 0.00), Top of Annular Sealant (618.89, 3.11), Static Water Level (615.97, 6.03), Top of Seal (597.20, 24.80), Top of Sand Pack (595.59, 26.41), Top of Screen (593.69, 28.31), Bottom of Screen (588.96, 33.04), Bottom of Well (588.50, 33.50), Bottom of Borehole (585.67, 36.33).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (31.67 feet), Bottom of Screen to End Cap (0.46 feet), Screen Length (4.73 feet), Total Length of Casing (36.86 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW11S

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-11a

State _____
Plane Coordinate: X 876,749.4 Y 2,515,971.2 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Darren E. Forgy IL Registration #: 035-003637

Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water

Logged By: Reynolds Drilling Corp. Date Started: 4/28/2006 Date Finished: 4/28/2006

Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Elevations (MSL)* Depths (BGS) (0.01 ft.)

Type of Surface Seal: Concrete

Type of Annular Sealant: Bentonite chips

Installation Method: Gravity

Setting Time: +24 hr.

Type of Bentonite Seal -- Granular Pellet Slurry (choose one)

Installation Method: Gravity

Setting Time: 26 min.

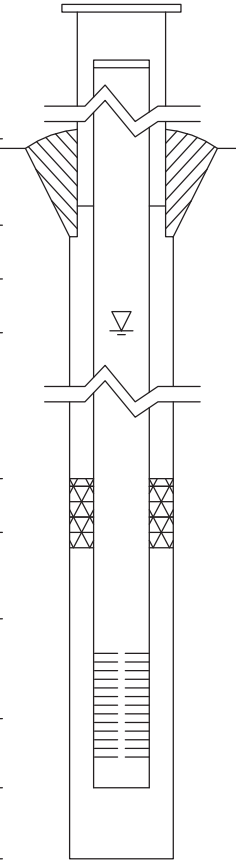
Type of Sand Pack: Quartz sand

Grain Size: #JC50FS (sieve size)

Installation Method: Gravity

Type of Backfill Material: n/a (if applicable)

Installation Method: n/a



<u>625.47</u>	<u>-3.47</u>	Top of Protective Casing
<u>625.16</u>	<u>-3.16</u>	Top of Riser Pipe
<u>622.00</u>	<u>0.00</u>	Ground Surface
<u>620.00</u>	<u>2.00</u>	Top of Annular Sealant
<u>616.58</u>	<u>5.42</u>	Static Water Level (After Completion) 6/1/2006
<u>620.00</u>	<u>2.00</u>	Top of Seal
<u>615.25</u>	<u>6.75</u>	Top of Sand Pack
<u>613.11</u>	<u>8.89</u>	Top of Screen
<u>608.37</u>	<u>13.63</u>	Bottom of Screen
<u>607.92</u>	<u>14.08</u>	Bottom of Well
<u>607.92</u>	<u>14.08</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

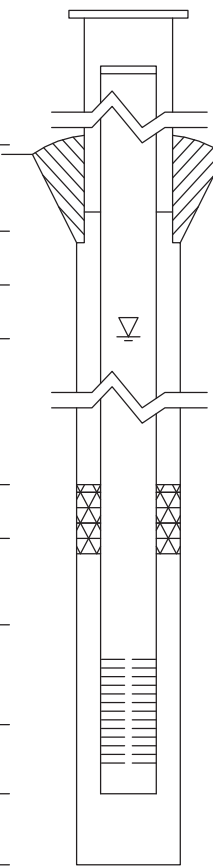
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	12.04
Bottom of Screen to End Cap	(feet)	0.46
Screen Length (1st slot to last slot)	(feet)	4.74
Total Length of Casing	(feet)	17.24
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: MW12D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-12
State _____
Plane Coordinate: X 875,515.1 Y 2,515,900.6 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/10/2006 Date Finished: 5/10/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.)



WELL CONSTRUCTION MATERIALS

Table for well construction materials with columns for material type and selection options.

CASING MEASUREMENTS

Table for casing measurements with columns for measurement type and value.



Site #: _____ County: Montgomery Well #: MW12S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-12a
State _____
Plane Coordinate: X 875,520.1 Y 2,515,900.5 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/10/2006 Date Finished: 5/10/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

WELL CONSTRUCTION MATERIALS

Table with columns for material type (Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen) and material options (SS304, SS316, PTFE, PVC, OTHER).

CASING MEASUREMENTS

Table with columns for measurement (Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, Screen Slot Size) and values.



Site #: County: Montgomery Well #: MW13D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-13
State Plane Coordinate: X 874,694.3 Y 2,513,929.9 (or) Latitude: Longitude:
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/9/2006 Date Finished: 5/9/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (626.33, -3.63), Top of Riser Pipe (625.87, -3.17), Ground Surface (622.70, 0.00), Top of Annular Sealant (619.64, 3.06), Static Water Level (566.67, 56.03), Top of Seal (577.48, 45.22), Top of Sand Pack (574.76, 47.94), Top of Screen (572.89, 49.81), Bottom of Screen (568.10, 54.60), Bottom of Well (567.70, 55.00), Bottom of Borehole (567.70, 55.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (52.98 feet), Bottom of Screen to End Cap (0.40 feet), Screen Length (4.79 feet), Total Length of Casing (58.17 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW13S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-13a
State _____
Plane Coordinate: X 874,695.7 Y 2,513,925.3 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/9/2006 Date Finished: 5/9/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: MW14S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: Sb-14a
State _____
Plane Coordinate: X 875,737.8 Y 2,514,125.9 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/2/2006 Date Finished: 5/2/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
DIVISION OF ENVIRONMENTAL HEALTH
525 W. JEFFERSON ST.
SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)**~~(S)~~ **Range** 3 ~~(E)~~**(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: MW15D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-15b
State _____
Plane Coordinate: X 875,970.5 Y 2,515,080.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 4/24/2006 Date Finished: 4/25/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
 DIVISION OF ENVIRONMENTAL HEALTH
 525 W. JEFFERSON ST.
 SPRINGFIELD, IL 62761

Sealing Form for MW15D - Original
 Mislabelled as MW15S

WATER WELL SEALING FORM

RETURN ALL COPIES TO IDPH OR
 LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

This form shall be submitted to this Department or the local health department not more than 30 days after a water well, boring or monitoring well is sealed. Such wells are to be sealed not more than 30 days after they are abandoned in accordance with the sealing requirements in the Water Well Construction Code. **THE LOCAL HEALTH DEPARTMENT OR REGIONAL PUBLIC HEALTH DEPARTMENT MUST BE NOTIFIED AT LEAST 48 HOURS PRIOR TO SEALING.**

1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)**~~(S)~~ **Range** 3 ~~(E)~~**(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: MW15S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-15a
State _____
Plane Coordinate: X 875,971.1 Y 2,515,076.3 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Testing Service Corporation Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 4/25/2006 Date Finished: 4/25/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (627.06, -3.26), Top of Riser Pipe (626.60, -2.80), Ground Surface (623.80, 0.00), Top of Annular Sealant (623.30, 0.50), Static Water Level (618.81, 4.99), Top of Seal (614.30, 9.50), Top of Sand Pack (611.60, 12.20), Top of Screen (609.39, 14.41), Bottom of Screen (604.64, 19.16), Bottom of Well (604.18, 19.62), Bottom of Borehole (604.18, 19.62).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.28 feet), Bottom of Screen to End Cap (19.62 feet), Screen Length (4.77 feet), Total Length of Casing (41.67 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).

ILLINOIS DEPARTMENT OF PUBLIC HEALTH
DIVISION OF ENVIRONMENTAL HEALTH
525 W. JEFFERSON ST.
SPRINGFIELD, IL 62761

WATER WELL SEALING FORM

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LOCAL HEALTH DEPARTMENT

TYPE OR PRESS FIRMLY

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1. **Ownership (Name of Controlling Party)** AEG Coffeen Power Station (G104)

2. **Well Location** 134 CIPS Trail Coffeen Montgomery
Address - Lot Number **City** **County**

General Description **Township** 7 **(N)**~~(S)~~ **Range** 3 ~~(E)~~**(W)** **Section** 10
SE **Quarter of the** NE **Quarter of the** NE **Quarter**

3. **Year Drilled** 2010

4. **Drilling Permit Number (and date, if known)** n/a

5. **Type of Well** **Bored** **Drilled** **Other**

6. **Total Depth** 20.0 ft. **Diameter (inches)** 2

7. **Formation clear of obstruction** **Yes** **No**

8. **DETAILS OF PLUGGING**

Filled with Bentonite grout **from** 0.5 **to** 20.0 **ft.**
(cement or other materials)

Kind of plug Random soil **from** 0 **to** 0.5 **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

Filled with **from** **to** **ft.**

Kind of plug **from** **to** **ft.**

9. **CASING RECORD** **Upper 2 feet of casing removed** **Yes** **No**

10. **Date well was sealed** **Month** October **Day** 8 **Year** 2010.

11. **Licensed water well driller or other person approved by the Department performing well sealing.**

Rhonald W. Hasenyager, L.P.G. 196-000246
Name **Complete License Number**

Hanson Professional Services Inc., 1525 S. 6th St. Springfield IL 62703
Address **City** **State/ZIP**

This state agency is requesting disclosure of information that is necessary to accomplish the statutory purpose as outlined under Public Act 85-0863. Disclosure of this information is mandatory. This form has been approved by the Forms Management Center. IL 482-0631



Site #: _____ County: Montgomery Well #: MW16D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-16b
State _____
Plane Coordinate: X 877,354.9 Y 2,515,079.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Rhonald W Hasenyager Date Started: 4/21/2006 Date Finished: 4/25/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: MW16S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-16a
State _____
Plane Coordinate: X 877,355.1 Y 2,515,088.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Rhonald W Hasenyager Date Started: 4/25/2006 Date Finished: 4/25/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (629.62, -3.52), Top of Riser Pipe (629.28, -3.18), Ground Surface (626.10, 0.00), Top of Annular Sealant (624.66, 1.44), Static Water Level (620.36, 5.74), Top of Seal (624.66, 1.44), Top of Sand Pack (612.40, 13.70), Top of Screen (611.51, 14.59), Bottom of Screen (606.69, 19.41), Bottom of Well (606.34, 19.76), Bottom of Borehole (606.20, 19.90).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.74 feet), Bottom of Screen to End Cap (0.38 feet), Screen Length (4.82 feet), Total Length of Casing (22.94 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW17D
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-17
State _____
Plane Coordinate: X 878,659.0 Y 2,515,090.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/4/2006 Date Finished: 5/4/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (630.62, -3.52), Top of Riser Pipe (630.29, -3.19), Ground Surface (627.10, 0.00), Top of Annular Sealant (624.92, 2.18), Static Water Level (572.65, 54.45), Top of Seal (581.55, 45.55), Top of Sand Pack (580.25, 46.85), Top of Screen (578.28, 48.82), Bottom of Screen (573.78, 53.32), Bottom of Well (573.23, 53.87), Bottom of Borehole (573.23, 53.87).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (52.01 feet), Bottom of Screen to End Cap (0.55 feet), Screen Length (4.50 feet), Total Length of Casing (57.06 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW17S
Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-17a
State _____
Plane Coordinate: X 878,658.5 Y 2,515,084.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Darren E. Forgy IL Registration #: 035-003637
Drilling Contractor: Reynolds Drilling Corp. Driller: K. Doetzel
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water
Logged By: Reynolds Drilling Corp. Date Started: 5/4/2006 Date Finished: 5/4/2006
Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (630.68 MSL, -3.58 BGS), Top of Riser Pipe (630.34 MSL, -3.24 BGS), Ground Surface (627.10 MSL, 0.00 BGS), Top of Annular Sealant (626.40 MSL, 0.70 BGS), Static Water Level (620.21 MSL, 6.89 BGS), Top of Seal (617.33 MSL, 9.77 BGS), Top of Sand Pack (614.80 MSL, 12.30 BGS), Top of Screen (613.08 MSL, 14.02 BGS), Bottom of Screen (603.54 MSL, 23.56 BGS), Bottom of Well (602.99 MSL, 24.11 BGS), Bottom of Borehole (602.99 MSL, 24.11 BGS).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.26 feet), Bottom of Screen to End Cap (0.55 feet), Screen Length (9.54 feet), Total Length of Casing (27.35 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: MW18S

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: SB-18a

State _____
Plane Coordinate: X 878,604.7 Y 2,513,745.2 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: Darren E. Forgy IL Registration #: 035-003637

Drilling Contractor: Reynolds Drilling Corp. Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W Hasenyager, LPG #196-000246

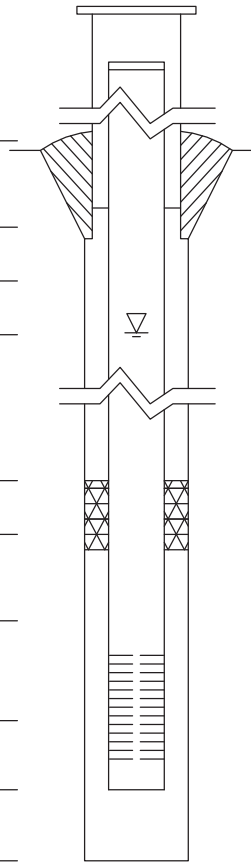
Drilling Method: Hollow stem auger Drilling Fluid (Type): Potable water

Logged By: Reynolds Drilling Corp. Date Started: 5/11/2006 Date Finished: 5/11/2006

Report Form Completed By: Rhonald W Hasenyager Date: 6/7/2006

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>629.02</u>	<u>-3.42</u>	Top of Protective Casing
	<u>628.71</u>	<u>-3.11</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>625.60</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>Bentonite chips</u>	<u>622.13</u>	<u>3.47</u>	Top of Annular Sealant
Installation Method: <u>Gravity</u>			
Setting Time: <u>+24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>618.73</u>	<u>6.87</u>	Static Water Level (After Completion) 6/1/2006
Installation Method: <u>Gravity</u>	<u>622.13</u>	<u>3.47</u>	Top of Seal
Setting Time: <u>25 min.</u>	<u>615.79</u>	<u>9.81</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>#JC50FS</u> (sieve size)	<u>614.29</u>	<u>11.31</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>609.81</u>	<u>15.79</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>609.20</u>	<u>16.40</u>	Bottom of Well
Installation Method: <u>n/a</u>	<u>609.20</u>	<u>16.40</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	14.42
Bottom of Screen to End Cap	(feet)	0.61
Screen Length (1st slot to last slot)	(feet)	4.48
Total Length of Casing	(feet)	19.51
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: MW20S

Site Name: Ash Pond Investigation Borehole #: SB20

State _____
Plane Coordinate: X 2,515,867.9 Y 874,226.4 (or) Latitude: _____ Longitude: _____

Surveyed By: Darren E. Forgy IL Registration #: 035-003637

Drilling Contractor: Reynolds Drilling Corp. Driller: A. Rachford

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): none

Logged By: Rhonald W. Hasenyager Date Started: 5/1/2007 Date Finished: 5/1/2007

Report Form Completed By: Rhonald W. Hasenyager Date: 5/2/2007

ANNULAR SPACE DETAILS

Elevations (MSL)* **Depths** (BGS) (0.01 ft.)

Type of Surface Seal: Concrete

Type of Annular Sealant: Bentonite chips

Installation Method: gravity

Setting Time: >12 hours

Type of Bentonite Seal -- Granular Pellet Slurry
(choose one)

Installation Method: _____

Setting Time: _____

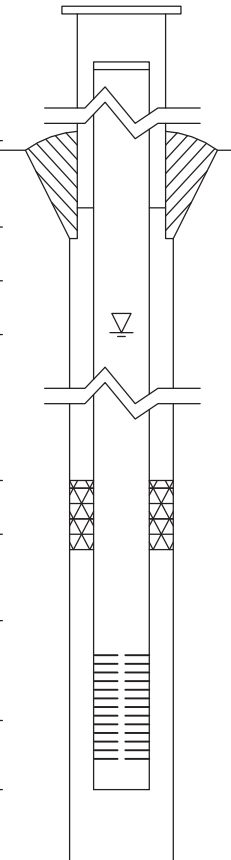
Type of Sand Pack: Quartz sand

Grain Size: 10/20 (sieve size)

Installation Method: gravity

Type of Backfill Material: _____
(if applicable)

Installation Method: _____



623.26	-2.96	Top of Protective Casing
623.00	-2.70	Top of Riser Pipe
620.30	0.00	Ground Surface
620.30	0.00	Top of Annular Sealant
616.02	4.28	Static Water Level (After Completion) 5/10/2007
n/a	n/a	Top of Seal
613.05	7.25	Top of Sand Pack
611.89	8.41	Top of Screen
607.08	13.22	Bottom of Screen
606.63	13.67	Bottom of Well
604.30	16.00	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	11.11
Bottom of Screen to End Cap	(feet)	0.45
Screen Length (1st slot to last slot)	(feet)	4.81
Total Length of Casing	(feet)	16.37
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: R104
Site Name: CCB Management Facility Borehole #: R104
State _____
Plane Coordinate: X 2,514,503.4 Y 875,857.8 (or) Latitude: 39° 4' 10.000" Longitude: -89° 23' 56.800"
Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507
Drilling Contractor: Layne-Western Co Driller: D. Mahurin
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a
Logged By: Rhonald W. Hasenyager Date Started: 10/8/2010 Date Finished: 10/8/2010
Report Form Completed By: Suzanna Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (632.02, -2.99), Top of Riser Pipe (631.84, -2.81), Ground Surface (629.03, 0.00), Top of Annular Sealant (627.03, 2.00), Static Water Level (612.85, 16.18), Top of Seal (617.03, 12.00), Top of Sand Pack (616.01, 13.02), Top of Screen (614.44, 14.59), Bottom of Screen (609.71, 19.32), Bottom of Well (609.18, 19.85), Bottom of Borehole (609.18, 19.85).

Type of Surface Seal: Concrete
Type of Annular Sealant: High-solids bentonite
Installation Method: Tremie
Setting Time: >24 hr.
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: 15 min
Type of Sand Pack: Quartz sand
Grain Size: 10/20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: n/a (if applicable)
Installation Method: n/a

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.60 feet), Bottom of Screen to End Cap (0.53 feet), Screen Length (4.53 feet), Total Length of Casing (22.66 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table with 2 columns: Material Area, Material Options. Rows include: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen.



Site #: _____ County: Montgomery Well #: R201

Site Name: CCB Management Facility Borehole #: R201

State _____
Plane Coordinate: X 2,514,842.0 Y 877,925.3 (or) Latitude: 39° 4' 30.500" Longitude: -89° 23' 52.300"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Rhonald W. Hasenyager Date Started: 10/15/2010 Date Finished: 10/15/2010

Report Form Completed By: Suzanna Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>626.51</u>	<u>-2.49</u>	Top of Protective Casing
	<u>626.34</u>	<u>-2.32</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>624.02</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>621.52</u>	<u>2.50</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>618.70</u>	<u>5.32</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>			
Setting Time: <u>48 min</u>			
Type of Sand Pack: <u>Quartz sand</u>	<u>614.47</u>	<u>9.55</u>	Top of Seal
Grain Size: <u>10/20</u> (sieve size)			
Installation Method: <u>Gravity</u>	<u>612.90</u>	<u>11.12</u>	Top of Sand Pack
Type of Backfill Material: <u>n/a</u> (if applicable)			
Installation Method: <u>n/a</u>			
	<u>611.75</u>	<u>12.27</u>	Top of Screen
	<u>607.36</u>	<u>16.66</u>	Bottom of Screen
	<u>606.80</u>	<u>17.22</u>	Bottom of Well
	<u>606.80</u>	<u>17.22</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	14.59
Bottom of Screen to End Cap	(feet)	0.56
Screen Length (1st slot to last slot)	(feet)	4.39
Total Length of Casing	(feet)	19.54
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>



Site #: _____ County: Montgomery Well #: R205
Site Name: Coffeen Power Station - Gypsum Mgmt Facility Borehole #: R205
State _____
Plane Coordinate: X 2,515,910.1 Y 875,548.8 (or) Latitude: 39° 4' 6.935" Longitude: -89° 23' 38.991"
Surveyed By: Gary C. Rogers IL Registration #: 035-02957
Drilling Contractor: Bulldog Drilling Driller: J. Gates
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow Stem Auger Drilling Fluid (Type): _____
Logged By: Rhonald W. Hasenyager Date Started: 3/20/2017 Date Finished: 3/20/2017
Report Form Completed By: Suzanna L. Keim Date: 3/27/2017

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.) descriptions. Includes a central diagram of the well casing and annular space details.

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Lists measurements such as Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, etc.

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Lists materials for Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.



Site #: _____ County: Montgomery Well #: T127

Site Name: CCB Management Facility Borehole #: T127

State _____
Plane Coordinate: X 2,513,911.0 Y 875,359.2 (or) Latitude: 39° 4' 5.200" Longitude: -89° 24' 4.400"

Surveyed By: _____ IL Registration #: _____

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: _____ Date Started: 2/10/2010 Date Finished: 2/10/2010

Report Form Completed By: _____ Date: 2/19/2010

ANNULAR SPACE DETAILS

Type of Surface Seal: Concrete

Type of Annular Sealant: High-solids bentonite

Installation Method: Tremie

Setting Time: >24 hr.

Type of Bentonite Seal -- Granular Pellet Slurry
(choose one)

Installation Method: Granular

Setting Time: 10 min

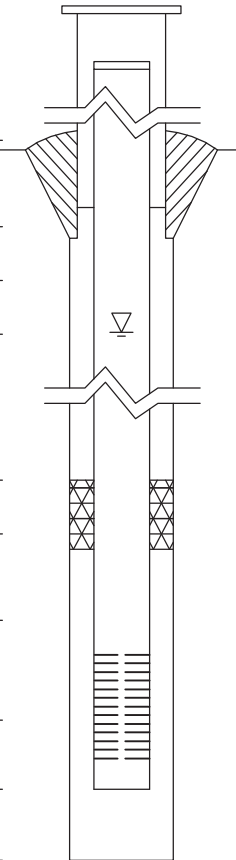
Type of Sand Pack: Quartz sand

Grain Size: 10/20 (sieve size)

Installation Method: Gravity

Type of Backfill Material: n/a
(if applicable)

Installation Method: n/a



Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
<u>631.29</u>	<u>-3.22</u>	Top of Protective Casing
<u>630.96</u>	<u>-2.89</u>	Top of Riser Pipe
<u>628.07</u>	<u>0.00</u>	Ground Surface
<u>622.43</u>	<u>5.64</u>	Top of Annular Sealant
<u>616.81</u>	<u>11.26</u>	Static Water Level (After Completion) 3/1/2010
<u>613.43</u>	<u>14.64</u>	Top of Seal
<u>612.32</u>	<u>15.75</u>	Top of Sand Pack
<u>610.54</u>	<u>17.53</u>	Top of Screen
<u>606.00</u>	<u>22.07</u>	Bottom of Screen
<u>605.43</u>	<u>22.64</u>	Bottom of Well
<u>605.43</u>	<u>22.64</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input type="text"/>
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER: <input type="text"/>

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	20.42
Bottom of Screen to End Cap	(feet)	0.57
Screen Length (1st slot to last slot)	(feet)	4.54
Total Length of Casing	(feet)	25.53
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: T128

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: T128

State- Plant
Plane Coordinate: X 2,513,909.5 Y 875,509.7 (or) Latitude: _____ ° _____ ' _____ " Longitude: _____ ° _____ ' _____ "

Surveyed By: _____ IL Registration #: _____

Drilling Contractor: Layne-Western Co Driller: T. List

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

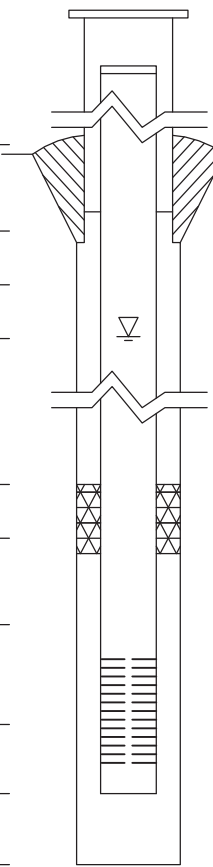
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 2/9/2010 Date Finished: 2/9/2010

Report Form Completed By: Suzanna L. Simpson Date: 2/18/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>631.23</u>	<u>-2.79</u>	Top of Protective Casing
	<u>630.93</u>	<u>-2.49</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>628.44</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>623.38</u>	<u>5.06</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>616.09</u>	<u>12.35</u>	Static Water Level (After Completion) 3/1/2010
Installation Method: <u>Granular</u>	<u>615.53</u>	<u>12.91</u>	Top of Seal
Setting Time: <u>10 min</u>	<u>614.13</u>	<u>14.31</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10/20</u> (sieve size)	<u>611.91</u>	<u>16.53</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>607.40</u>	<u>21.04</u>	Bottom of Screen
Type of Backfill Material: <u>Quartz sand</u> (if applicable)	<u>606.80</u>	<u>21.64</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>604.38</u>	<u>24.06</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	20.02
Bottom of Screen to End Cap	(feet)	0.60
Screen Length (1st slot to last slot)	(feet)	4.51
Total Length of Casing	(feet)	25.13
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: T202

Site Name: AEG Coffeen Power Station CCB Management Facility Borehole #: T202

State _____
Plane Coordinate: X 2,514,895.0 Y 876,699.4 (or) Latitude: 39° 4' 18.4" Longitude: -89° 23' 51.7"

Surveyed By: Jeffrey D. Emrick IL Registration #: 035-003507

Drilling Contractor: Layne-Western Co Driller: D. Mahurin

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

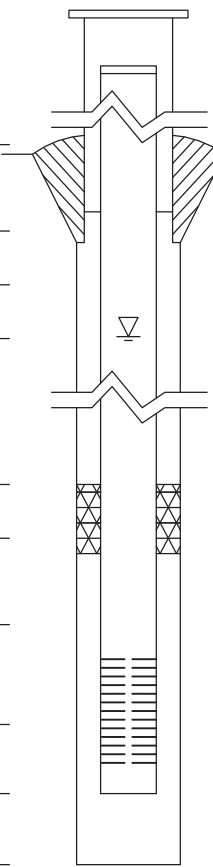
Drilling Method: Hollow stem auger Drilling Fluid (Type): n/a

Logged By: Suzanna L. Simpson Date Started: 10/15/2010 Date Finished: 10/15/2010

Report Form Completed By: Suzanna L. Simpson Date: 10/19/2010

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>628.79</u>	<u>-2.57</u>	Top of Protective Casing
	<u>628.63</u>	<u>-2.41</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>626.22</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>623.42</u>	<u>2.80</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>24 hr.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>613.72</u>	<u>12.50</u>	Static Water Level (After Completion) 11/15/2010
Installation Method: <u>Gravity</u>			
Setting Time: <u>15 min</u>			
Type of Sand Pack: <u>Quartz sand</u>	<u>616.50</u>	<u>9.72</u>	Top of Seal
Grain Size: <u>10/20</u> (sieve size)	<u>615.27</u>	<u>10.95</u>	Top of Sand Pack
Installation Method: <u>Gravity</u>			
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>613.95</u>	<u>12.27</u>	Top of Screen
Installation Method: <u>n/a</u>			
	<u>609.57</u>	<u>16.65</u>	Bottom of Screen
	<u>609.01</u>	<u>17.21</u>	Bottom of Well
	<u>608.22</u>	<u>18.00</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

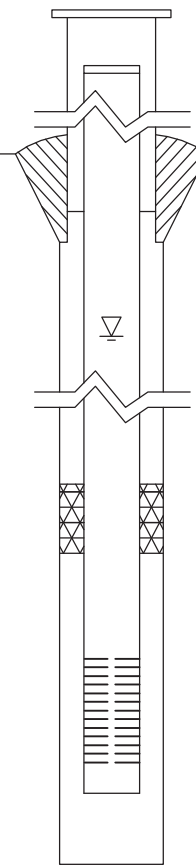
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	14.68
Bottom of Screen to End Cap	(feet)	0.56
Screen Length (1st slot to last slot)	(feet)	4.38
Total Length of Casing	(feet)	19.62
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: T408
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: T408
State _____
Plane Coordinate: X 2,515,314.9 Y 873,999.4 (or) Latitude: 39° 3' 51.671" Longitude: -89° 23' 46.704"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Bulldog Drilling, Inc. Driller: J. Dittmaier
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 8/17/2016 Date Finished: 8/17/2016
Report Form Completed By: Suzanna L. Keim Date: 8/24/2016

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes data for Top of Protective Casing, Top of Riser Pipe, Ground Surface, Top of Annular Sealant, Static Water Level, Top of Seal, Top of Sand Pack, Top of Screen, Bottom of Screen, Bottom of Well, and Bottom of Borehole.



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Includes Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (23.65 feet), Bottom of Screen to End Cap (0.43 feet), Screen Length (4.83 feet), Total Length of Casing (28.91 feet), and Screen Slot Size (0.010 inches).



Site #: _____ County: Montgomery Well #: T409
Site Name: Coffeen Power Station - Ash Pond 2 Borehole #: T409
State _____
Plane Coordinate: X 2,514,693.9 Y 872,517.8 (or) Latitude: 39° 3' 37.079" Longitude: -89° 23' 54.736"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Bulldog Drilling, Inc. Driller: J. Dittmaier
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): none
Logged By: Kristen L. Theesfeld Date Started: 8/19/2016 Date Finished: 8/19/2016
Report Form Completed By: Suzanna L. Keim Date: 8/24/2016

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.)

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>625.35</u>	<u>-3.50</u>	Top of Protective Casing
	<u>625.01</u>	<u>-3.16</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>621.85</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>619.85</u>	<u>2.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			Static Water Level (After Completion)
Setting Time: <u>>24 hours</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>602.65</u>	<u>19.20</u>	Top of Seal
Installation Method: <u>Gravity</u>	<u>601.30</u>	<u>20.55</u>	Top of Sand Pack
Setting Time: <u>30 minutes</u>	<u>600.06</u>	<u>21.79</u>	Top of Screen
Type of Sand Pack: <u>Quartz Sand</u>	<u>595.26</u>	<u>26.59</u>	Bottom of Screen
Grain Size: <u>10-20</u> (sieve size)	<u>594.86</u>	<u>26.99</u>	Bottom of Well
Installation Method: <u>Gravity</u>	<u>594.86</u>	<u>26.99</u>	Bottom of Borehole
Type of Backfill Material: _____ (if applicable)			
Installation Method: _____			

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	24.95
Bottom of Screen to End Cap	(feet)	0.40
Screen Length (1st slot to last slot)	(feet)	4.80
Total Length of Casing	(feet)	30.15
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:



Site #: _____ County: _____ Well #: TA31
Site Name: Coffeen Power Station Borehole #: TA31
State _____
Plane Coordinate: X 2,513,856.8 Y 876,542.2 (or) Latitude: 39° 4' 16.930" Longitude: 89° 24' 4.920"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow Stem Auger Drilling Fluid (Type): none
Logged By: Rhonald W. Hasenyager Date Started: 10/28/2014 Date Finished: 10/28/2014
Report Form Completed By: Rhonald W. Hasenyager Date: 11/5/2014

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER: Steel. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: _____ Well #: TA32
Site Name: Coffeen Power Station Borehole #: TA32
State _____
Plane Coordinate: X 2,513,605.2 Y 877,532.6 (or) Latitude: 39° 4' 26.730" Longitude: 89° 24' 8.000"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow Stem Auger Drilling Fluid (Type): none
Logged By: Rhonald W. Hasenyager Date Started: 10/27/2014 Date Finished: 10/27/2014
Report Form Completed By: Rhonald W. Hasenyager Date: 11/5/2014

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths. Includes data for Protective Casing, Riser Pipe, Ground Surface, Annular Sealant, Static Water Level, Seal, Sand Pack, Screen, and Bottom of Well. Elevation (MSL)* and Depth (BGS) are provided for each level.

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (13.94 feet), Bottom of Screen to End Cap (0.65 feet), Screen Length (4.37 feet), Total Length of Casing (18.96 feet), and Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.



Site #: _____ County: Montgomery Well #: TA33

Site Name: Coffeen Well Sealing & Assmt Well Install Borehole #: TA33b

State _____
Plane Coordinate: X 2,513,248.7 Y 876,605.4 (or) Latitude: 39° 4' 17.500" Longitude: 89° 24' 12.700"

Surveyed By: Gary C. Rogers IL Registration #: 035-002957

Drilling Contractor: Ramsey Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

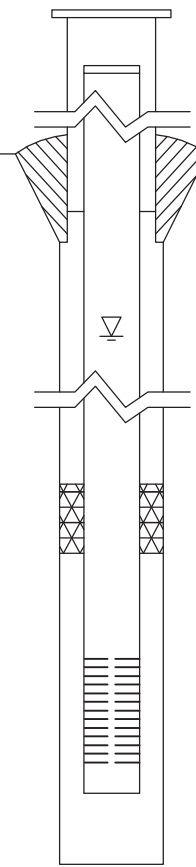
Drilling Method: Hollow Stem Auger Drilling Fluid (Type): None

Logged By: Suzanna L. Keim Date Started: 6/2/2015 Date Finished: 6/2/2015

Report Form Completed By: Suzanna L. Keim Date: 6/4/2015

ANNULAR SPACE DETAILS

	Elevations (MSL)*	Depths (BGS)	(0.01 ft.)
	<u>625.05</u>	<u>-2.54</u>	Top of Protective Casing
	<u>625.27</u>	<u>-2.76</u>	Top of Riser Pipe
Type of Surface Seal: <u>Concrete</u>	<u>622.51</u>	<u>0.00</u>	Ground Surface
Type of Annular Sealant: <u>High-solids bentonite</u>	<u>620.51</u>	<u>2.00</u>	Top of Annular Sealant
Installation Method: <u>Tremie</u>			
Setting Time: <u>>48 hrs.</u>			
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)	<u>615.51</u>	<u>7.00</u>	Static Water Level (After Completion) 6/16/2015
Installation Method: <u>Gravity</u>	<u>614.51</u>	<u>8.00</u>	Top of Seal
Setting Time: <u>30 minutes</u>	<u>612.11</u>	<u>10.40</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>			
Grain Size: <u>10-20</u> (sieve size)	<u>610.28</u>	<u>12.23</u>	Top of Screen
Installation Method: <u>Gravity</u>	<u>605.62</u>	<u>16.89</u>	Bottom of Screen
Type of Backfill Material: <u>n/a</u> (if applicable)	<u>605.07</u>	<u>17.44</u>	Bottom of Well
Installation Method: _____	<u>605.07</u>	<u>17.44</u>	Bottom of Borehole



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	<u>7.5</u>
ID of Riser Pipe	(inches)	<u>2.0</u>
Protective Casing Length	(feet)	<u>5.0</u>
Riser Pipe Length	(feet)	<u>14.99</u>
Bottom of Screen to End Cap	(feet)	<u>0.55</u>
Screen Length (1st slot to last slot)	(feet)	<u>4.66</u>
Total Length of Casing	(feet)	<u>20.20</u>
Screen Slot Size **	(inches)	<u>0.010</u>

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:



Site #: _____ County: Montgomery Well #: TA34
Site Name: Coffeen Well Sealing & Assmt Well Install Borehole #: TA34
State _____
Plane Coordinate: X 2,513,466.7 Y 875,906.1 (or) Latitude: 39° 4' 10.500" Longitude: 89° 24' 10.000"
Surveyed By: Gary C. Rogers IL Registration #: 035-002957
Drilling Contractor: Ramsey Driller: B. Williamson
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow Stem Auger Drilling Fluid (Type): None
Logged By: Suzanna L. Keim Date Started: 6/3/2015 Date Finished: 6/3/2015
Report Form Completed By: Suzanna L. Keim Date: 6/4/2015

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (626.77, -2.67), Top of Riser Pipe (626.52, -2.42), Ground Surface (624.10, 0.00), Top of Annular Sealant (623.10, 1.00), Static Water Level (616.00, 8.10), Top of Seal (n/a, n/a), Top of Sand Pack (615.10, 9.00), Top of Screen (613.18, 10.92), Bottom of Screen (608.69, 15.41), Bottom of Well (608.00, 16.10), Bottom of Borehole (608.00, 16.10).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement and Value. Measurements include: Diameter of Borehole (7.5 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (13.34 feet), Bottom of Screen to End Cap (0.69 feet), Screen Length (4.49 feet), Total Length of Casing (18.52 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 2 columns: Material Type and Material Selection. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: _____ Well #: TR32

Site Name: Coffeen Power Station Borehole #: TR32

State _____
Plan Coordinate: X 2,513,605.0 Y 877,523.7 (or) Latitude: 39° 4' 2663.000" Longitude: 89° 24' 8.070"

Surveyed By: Kyle J. Nolan IL Registration #: 035-003919

Drilling Contractor: Ramsey Geotechnical Engineering, LLC Driller: B. Williamson

Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246

Drilling Method: Hollow Stem Auger Drilling Fluid (Type): none

Logged By: Rhonald W. Hasenyager Date Started: 7/2/2019 Date Finished: 7/2/2019

Report Form Completed By: Rhonald W. Hasenyager Date: 7/3/2019

ANNULAR SPACE DETAILS

Elevations (MSL)* **Depths** (BGS) (0.01 ft.)

Type of Surface Seal: Concrete

Type of Annular Sealant: Bentonite

Installation Method: Gravity

Setting Time: 30 min.

Type of Bentonite Seal -- Granular Pellet Slurry
(choose one)

Installation Method: _____

Setting Time: _____

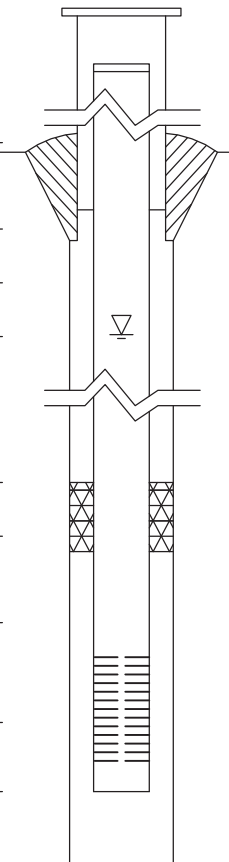
Type of Sand Pack: Quartz Sand

Grain Size: 10/20 (sieve size)

Installation Method: Gravity

Type of Backfill Material: n/a
(if applicable)

Installation Method: _____



<u>621.97</u>	<u>-2.69</u>	Top of Protective Casing
<u>621.68</u>	<u>-2.40</u>	Top of Riser Pipe
<u>619.28</u>	<u>0.00</u>	Ground Surface
<u>616.28</u>	<u>3.00</u>	Top of Annular Sealant
_____	_____	Static Water Level (After Completion)
<u>n/a</u>	<u>n/a</u>	Top of Seal
<u>609.77</u>	<u>9.51</u>	Top of Sand Pack
<u>608.28</u>	<u>11.00</u>	Top of Screen
<u>603.60</u>	<u>15.68</u>	Bottom of Screen
<u>603.11</u>	<u>16.17</u>	Bottom of Well
<u>603.11</u>	<u>16.17</u>	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="radio"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input type="radio"/> PVC	OTHER: _____
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input type="radio"/> PVC	OTHER: _____
Screen	SS304	SS316	PTFE	<input type="radio"/> PVC	OTHER: _____

CASING MEASUREMENTS

Diameter of Borehole	(inches)	<u>8.0</u>
ID of Riser Pipe	(inches)	<u>2.0</u>
Protective Casing Length	(feet)	<u>5.0</u>
Riser Pipe Length	(feet)	<u>13.40</u>
Bottom of Screen to End Cap	(feet)	<u>0.49</u>
Screen Length (1st slot to last slot)	(feet)	<u>4.68</u>
Total Length of Casing	(feet)	<u>18.57</u>
Screen Slot Size **	(inches)	<u>0.010</u>

**Hand-Slotted Well Screens Are Unacceptable



Site #: _____ County: Montgomery Well #: G307D
Site Name: Coffeen Part 845 Groundwater Borehole #: G307D
State _____
Plane Coordinate: X 2,515,560.3 Y 871,397.2 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/9/2021 Date Finished: 2/9/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: G308
Site Name: Coffeen Part 845 Groundwater Borehole #: G308
State _____
Plane Coordinate: X 2,515,101.4 Y 871,454.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/13/2021 Date Finished: 1/13/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.) descriptions. Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (624.96, -3.37), Top of Riser Pipe (624.59, -3.00), Ground Surface (621.59, 0.00), Top of Annular Sealant (621.39, 0.20), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (612.59, 9.00), Top of Screen (611.49, 10.10), Bottom of Screen (606.70, 14.89), Bottom of Well (606.35, 15.24), Bottom of Borehole (605.79, 15.80).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement and Value. Measurements include Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (13.10 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (4.79 feet), Total Length of Casing (18.24 feet), and Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 2 columns: Material Type and Material Selection. Rows include Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), and Screen (PVC).



Site #: _____ County: Montgomery Well #: G309
Site Name: Coffeen Part 845 Groundwater Borehole #: G309
State _____
Plane Coordinate: X 2,515,067.1 Y 871,865.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/12/2021 Date Finished: 1/12/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (626.20, -3.43), Top of Riser Pipe (625.88, -3.11), Ground Surface (622.77, 0.00), Top of Annular Sealant (622.27, 0.50), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (610.77, 12.00), Top of Screen (609.80, 12.97), Bottom of Screen (605.02, 17.75), Bottom of Well (604.67, 18.10), Bottom of Borehole (604.67, 18.10).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (16.08 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (4.78 feet), Total Length of Casing (21.21 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G310
Site Name: Coffeen Part 845 Groundwater Borehole #: G310
State _____
Plane Coordinate: X 2,515,159.4 Y 872,239.4 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/15/2021 Date Finished: 1/15/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Key data points include: Top of Protective Casing (623.32 MSL, -3.43 BGS), Top of Riser Pipe (622.87 MSL, -2.98 BGS), Ground Surface (619.89 MSL, 0.00 BGS), Top of Annular Sealant (619.39 MSL, 0.50 BGS), Top of Seal (n/a MSL, n/a BGS), Top of Sand Pack (610.69 MSL, 9.20 BGS), Top of Screen (609.65 MSL, 10.24 BGS), Bottom of Screen (604.86 MSL, 15.03 BGS), Bottom of Well (604.51 MSL, 15.38 BGS), Bottom of Borehole (603.99 MSL, 15.90 BGS).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Measurements include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (13.22 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (4.79 feet), Total Length of Casing (18.36 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G311
Site Name: Coffeen Part 845 Groundwater Borehole #: G311
State _____
Plane Coordinate: X 2,515,881.8 Y 872,238.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/5/2021 Date Finished: 2/5/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, sand pack, and screen.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Component, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: G311D
Site Name: Coffeen Part 845 Groundwater Borehole #: G311D
State _____
Plane Coordinate: X 2,515,881.8 Y 872,238.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/5/2021 Date Finished: 2/5/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well cross-section. Rows include: Top of Protective Casing (621.75, -3.36), Top of Riser Pipe (621.24, -2.85), Ground Surface (618.39, 0.00), Top of Annular Sealant (617.09, 1.30), Static Water Level (After Completion), Top of Seal (572.39, 46.00), Top of Sand Pack (569.39, 49.00), Top of Screen (568.23, 50.16), Bottom of Screen (558.29, 60.10), Bottom of Well (557.81, 60.58), Bottom of Borehole (557.81, 60.58).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (6.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (53.01 feet), Bottom of Screen to End Cap (0.48 feet), Screen Length (9.94 feet), Total Length of Casing (63.43 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G312
Site Name: Coffeen Part 845 Groundwater Borehole #: G312
State _____
Plane Coordinate: X 2,516,557.4 Y 872,260.9 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/4/2021 Date Finished: 2/4/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (620.11, -3.19), Top of Riser Pipe (619.78, -2.86), Ground Surface (616.92, 0.00), Top of Annular Sealant (615.92, 1.00), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (608.92, 8.00), Top of Screen (607.13, 9.79), Bottom of Screen (602.34, 14.58), Bottom of Well (601.99, 14.93), Bottom of Borehole (601.67, 15.25).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, and Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (12.65 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (4.79 feet), Total Length of Casing (17.79 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G313
Site Name: Coffeen Part 845 Groundwater Borehole #: G313
State _____
Plane Coordinate: X 2,516,803.7 Y 871,976.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/14/2021 Date Finished: 1/14/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (614.62, -3.11), Top of Riser Pipe (614.30, -2.79), Ground Surface (611.51, 0.00), Top of Annular Sealant (611.11, 0.40), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (606.51, 5.00), Top of Screen (605.21, 6.30), Bottom of Screen (600.40, 11.11), Bottom of Well (600.05, 11.46), Bottom of Borehole (599.51, 12.00).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement and Value. Measurements include Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (9.09 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (4.81 feet), Total Length of Casing (14.25 feet), and Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 2 columns: Material Type and Material Selection. Rows include Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), and Screen (PVC).



Site #: _____ County: Montgomery Well #: G314
Site Name: Coffeen Part 845 Groundwater Borehole #: G314
State _____
Plane Coordinate: X 2,516,852.1 Y 871,630.2 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Holcomb Foundation Engineering Co. Driller: Steve
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/26/2021 Date Finished: 2/26/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Data points include: Top of Protective Casing (614.28, -3.17), Top of Riser Pipe (613.88, -2.77), Ground Surface (611.11, 0.00), Top of Annular Sealant (609.11, 2.00), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (597.61, 13.50), Top of Screen (596.55, 14.56), Bottom of Screen (591.53, 19.58), Bottom of Well (591.09, 20.02), Bottom of Borehole (591.06, 20.05).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (17.30 feet), Bottom of Screen to End Cap (0.47 feet), Screen Length (5.02 feet), Total Length of Casing (22.79 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

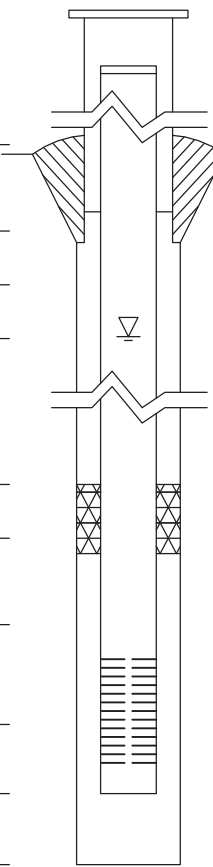
Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G314D
Site Name: Coffeen Part 845 Groundwater Borehole #: G314D
State _____
Plane Coordinate: X 2,516,853.9 Y 871,642.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/10/2021 Date Finished: 2/12/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.)
Type of Surface Seal: Concrete
Type of Annular Sealant: High-solids bentonite
Installation Method: Tremie
Setting Time: >24 hours
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: Gravity
Setting Time: 30 minutes
Type of Sand Pack: Quartz sand
Grain Size: 10-20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: N/A (if applicable)
Installation Method: _____



WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER: [selected material]

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value



Site #: _____ County: Montgomery Well #: G315
Site Name: Coffeen Part 845 Groundwater Borehole #: G315
State _____
Plane Coordinate: X 2,516,086.6 Y 871,385.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/13/2021 Date Finished: 1/13/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 3 columns: Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen. Data points include: Top of Protective Casing (623.89, -2.95), Top of Riser Pipe (623.52, -2.58), Ground Surface (620.94, 0.00), Top of Annular Sealant (620.54, 0.40), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (612.64, 8.30), Top of Screen (611.25, 9.69), Bottom of Screen (606.46, 14.48), Bottom of Well (606.09, 14.85), Bottom of Borehole (605.04, 15.90).

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (12.27 feet), Bottom of Screen to End Cap (0.37 feet), Screen Length (4.79 feet), Total Length of Casing (17.43 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

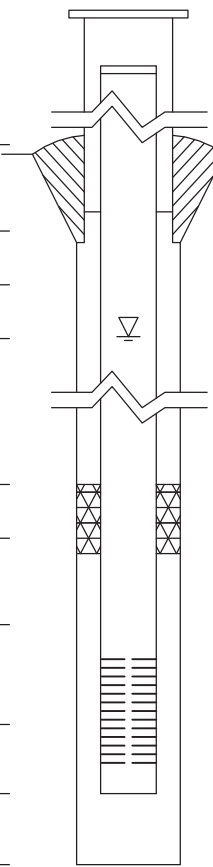
Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G316
Site Name: Coffeen Part 845 Groundwater Borehole #: G316
State _____
Plane Coordinate: X 2,517,211.6 Y 871,643.1 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/14/2021 Date Finished: 1/14/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 3 columns: Description, Elevations (MSL)*, Depths (BGS) (0.01 ft.). Includes data for Protective Casing, Riser Pipe, Ground Surface, Annular Sealant, Static Water Level, Seal, Sand Pack, Screen, and Bottom of Well/Borehole.



* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Includes Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: G317
Site Name: Coffeen Part 845 Groundwater Borehole #: G317
State _____
Plane Coordinate: X 2,517,087.4 Y 871,234.2 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/13/2021 Date Finished: 1/14/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes details for surface seal, annular sealant, bentonite seal, sand pack, and screen.

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen.

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, and Screen Slot Size.



Site #: _____ County: Montgomery Well #: XPW01
Site Name: Coffeen Part 845 Groundwater Borehole #: XPW01
State _____
Plane Coordinate: X 2,515,366.3 Y 871,638.7 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/8/2021 Date Finished: 2/8/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Table with 4 columns: Description, Elevations (MSL)*, Depths (BGS), and (0.01 ft.). Includes a central diagram of a well casing and screen assembly. Rows include: Top of Protective Casing (634.92, -3.07), Top of Riser Pipe (634.57, -2.72), Ground Surface (631.85, 0.00), Top of Annular Sealant (631.15, 0.70), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (624.85, 7.00), Top of Screen (623.64, 8.21), Bottom of Screen (618.87, 12.98), Bottom of Well (618.49, 13.36), Bottom of Borehole (617.85, 14.00).

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (10.93 feet), Bottom of Screen to End Cap (0.38 feet), Screen Length (4.77 feet), Total Length of Casing (16.08 feet), Screen Slot Size (0.010 inches).



Site #: _____ County: Montgomery Well #: XPW02
Site Name: Coffeen Part 845 Groundwater Borehole #: XPW02
State _____
Plane Coordinate: X 2,515,627.3 Y 871,987.1 (or) Latitude: _____ Longitude: _____
Surveyed By: Kyle J. Nolan IL Registration #: 035-003919
Drilling Contractor: Roberts Env. Drilling Inc. Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 2/8/2021 Date Finished: 2/8/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

ANNULAR SPACE DETAILS

Diagram of well construction with elevations and depths table. Includes fields for sealant, sand pack, and backfill materials.

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (11.10 feet), Bottom of Screen to End Cap (0.35 feet), Screen Length (9.80 feet), Total Length of Casing (21.25 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

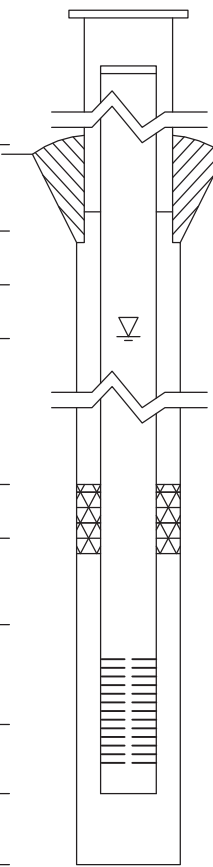
Table for material selection with columns for Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen. Options include SS304, SS316, PTFE, PVC, and OTHER (Steel, PVC).



Site #: _____ County: Montgomery Well #: G275D
 Site Name: Coffeen Part 845 Groundwater Borehole #: G275D
 State _____
 Plane Coordinate: X 2,516,366.5 Y 874,285.3 (or) Latitude: _____ Longitude: _____
 Surveyed By: Michael J. Graminski IL Registration #: 035-003919
 Drilling Contractor: Roberts Driller: Matt
 Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
 Drilling Method: Hollow stem auger Drilling Fluid (Type): None
 Logged By: Colin Winter Date Started: 1/28/2021 Date Finished: 2/3/2021
 Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Construction

Well Completion (MSL)* Riser Pipe (BGS) (0.01 ft.)

Type of Surface Seal: <u>Concrete</u>		<u>620.69</u>	<u>-3.17</u>	Top of Protective Casing
Type of Annular Sealant: <u>High-solids bentonite</u>		<u>620.31</u>	<u>-2.79</u>	Top of Riser Pipe
Installation Method: <u>Tremie</u>		<u>617.52</u>	<u>0.00</u>	Ground Surface
Setting Time: <u>>24 hours</u>		<u>617.02</u>	<u>0.50</u>	Top of Annular Sealant
Type of Bentonite Seal -- <input checked="" type="checkbox"/> Granular <input type="checkbox"/> Pellet <input type="checkbox"/> Slurry (choose one)				Static Water Level (After Completion)
Installation Method: <u>Gravity</u>		<u>572.02</u>	<u>45.50</u>	Top of Seal
Setting Time: <u>30 minutes</u>		<u>571.02</u>	<u>46.50</u>	Top of Sand Pack
Type of Sand Pack: <u>Quartz sand</u>		<u>567.76</u>	<u>49.76</u>	Top of Screen
Grain Size: <u>10-20</u> (sieve size)		<u>557.97</u>	<u>59.55</u>	Bottom of Screen
Installation Method: <u>Gravity</u>		<u>557.63</u>	<u>59.89</u>	Bottom of Well
Type of Backfill Material: <u>N/A</u> (if applicable)		<u>517.82</u>	<u>99.70</u>	Bottom of Borehole
Installation Method: _____				

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	52.55
Bottom of Screen to End Cap	(feet)	0.34
Screen Length (1st slot to last slot)	(feet)	9.79
Total Length of Casing	(feet)	62.68
Screen Slot Size **	(inches)	0.010

WELL CONSTRUCTION MATERIALS
(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:



Department of Natural Resources

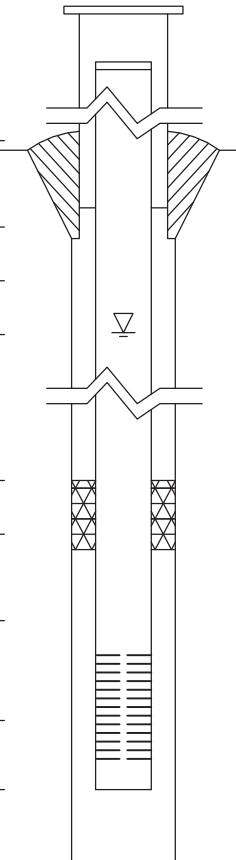
Well Completion Report

Site #: _____ County: Montgomery Well #: G206D
 Site Name: Coffeen Part 845 Groundwater Borehole #: G282D
 State _____
 Plane Coordinate: X 2,514,684.6 Y 875,111.4 (or) Latitude: _____ Longitude: _____
 Surveyed By: Michael J. Graminski IL Registration #: 035-003919
 Drilling Contractor: Roberts Driller: Matt
 Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
 Drilling Method: Hollow stem auger Drilling Fluid (Type): None
 Logged By: Colin Winter Date Started: 1/25/2021 Date Finished: 1/25/2021
 Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Completion Data

Well Completion (MSL)*
 Riser Pipe (BGS)
 (0.01 ft.)

Type of Surface Seal: Concrete
 Type of Annular Sealant: High-solids bentonite
 Installation Method: Tremie
 Setting Time: >24 hours
 Type of Bentonite Seal -- Granular Pellet Slurry
 (choose one)
 Installation Method: Gravity
 Setting Time: 15 hours
 Type of Sand Pack: Quartz sand
 Grain Size: 10-20 (sieve size)
 Installation Method: Gravity
 Type of Backfill Material: N/A
 (if applicable)
 Installation Method: _____



634.35	-2.94	Top of Protective Casing
634.14	-2.73	Top of Riser Pipe
631.41	0.00	Ground Surface
631.11	0.30	Top of Annular Sealant
		Static Water Level (After Completion)
585.81	45.60	Top of Seal
583.41	48.00	Top of Sand Pack
582.21	49.20	Top of Screen
572.41	59.00	Bottom of Screen
572.02	59.39	Bottom of Well
571.41	60.00	Bottom of Borehole

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS

(Choose one type of material for each area)

Protective Casing	SS304	SS316	PTFE	PVC	OTHER: <input checked="" type="checkbox"/> Steel
Riser Pipe Above W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Riser Pipe Below W.T.	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:
Screen	SS304	SS316	PTFE	<input checked="" type="checkbox"/> PVC	OTHER:

CASING MEASUREMENTS

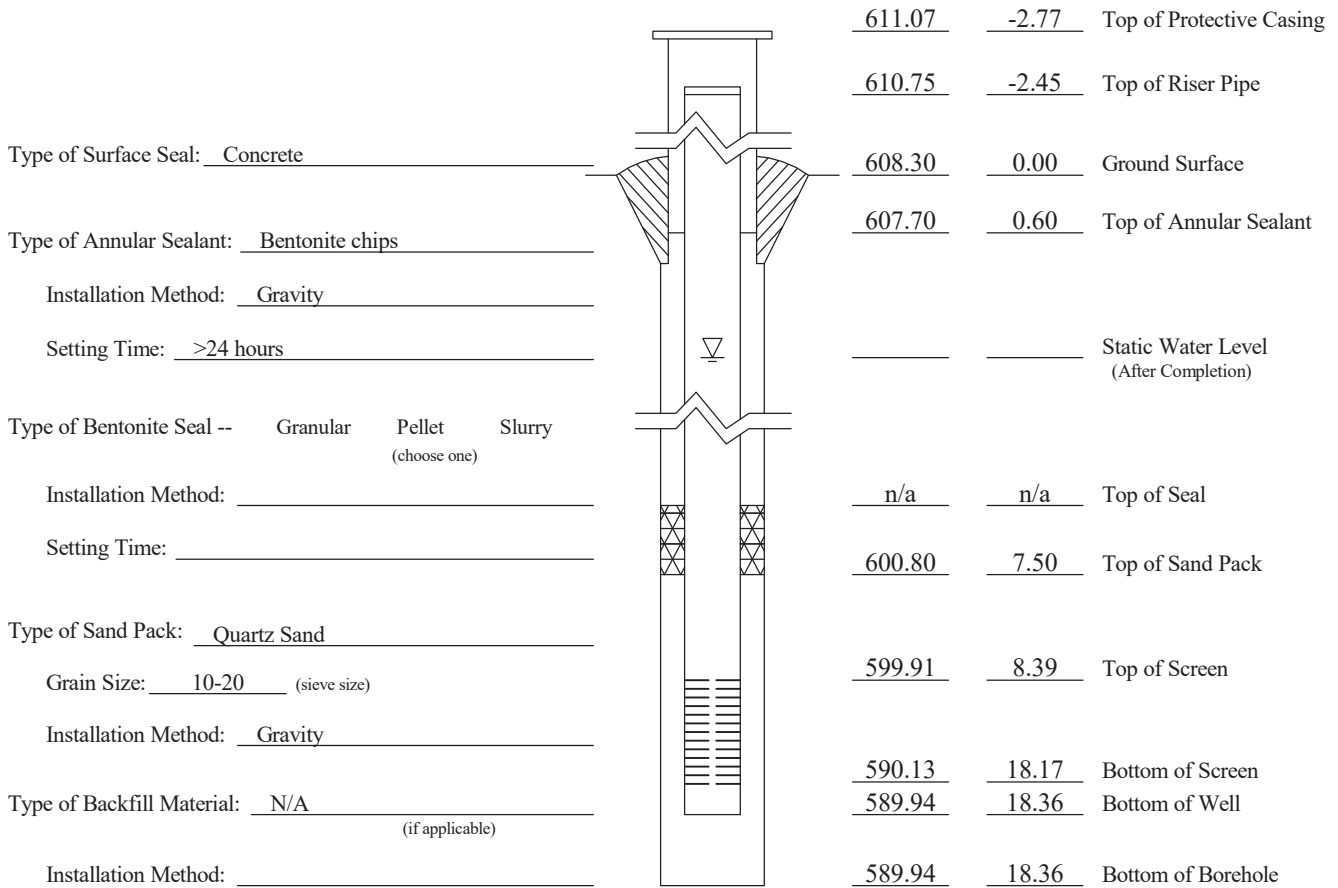
Diameter of Borehole	(inches)	8.0
ID of Riser Pipe	(inches)	2.0
Protective Casing Length	(feet)	5.0
Riser Pipe Length	(feet)	51.93
Bottom of Screen to End Cap	(feet)	0.39
Screen Length (1st slot to last slot)	(feet)	9.77
Total Length of Casing	(feet)	62.09
Screen Slot Size **	(inches)	0.010



Site #: _____ County: Montgomery Well #: G283
Site Name: Coffeen Part 845 Groundwater Borehole #: G283
State _____
Plane Coordinate: X 2,516,503.0 Y 874,113.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Michael J. Graminski IL Registration #: 035-003919
Drilling Contractor: Roberts Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/18/2021 Date Finished: 1/18/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Construction

Well Completion Elevations (0.01 ft.)



Type of Surface Seal: Concrete
Type of Annular Sealant: Bentonite chips
Installation Method: Gravity
Setting Time: >24 hours
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: _____
Setting Time: _____
Type of Sand Pack: Quartz Sand
Grain Size: 10-20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: N/A (if applicable)
Installation Method: _____

Table with 3 columns: Elevation (MSL)*, Elevation (BGS), and Description. Rows include Top of Protective Casing, Top of Riser Pipe, Ground Surface, Top of Annular Sealant, Static Water Level, Top of Seal, Top of Sand Pack, Top of Screen, Bottom of Screen, Bottom of Well, and Bottom of Borehole.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table for selecting well construction materials: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen. Options include SS304, SS316, PTFE, PVC, and OTHER (Steel, PVC).

CASING MEASUREMENTS

Table of casing measurements: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (10.84 feet), Bottom of Screen to End Cap (0.38 feet), Screen Length (9.78 feet), Total Length of Casing (21.00 feet), and Screen Slot Size (0.010 inches).

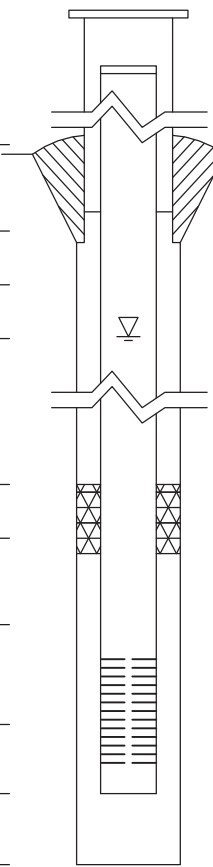


Site #: _____ County: Montgomery Well #: G284
Site Name: Coffeen Part 845 Groundwater Borehole #: G284
State _____
Plane Coordinate: X 2,516,922.9 Y 874,423.6 (or) Latitude: _____ Longitude: _____
Surveyed By: Michael J. Graminski IL Registration #: 035-003919
Drilling Contractor: Roberts Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/20/2021 Date Finished: 1/20/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Construction

Well Completion Elevations (0.01 ft.)

Table with 3 columns: Description, Elevation (MSL)*, Elevation (BGS). Rows include: Top of Protective Casing (618.66, -3.33), Top of Riser Pipe (618.42, -3.09), Ground Surface (615.33, 0.00), Top of Annular Sealant (614.33, 1.00), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (608.33, 7.00), Top of Screen (607.25, 8.08), Bottom of Screen (602.48, 12.85), Bottom of Well (602.10, 13.23), Bottom of Borehole (601.33, 14.00).



Type of Surface Seal: Concrete
Type of Annular Sealant: Bentonite chips
Installation Method: Gravity
Setting Time: >24 hours
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: _____
Setting Time: _____
Type of Sand Pack: Quartz Sand
Grain Size: 10-20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: N/A (if applicable)
Installation Method: _____

* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (11.17 feet), Bottom of Screen to End Cap (0.38 feet), Screen Length (4.77 feet), Total Length of Casing (16.32 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS

(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G285
Site Name: Coffeen Part 845 Groundwater Borehole #: G285
State _____
Plane Coordinate: X 2,516,680.4 Y 874,795.0 (or) Latitude: _____ Longitude: _____
Surveyed By: Michael J. Graminski IL Registration #: 035-002901
Drilling Contractor: Roberts Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/19/2021 Date Finished: 1/19/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Completion Data

Well Completion (MSL)* (0.01 ft.)

Riser Pipe (BGS)

(0.01 ft.)

Type of Surface Seal: Concrete
Type of Annular Sealant: Bentonite chips
Installation Method: Gravity
Setting Time: >24 hours
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: _____
Setting Time: _____
Type of Sand Pack: Quartz Sand
Grain Size: 10-20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: N/A (if applicable)
Installation Method: _____

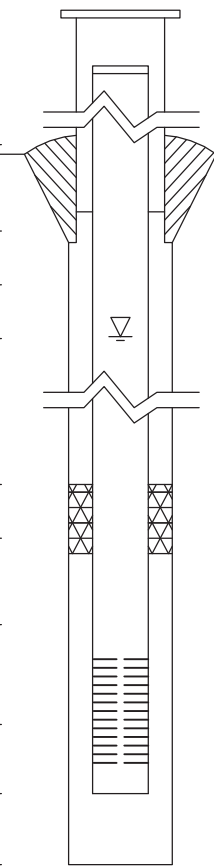


Table with 3 columns: Well Completion (MSL)*, Riser Pipe (BGS), and (0.01 ft.). Rows include: Top of Protective Casing (613.90, -3.36), Top of Riser Pipe (613.52, -2.98), Ground Surface (610.54, 0.00), Top of Annular Sealant (610.24, 0.30), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (598.24, 12.30), Top of Screen (596.86, 13.68), Bottom of Screen (587.09, 23.45), Bottom of Well (586.71, 23.83), Bottom of Borehole (584.54, 26.00).

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS

(Choose one type of material for each area)

Table with 6 columns: Material Type, SS304, SS316, PTFE, PVC, OTHER. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).

CASING MEASUREMENTS

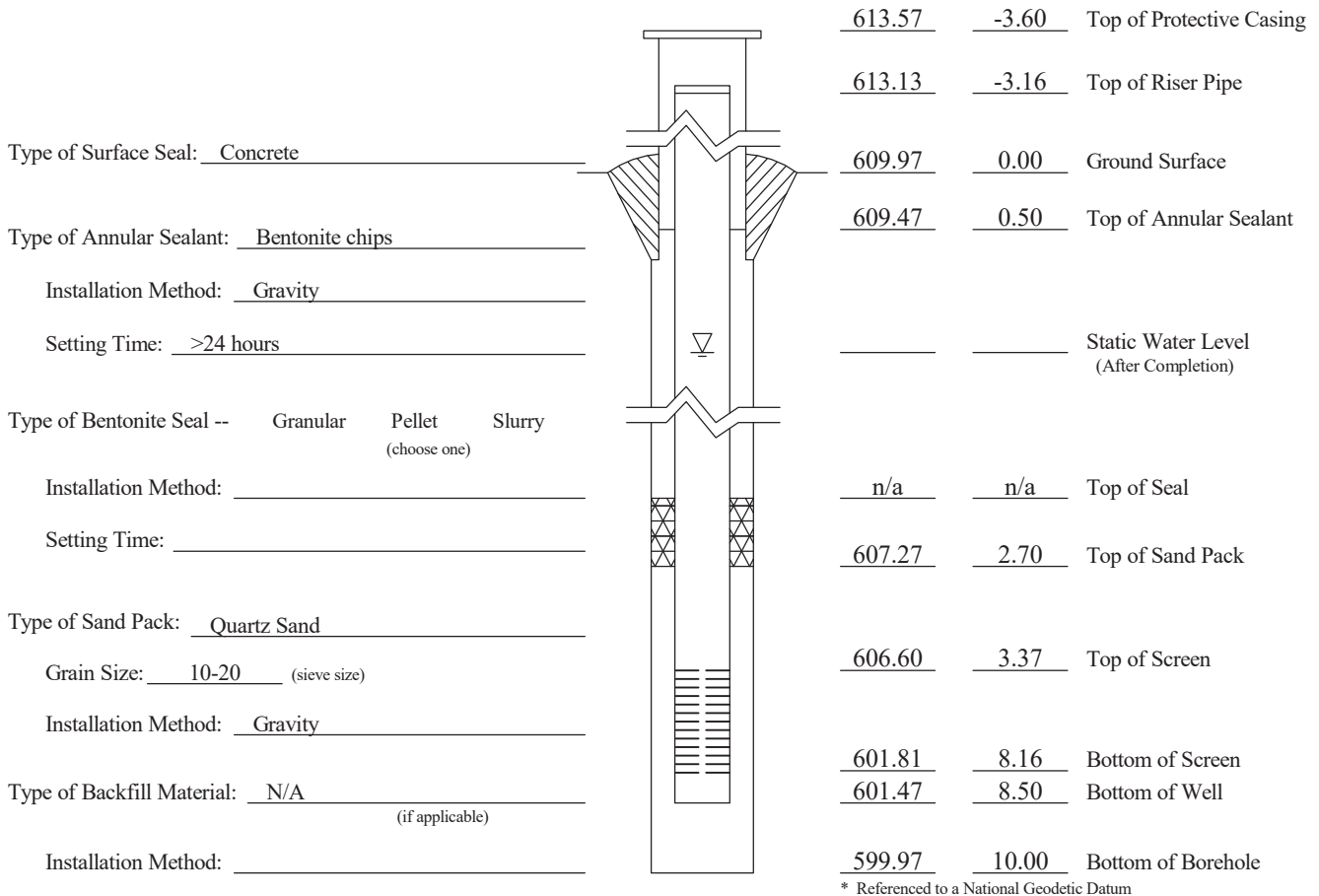
Table with 3 columns: Measurement, Unit, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (16.66 feet), Bottom of Screen to End Cap (0.38 feet), Screen Length (9.77 feet), Total Length of Casing (26.81 feet), Screen Slot Size (0.010 inches).



Site #: County: Montgomery Well #: G286
Site Name: Coffeen Part 845 Groundwater Borehole #: G286
State: Plane Coordinate: X 2,516,561.8 Y 875,072.2 (or) Latitude: Longitude:
Surveyed By: Michael J. Graminski IL Registration #: 035-002901
Drilling Contractor: Roberts Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/19/2021 Date Finished: 1/19/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Construction

Well Completion Elevations (0.01 ft.)



Type of Surface Seal: Concrete
Type of Annular Sealant: Bentonite chips
Installation Method: Gravity
Setting Time: >24 hours
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method:
Setting Time:
Type of Sand Pack: Quartz Sand
Grain Size: 10-20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: N/A (if applicable)
Installation Method:

Table with 3 columns: Elevation (MSL)*, Elevation (BGS), and Description. Rows include Top of Protective Casing, Top of Riser Pipe, Ground Surface, Top of Annular Sealant, Static Water Level, Top of Seal, Top of Sand Pack, Top of Screen, Bottom of Screen, Bottom of Well, and Bottom of Borehole.

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

Table for selecting well construction materials: Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., and Screen. Options include SS304, SS316, PTFE, PVC, and OTHER (Steel, PVC).

CASING MEASUREMENTS

Table for casing measurements: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (6.53 feet), Bottom of Screen to End Cap (0.34 feet), Screen Length (4.79 feet), Total Length of Casing (11.66 feet), and Screen Slot Size (0.010 inches).

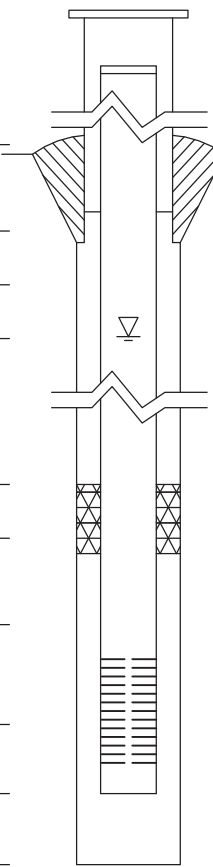


Site #: _____ County: Montgomery Well #: G287
Site Name: Coffeen Part 845 Groundwater Borehole #: G287
State _____
Plane Coordinate: X 2,516,415.5 Y 875,442.8 (or) Latitude: _____ Longitude: _____
Surveyed By: Michael J. Graminski IL Registration #: 035-002901
Drilling Contractor: Roberts Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/18/2021 Date Finished: 1/18/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Completion Data

Well Completion Data (0.01 ft.)

Table with 3 columns: Description, Elevation (MSL)*, Elevation (BGS). Rows include: Top of Protective Casing (617.88, -3.54), Top of Riser Pipe (617.45, -3.11), Ground Surface (614.34, 0.00), Top of Annular Sealant (613.74, 0.60), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (609.84, 4.50), Top of Screen (608.91, 5.43), Bottom of Screen (604.09, 10.25), Bottom of Well (603.75, 10.59), Bottom of Borehole (602.54, 11.80).



* Referenced to a National Geodetic Datum

CASING MEASUREMENTS

Table with 2 columns: Measurement, Value. Rows include: Diameter of Borehole (8.0 inches), ID of Riser Pipe (2.0 inches), Protective Casing Length (5.0 feet), Riser Pipe Length (8.54 feet), Bottom of Screen to End Cap (0.34 feet), Screen Length (4.82 feet), Total Length of Casing (13.70 feet), Screen Slot Size (0.010 inches).

WELL CONSTRUCTION MATERIALS

Table with 2 columns: Material Type, Material Selection. Rows include: Protective Casing (Steel), Riser Pipe Above W.T. (PVC), Riser Pipe Below W.T. (PVC), Screen (PVC).



Site #: _____ County: Montgomery Well #: G288
Site Name: Coffeen Part 845 Groundwater Borehole #: G288
State _____
Plane Coordinate: X 2,517,071.4 Y 875,279.6 (or) Latitude: _____ Longitude: _____
Surveyed By: Michael J. Graminski IL Registration #: 035-002901
Drilling Contractor: Roberts Driller: Matt
Consulting Firm: Hanson Professional Services Inc. Geologist: Rhonald W. Hasenyager, LPG #196-000246
Drilling Method: Hollow stem auger Drilling Fluid (Type): None
Logged By: Colin Winter Date Started: 1/21/2021 Date Finished: 1/21/2021
Report Form Completed By: Colin Winter Date: 5/3/2021

Vertical Well Construction

Well Completion (MSL)* Riser Pipe (BGS) (0.01 ft.)

Type of Surface Seal: Concrete
Type of Annular Sealant: Bentonite chips
Installation Method: Gravity
Setting Time: >24 hours
Type of Bentonite Seal -- Granular Pellet Slurry (choose one)
Installation Method: _____
Setting Time: _____
Type of Sand Pack: Quartz Sand
Grain Size: 10-20 (sieve size)
Installation Method: Gravity
Type of Backfill Material: N/A (if applicable)
Installation Method: _____

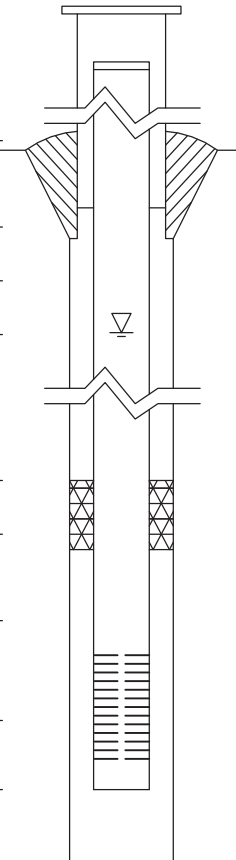


Table with 3 columns: Well Completion (MSL)*, Riser Pipe (BGS), and (0.01 ft.). Rows include: Top of Protective Casing (620.37, -3.29), Top of Riser Pipe (620.07, -2.99), Ground Surface (617.08, 0.00), Top of Annular Sealant (616.78, 0.30), Static Water Level (After Completion), Top of Seal (n/a, n/a), Top of Sand Pack (611.08, 6.00), Top of Screen (609.49, 7.59), Bottom of Screen (604.82, 12.26), Bottom of Well (604.33, 12.75), Bottom of Borehole (603.08, 14.00).

* Referenced to a National Geodetic Datum

WELL CONSTRUCTION MATERIALS (Choose one type of material for each area)

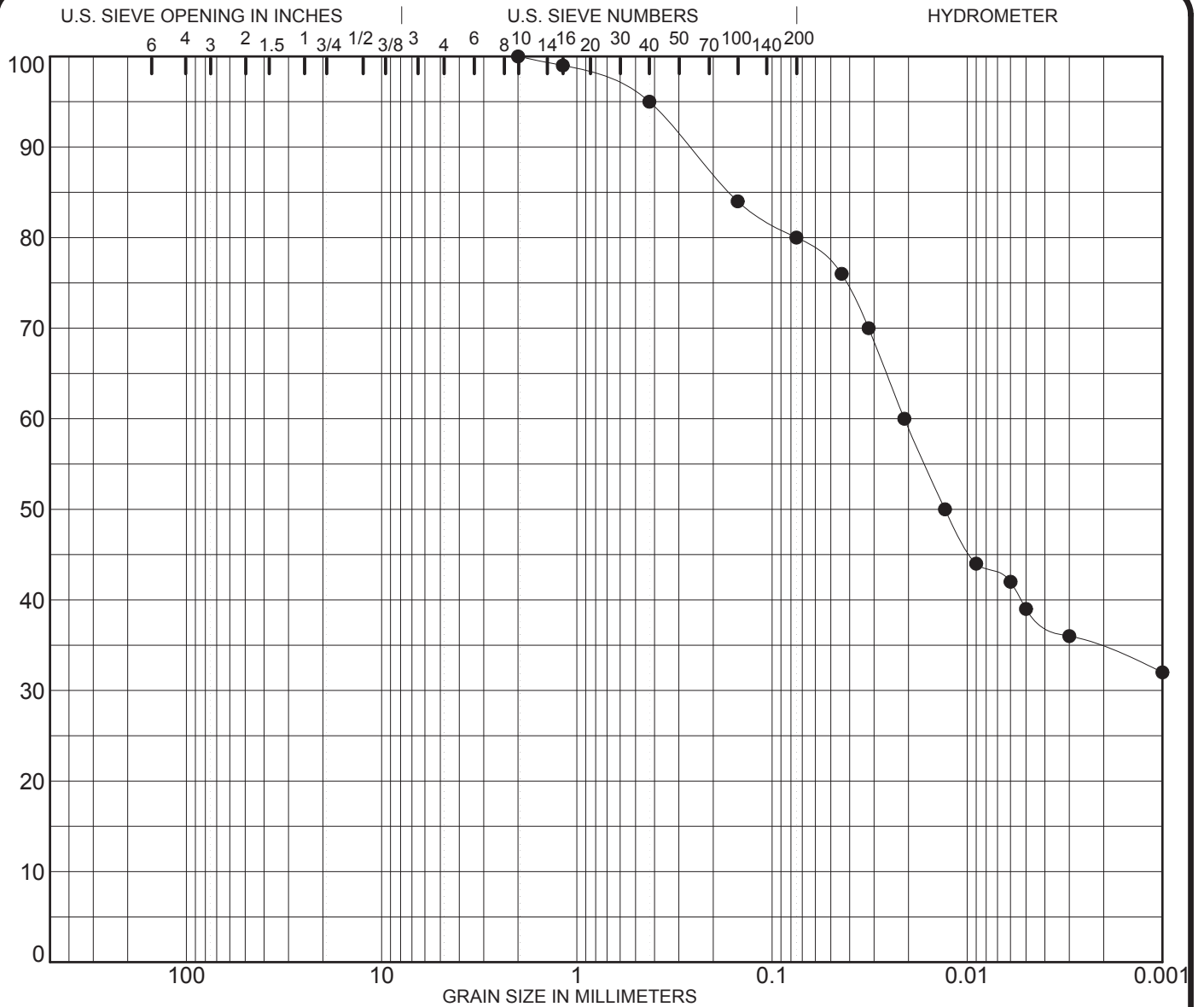
Table with 6 columns: Material type (Protective Casing, Riser Pipe Above W.T., Riser Pipe Below W.T., Screen) and 5 options (SS304, SS316, PTFE, PVC, OTHER: Steel/PVC).

CASING MEASUREMENTS

Table with 2 columns: Measurement (Diameter of Borehole, ID of Riser Pipe, Protective Casing Length, Riser Pipe Length, Bottom of Screen to End Cap, Screen Length, Total Length of Casing, Screen Slot Size) and Value (8.0, 2.0, 5.0, 10.58, 0.49, 4.67, 15.74, 0.010).

APPENDIX D
GEOTECHNICAL LABORATORY REPORTS

RAMSEY GEOTECHNICAL REPORT



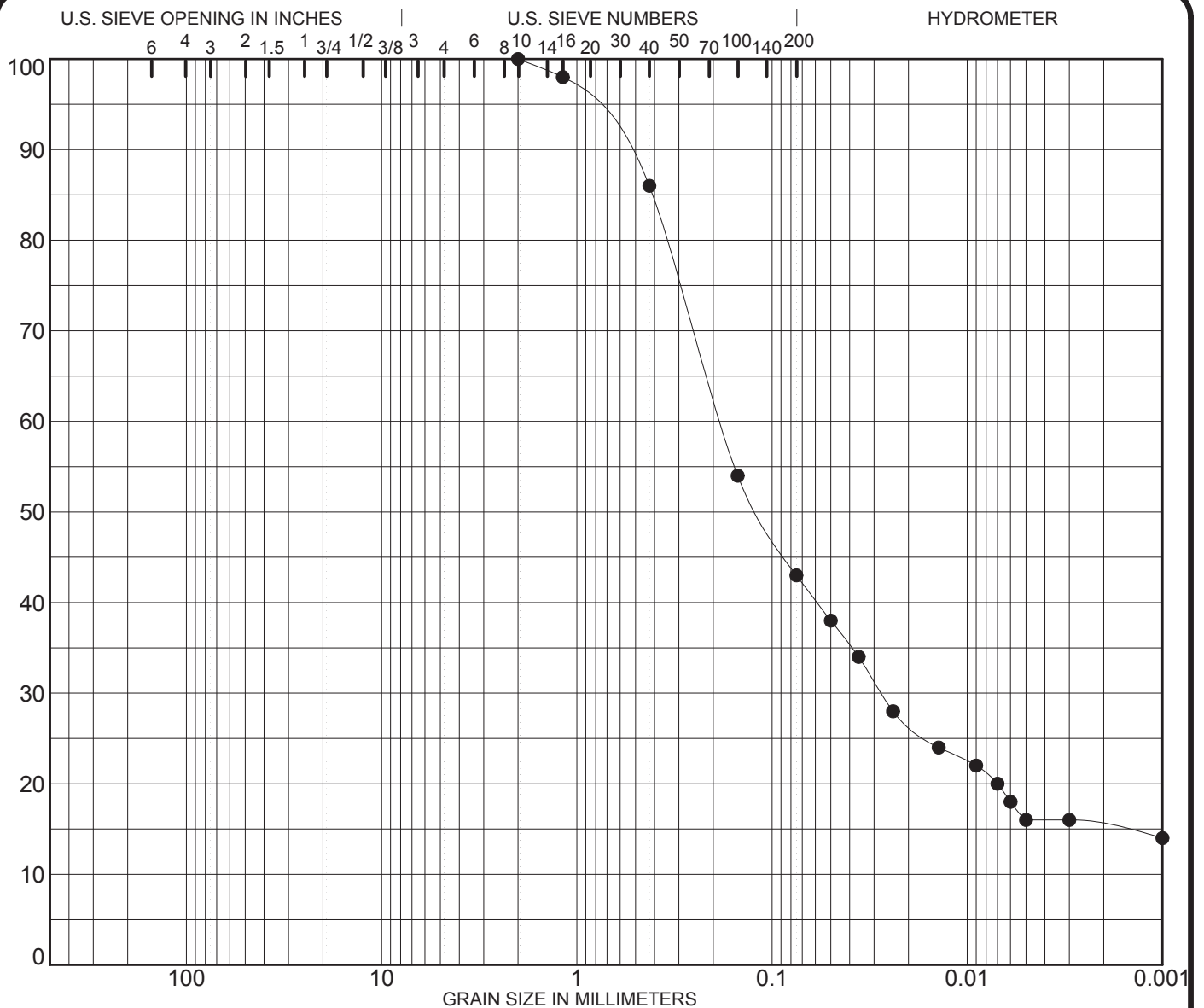
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G282D/Comp1	3/8	100.0	Lean Clay, with sand				
Identified as G206D	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	99.0	0	20	46	34	
Specific Gravity - 2.60	# 30						
	# 40	95.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		20.5	105.3	42	18	24
	# 80						
	# 100	84.0	Density/Moisture Relationship: Standard				
	# 200	80.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility - Montgomery County, Illinois** JOB NO. **21-055**
 DATE **March 18, 2021**

G282D/Comp1

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701



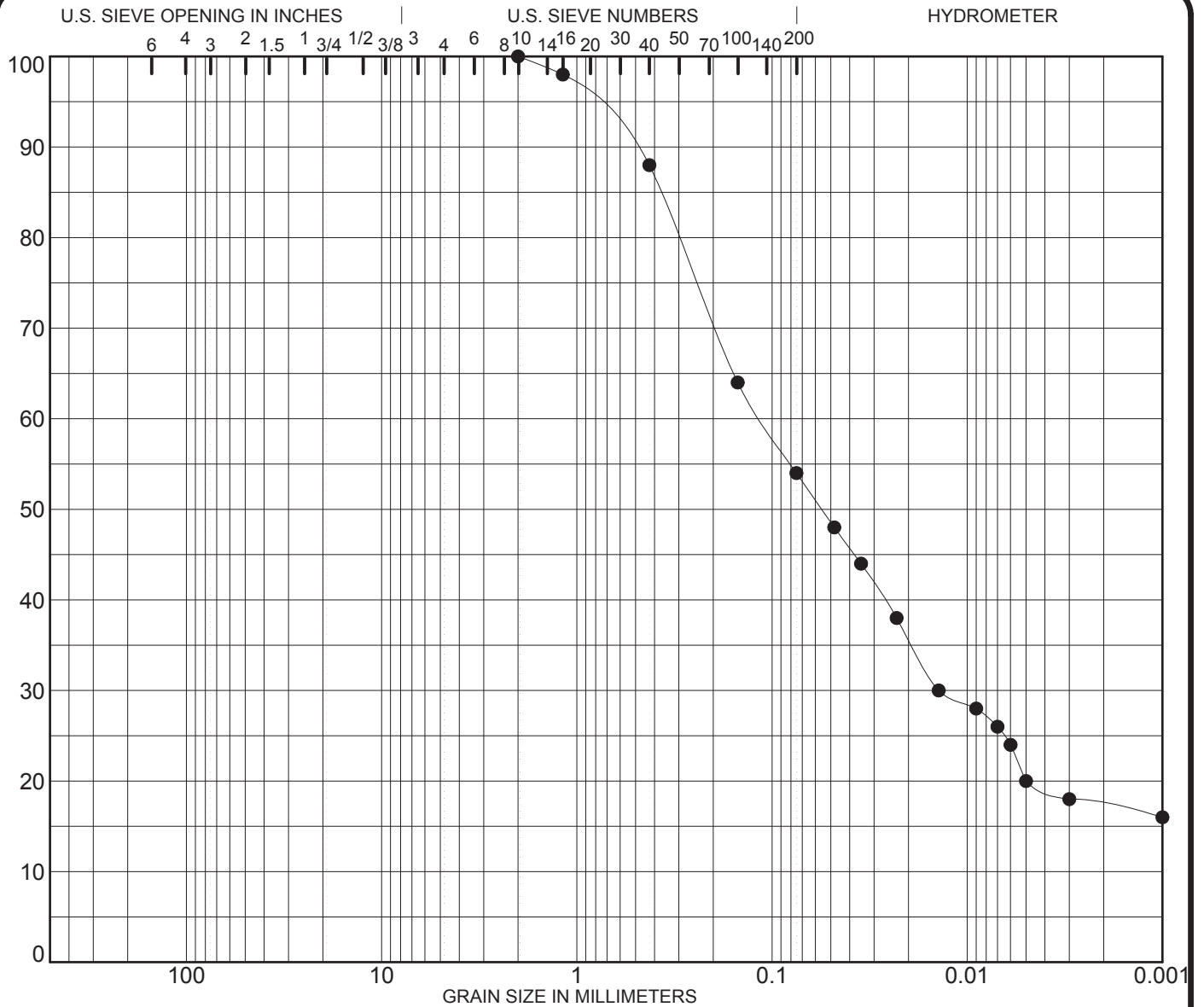
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G282D/Comp2	3/8	100.0	Clayey Fine to Medium Sand				
Identified as G206D	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	98.0	0	57	28	15	
Specific Gravity - 2.56	# 30						
	# 40	86.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		10.1	117.4	22	15	7
	# 80						
	# 100	54.0	Density/Moisture Relationship: Standard				
	# 200	43.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility -** JOB NO. **21-055**
Montgomery County, Illinois DATE **March 18, 2021**

G282D/Comp2

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701

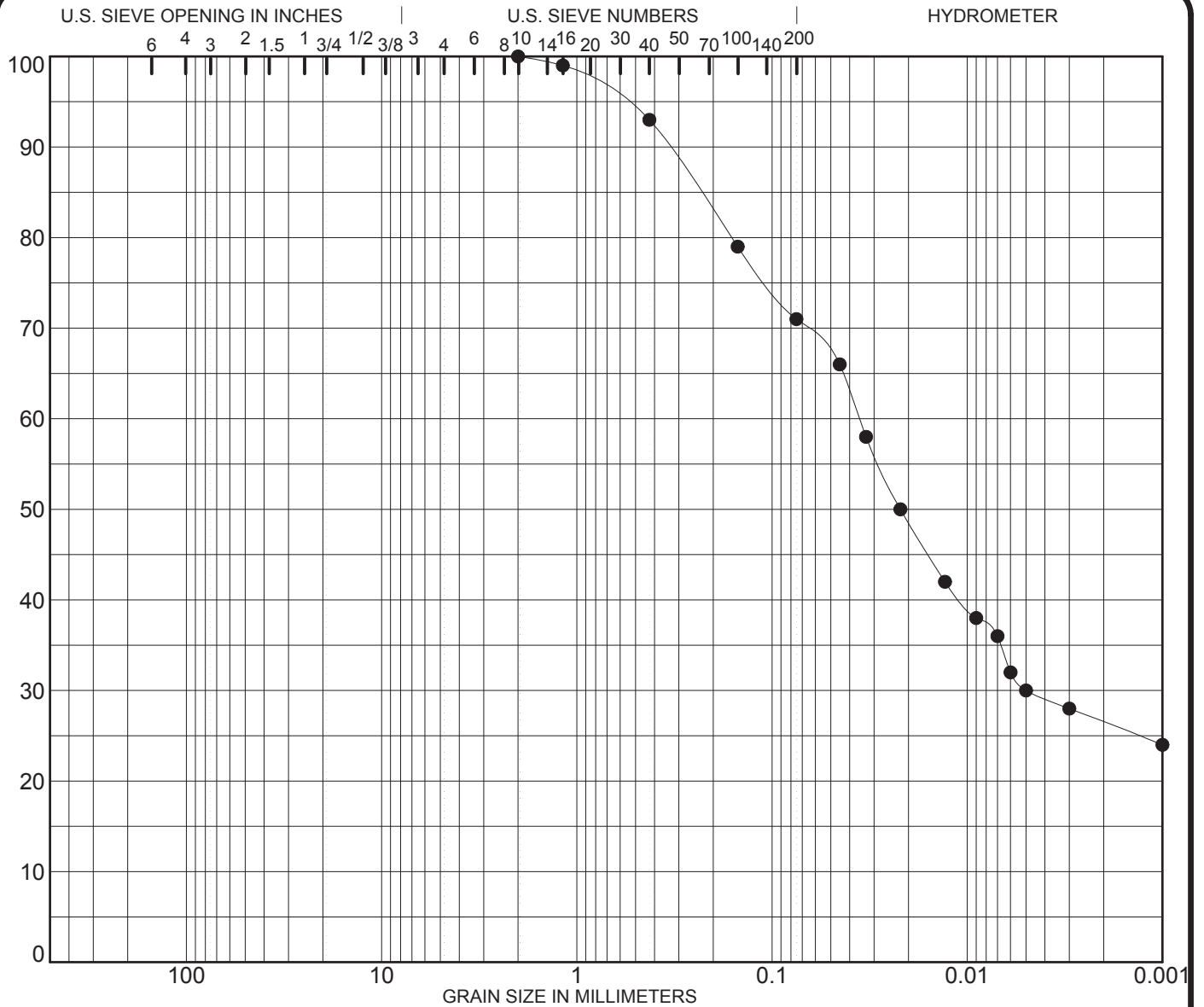


COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G282D/Comp3	3/8	100.0	Sandy Lean Clay				
Identified as G206D	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	98.0	0	46	37	17	
Specific Gravity - 2.56	# 30						
	# 40	88.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		8.3	131.4	21	13	8
	# 80						
	# 100	64.0	Density/Moisture Relationship: Standard				
	# 200	54.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility -** JOB NO. **21-055**
Montgomery County, Illinois DATE **March 18, 2021**
 G282D/Comp3

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701



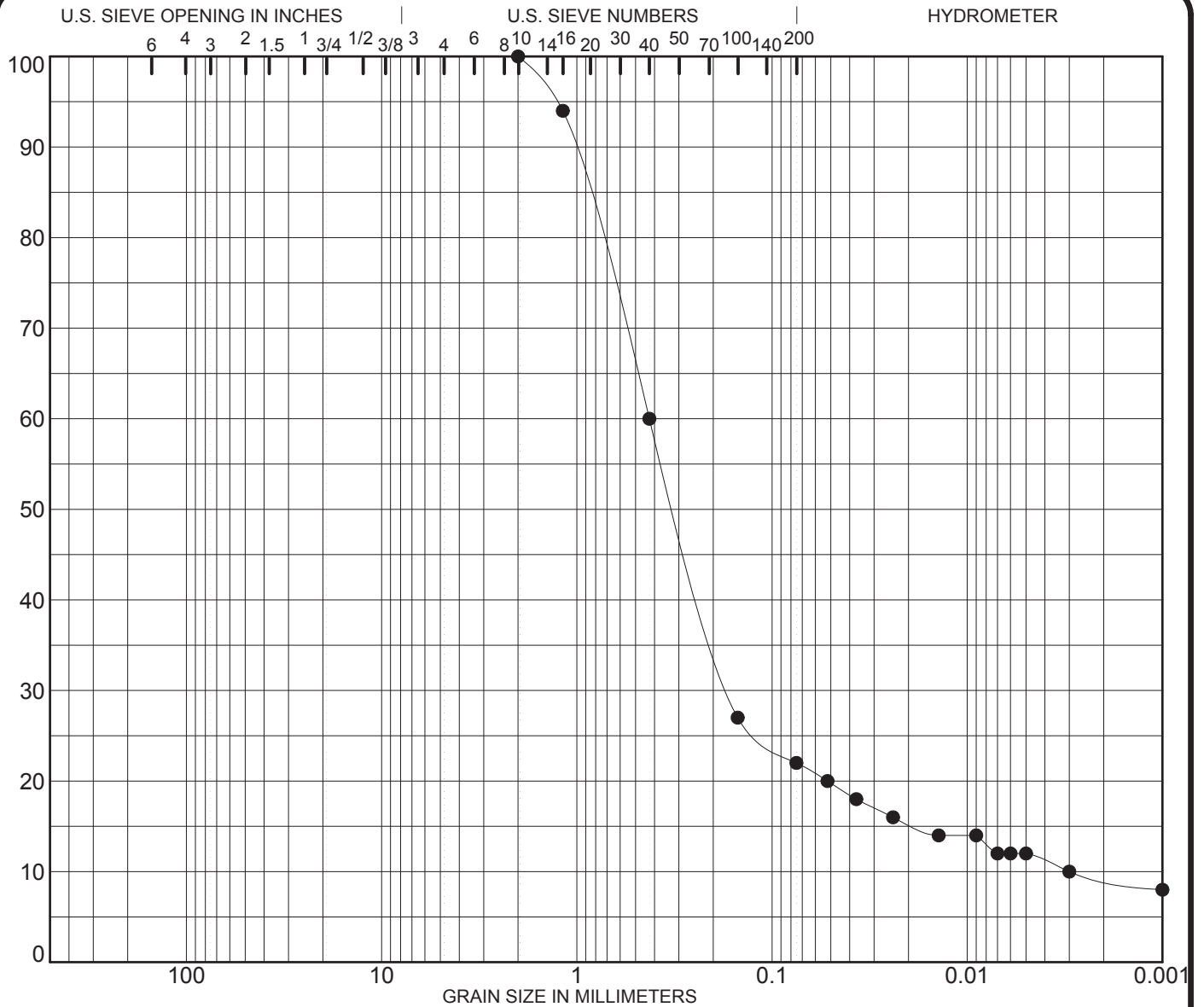
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G282D/Comp4	3/8	100.0	Lean Clay, with sand				
Identified as G206D	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	99.0	0	29	46	26	
Specific Gravity - 2.62	# 30						
	# 40	93.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		13.4	120.6	29	14	15
	# 80						
	# 100	79.0	Density/Moisture Relationship: Standard				
	# 200	71.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility -** JOB NO. **21-055**
Montgomery County, Illinois DATE **March 18, 2021**

G282D/Comp4

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701



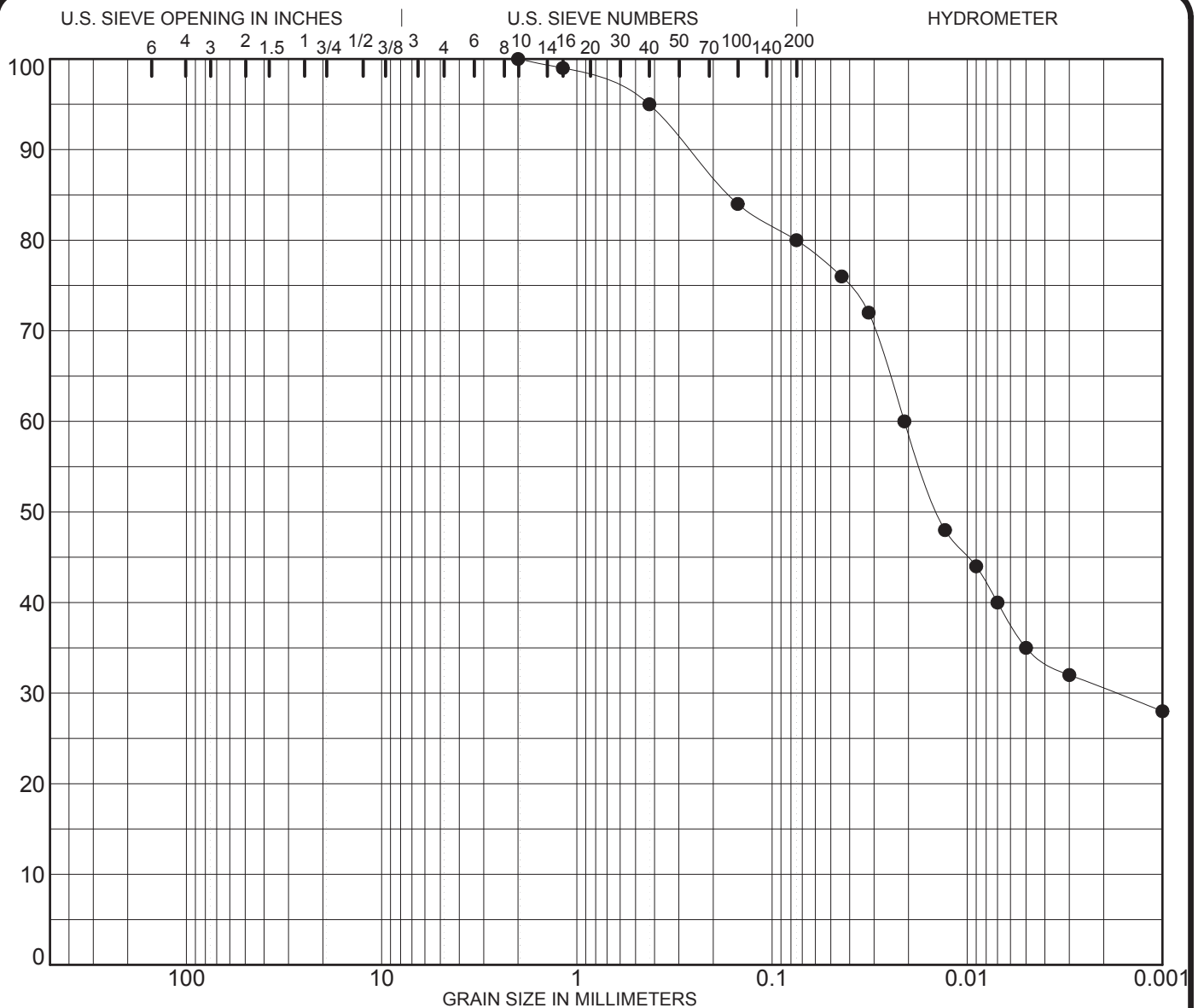
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G282D/Comp5	3/8	100.0	Silty Fine to Medium Sand				
Identified as G206D	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	94.0	0	78	13	9	
Specific Gravity - 2.75	# 30						
	# 40	60.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		14.9		18	12	6
	# 80						
	# 100	27.0	Density/Moisture Relationship: Standard				
	# 200	22.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility - Montgomery County, Illinois** JOB NO. **21-055**
 DATE **March 18, 2021**

G282D/Comp5

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701



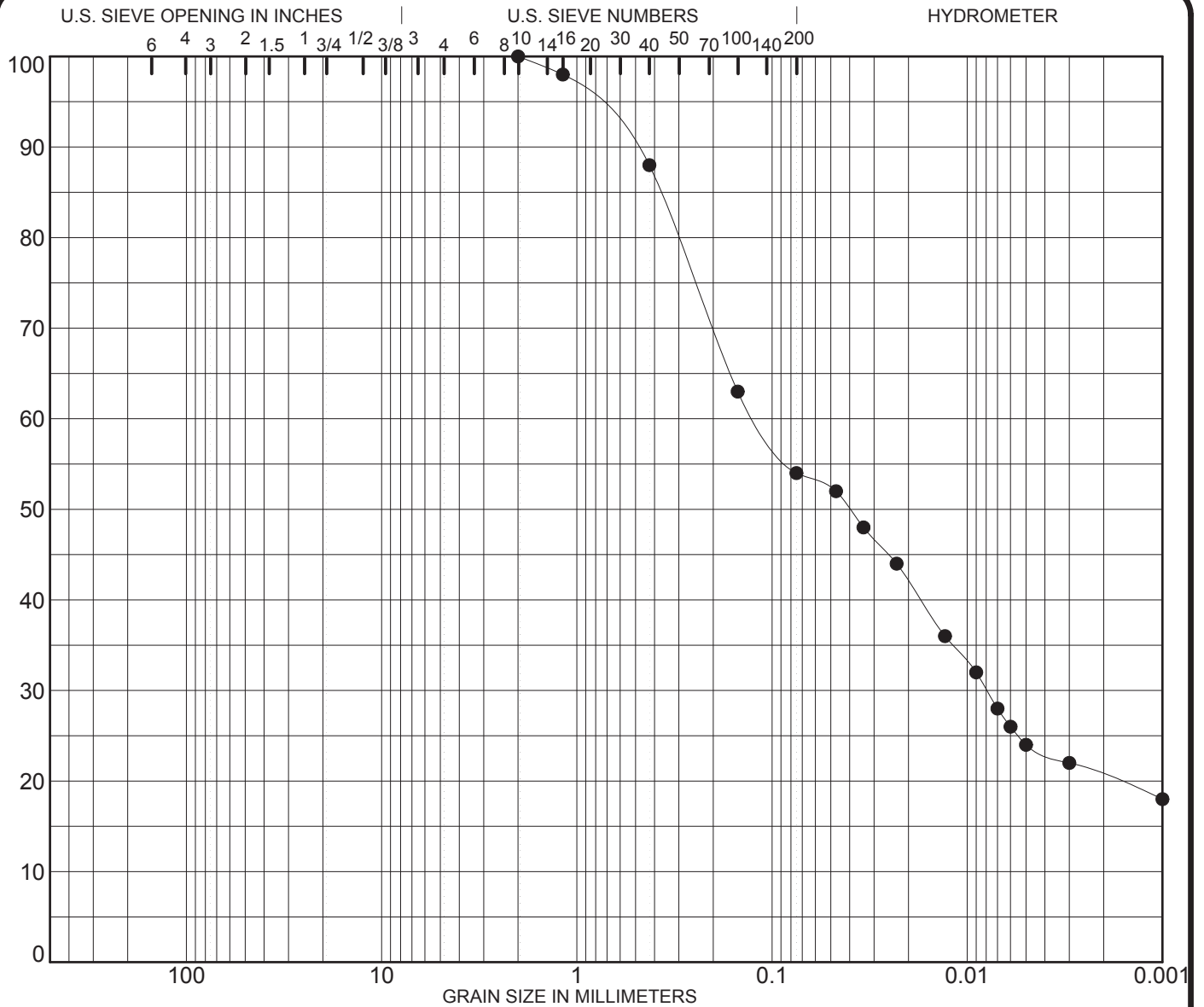
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G289D/Comp1	3/8	100.0	Lean Clay, with sand				
Identified as SB289	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	99.0	0	20	50	30	
Specific Gravity - 2.56	# 30						
	# 40	95.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		21.2	103.4	40	18	22
	# 80						
	# 100	84.0	Density/Moisture Relationship: Standard				
	# 200	80.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility - Montgomery County, Illinois** JOB NO. **21-055**
 DATE **March 22, 2021**

G289D/Comp1

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701



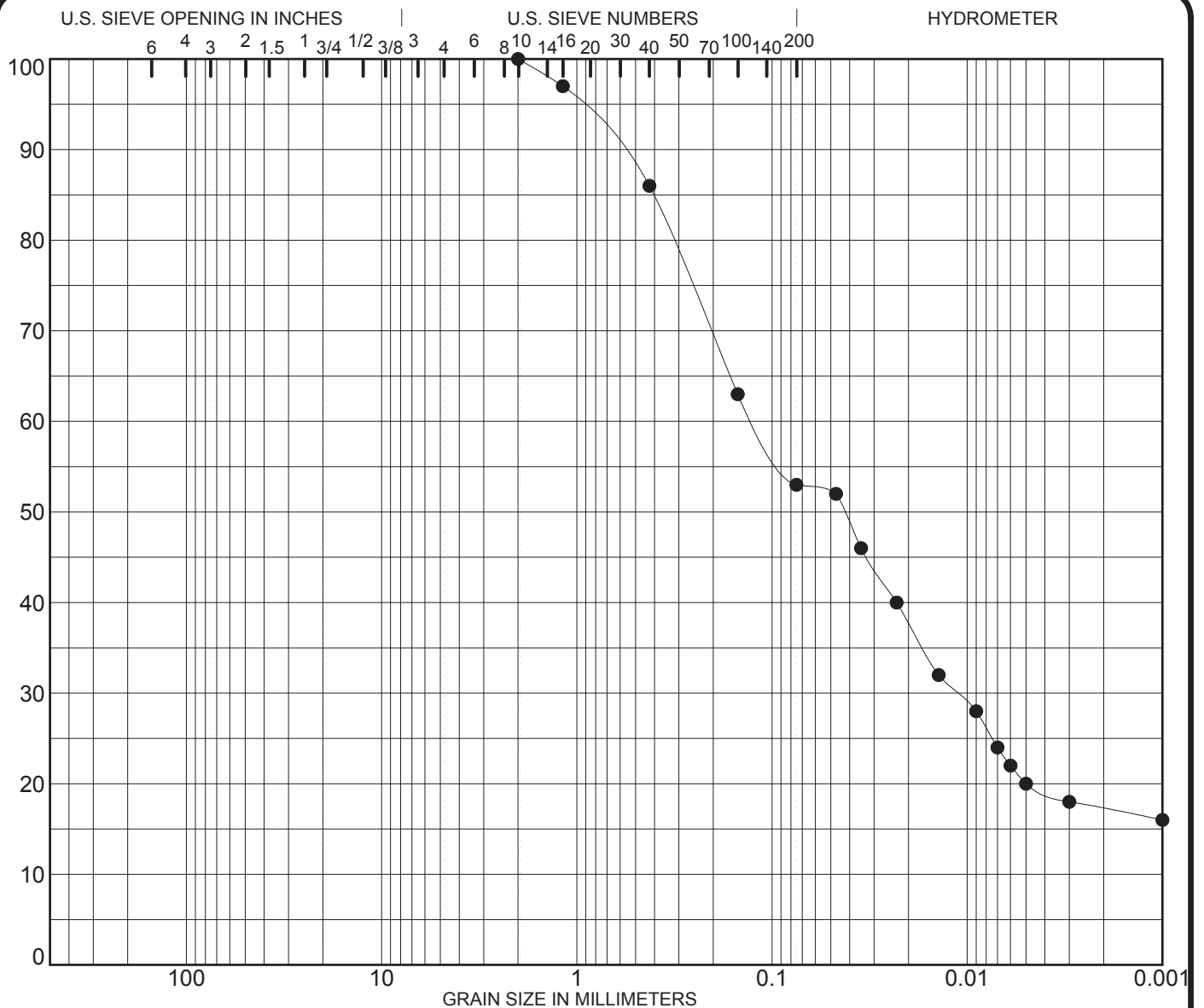
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

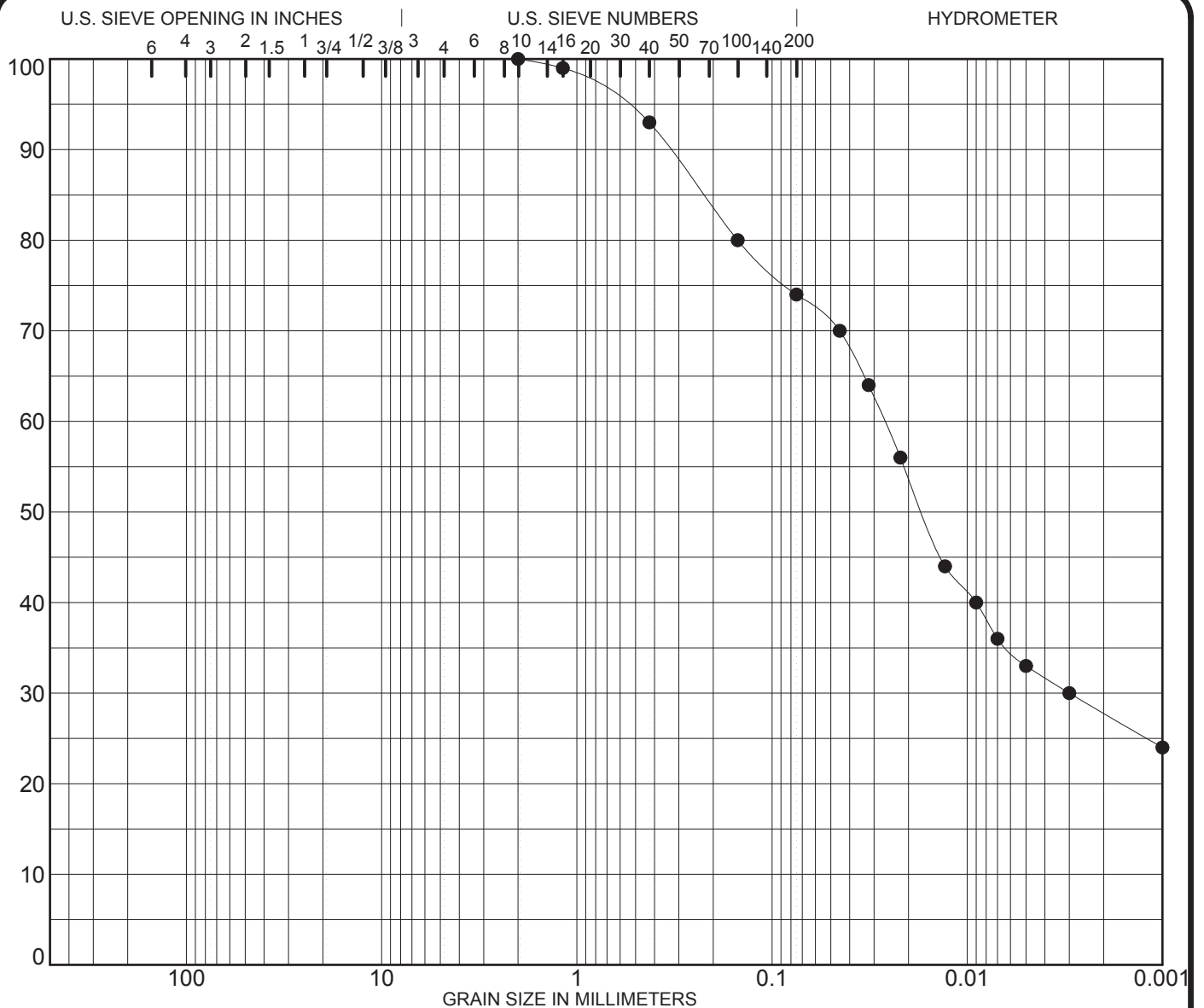
SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G289D/Comp2	3/8	100.0	Sandy Lean Clay				
Identified as SB289	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	98.0	0	46	34	20	
Specific Gravity - 2.60	# 30						
	# 40	88.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		16.6	112.6	23	14	9
	# 80						
	# 100	63.0	Density/Moisture Relationship: Standard				
	# 200	54.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility -** JOB NO. **21-055**
Montgomery County, Illinois DATE **March 22, 2021**

G289D/Comp2

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701





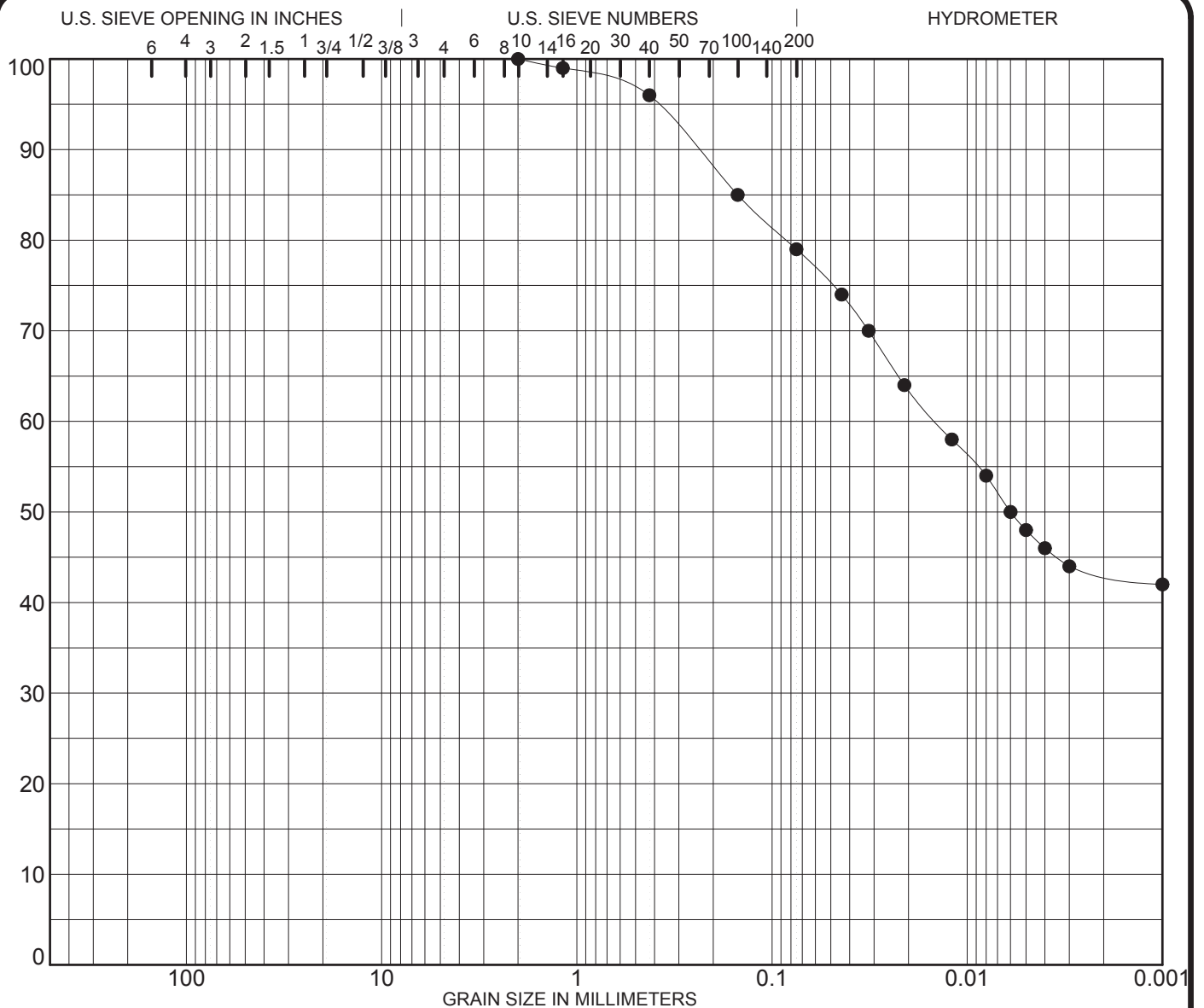
COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G289D/Comp4	3/8	100.0	Lean Clay, with sand				
Identified as SB289	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	99.0	0	26	47	27	
Specific Gravity - 2.61	# 30						
	# 40	93.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		14.9	116.6	31	15	16
	# 80						
	# 100	80.0	Density/Moisture Relationship: Standard				
	# 200	74.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility - Montgomery County, Illinois** JOB NO. **21-055**
 DATE **March 22, 2021**

G289D/Comp4

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

SPECIMEN IDENTIFICATION	SIEVE	% PASS	CLASSIFICATION				
G289D/Comp5	3/8	100.0	Lean Clay, with sand				
Identified as SB289	#4	100.0					
	#8	100.0					
	#10	100.0	%GRAVEL	%SAND	%SILT	%CLAY	
NOTES:	# 16	99.0	0	21	36	43	
Specific Gravity - 2.63	# 30						
	# 40	96.0	MC%	γ _{dry} (pcf)	LL	PL	PI
	# 50		25.0	98.4	48	18	30
	# 80						
	# 100	85.0	Density/Moisture Relationship: Standard				
	# 200	79.0	Max PCF		Opt. WC%		

PROJECT **Coffeen Gypsum Management Facility -** JOB NO. **21-055**
Montgomery County, Illinois DATE **March 22, 2021**
 G289D/Comp5

SOIL DATA SHEET
Ramsey Geotechnical Engineering
 Bloomington, IL 61701

GEOTECHNOLOGY GEOTECHNICAL REPORT



Via email: dramsey@ramgeoeng.com

April 5, 2021

J037264.01.6002

Mr. Douglas P. Ramsey, P.E.
Ramsey Geotechnical Engineering
1701 W. Market Street
Bloomington, Illinois 61701

Re: Coffeen Gypsum Management
Montgomery County, Illinois

Dear Mr. Ramsey:

Included in this report are the test results from two Shelby tubes and one bulk sample of gypsum received in our laboratory on March 15, 2021. The samples were tested in general accordance with the test method listed below.

<u>Test to Determine</u>	<u>Method of Test</u>
Water (Moisture) Content of Soils	ASTM D2216
Description and Identification of Soils (Visual-Manual)	ASTM D2488
Hydraulic Conductivity of Soil Using Flexible Wall Permeameter	ASTM D5084
Density (Unit Weight) of Soil Specimens	ASTM D7263

We trust this is the information you require. Please contact the undersigned if you have any questions regarding this report.

Respectfully submitted,

GEOTECHNOLOGY, INC.

Janet M. May
Illinois Laboratory Manager

JMM/LPH:jmm

Attachment: Test Result Summary
Hydraulic Conductivity Test Data Sheets
Proctor Curve (Provided by Ramsey)
Shelby Tube Logs
Testing Assignment Sheets

TEST RESULT SUMMARY

**Coffeen Power Station
 Gypsum Management Facility
 Montgomery County, Illinois**

Boring Number	Sample Number	Depth, feet	ASTM D2216	ASTM D7263	ASTM D5084	
			Moisture Content, %	Dry Unit Weight, pcf	Hydraulic Conductivity, cm/sec	Range of Hydraulic Gradient
G275D	ST-7	12.0-14.0	15.8	115.9	1.6×10^{-4}	0 - 1.5
G289D	ST-5	8.0-10.0	20.2	105.9	1.1×10^{-8}	11.1 - 19.3

Sample Number/ Material	Optimum Moisture Content, %	Maximum Dry Unit Weight, pcf	Percent Compact	Moisture Content, %	Dry Unit Weight, pcf	Hydraulic Conductivity, cm/sec	Range of Hydraulic Gradient
LSN-3783 Gypsum	24.1	85.8	90.9	25.3	78.0	8.9×10^{-4}	0.2 - 2.0

Boring Number	Sample Number	Depth, feet	ASTM D2488			
			Material Description			
G275D	ST-7	12.0-14.0	Brown CLAYEY SAND, some gravel – SC			
G289D	ST-5	8.0-10.0	Dark to very dark olive-brown, LEAN to FAT CLAY – CL/CH			

Notes and abbreviations:

% - Percent

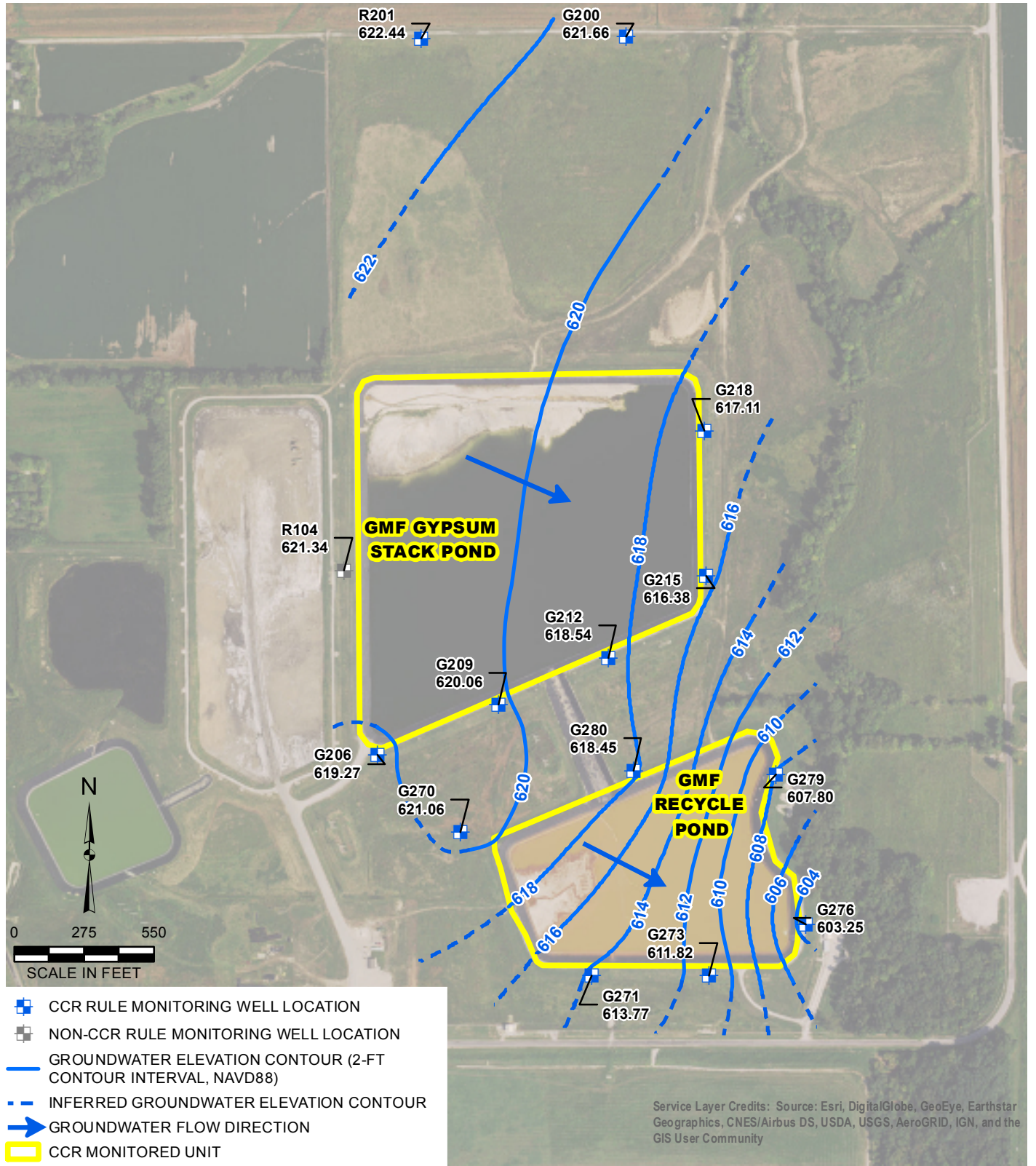
cm/sec - Centimeters per second

pcf - Pounds per cubic foot

**APPENDIX E
GROUNDWATER CONTOUR MAPS, ELEVATIONS, AND
VERTICAL GRADIENTS**

GROUNDWATER CONTOUR MAPS

Y:\Mapping\Projects\22\2285\MXD\GW_Contours\Round_01\R1_CoffeenGMF_GSP_GW_Contours.mxd Author: sstolz Date/Time: 3/2/2017, 2:40:46 PM



**COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 1: NOVEMBER 16, 2015**

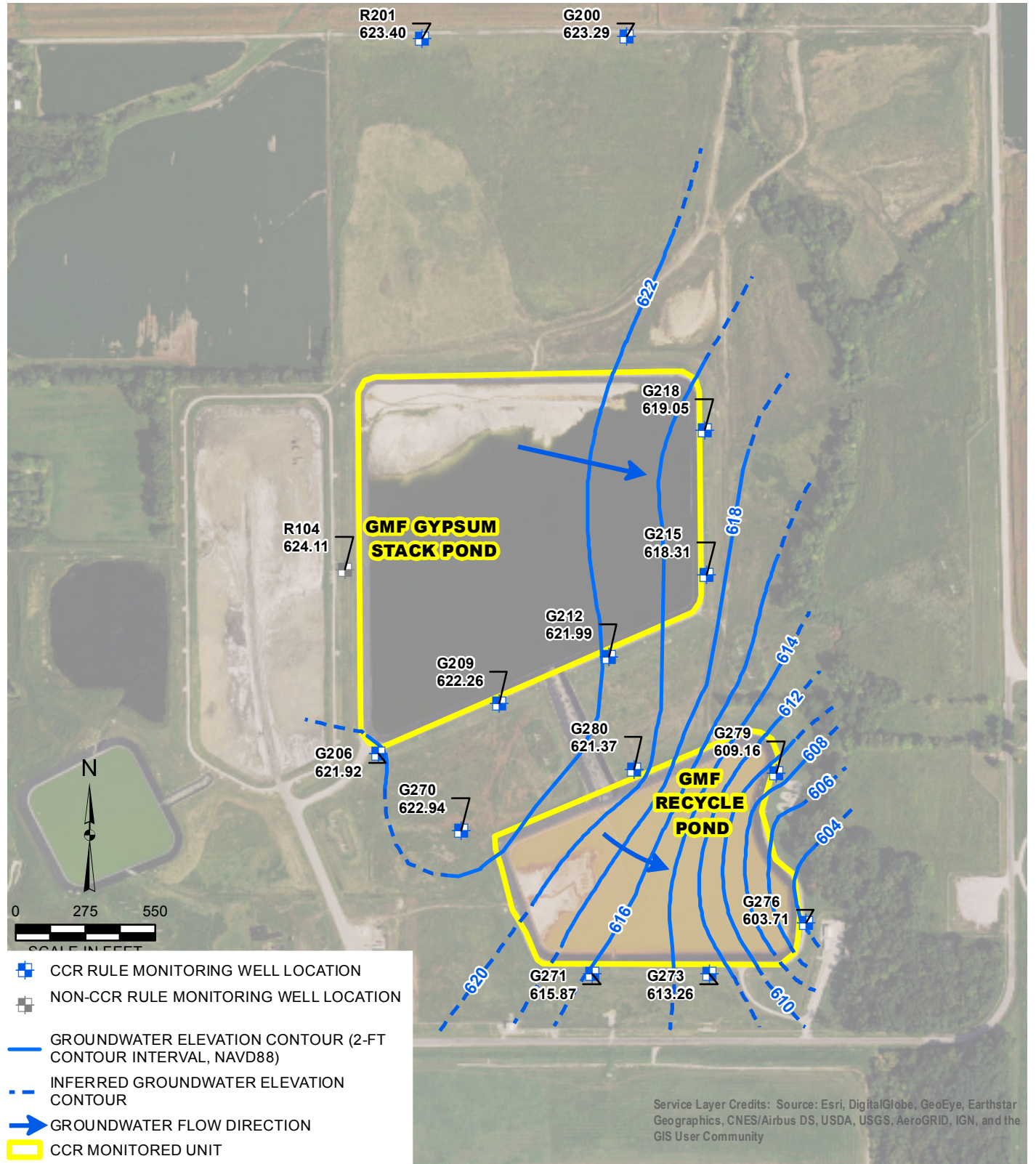
PROJECT NO: 2285
FIGURE NO: 1

DRAWN BY/DATE:
SDS 1/23/17
REVIEWED BY/DATE:
TBN 1/25/17
APPROVED BY/DATE:
JJW 2/7/17

DYNEGY CCR RULE GROUNDWATER MONITORING
COFFEEN POWER STATION
COFFEEN, ILLINOIS



Y:\Mapping\Projects\22\2285\MXD\GW_Contours\Round_02\R2_CoffeenGMF_GSP_GW_Contours.mxd Author: sstolz Date/Time: 3/2/2017, 3:26:24 PM



**COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 2: FEBRUARY 8, 2016**

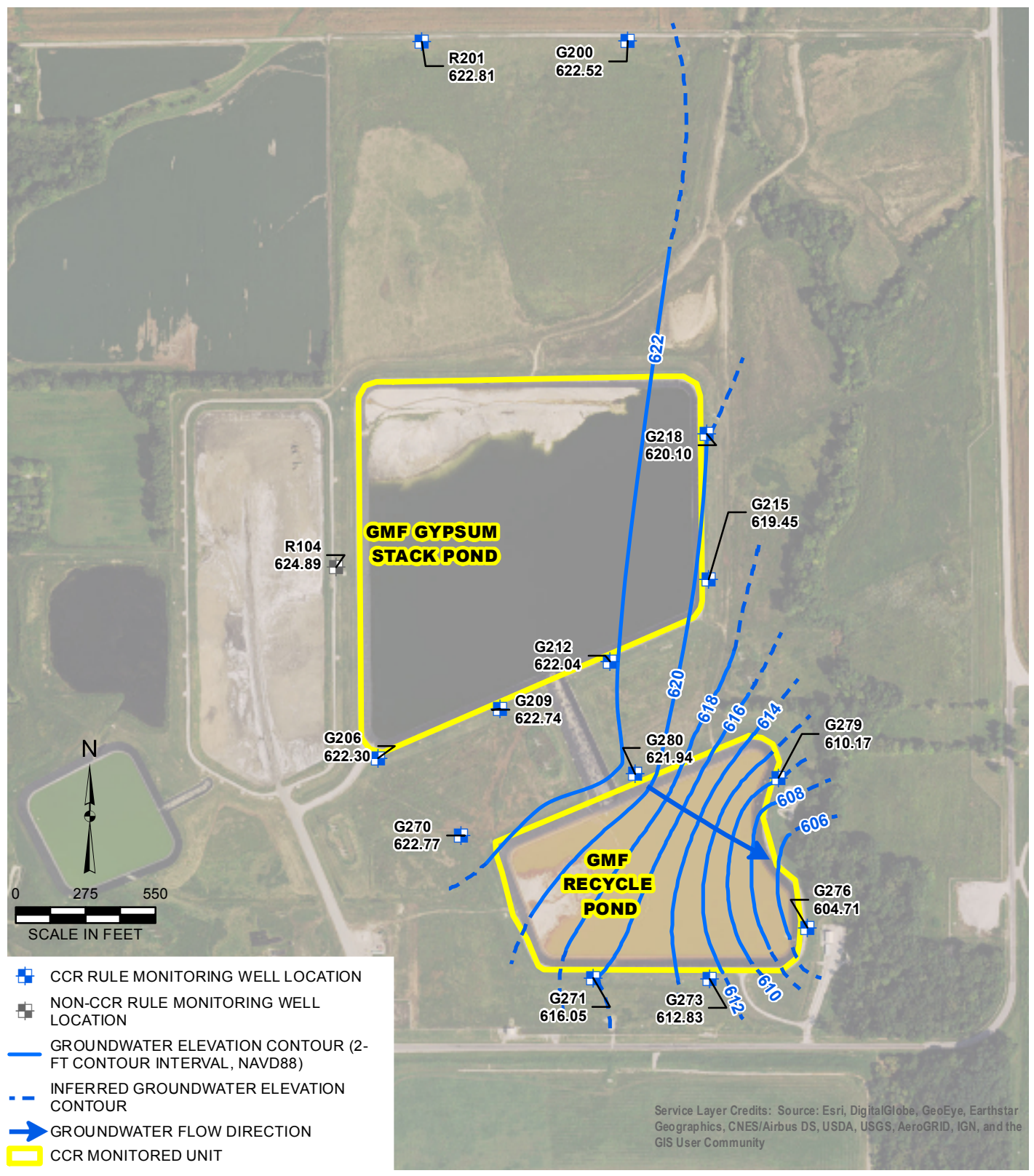
PROJECT NO: 2285
FIGURE NO: 1

DRAWN BY/DATE:
SDS 1/23/17
REVIEWED BY/DATE:
TBN 1/25/17
APPROVED BY/DATE:
JJW 2/8/17

DYNEGY CCR RULE GROUNDWATER MONITORING
COFFEEN POWER STATION
COFFEEN, ILLINOIS



Y:\Mapping\Projects\22\2285\MXD\GW_Contours\Round_03\R3_CoffeenGMF_GSP_GW_Contours.mxd - Author: sstolz - Date/Time: 3/2/2017, 6:59:32 PM



**COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 3: MAY 9, 2016**

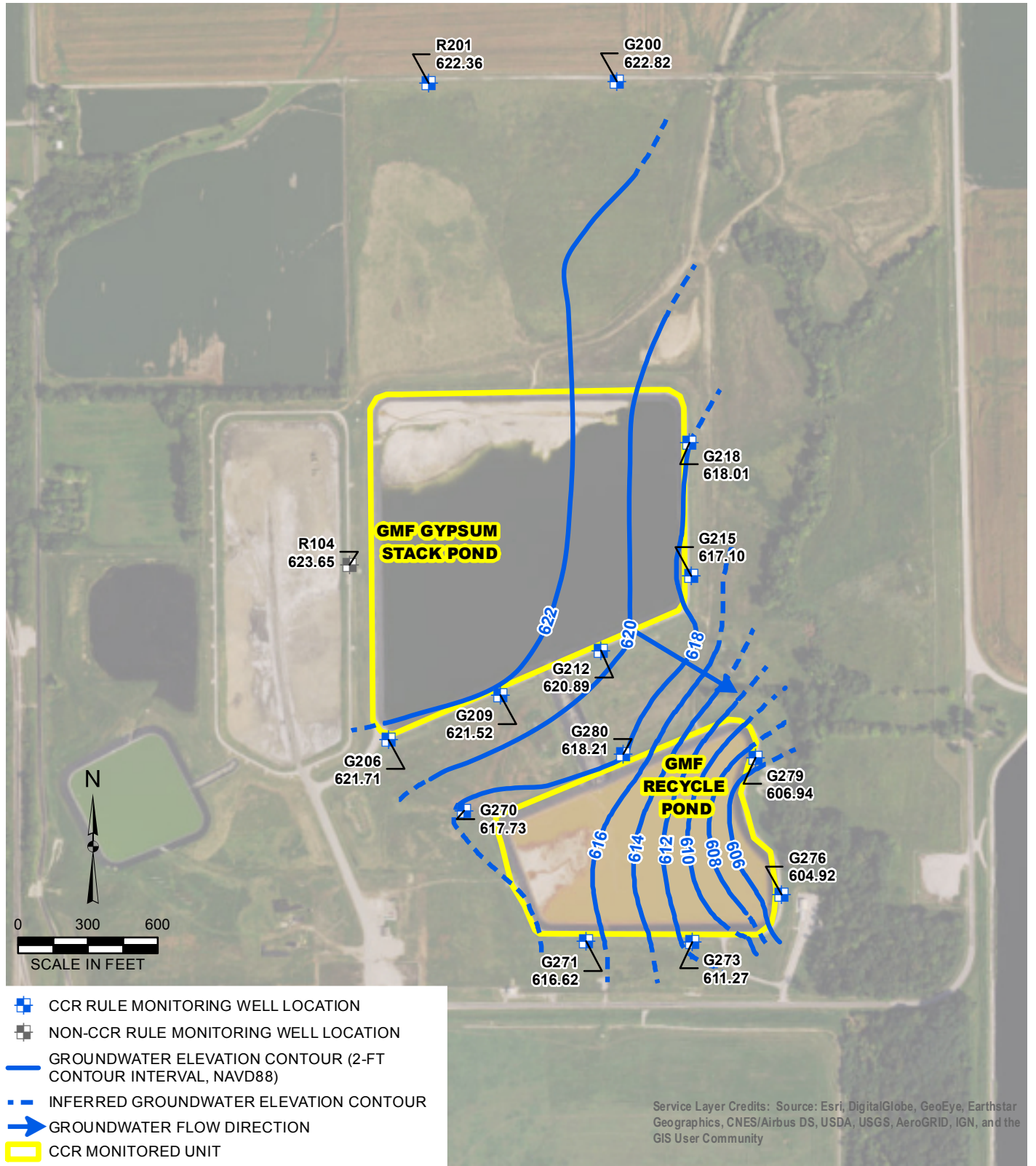
PROJECT NO: 2285
FIGURE NO: 1



DRAWN BY/DATE:
SDS 1/23/17
REVIEWED BY/DATE:
TBN 1/25/17
APPROVED BY/DATE:
JJW 2/8/17

DYNEGY CCR RULE GROUNDWATER MONITORING
COFFEEN POWER STATION
COFFEEN, ILLINOIS

Y:\Mapping\Projects\22\2285\MXD\GW_Contours\Round_04\R4_CoffeenGMF_GSP_GW_Contours.mxd Author: sstolz Date/Time: 3/3/2017, 1:46:48 PM



- CCR RULE MONITORING WELL LOCATION
- NON-CCR RULE MONITORING WELL LOCATION
- GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
- - - INFERRED GROUNDWATER ELEVATION CONTOUR
- GROUNDWATER FLOW DIRECTION
- CCR MONITORED UNIT

Service Layer Credits: Source: Esri, DigitalGlobe, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

**COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103)
AND COFFEEN GMF RECYCLE POND (UNIT ID: 104)
UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 4: JULY 25, 2016**

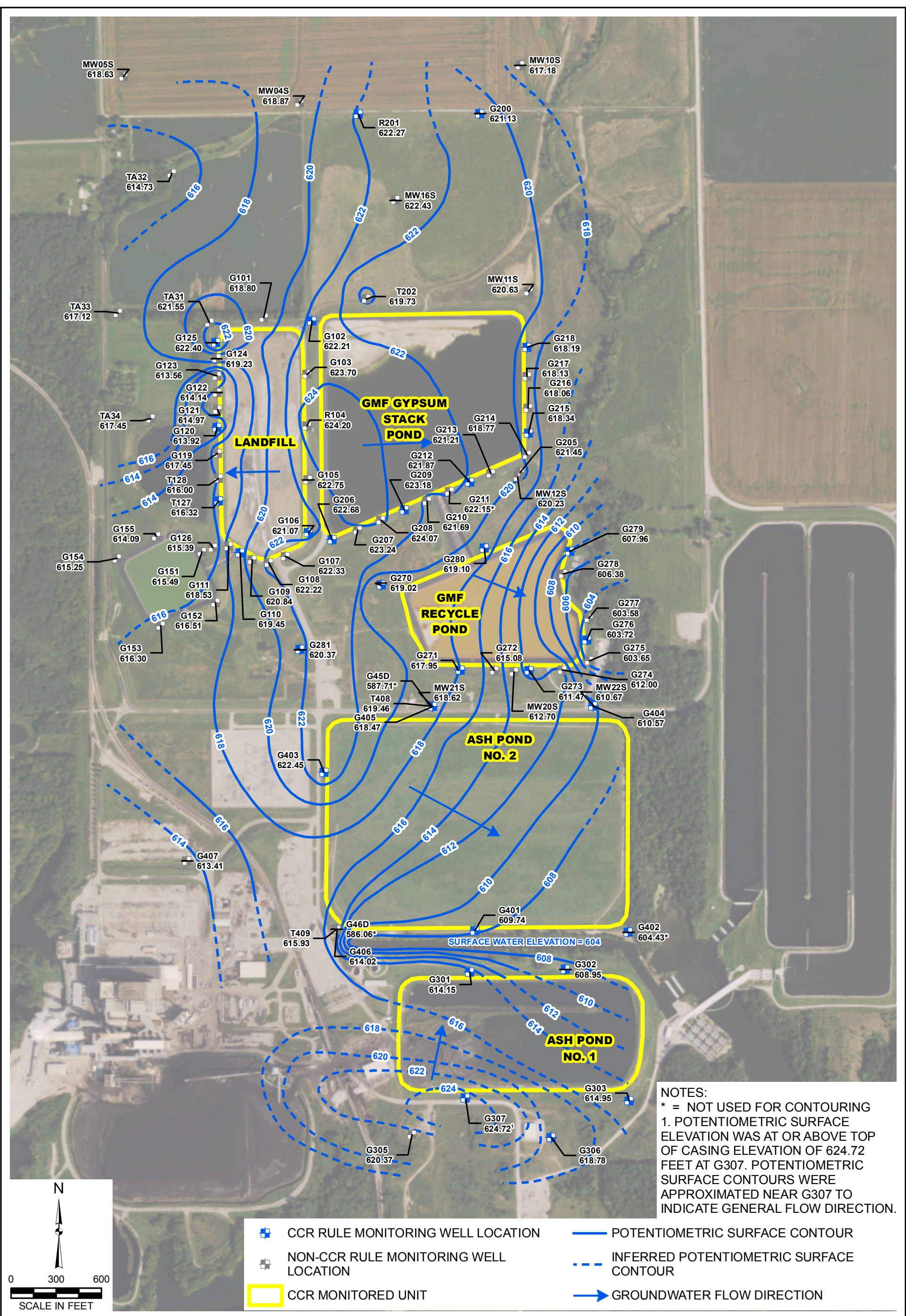
PROJECT NO: 2285
FIGURE NO: 1



DRAWN BY/DATE:
SDS 1/23/17
REVIEWED BY/DATE:
TBN 1/25/17
APPROVED BY/DATE:
JJW 2/8/17

DYNEGY CCR RULE GROUNDWATER MONITORING
COFFEEN POWER STATION
COFFEEN, ILLINOIS

Y:\Mapping\Projects\22285\MXD\GW_Contours\Round_06\B6_Coffeen_GW_Contours.mxd Author: stolzsd Date/Time: 9/1/2017, 4:55:10 PM

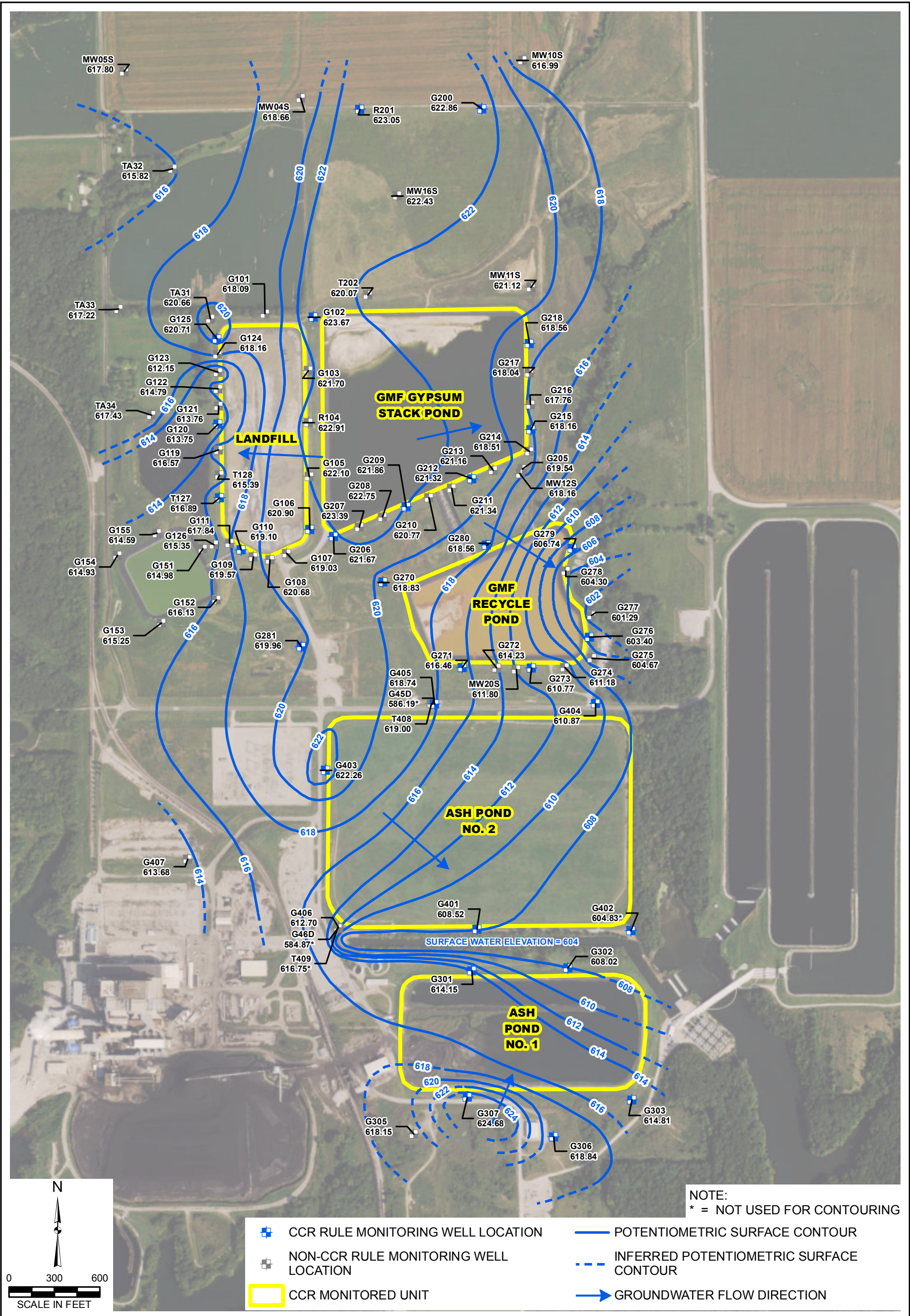


COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105) UPPERMOST AQUIFER UNIT GROUNDWATER ELEVATION CONTOUR MAP
ROUND 6: FEBRUARY 4, 2017
 DYNEGY CCR RULE GROUNDWATER MONITORING
 COFFEEN POWER STATION
 COFFEEN, ILLINOIS

DRAWN BY/DATE:
SDS 4/14/17
 REVIEWED BY/DATE:
TBN 4/14/17
 APPROVED BY/DATE:
JJW 8/30/17

PROJECT NO: 2285
 FIGURE NO: 1

 AN OBG COMPANY




NOTE:
* = NOT USED FOR CONTOURING

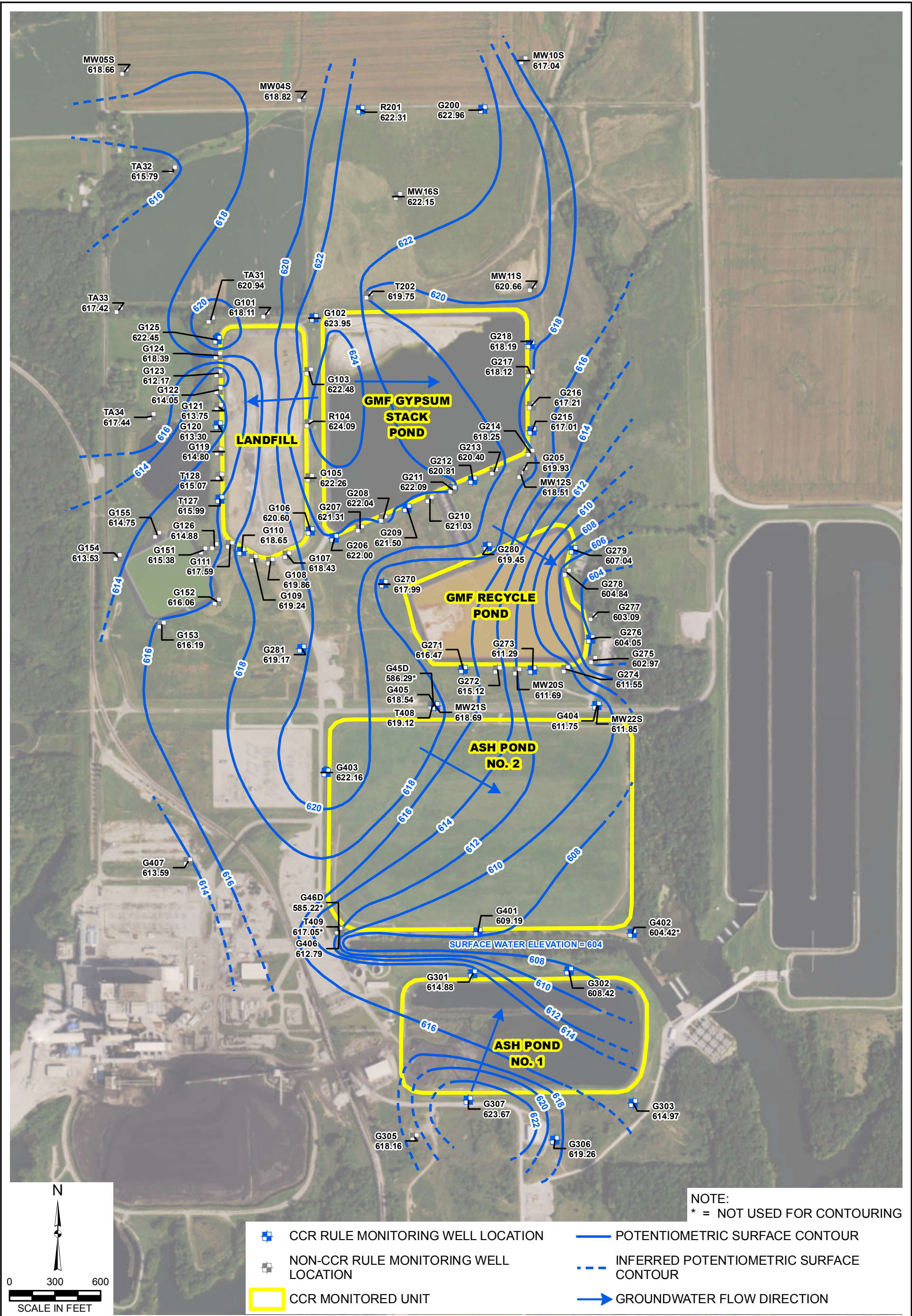
- CCR RULE MONITORING WELL LOCATION
- NON-CCR RULE MONITORING WELL LOCATION
- CCR MONITORED UNIT
- POTENTIOMETRIC SURFACE CONTOUR
- - - INFERRED POTENTIOMETRIC SURFACE CONTOUR
- GROUNDWATER FLOW DIRECTION

DRAWN BY/DATE:
SDS 7/12/17
REVIEWED BY/DATE:
TBN 7/12/17
APPROVED BY/DATE:
JJW 8/30/17

**COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102),
COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND
(UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105) UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 7: MAY 13, 2017
DYNEGY CCR RULE GROUNDWATER MONITORING
COFFEEN POWER STATION
COFFEEN, ILLINOIS**

PROJECT NO: 2285
FIGURE NO: 1

AN OBG COMPANY


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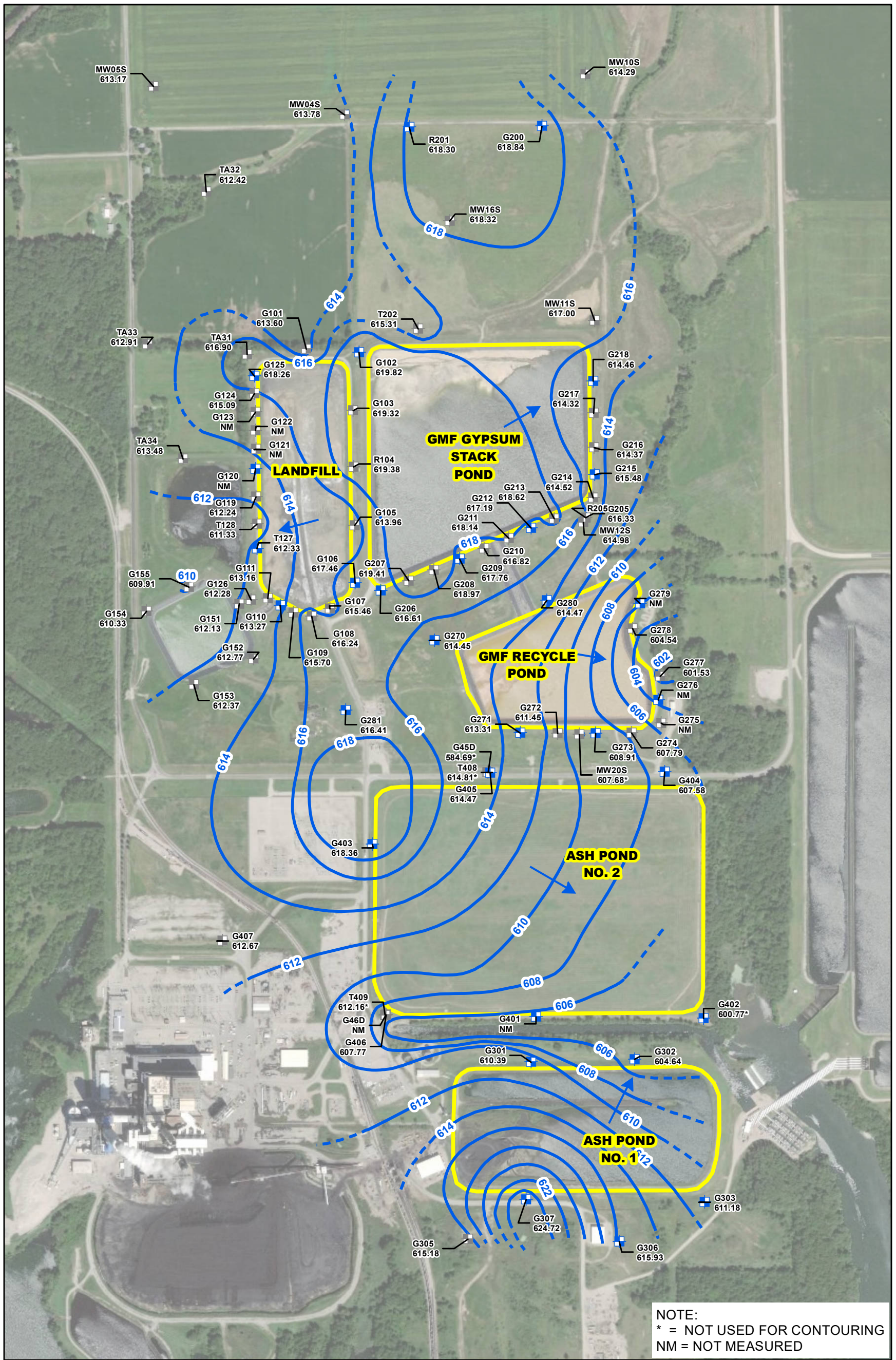
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DRAWN BY/DATE:
SDS 8/12/17
REVIEWED BY/DATE:
TBN 8/10/17
APPROVED BY/DATE:
JJW 8/30/17

**COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102),
COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND
(UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105) UPPERMOST AQUIFER UNIT
GROUNDWATER ELEVATION CONTOUR MAP
ROUND 8: JULY 8, 2017
DYNEGY CCR RULE GROUNDWATER MONITORING
COFFEEN POWER STATION
COFFEEN, ILLINOIS**

PROJECT NO: 2285
FIGURE NO: 1

AN OBG COMPANY

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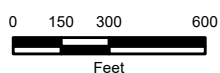
NOTE:
 * = NOT USED FOR CONTOURING
 NM = NOT MEASURED

LEGEND

- CCR RULE MONITORING WELL LOCATION
- NON-CCR RULE MONITORING WELL LOCATION
- GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
- - - INFERRED GROUNDWATER ELEVATION CONTOUR
- GROUNDWATER FLOW DIRECTION
- ▭ CCR MONITORED UNIT

COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102), COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
 GROUNDWATER ELEVATION CONTOUR MAP
 OCTOBER 21, 2017

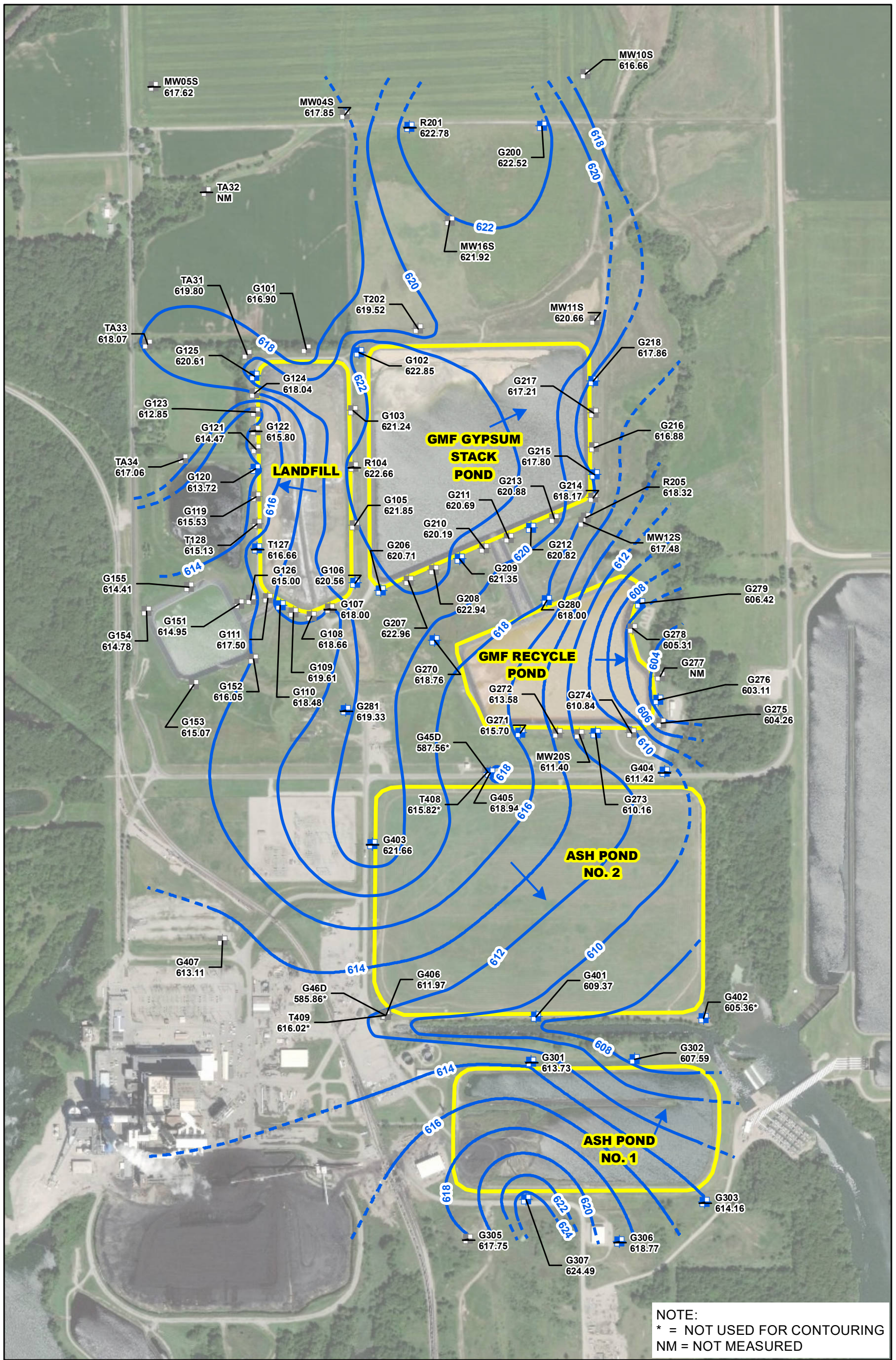
CCR RULE GROUNDWATER MONITORING COFFEEN POWER STATION COFFEEN, ILLINOIS



O'BRIEN & GERE ENGINEERS, INC.

FILE NO. 70099
 DATE 8/1/2018

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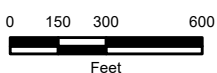
NOTE:
 * = NOT USED FOR CONTOURING
 NM = NOT MEASURED

LEGEND

- CCR RULE MONITORING WELL LOCATION
- NON-CCR RULE MONITORING WELL LOCATION
- GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
- - - INFERRED GROUNDWATER ELEVATION CONTOUR
- GROUNDWATER FLOW DIRECTION
- CCR MONITORED UNIT

COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102),
 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND
 (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
 GROUNDWATER ELEVATION CONTOUR MAP
 MAY 8, 2018

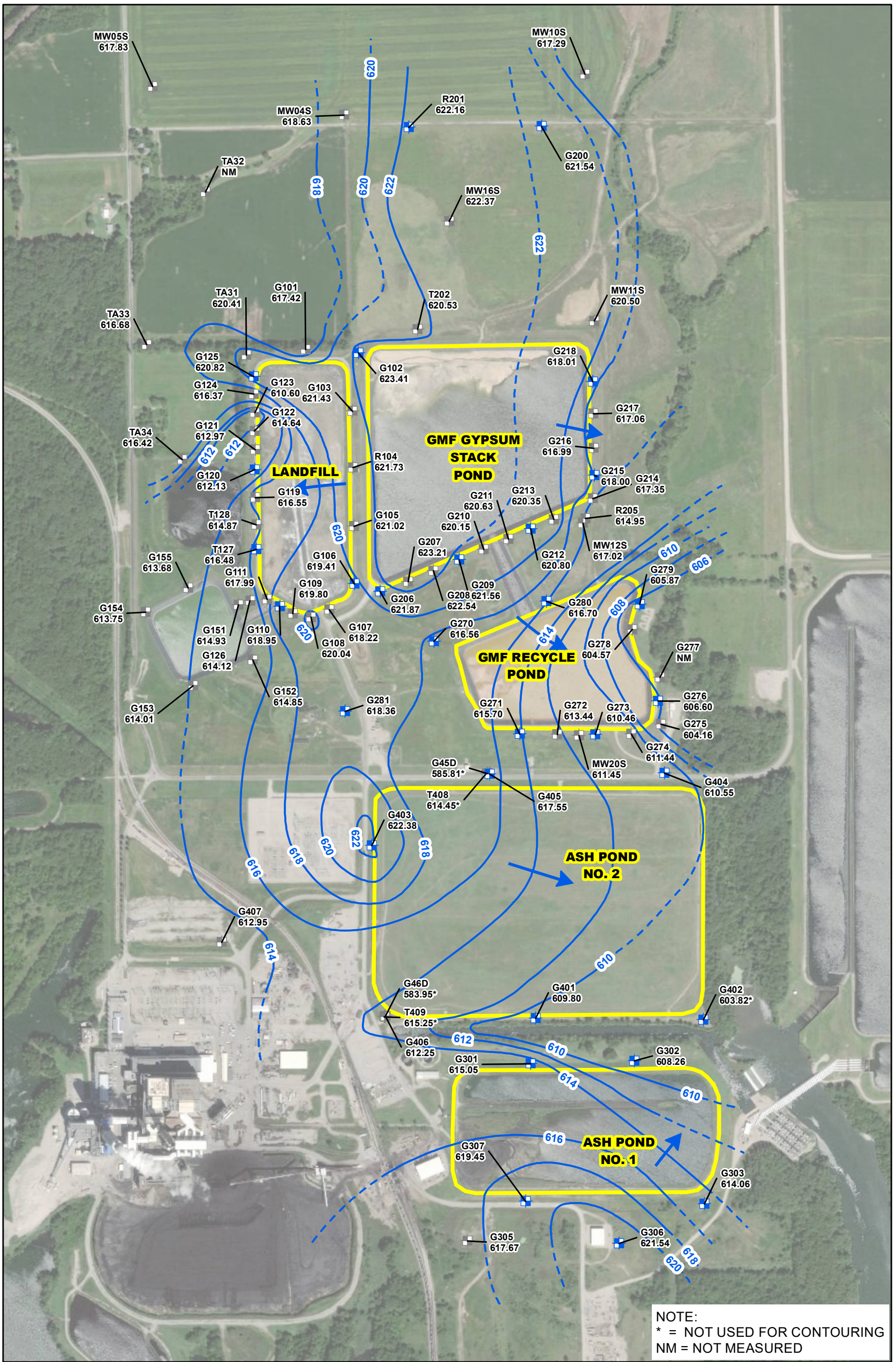
CCR RULE GROUNDWATER MONITORING
 COFFEEN POWER STATION
 COFFEEN, ILLINOIS



O'BRIEN & GERE ENGINEERS, INC.

FILE NO. 70099
 DATE 8/1/2018



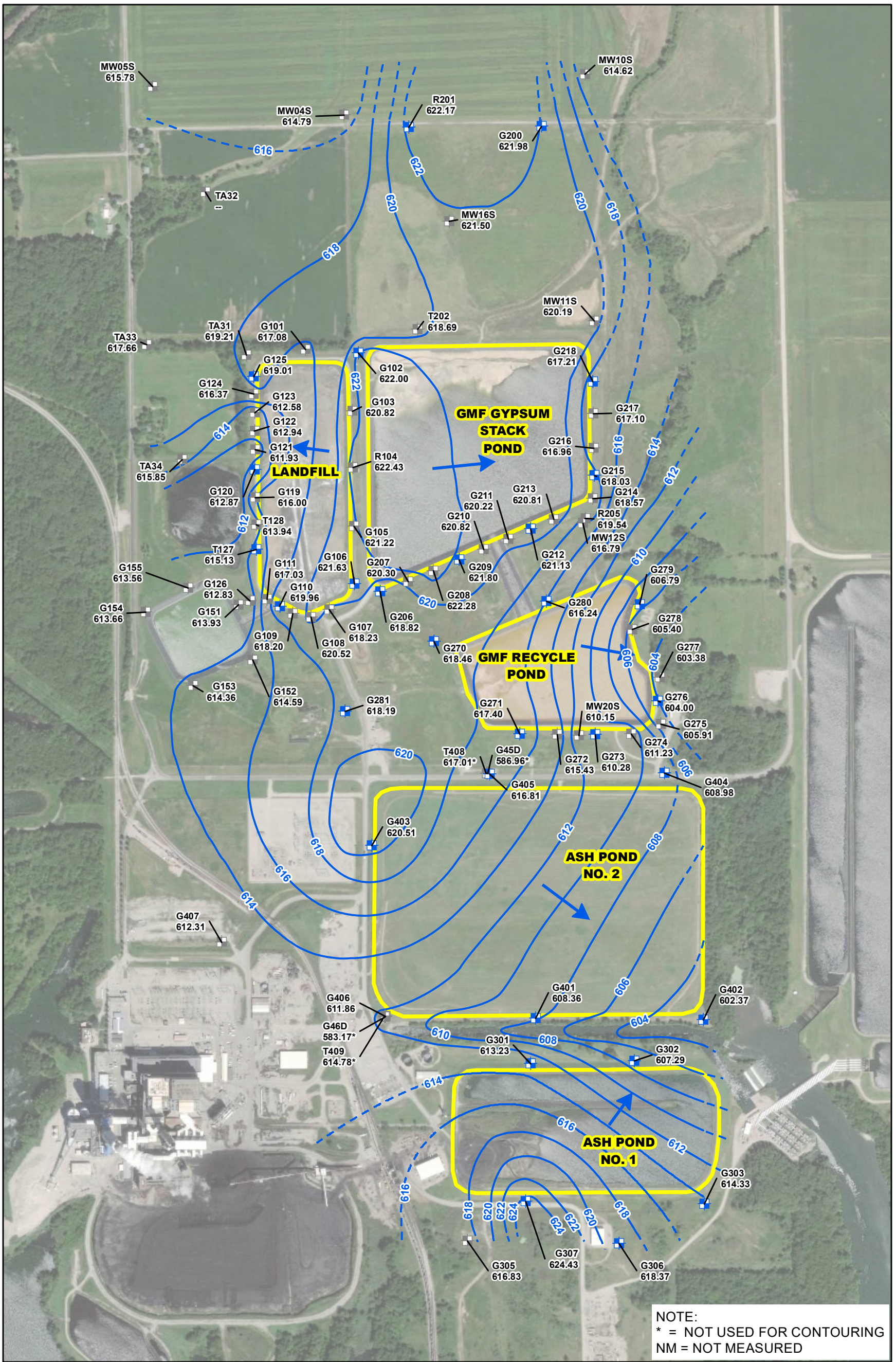


NOTE:
 * = NOT USED FOR CONTOURING
 NM = NOT MEASURED

- CCR RULE MONITORING WELL LOCATION
 - NON-CCR RULE MONITORING WELL LOCATION
 - GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
 - - - INFERRED GROUNDWATER ELEVATION CONTOUR
 - GROUNDWATER FLOW DIRECTION
 - CCR MONITORED UNIT
- COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102),
 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND
 (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
 GROUNDWATER ELEVATION CONTOUR MAP
 AUGUST 2, 2018
- CCR RULE GROUNDWATER MONITORING
 COFFEEN POWER STATION
 COFFEEN, ILLINOIS
- 0 150 300 600
 Feet

N

O'BRIEN & GERE ENGINEERS, INC.



■ CCR RULE MONITORING WELL LOCATION
■ NON-CCR RULE MONITORING WELL LOCATION
— GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
- - - INFERRED GROUNDWATER ELEVATION CONTOUR
→ GROUNDWATER FLOW DIRECTION
 CCR MONITORED UNIT

COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102),
 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND
 (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
GROUNDWATER ELEVATION CONTOUR MAP
 JANUARY 15, 2019

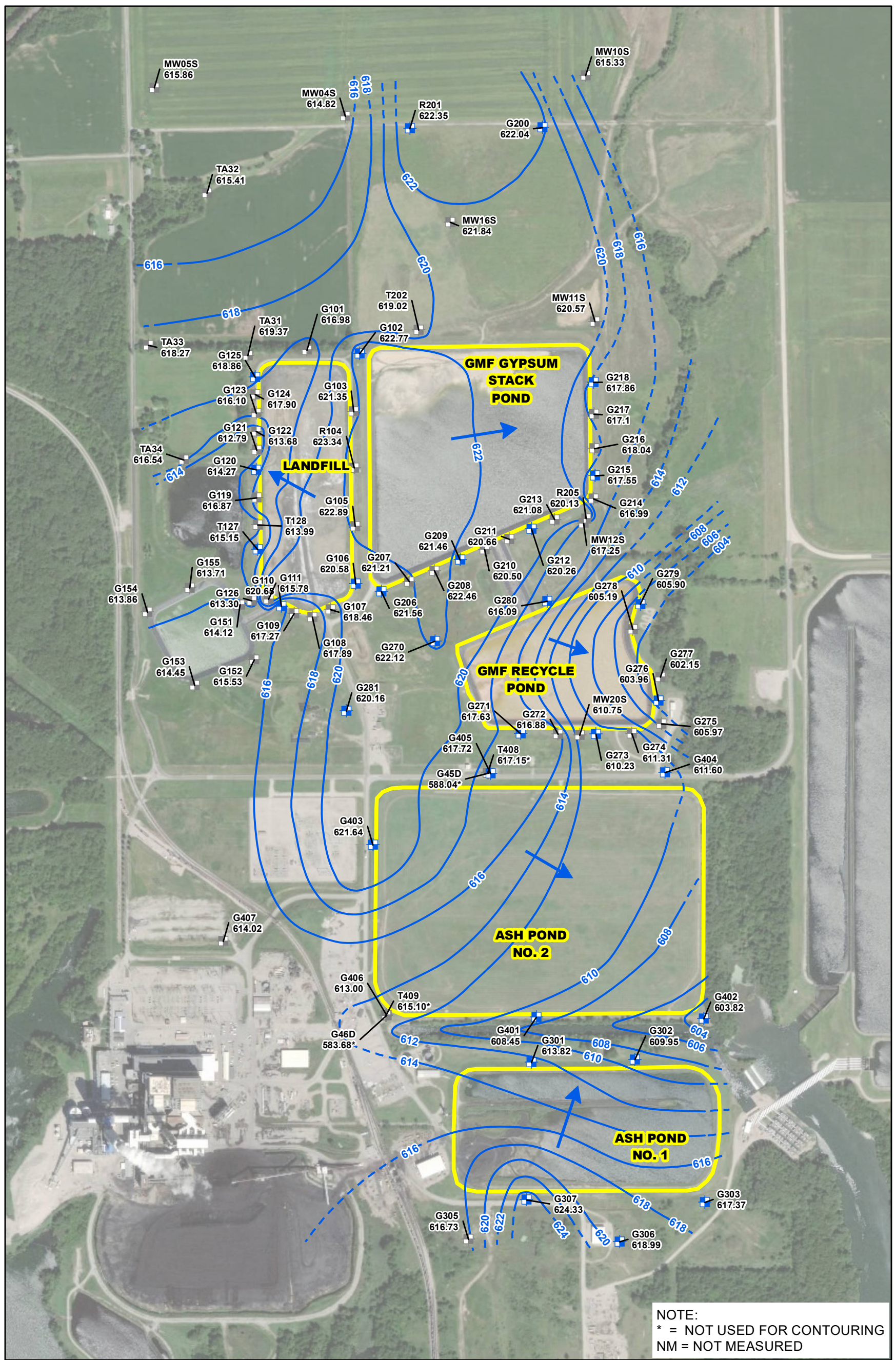
CCR RULE GROUNDWATER MONITORING
 COFFEEN POWER STATION
 COFFEEN, ILLINOIS

0 150 300 600
 Feet

NOTE:
 * = NOT USED FOR CONTOURING
 NM = NOT MEASURED

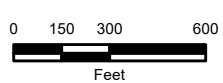
G
 O'BRIEN & GERE ENGINEERS, INC.

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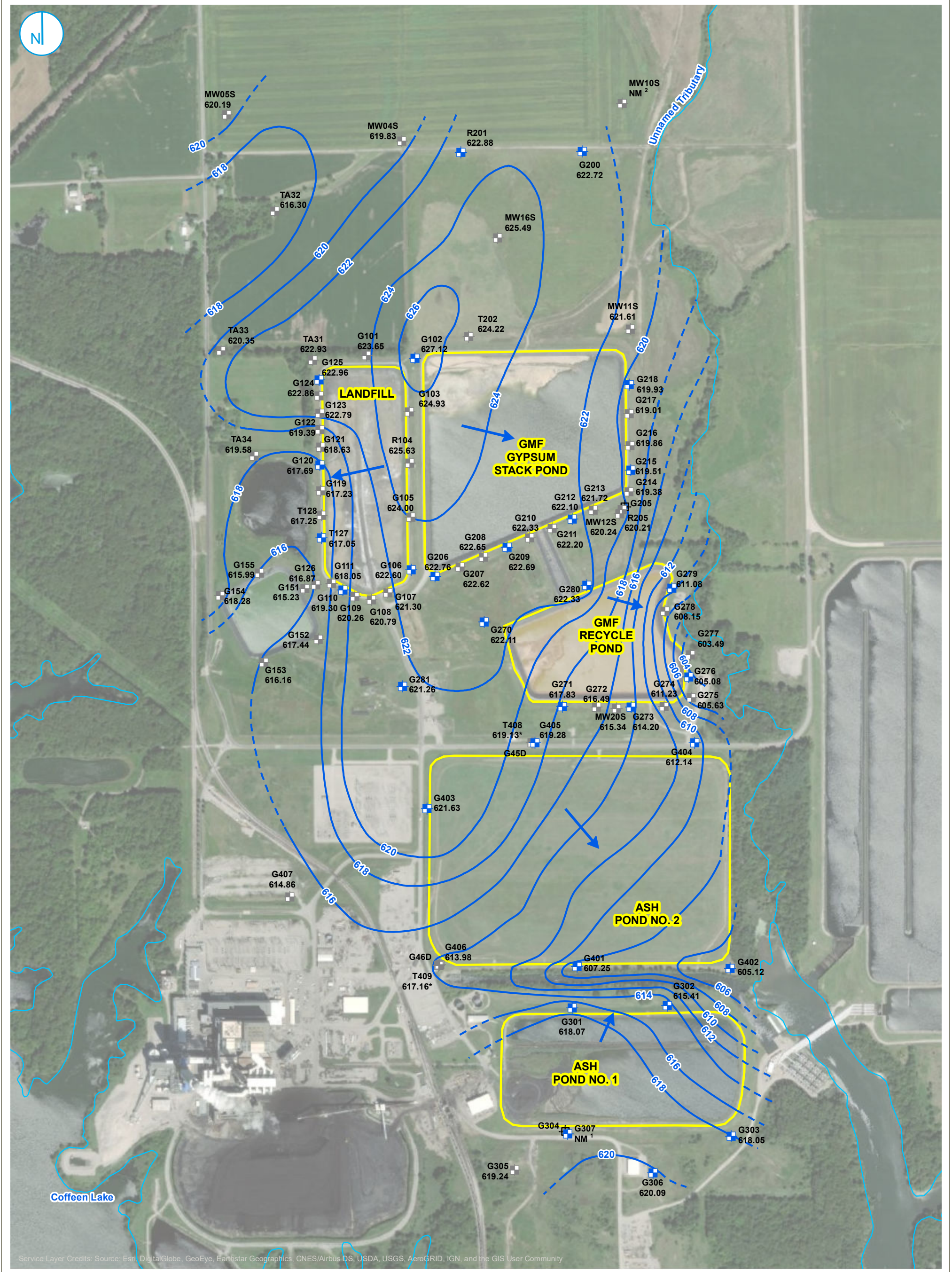


NOTE:
 * = NOT USED FOR CONTOURING
 NM = NOT MEASURED

- CCR RULE MONITORING WELL LOCATION
 - NON-CCR RULE MONITORING WELL LOCATION
 - GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
 - - - INFERRED GROUNDWATER ELEVATION CONTOUR
 - GROUNDWATER FLOW DIRECTION
 - CCR MONITORED UNIT
- COFFEEN ASH POND NO. 1 (UNIT ID: 101), COFFEEN ASH POND NO. 2 (UNIT ID: 102),
 COFFEEN GMF GYPSUM STACK POND (UNIT ID: 103), COFFEEN GMF RECYCLE POND
 (UNIT ID: 104) AND COFFEEN LANDFILL (UNIT ID: 105)
- GROUNDWATER ELEVATION CONTOUR MAP
 AUGUST 5, 2019
- CCR RULE GROUNDWATER MONITORING
 COFFEEN POWER STATION
 COFFEEN, ILLINOIS



O'BRIEN & GERE ENGINEERS, INC.



Service Layer Credits: Source: Esri, DigitalGlobe, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- CCR RULE MONITORING WELL LOCATION
- NON-CCR RULE MONITORING WELL LOCATION
- ABANDONED MONITORING WELL
- GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
- - - INFERRED GROUNDWATER ELEVATION CONTOUR
- GROUNDWATER FLOW DIRECTION
- CCR UNIT BOUNDARY
- SURFACE WATER FEATURE

NOTE:
 * = NOT USED FOR CONTOURING
 NM = NOT MEASURED
¹ G307 WAS FROZEN DURING THE JANUARY 20, 2020 SAMPLING EVENT AND WATER LEVEL COULD NOT BE COLLECTED.
² MW10S WAS DAMAGED PRIOR TO THE JANUARY 20, 2020 SAMPLING EVENT AND WATER LEVEL COULD NOT BE COLLECTED.

GROUNDWATER ELEVATION CONTOUR MAP JANUARY 20, 2020

CCR RULE GROUNDWATER MONITORING
 COFFEEN POWER STATION
 COFFEEN, ILLINOIS

RAMBOLL US CORPORATION
 A RAMBOLL COMPANY



TABLE E-1. GROUNDWATER ELEVATIONS

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G045D	11/12/2016	584.91
G045D	02/04/2017	587.71
G045D	05/13/2017	586.19
G045D	07/08/2017	586.29
G045D	10/21/2017	584.69
G045D	05/08/2018	587.56
G045D	08/02/2018	585.81
G045D	10/23/2018	584.60
G045D	01/15/2019	586.96
G045D	08/05/2019	588.04
G045D	08/10/2020	614.21
G045D	01/20/2021	614.60
G045D	04/20/2021	614.32
G045D	07/26/2021	613.58
G045D	08/16/2021	613.83
G046D	11/12/2016	583.59
G046D	02/04/2017	586.06
G046D	05/13/2017	584.87
G046D	07/08/2017	585.22
G046D	05/08/2018	585.86
G046D	08/02/2018	583.95
G046D	10/23/2018	582.05
G046D	01/15/2019	583.17
G046D	08/05/2019	583.68
G046D	08/10/2020	609.00
G046D	01/20/2021	610.49
G046D	04/20/2021	611.06
G046D	07/26/2021	607.21
G046D	08/16/2021	608.17
G101	01/20/2015	614.48
G101	04/08/2015	618.87
G101	07/23/2015	618.53
G101	10/06/2015	617.15
G101	11/16/2015	612.95
G101	02/08/2016	618.46
G101	05/09/2016	618.89
G101	07/25/2016	618.44
G101	11/12/2016	617.65
G101	02/04/2017	618.80
G101	05/13/2017	618.09
G101	07/08/2017	618.11
G101	10/21/2017	613.60
G101	05/08/2018	616.90
G101	08/02/2018	617.42
G101	10/23/2018	616.12
G101	01/15/2019	617.08

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G101	08/05/2019	616.98
G101	01/20/2020	623.65
G101	08/10/2020	616.70
G101	10/15/2020	613.61
G101	01/20/2021	617.20
G101	01/28/2021	617.80
G101	04/20/2021	622.85
G101	07/26/2021	619.94
G101	08/16/2021	619.95
G102	01/20/2015	619.18
G102	04/08/2015	622.06
G102	10/06/2015	622.02
G102	11/16/2015	618.96
G102	02/08/2016	624.04
G102	05/09/2016	625.34
G102	07/25/2016	623.92
G102	11/12/2016	623.39
G102	02/04/2017	622.21
G102	05/13/2017	623.67
G102	07/08/2017	623.95
G102	10/21/2017	619.82
G102	01/26/2018	621.79
G102	05/08/2018	622.85
G102	08/02/2018	623.41
G102	10/23/2018	621.71
G102	01/15/2019	622.00
G102	08/05/2019	622.77
G102	01/20/2020	627.12
G102	08/10/2020	621.72
G102	10/15/2020	618.94
G102	01/20/2021	619.79
G102	01/26/2021	621.71
G102	04/20/2021	623.86
G102	05/03/2021	624.28
G102	05/17/2021	623.83
G102	06/09/2021	623.09
G102	06/23/2021	621.22
G102	07/12/2021	622.92
G102	07/26/2021	622.97
G102	08/16/2021	622.69
G103	01/20/2015	620.82
G103	04/08/2015	622.58
G103	07/23/2015	621.70
G103	10/06/2015	620.69
G103	02/08/2016	621.68
G103	05/09/2016	623.26

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G103	07/25/2016	622.88
G103	11/12/2016	621.21
G103	02/04/2017	623.70
G103	05/13/2017	621.70
G103	07/08/2017	622.48
G103	10/21/2017	619.32
G103	05/08/2018	621.24
G103	08/02/2018	621.43
G103	10/23/2018	617.95
G103	01/15/2019	620.82
G103	08/05/2019	621.35
G103	01/20/2020	624.93
G103	08/10/2020	622.45
G103	10/15/2020	618.91
G103	01/20/2021	621.01
G103	01/28/2021	621.38
G103	04/20/2021	623.84
G103	07/26/2021	624.14
G103	08/16/2021	624.29
G105	01/20/2015	621.95
G105	04/08/2015	623.73
G105	07/23/2015	622.72
G105	10/06/2015	621.65
G105	02/08/2016	623.03
G105	05/09/2016	623.60
G105	07/25/2016	622.08
G105	11/12/2016	622.13
G105	02/04/2017	622.75
G105	05/13/2017	622.10
G105	07/08/2017	622.26
G105	10/21/2017	613.96
G105	05/08/2018	621.85
G105	08/02/2018	621.02
G105	10/23/2018	620.78
G105	01/15/2019	621.22
G105	08/05/2019	622.89
G105	01/20/2020	624.00
G105	08/10/2020	623.11
G105	10/15/2020	620.10
G105	01/20/2021	622.21
G105	01/28/2021	622.33
G105	04/20/2021	623.23
G105	07/26/2021	623.76
G105	08/16/2021	623.70
G106	01/20/2015	620.45
G106	04/08/2015	622.19

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G106	07/23/2015	621.43
G106	10/06/2015	620.50
G106	11/16/2015	619.32
G106	02/08/2016	621.55
G106	05/09/2016	622.11
G106	07/25/2016	620.62
G106	11/12/2016	620.65
G106	02/04/2017	621.07
G106	05/13/2017	620.90
G106	07/08/2017	620.60
G106	10/21/2017	617.46
G106	05/08/2018	620.56
G106	08/02/2018	619.41
G106	10/23/2018	619.35
G106	01/15/2019	621.63
G106	08/05/2019	620.58
G106	01/20/2020	622.60
G106	08/10/2020	620.48
G106	10/14/2020	618.19
G106	01/20/2021	620.90
G106	01/26/2021	620.90
G106	04/20/2021	621.69
G106	06/29/2021	621.95
G106	07/26/2021	621.88
G106	08/16/2021	621.90
G107	01/20/2015	619.23
G107	04/08/2015	620.85
G107	07/23/2015	620.15
G107	10/06/2015	619.10
G107	02/08/2016	620.26
G107	05/09/2016	620.78
G107	07/25/2016	618.37
G107	11/12/2016	618.72
G107	02/04/2017	622.33
G107	05/13/2017	619.03
G107	07/08/2017	618.43
G107	10/21/2017	615.46
G107	05/08/2018	618.00
G107	08/02/2018	618.22
G107	10/23/2018	616.28
G107	01/15/2019	618.23
G107	08/05/2019	618.46
G107	01/20/2020	621.30
G107	08/10/2020	618.68
G107	10/14/2020	616.56
G107	01/20/2021	619.58

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G107	01/28/2021	619.74
G107	04/20/2021	620.19
G107	07/26/2021	620.26
G107	08/16/2021	620.39
G108	01/19/2015	618.42
G108	04/08/2015	620.31
G108	07/24/2015	621.22
G108	10/07/2015	618.92
G108	02/08/2016	619.53
G108	05/09/2016	620.15
G108	07/25/2016	619.78
G108	11/12/2016	620.46
G108	02/04/2017	622.22
G108	05/13/2017	620.68
G108	07/08/2017	619.86
G108	10/21/2017	616.24
G108	05/08/2018	618.66
G108	08/02/2018	620.04
G108	10/23/2018	618.66
G108	01/15/2019	620.52
G108	08/05/2019	617.89
G108	01/20/2020	620.79
G108	08/10/2020	617.86
G108	10/14/2020	616.02
G108	01/20/2021	618.72
G108	01/28/2021	618.82
G108	04/20/2021	619.37
G108	07/26/2021	619.40
G108	08/16/2021	619.66
G109	01/19/2015	617.78
G109	04/08/2015	619.71
G109	07/24/2015	620.41
G109	10/06/2015	618.12
G109	02/08/2016	618.94
G109	05/09/2016	619.56
G109	07/25/2016	619.11
G109	11/12/2016	619.35
G109	02/04/2017	620.84
G109	05/13/2017	619.57
G109	07/08/2017	619.24
G109	10/21/2017	615.70
G109	05/08/2018	619.61
G109	08/02/2018	619.80
G109	10/23/2018	617.11
G109	01/15/2019	618.20
G109	08/05/2019	617.27

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G109	01/20/2020	620.26
G109	08/10/2020	617.16
G109	10/14/2020	615.52
G109	01/20/2021	617.91
G109	01/28/2021	618.58
G109	04/20/2021	618.59
G109	07/26/2021	618.68
G109	08/16/2021	618.96
G110	01/19/2015	616.76
G110	04/08/2015	618.60
G110	07/24/2015	619.55
G110	10/07/2015	617.70
G110	11/16/2015	616.55
G110	02/08/2016	617.88
G110	05/09/2016	618.53
G110	07/25/2016	617.64
G110	11/12/2016	618.86
G110	02/04/2017	619.45
G110	05/13/2017	619.10
G110	07/08/2017	618.65
G110	10/21/2017	613.27
G110	01/26/2018	616.74
G110	05/08/2018	618.48
G110	08/02/2018	618.95
G110	10/23/2018	618.42
G110	01/15/2019	619.96
G110	08/05/2019	620.65
G110	01/20/2020	619.30
G110	08/10/2020	616.14
G110	10/14/2020	614.90
G110	01/20/2021	616.81
G110	01/28/2021	616.81
G110	04/20/2021	617.71
G110	07/26/2021	617.76
G110	08/16/2021	617.97
G111	01/19/2015	615.93
G111	04/08/2015	617.48
G111	07/24/2015	618.03
G111	10/07/2015	616.79
G111	02/08/2016	616.92
G111	05/09/2016	617.52
G111	07/25/2016	617.35
G111	11/12/2016	617.61
G111	02/04/2017	618.53
G111	05/13/2017	617.84
G111	07/08/2017	617.59

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G111	10/21/2017	613.16
G111	05/08/2018	617.50
G111	08/02/2018	617.99
G111	10/23/2018	615.72
G111	01/15/2019	617.03
G111	08/05/2019	615.78
G111	01/20/2020	618.05
G111	08/10/2020	615.59
G111	10/14/2020	614.40
G111	01/20/2021	615.84
G111	01/28/2021	616.67
G111	04/20/2021	616.73
G111	07/26/2021	616.84
G111	08/16/2021	618.10
G119	01/19/2015	615.64
G119	04/08/2015	615.86
G119	07/23/2015	616.55
G119	10/06/2015	615.31
G119	02/08/2016	615.83
G119	05/09/2016	615.87
G119	07/25/2016	614.73
G119	11/12/2016	616.03
G119	02/04/2017	617.45
G119	05/13/2017	616.57
G119	07/08/2017	614.80
G119	10/21/2017	612.24
G119	05/08/2018	615.53
G119	08/02/2018	616.55
G119	10/23/2018	615.43
G119	01/15/2019	616.00
G119	08/05/2019	616.87
G119	01/20/2020	617.23
G119	08/10/2020	616.02
G119	10/13/2020	615.16
G119	01/20/2021	616.09
G119	01/28/2021	616.14
G119	04/20/2021	616.80
G119	07/26/2021	616.62
G119	08/16/2021	616.75
G120	01/19/2015	612.75
G120	04/08/2015	613.43
G120	07/23/2015	613.47
G120	10/06/2015	612.94
G120	11/16/2015	612.37
G120	02/08/2016	613.06
G120	05/09/2016	613.37

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G120	07/25/2016	612.87
G120	11/12/2016	613.45
G120	02/04/2017	613.92
G120	05/13/2017	613.75
G120	07/08/2017	613.30
G120	01/26/2018	612.69
G120	05/08/2018	613.72
G120	08/02/2018	612.13
G120	10/23/2018	612.68
G120	01/15/2019	612.87
G120	05/03/2019	618.15
G120	08/05/2019	614.27
G120	01/20/2020	617.69
G120	05/05/2020	618.23
G120	08/10/2020	615.22
G120	10/13/2020	614.39
G120	01/20/2021	615.80
G120	01/27/2021	615.80
G120	04/20/2021	617.55
G120	07/26/2021	616.95
G120	08/16/2021	617.19
G121	01/19/2015	613.63
G121	04/08/2015	614.63
G121	07/23/2015	614.09
G121	10/06/2015	613.31
G121	02/08/2016	614.10
G121	05/09/2016	614.81
G121	07/25/2016	613.62
G121	11/12/2016	613.33
G121	02/04/2017	614.97
G121	05/13/2017	613.76
G121	07/08/2017	613.75
G121	05/08/2018	614.47
G121	08/02/2018	612.97
G121	10/23/2018	611.97
G121	01/15/2019	611.93
G121	08/05/2019	612.79
G121	01/20/2020	618.63
G121	08/10/2020	615.02
G121	10/13/2020	613.69
G121	01/20/2021	615.44
G121	01/27/2021	616.14
G121	04/20/2021	618.73
G121	07/26/2021	616.79
G121	08/16/2021	617.27
G122	01/19/2015	610.79

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G122	04/08/2015	615.94
G122	07/23/2015	615.26
G122	10/06/2015	614.39
G122	02/08/2016	615.52
G122	05/09/2016	616.84
G122	07/25/2016	614.06
G122	11/12/2016	614.64
G122	02/04/2017	614.14
G122	05/13/2017	614.79
G122	07/08/2017	614.05
G122	05/08/2018	615.80
G122	08/02/2018	614.64
G122	10/23/2018	613.34
G122	01/15/2019	612.94
G122	08/05/2019	613.68
G122	01/20/2020	619.39
G122	08/10/2020	613.48
G122	10/13/2020	611.41
G122	01/20/2021	613.99
G122	01/27/2021	614.08
G122	04/20/2021	620.41
G122	07/26/2021	616.92
G122	08/16/2021	617.28
G123	01/19/2015	610.84
G123	04/08/2015	612.41
G123	07/23/2015	612.76
G123	10/06/2015	611.89
G123	02/08/2016	611.74
G123	05/09/2016	611.73
G123	07/25/2016	611.91
G123	11/12/2016	612.03
G123	02/04/2017	613.56
G123	05/13/2017	612.15
G123	07/08/2017	612.17
G123	05/08/2018	612.85
G123	08/02/2018	610.60
G123	10/23/2018	610.31
G123	01/15/2019	612.58
G123	08/05/2019	616.10
G123	01/20/2020	622.79
G123	08/10/2020	615.96
G123	10/14/2020	613.01
G123	01/20/2021	615.92
G123	01/27/2021	616.24
G123	04/20/2021	622.41
G123	07/26/2021	619.35

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G123	08/16/2021	617.73
G124	01/19/2015	615.27
G124	04/08/2015	617.85
G124	07/23/2015	618.25
G124	10/06/2015	617.27
G124	02/08/2016	616.47
G124	05/09/2016	616.81
G124	07/25/2016	618.27
G124	11/12/2016	617.85
G124	02/04/2017	619.23
G124	05/13/2017	618.16
G124	07/08/2017	618.39
G124	10/21/2017	615.09
G124	05/08/2018	618.04
G124	08/02/2018	616.37
G124	10/23/2018	615.61
G124	01/15/2019	616.37
G124	08/05/2019	617.90
G124	01/20/2020	622.86
G124	08/10/2020	615.53
G124	10/14/2020	612.59
G124	01/20/2021	615.96
G124	01/27/2021	616.10
G124	04/20/2021	622.44
G124	07/26/2021	619.05
G124	08/16/2021	619.43
G125	01/19/2015	617.83
G125	04/08/2015	620.45
G125	07/23/2015	620.71
G125	10/06/2015	619.66
G125	11/16/2015	614.60
G125	02/08/2016	619.95
G125	05/09/2016	620.22
G125	07/25/2016	621.53
G125	11/12/2016	620.46
G125	02/04/2017	622.40
G125	05/13/2017	620.71
G125	07/08/2017	622.45
G125	10/21/2017	618.26
G125	01/26/2018	613.88
G125	05/08/2018	620.61
G125	08/02/2018	620.82
G125	10/23/2018	618.58
G125	01/15/2019	619.01
G125	08/05/2019	618.86
G125	01/20/2020	622.96

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G125	05/05/2020	623.39
G125	08/10/2020	615.38
G125	10/14/2020	612.46
G125	01/20/2021	615.66
G125	01/27/2021	615.66
G125	04/20/2021	622.46
G125	07/26/2021	619.11
G125	08/16/2021	619.49
G126	01/20/2015	615.22
G126	04/08/2015	616.45
G126	07/23/2015	616.34
G126	10/07/2015	614.13
G126	02/08/2016	616.12
G126	05/09/2016	616.58
G126	07/25/2016	614.82
G126	11/12/2016	615.19
G126	02/04/2017	615.39
G126	05/13/2017	615.35
G126	07/08/2017	614.88
G126	10/21/2017	612.28
G126	05/08/2018	615.00
G126	08/02/2018	614.12
G126	10/23/2018	612.36
G126	01/15/2019	612.83
G126	08/05/2019	613.30
G126	01/20/2020	616.87
G126	08/10/2020	614.91
G126	10/14/2020	613.97
G126	01/20/2021	614.95
G126	01/29/2021	615.98
G126	04/20/2021	615.68
G126	07/26/2021	615.85
G126	08/16/2021	616.05
G151	07/23/2015	615.43
G151	10/06/2015	614.86
G151	11/12/2016	614.85
G151	02/04/2017	615.49
G151	05/13/2017	614.98
G151	07/08/2017	615.38
G151	10/21/2017	612.13
G151	05/08/2018	614.95
G151	08/02/2018	614.93
G151	10/23/2018	613.11
G151	01/15/2019	613.93
G151	08/05/2019	614.12
G151	01/20/2020	615.23

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G151	08/10/2020	614.11
G151	10/13/2020	613.39
G151	01/20/2021	613.86
G151	02/01/2021	615.07
G151	04/20/2021	614.53
G151	07/26/2021	614.83
G151	08/16/2021	615.06
G152	07/23/2015	616.47
G152	10/06/2015	614.06
G152	11/12/2016	615.74
G152	02/04/2017	616.51
G152	05/13/2017	616.13
G152	07/08/2017	616.06
G152	10/21/2017	612.77
G152	05/08/2018	616.05
G152	08/02/2018	614.85
G152	10/23/2018	614.35
G152	01/15/2019	614.59
G152	08/05/2019	615.53
G152	01/20/2020	617.44
G152	08/10/2020	614.46
G152	10/13/2020	613.13
G152	01/20/2021	614.87
G152	02/01/2021	613.13
G152	04/20/2021	615.34
G152	07/26/2021	616.76
G152	08/16/2021	615.30
G153	07/23/2015	615.93
G153	10/06/2015	614.45
G153	11/12/2016	615.15
G153	02/04/2017	616.30
G153	05/13/2017	615.25
G153	07/08/2017	616.19
G153	10/21/2017	612.37
G153	05/08/2018	615.07
G153	08/02/2018	614.01
G153	10/23/2018	613.31
G153	01/15/2019	614.36
G153	08/05/2019	614.45
G153	01/20/2020	616.16
G153	08/10/2020	613.72
G153	10/13/2020	612.16
G153	01/20/2021	612.66
G153	02/01/2021	613.18
G153	04/20/2021	615.52
G153	07/26/2021	613.97

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G153	08/16/2021	614.19
G154	07/23/2015	614.85
G154	10/06/2015	612.24
G154	11/12/2016	614.68
G154	02/04/2017	615.25
G154	05/13/2017	614.93
G154	07/08/2017	613.53
G154	10/21/2017	610.33
G154	05/08/2018	614.78
G154	08/02/2018	613.75
G154	10/23/2018	613.06
G154	01/15/2019	613.66
G154	08/05/2019	613.86
G154	01/20/2020	618.28
G154	08/10/2020	612.57
G154	10/13/2020	610.84
G154	01/20/2021	612.41
G154	02/01/2021	617.01
G154	04/20/2021	614.81
G154	07/26/2021	615.21
G154	08/16/2021	615.45
G155	07/23/2015	614.45
G155	10/06/2015	613.51
G155	11/12/2016	613.93
G155	02/04/2017	614.09
G155	05/13/2017	614.59
G155	07/08/2017	614.75
G155	10/21/2017	609.91
G155	05/08/2018	614.41
G155	08/02/2018	613.68
G155	10/23/2018	612.80
G155	01/15/2019	613.56
G155	08/05/2019	613.71
G155	01/20/2020	615.99
G155	08/10/2020	613.09
G155	10/13/2020	612.10
G155	01/20/2021	612.72
G155	02/01/2021	614.59
G155	04/20/2021	613.94
G155	07/26/2021	613.81
G155	08/16/2021	614.01
G200	10/05/2015	621.05
G200	11/16/2015	621.66
G200	02/08/2016	623.29
G200	05/09/2016	622.52
G200	07/25/2016	622.82

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G200	11/12/2016	622.82
G200	02/04/2017	621.13
G200	05/13/2017	622.86
G200	07/08/2017	622.96
G200	10/21/2017	618.84
G200	01/25/2018	620.39
G200	05/08/2018	622.52
G200	08/02/2018	621.54
G200	10/23/2018	621.14
G200	01/15/2019	621.98
G200	08/05/2019	622.04
G200	01/20/2020	622.72
G200	08/10/2020	618.16
G200	10/13/2020	615.63
G200	01/20/2021	619.63
G200	01/29/2021	619.63
G200	03/29/2021	623.27
G200	04/20/2021	621.86
G200	04/21/2021	622.19
G200	05/03/2021	622.69
G200	05/06/2021	623.36
G200	05/17/2021	622.10
G200	06/09/2021	620.84
G200	06/23/2021	619.38
G200	07/12/2021	620.52
G200	07/26/2021	619.74
G200	07/28/2021	619.56
G200	08/16/2021	619.88
G205	02/08/2016	620.10
G205	05/09/2016	620.48
G205	07/25/2016	619.81
G205	11/12/2016	620.04
G205	02/04/2017	621.45
G205	05/13/2017	619.54
G205	07/08/2017	619.93
G205	10/21/2017	616.33
G206	10/07/2015	620.69
G206	11/16/2015	619.27
G206	02/08/2016	621.92
G206	05/09/2016	622.30
G206	06/27/2016	620.51
G206	07/25/2016	621.71
G206	11/12/2016	621.44
G206	02/04/2017	622.68
G206	05/13/2017	621.67
G206	07/08/2017	622.00

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G206	10/21/2017	616.61
G206	05/08/2018	620.71
G206	08/02/2018	621.87
G206	10/23/2018	619.71
G206	01/15/2019	618.82
G206	08/05/2019	621.56
G206	01/20/2020	622.76
G206	05/05/2020	623.02
G206	08/10/2020	619.92
G206	10/13/2020	617.84
G206	01/20/2021	621.50
G206	01/27/2021	621.50
G206	04/20/2021	622.07
G206	05/03/2021	622.60
G206	05/17/2021	622.31
G206	06/09/2021	621.71
G206	06/23/2021	620.54
G206	07/12/2021	622.39
G206	07/26/2021	622.00
G206	08/16/2021	622.08
G206D	03/29/2021	583.94
G206D	03/30/2021	584.34
G206D	04/20/2021	585.96
G206D	04/22/2021	584.64
G206D	05/03/2021	587.42
G206D	05/05/2021	586.96
G206D	05/17/2021	587.81
G206D	05/18/2021	587.82
G206D	06/09/2021	584.19
G206D	06/23/2021	589.66
G206D	07/12/2021	590.72
G206D	07/26/2021	591.14
G206D	07/27/2021	591.15
G206D	08/16/2021	592.00
G207	10/07/2015	620.72
G207	02/08/2016	622.18
G207	05/09/2016	622.56
G207	07/25/2016	622.06
G207	11/12/2016	622.54
G207	02/04/2017	623.24
G207	05/13/2017	623.39
G207	07/08/2017	621.31
G207	10/21/2017	619.41
G207	05/08/2018	622.96
G207	08/02/2018	623.21
G207	10/23/2018	621.64

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G207	01/15/2019	620.30
G207	08/05/2019	621.21
G207	01/20/2020	622.62
G207	08/10/2020	619.71
G207	10/13/2020	617.71
G207	01/20/2021	621.85
G207	01/28/2021	621.86
G207	04/20/2021	622.30
G207	07/26/2021	622.25
G207	08/16/2021	622.36
G208	10/07/2015	620.62
G208	02/08/2016	622.19
G208	05/09/2016	622.63
G208	07/25/2016	622.20
G208	11/12/2016	622.61
G208	02/04/2017	624.07
G208	05/13/2017	622.75
G208	07/08/2017	622.04
G208	10/21/2017	618.97
G208	05/08/2018	622.94
G208	08/02/2018	622.54
G208	10/23/2018	620.66
G208	01/15/2019	622.28
G208	08/05/2019	622.46
G208	01/20/2020	622.65
G208	08/10/2020	619.56
G208	10/13/2020	617.65
G208	01/20/2021	622.09
G208	01/27/2021	622.13
G208	04/20/2021	622.37
G208	07/26/2021	622.37
G208	08/16/2021	622.50
G209	10/07/2015	620.56
G209	11/16/2015	620.06
G209	02/08/2016	622.26
G209	05/09/2016	622.74
G209	07/25/2016	621.52
G209	11/12/2016	621.65
G209	02/04/2017	623.18
G209	05/13/2017	621.86
G209	07/08/2017	621.50
G209	10/21/2017	617.76
G209	01/25/2018	619.95
G209	05/08/2018	621.35
G209	08/02/2018	621.56
G209	10/23/2018	619.91

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G209	01/15/2019	621.80
G209	05/03/2019	623.09
G209	08/05/2019	621.46
G209	01/20/2020	622.69
G209	05/05/2020	622.96
G209	08/10/2020	619.59
G209	10/13/2020	617.69
G209	01/20/2021	621.94
G209	01/27/2021	621.94
G209	04/20/2021	622.40
G209	05/03/2021	622.82
G209	05/17/2021	622.51
G209	06/09/2021	622.03
G209	06/23/2021	620.89
G209	07/12/2021	622.08
G209	07/26/2021	622.41
G209	08/16/2021	622.56
G210	10/07/2015	619.83
G210	02/08/2016	621.72
G210	05/09/2016	622.50
G210	07/25/2016	620.94
G210	11/12/2016	621.05
G210	02/04/2017	621.69
G210	05/13/2017	620.77
G210	07/08/2017	621.03
G210	10/21/2017	616.82
G210	05/08/2018	620.19
G210	08/02/2018	620.15
G210	10/23/2018	618.92
G210	01/15/2019	620.82
G210	08/05/2019	620.50
G210	01/20/2020	622.33
G210	08/10/2020	619.97
G210	10/13/2020	618.00
G210	01/20/2021	620.58
G210	01/27/2021	620.40
G210	04/20/2021	622.18
G210	07/26/2021	622.18
G210	08/16/2021	622.28
G211	10/07/2015	619.00
G211	02/08/2016	622.08
G211	05/09/2016	622.45
G211	07/25/2016	621.81
G211	11/12/2016	621.28
G211	02/04/2017	622.15
G211	05/13/2017	621.34

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G211	07/08/2017	622.09
G211	10/21/2017	618.14
G211	05/08/2018	620.69
G211	08/02/2018	620.63
G211	10/23/2018	619.52
G211	01/15/2019	620.22
G211	08/05/2019	620.66
G211	01/20/2020	622.20
G211	08/10/2020	619.83
G211	10/13/2020	617.77
G211	01/20/2021	620.22
G211	01/27/2021	620.50
G211	04/20/2021	622.04
G211	07/26/2021	621.82
G211	08/16/2021	621.91
G212	10/07/2015	620.76
G212	11/16/2015	618.54
G212	02/08/2016	621.99
G212	05/09/2016	622.04
G212	07/25/2016	620.89
G212	11/12/2016	621.00
G212	02/04/2017	621.87
G212	05/13/2017	621.32
G212	07/08/2017	620.81
G212	10/21/2017	617.19
G212	05/08/2018	620.82
G212	08/02/2018	620.80
G212	10/23/2018	619.43
G212	01/15/2019	621.13
G212	08/05/2019	620.26
G212	01/20/2020	622.10
G212	08/10/2020	619.14
G212	10/13/2020	616.90
G212	01/20/2021	620.08
G212	01/26/2021	620.08
G212	04/20/2021	621.60
G212	05/03/2021	622.12
G212	05/17/2021	621.74
G212	06/09/2021	621.19
G212	06/23/2021	619.96
G212	06/29/2021	620.08
G212	07/12/2021	620.55
G212	07/26/2021	621.13
G212	08/16/2021	621.41
G213	10/07/2015	620.21
G213	02/08/2016	621.20

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G213	05/09/2016	621.69
G213	07/25/2016	620.36
G213	11/12/2016	620.78
G213	02/04/2017	621.21
G213	05/13/2017	621.16
G213	07/08/2017	620.40
G213	10/21/2017	618.62
G213	05/08/2018	620.88
G213	08/02/2018	620.35
G213	10/23/2018	619.83
G213	01/15/2019	620.81
G213	08/05/2019	621.08
G213	01/20/2020	621.72
G213	08/10/2020	618.66
G213	10/13/2020	616.56
G213	01/20/2021	619.61
G213	01/27/2021	619.97
G213	04/20/2021	621.28
G213	07/26/2021	620.96
G213	08/16/2021	621.20
G214	10/07/2015	617.56
G214	02/08/2016	618.11
G214	05/09/2016	619.39
G214	07/25/2016	617.75
G214	11/12/2016	618.16
G214	02/04/2017	618.77
G214	05/13/2017	618.51
G214	07/08/2017	618.25
G214	10/21/2017	614.52
G214	05/08/2018	618.17
G214	08/02/2018	617.35
G214	10/23/2018	616.87
G214	01/15/2019	618.57
G214	08/05/2019	616.99
G214	01/20/2020	619.38
G214	08/10/2020	616.32
G214	10/13/2020	614.47
G214	01/20/2021	616.45
G214	01/27/2021	616.64
G214	04/20/2021	618.60
G214	07/26/2021	618.39
G214	08/16/2021	618.55
G215	10/07/2015	616.56
G215	11/16/2015	616.38
G215	02/08/2016	618.31
G215	05/09/2016	619.45

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G215	07/25/2016	617.10
G215	11/12/2016	617.91
G215	02/04/2017	618.34
G215	05/13/2017	618.16
G215	07/08/2017	617.01
G215	10/21/2017	615.48
G215	05/08/2018	617.80
G215	08/02/2018	618.00
G215	10/23/2018	616.26
G215	01/15/2019	618.03
G215	08/05/2019	617.55
G215	01/20/2020	619.51
G215	08/10/2020	617.11
G215	10/14/2020	618.58
G215	01/20/2021	617.19
G215	01/26/2021	617.19
G215	04/20/2021	618.83
G215	05/03/2021	619.20
G215	05/17/2021	619.10
G215	06/09/2021	618.65
G215	06/23/2021	617.45
G215	06/29/2021	617.72
G215	07/12/2021	618.24
G215	07/26/2021	618.79
G215	08/16/2021	618.91
G216	10/07/2015	616.66
G216	02/08/2016	618.74
G216	05/09/2016	619.81
G216	07/25/2016	617.68
G216	11/12/2016	617.68
G216	02/04/2017	618.06
G216	05/13/2017	617.76
G216	07/08/2017	617.21
G216	10/21/2017	614.37
G216	05/08/2018	616.88
G216	08/02/2018	616.99
G216	10/23/2018	615.92
G216	01/15/2019	616.96
G216	08/05/2019	618.04
G216	01/20/2020	619.86
G216	08/10/2020	617.54
G216	10/14/2020	615.85
G216	01/20/2021	617.65
G216	01/28/2021	617.48
G216	04/20/2021	619.25
G216	07/26/2021	619.20

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G216	08/16/2021	619.30
G217	10/07/2015	616.71
G217	02/08/2016	618.25
G217	05/09/2016	619.13
G217	07/25/2016	617.81
G217	11/12/2016	617.81
G217	02/04/2017	618.13
G217	05/13/2017	618.04
G217	07/08/2017	618.12
G217	10/21/2017	614.32
G217	05/08/2018	617.21
G217	08/02/2018	617.06
G217	10/23/2018	616.17
G217	01/15/2019	617.10
G217	08/05/2019	617.10
G217	01/20/2020	619.01
G217	08/10/2020	616.20
G217	10/14/2020	614.57
G217	01/20/2021	616.74
G217	01/28/2021	616.84
G217	04/20/2021	618.45
G217	07/26/2021	617.93
G217	08/16/2021	618.04
G218	10/07/2015	616.93
G218	11/16/2015	617.11
G218	02/08/2016	619.05
G218	05/09/2016	620.10
G218	07/25/2016	618.01
G218	11/12/2016	618.39
G218	02/04/2017	618.19
G218	05/13/2017	618.56
G218	07/08/2017	618.19
G218	10/21/2017	614.46
G218	01/26/2018	616.46
G218	05/08/2018	617.87
G218	08/02/2018	618.01
G218	10/23/2018	616.66
G218	01/15/2019	617.21
G218	08/05/2019	617.86
G218	01/20/2020	619.93
G218	08/10/2020	617.42
G218	10/14/2020	615.65
G218	01/20/2021	617.53
G218	01/26/2021	617.53
G218	04/20/2021	619.53
G218	05/03/2021	619.90

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G218	05/17/2021	619.72
G218	06/09/2021	619.32
G218	06/23/2021	617.87
G218	07/12/2021	618.60
G218	07/26/2021	619.11
G218	08/16/2021	619.33
G270	10/05/2015	616.07
G270	11/16/2015	621.06
G270	02/08/2016	622.94
G270	05/09/2016	622.77
G270	07/25/2016	617.73
G270	11/12/2016	618.31
G270	02/04/2017	619.02
G270	05/13/2017	618.83
G270	07/08/2017	617.99
G270	10/21/2017	614.45
G270	05/08/2018	618.76
G270	08/02/2018	616.56
G270	10/23/2018	617.01
G270	01/15/2019	618.46
G270	08/05/2019	622.12
G270	01/20/2020	622.11
G270	08/10/2020	618.11
G270	10/14/2020	616.17
G270	01/20/2021	622.51
G270	01/21/2021	622.57
G270	03/29/2021	623.38
G270	03/30/2021	623.44
G270	04/20/2021	622.74
G270	04/21/2021	622.85
G270	05/03/2021	623.08
G270	05/06/2021	623.27
G270	05/17/2021	622.87
G270	05/19/2021	623.30
G270	06/09/2021	621.75
G270	06/15/2021	620.09
G270	06/23/2021	619.06
G270	06/29/2021	621.69
G270	07/12/2021	622.56
G270	07/26/2021	622.39
G270	07/27/2021	622.30
G270	08/16/2021	622.54
G271	10/08/2015	614.12
G271	11/16/2015	613.77
G271	02/08/2016	615.87
G271	05/09/2016	616.05

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G271	07/25/2016	616.62
G271	11/12/2016	616.49
G271	02/04/2017	617.95
G271	05/13/2017	616.46
G271	07/08/2017	616.47
G271	07/17/2017	616.47
G271	10/21/2017	613.31
G271	05/08/2018	615.70
G271	08/02/2018	615.70
G271	10/23/2018	614.73
G271	01/15/2019	617.40
G271	08/05/2019	617.63
G271	01/20/2020	617.83
G271	08/10/2020	614.18
G271	08/13/2020	614.18
G271	10/14/2020	612.90
G271	01/20/2021	613.91
G271	02/01/2021	613.91
G271	04/20/2021	615.51
G271	05/03/2021	615.96
G271	05/17/2021	615.78
G271	06/09/2021	615.52
G271	06/23/2021	615.02
G271	07/12/2021	615.57
G271	07/26/2021	615.67
G271	08/16/2021	615.78
G272	10/08/2015	612.56
G272	02/08/2016	614.93
G272	05/09/2016	614.96
G272	07/25/2016	614.79
G272	11/12/2016	614.34
G272	02/04/2017	615.08
G272	05/13/2017	614.23
G272	07/08/2017	615.12
G272	10/21/2017	611.45
G272	05/08/2018	613.58
G272	08/02/2018	613.44
G272	10/23/2018	612.96
G272	01/15/2019	615.43
G272	08/05/2019	616.88
G272	01/20/2020	616.49
G272	08/10/2020	613.19
G272	10/14/2020	611.89
G272	01/20/2021	613.01
G272	02/01/2021	616.48
G272	04/20/2021	614.50

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G272	07/26/2021	614.44
G272	08/16/2021	614.47
G273	10/08/2015	610.41
G273	11/16/2015	611.82
G273	02/08/2016	613.26
G273	05/09/2016	612.83
G273	07/25/2016	611.27
G273	11/12/2016	610.83
G273	02/04/2017	611.47
G273	05/13/2017	610.77
G273	07/08/2017	611.29
G273	07/17/2017	611.29
G273	10/21/2017	608.91
G273	05/08/2018	610.16
G273	08/02/2018	610.46
G273	10/23/2018	608.82
G273	01/15/2019	610.28
G273	08/05/2019	610.23
G273	01/20/2020	614.20
G273	08/10/2020	611.52
G273	08/13/2020	611.52
G273	10/14/2020	610.31
G273	01/20/2021	611.52
G273	02/01/2021	611.52
G273	04/20/2021	612.42
G273	05/03/2021	612.90
G273	05/17/2021	612.63
G273	06/09/2021	612.24
G273	06/23/2021	611.79
G273	07/12/2021	612.22
G273	07/26/2021	612.35
G273	08/16/2021	613.52
G274	10/08/2015	610.06
G274	02/08/2016	610.22
G274	05/09/2016	609.97
G274	07/25/2016	611.06
G274	11/12/2016	610.86
G274	02/04/2017	612.00
G274	05/13/2017	611.18
G274	07/08/2017	611.55
G274	10/21/2017	607.79
G274	05/08/2018	610.84
G274	08/02/2018	611.44
G274	10/23/2018	609.52
G274	01/15/2019	611.23
G274	08/05/2019	611.31

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G274	01/20/2020	611.23
G274	08/10/2020	609.29
G274	10/14/2020	608.49
G274	01/20/2021	610.36
G274	02/01/2021	611.18
G274	04/20/2021	609.89
G274	07/26/2021	609.82
G274	08/16/2021	609.88
G275	02/08/2016	604.71
G275	05/09/2016	604.76
G275	07/25/2016	603.17
G275	11/12/2016	604.28
G275	02/04/2017	603.65
G275	05/13/2017	604.67
G275	07/08/2017	602.97
G275	05/08/2018	604.26
G275	08/02/2018	604.16
G275	10/23/2018	604.46
G275	01/15/2019	605.91
G275	08/05/2019	605.97
G275	01/20/2020	605.63
G275	08/10/2020	604.95
G275	01/20/2021	605.02
G275	04/20/2021	605.00
G275	07/13/2021	605.63
G275	07/26/2021	605.05
G275	08/16/2021	605.09
G275D	03/30/2021	570.32
G275D	04/20/2021	570.98
G275D	04/22/2021	568.33
G275D	05/03/2021	569.75
G275D	05/05/2021	570.26
G275D	05/17/2021	568.67
G275D	05/18/2021	569.00
G275D	06/09/2021	570.31
G275D	06/23/2021	569.71
G275D	07/12/2021	570.43
G275D	07/26/2021	570.35
G275D	07/28/2021	570.68
G275D	08/16/2021	571.48
G276	11/16/2015	603.25
G276	02/08/2016	603.71
G276	05/09/2016	604.71
G276	07/25/2016	604.92
G276	11/12/2016	603.60
G276	02/04/2017	603.72

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G276	05/13/2017	603.40
G276	07/08/2017	604.05
G276	07/18/2017	604.05
G276	05/08/2018	603.11
G276	08/02/2018	606.60
G276	10/23/2018	603.35
G276	01/15/2019	604.00
G276	08/05/2019	603.96
G276	01/20/2020	605.08
G276	08/10/2020	604.63
G276	08/12/2020	604.63
G276	10/14/2020	603.59
G276	01/20/2021	603.71
G276	04/20/2021	604.65
G276	05/03/2021	604.71
G276	05/17/2021	604.88
G276	06/09/2021	604.93
G276	06/23/2021	604.53
G276	06/28/2021	604.58
G276	07/12/2021	604.55
G276	07/26/2021	604.68
G276	08/16/2021	604.73
G277	02/08/2016	602.98
G277	05/09/2016	603.79
G277	07/25/2016	602.08
G277	11/12/2016	601.23
G277	02/04/2017	603.58
G277	05/13/2017	601.29
G277	07/08/2017	603.09
G277	10/21/2017	601.53
G277	10/23/2018	601.28
G277	01/15/2019	603.38
G277	08/05/2019	602.15
G277	01/20/2020	603.49
G277	08/10/2020	603.29
G277	04/20/2021	603.33
G277	07/26/2021	603.33
G278	02/08/2016	606.56
G278	05/09/2016	607.00
G278	07/25/2016	604.57
G278	11/12/2016	604.29
G278	02/04/2017	606.38
G278	05/13/2017	604.30
G278	07/08/2017	604.84
G278	10/21/2017	604.54
G278	05/08/2018	605.31

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G278	08/02/2018	604.57
G278	10/23/2018	604.29
G278	01/15/2019	605.40
G278	08/05/2019	605.19
G278	01/20/2020	608.15
G278	08/10/2020	606.83
G278	10/14/2020	605.55
G278	01/20/2021	605.18
G278	04/20/2021	606.47
G278	07/26/2021	607.49
G278	08/16/2021	607.62
G279	10/08/2015	608.14
G279	11/16/2015	607.80
G279	02/08/2016	609.16
G279	05/09/2016	610.17
G279	07/25/2016	606.94
G279	11/12/2016	606.93
G279	02/04/2017	607.96
G279	05/13/2017	606.74
G279	07/08/2017	607.04
G279	07/18/2017	607.04
G279	05/08/2018	606.42
G279	08/02/2018	605.87
G279	10/23/2018	604.87
G279	01/15/2019	606.79
G279	08/05/2019	605.90
G279	01/20/2020	611.08
G279	08/10/2020	607.17
G279	08/12/2020	607.17
G279	10/14/2020	605.54
G279	01/20/2021	607.07
G279	01/28/2021	607.07
G279	04/20/2021	608.97
G279	05/03/2021	609.38
G279	05/17/2021	609.22
G279	06/09/2021	599.69
G279	06/23/2021	607.74
G279	07/12/2021	608.18
G279	07/26/2021	608.57
G279	08/16/2021	608.95
G280	10/08/2015	614.54
G280	11/16/2015	618.45
G280	02/08/2016	621.37
G280	05/09/2016	621.94
G280	07/25/2016	618.21
G280	11/12/2016	618.46

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G280	02/04/2017	619.10
G280	05/13/2017	618.56
G280	07/08/2017	619.45
G280	07/18/2017	619.45
G280	10/21/2017	614.47
G280	05/08/2018	618.00
G280	08/02/2018	616.70
G280	10/23/2018	615.75
G280	01/15/2019	616.24
G280	08/05/2019	616.09
G280	01/20/2020	622.33
G280	08/10/2020	619.50
G280	08/11/2020	619.50
G280	10/14/2020	617.45
G280	01/20/2021	618.20
G280	01/28/2021	618.70
G280	03/29/2021	620.61
G280	03/30/2021	621.22
G280	04/20/2021	619.76
G280	04/22/2021	620.13
G280	05/03/2021	620.21
G280	05/06/2021	620.89
G280	05/17/2021	619.98
G280	05/19/2021	620.72
G280	06/09/2021	619.75
G280	06/23/2021	618.93
G280	06/28/2021	619.02
G280	07/12/2021	619.26
G280	07/13/2021	619.50
G280	07/26/2021	619.75
G280	07/27/2021	619.66
G280	08/16/2021	620.00
G281	11/16/2015	619.56
G281	02/08/2016	621.21
G281	05/09/2016	620.93
G281	07/25/2016	620.30
G281	11/12/2016	620.01
G281	02/04/2017	620.37
G281	05/13/2017	619.96
G281	07/08/2017	619.17
G281	10/21/2017	616.41
G281	05/08/2018	619.33
G281	08/02/2018	618.36
G281	10/23/2018	617.26
G281	01/15/2019	618.19
G281	08/05/2019	620.16

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G281	01/20/2020	621.26
G281	08/10/2020	619.26
G281	01/20/2021	619.36
G281	01/29/2021	619.36
G281	03/29/2021	621.68
G281	03/31/2021	621.29
G281	04/20/2021	619.62
G281	04/21/2021	619.77
G281	05/03/2021	620.60
G281	05/05/2021	620.85
G281	05/17/2021	620.13
G281	06/09/2021	619.65
G281	06/14/2021	619.46
G281	06/23/2021	618.71
G281	06/28/2021	619.77
G281	07/12/2021	620.23
G281	07/26/2021	620.02
G281	07/27/2021	619.92
G281	08/16/2021	619.81
G283	03/29/2021	607.80
G283	03/31/2021	607.34
G283	04/20/2021	606.34
G283	04/22/2021	606.09
G283	05/03/2021	606.81
G283	05/06/2021	606.79
G283	05/17/2021	606.30
G283	05/18/2021	606.54
G283	06/09/2021	605.13
G283	06/15/2021	604.95
G283	06/23/2021	604.56
G283	06/29/2021	605.29
G283	07/12/2021	605.50
G283	07/13/2021	605.82
G283	07/26/2021	605.18
G283	07/27/2021	605.08
G283	08/16/2021	605.12
G284	03/29/2021	611.14
G284	03/30/2021	610.95
G284	04/20/2021	608.16
G284	04/21/2021	607.65
G284	05/03/2021	609.33
G284	05/06/2021	610.72
G284	05/17/2021	608.16
G284	05/18/2021	609.49
G284	06/09/2021	607.07
G284	06/14/2021	606.95

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G284	06/23/2021	606.17
G284	06/28/2021	608.02
G284	07/12/2021	607.68
G284	07/13/2021	607.69
G284	07/26/2021	607.11
G284	07/27/2021	606.95
G284	08/16/2021	606.98
G285	03/29/2021	608.62
G285	03/30/2021	608.81
G285	04/20/2021	608.13
G285	04/22/2021	603.79
G285	05/03/2021	606.99
G285	05/06/2021	607.57
G285	05/17/2021	607.47
G285	05/18/2021	607.51
G285	06/09/2021	607.39
G285	06/15/2021	607.08
G285	06/23/2021	604.33
G285	06/28/2021	604.93
G285	07/12/2021	604.80
G285	07/13/2021	604.92
G285	07/26/2021	605.27
G285	07/27/2021	605.37
G285	08/16/2021	606.28
G286	03/29/2021	609.08
G286	03/31/2021	608.22
G286	04/20/2021	606.63
G286	04/22/2021	606.15
G286	05/03/2021	606.97
G286	05/06/2021	608.56
G286	05/17/2021	606.44
G286	05/18/2021	606.57
G286	06/09/2021	604.68
G286	06/15/2021	602.98
G286	07/12/2021	605.90
G286	07/13/2021	606.00
G287	03/29/2021	610.22
G287	04/20/2021	608.67
G287	04/22/2021	608.03
G287	05/03/2021	609.28
G287	05/06/2021	610.29
G287	05/17/2021	608.41
G287	05/18/2021	609.32
G287	06/09/2021	607.59
G287	06/14/2021	617.45
G287	07/12/2021	610.83

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G287	07/13/2021	607.78
G287	08/16/2021	607.76
G288	03/29/2021	616.32
G288	03/30/2021	615.89
G288	04/20/2021	613.90
G288	04/21/2021	613.56
G288	05/03/2021	614.51
G288	05/06/2021	616.00
G288	05/17/2021	613.87
G288	05/18/2021	616.15
G288	06/09/2021	612.90
G288	06/15/2021	612.47
G288	06/23/2021	611.90
G288	06/28/2021	612.91
G288	07/12/2021	613.59
G288	07/13/2021	615.11
G288	07/26/2021	612.85
G288	07/27/2021	612.75
G288	08/16/2021	612.98
G301	11/16/2015	616.51
G301	02/08/2016	617.21
G301	05/09/2016	616.75
G301	07/25/2016	614.65
G301	11/12/2016	614.08
G301	02/04/2017	614.15
G301	05/13/2017	614.15
G301	07/08/2017	614.88
G301	10/21/2017	610.39
G301	05/08/2018	613.73
G301	08/02/2018	615.05
G301	10/23/2018	612.46
G301	01/15/2019	613.23
G301	08/05/2019	613.82
G301	01/20/2020	618.07
G301	08/10/2020	615.16
G301	01/20/2021	616.03
G301	01/27/2021	616.03
G301	04/20/2021	616.05
G301	05/03/2021	616.12
G301	05/17/2021	615.99
G301	06/09/2021	615.63
G301	06/23/2021	615.02
G301	07/12/2021	615.79
G301	07/26/2021	615.31
G301	08/16/2021	615.45
G302	11/16/2015	610.74

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G302	02/08/2016	613.14
G302	05/09/2016	614.60
G302	07/25/2016	608.16
G302	11/12/2016	607.92
G302	02/04/2017	608.95
G302	05/13/2017	608.02
G302	07/08/2017	608.42
G302	10/21/2017	604.64
G302	05/08/2018	607.59
G302	08/02/2018	608.26
G302	10/23/2018	605.54
G302	01/15/2019	607.29
G302	08/05/2019	609.95
G302	01/20/2020	615.41
G302	08/10/2020	608.05
G302	01/20/2021	609.99
G302	01/27/2021	609.99
G302	04/20/2021	611.85
G302	05/03/2021	612.07
G302	05/17/2021	612.06
G302	06/09/2021	610.29
G302	06/23/2021	608.79
G302	07/12/2021	611.79
G302	07/26/2021	610.98
G302	08/16/2021	611.77
G303	11/16/2015	616.70
G303	02/08/2016	617.87
G303	05/09/2016	617.97
G303	07/25/2016	614.92
G303	11/12/2016	614.38
G303	02/04/2017	614.95
G303	05/13/2017	614.81
G303	07/08/2017	614.97
G303	10/21/2017	611.18
G303	05/08/2018	614.16
G303	08/02/2018	614.06
G303	10/23/2018	613.05
G303	01/15/2019	614.33
G303	08/05/2019	617.37
G303	01/20/2020	618.05
G303	08/10/2020	615.16
G303	01/20/2021	616.17
G303	01/26/2021	616.17
G303	04/20/2021	617.27
G303	05/03/2021	618.02
G303	05/17/2021	617.37

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G303	06/09/2021	616.52
G303	06/23/2021	614.92
G303	07/12/2021	617.15
G303	07/26/2021	616.44
G303	08/16/2021	616.58
G304	11/16/2015	623.78
G304	02/08/2016	624.07
G304	05/09/2016	623.91
G304	07/25/2016	626.72
G305	05/09/2016	618.48
G305	07/01/2016	616.28
G305	07/25/2016	618.24
G305	09/29/2016	617.33
G305	11/12/2016	618.06
G305	02/04/2017	620.49
G305	05/13/2017	618.27
G305	07/08/2017	618.28
G305	10/21/2017	615.30
G305	05/08/2018	617.87
G305	08/02/2018	617.79
G305	10/23/2018	616.56
G305	01/15/2019	616.95
G305	08/05/2019	616.85
G305	01/20/2020	619.36
G305	08/10/2020	617.02
G305	01/20/2021	618.63
G305	04/20/2021	618.77
G305	05/03/2021	619.11
G305	05/17/2021	618.90
G305	06/09/2021	618.04
G305	06/23/2021	616.94
G305	07/12/2021	618.55
G305	07/26/2021	618.18
G305	08/16/2021	618.31
G306	05/09/2016	619.74
G306	07/01/2016	615.11
G306	07/25/2016	619.26
G306	09/29/2016	617.64
G306	11/12/2016	618.77
G306	02/04/2017	618.97
G306	05/13/2017	619.03
G306	07/08/2017	619.45
G306	10/21/2017	616.12
G306	05/08/2018	618.96
G306	08/02/2018	621.73
G306	10/23/2018	617.24

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G306	01/15/2019	618.56
G306	08/05/2019	619.18
G306	01/20/2020	620.28
G306	08/10/2020	617.26
G306	01/20/2021	619.17
G306	01/26/2021	618.98
G306	03/29/2021	620.42
G306	04/20/2021	619.30
G306	04/21/2021	619.53
G306	05/03/2021	619.96
G306	05/05/2021	620.27
G306	05/17/2021	619.44
G306	05/18/2021	619.56
G306	06/09/2021	618.04
G306	06/15/2021	617.29
G306	06/23/2021	616.32
G306	06/28/2021	618.31
G306	07/12/2021	620.59
G306	07/14/2021	620.17
G306	07/26/2021	618.84
G306	07/27/2021	618.70
G306	08/16/2021	618.92
G307	07/25/2016	624.30
G307	09/29/2016	623.85
G307	11/12/2016	624.44
G307	02/04/2017	624.60
G307	05/13/2017	624.56
G307	07/08/2017	623.55
G307	10/21/2017	624.60
G307	05/08/2018	624.37
G307	08/02/2018	619.33
G307	10/23/2018	623.95
G307	01/15/2019	624.31
G307	08/05/2019	624.21
G307	05/06/2020	624.72
G307	08/10/2020	624.36
G307	01/20/2021	624.10
G307	01/27/2021	624.10
G307	04/20/2021	624.50
G307	05/17/2021	624.45
G307	07/12/2021	624.45
G307	08/16/2021	624.46
G307D	03/29/2021	622.43
G307D	04/20/2021	622.48
G307D	04/21/2021	622.46
G307D	05/03/2021	622.47

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G307D	05/04/2021	622.44
G307D	05/17/2021	622.44
G307D	05/18/2021	622.46
G307D	06/09/2021	622.43
G307D	06/15/2021	622.42
G307D	06/23/2021	622.42
G307D	07/12/2021	622.59
G307D	07/26/2021	622.26
G307D	07/27/2021	622.51
G307D	08/16/2021	621.49
G308	03/29/2021	621.03
G308	04/20/2021	619.67
G308	04/21/2021	620.15
G308	05/03/2021	620.04
G308	05/05/2021	621.01
G308	05/17/2021	619.93
G308	06/09/2021	619.17
G308	06/14/2021	619.06
G308	06/23/2021	618.54
G308	06/28/2021	620.44
G308	07/12/2021	620.22
G308	07/14/2021	620.67
G308	07/26/2021	619.68
G308	07/27/2021	619.44
G308	08/16/2021	619.45
G309	03/29/2021	621.09
G309	04/20/2021	618.88
G309	04/21/2021	618.88
G309	04/22/2021	618.88
G309	05/03/2021	619.04
G309	05/05/2021	619.84
G309	05/17/2021	618.83
G309	06/09/2021	618.43
G309	06/14/2021	618.25
G309	06/23/2021	617.89
G309	06/28/2021	618.95
G309	07/12/2021	619.31
G309	07/13/2021	620.17
G309	07/26/2021	618.88
G309	07/27/2021	618.78
G309	08/16/2021	618.91
G310	03/29/2021	617.27
G310	04/20/2021	614.41
G310	04/22/2021	614.40
G310	05/03/2021	614.61
G310	05/04/2021	615.01

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G310	05/17/2021	614.47
G310	05/19/2021	616.01
G310	06/09/2021	613.83
G310	06/15/2021	613.54
G310	06/23/2021	613.20
G310	06/28/2021	614.15
G310	07/12/2021	614.81
G310	07/13/2021	615.88
G310	07/26/2021	614.13
G310	07/28/2021	614.00
G310	08/16/2021	614.29
G311	03/29/2021	616.54
G311	03/30/2021	616.21
G311	04/20/2021	613.75
G311	04/22/2021	613.68
G311	05/03/2021	614.01
G311	05/04/2021	615.13
G311	05/17/2021	613.86
G311	05/19/2021	615.78
G311	06/09/2021	613.13
G311	06/15/2021	612.78
G311	06/23/2021	612.45
G311	06/29/2021	613.31
G311	07/12/2021	613.75
G311	07/14/2021	615.37
G311	07/26/2021	613.05
G311	07/27/2021	612.94
G311	08/16/2021	613.30
G311D	03/29/2021	575.42
G311D	03/30/2021	575.73
G311D	04/20/2021	575.29
G311D	04/22/2021	575.74
G311D	05/03/2021	573.09
G311D	05/04/2021	573.23
G311D	05/17/2021	572.40
G311D	05/19/2021	572.91
G311D	06/09/2021	573.85
G311D	06/15/2021	575.25
G311D	06/23/2021	571.74
G311D	07/12/2021	571.63
G311D	07/26/2021	569.74
G311D	07/28/2021	569.98
G311D	08/16/2021	570.34
G312	03/29/2021	612.19
G312	03/30/2021	611.97
G312	04/20/2021	609.11

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G312	04/22/2021	608.97
G312	05/03/2021	609.47
G312	05/04/2021	610.07
G312	05/17/2021	609.27
G312	05/19/2021	610.89
G312	06/09/2021	608.31
G312	06/15/2021	607.64
G312	06/23/2021	606.99
G312	06/29/2021	608.07
G312	07/12/2021	608.70
G312	07/13/2021	610.23
G312	07/26/2021	608.56
G312	07/27/2021	608.47
G312	08/16/2021	609.09
G313	03/29/2021	611.78
G313	03/30/2021	611.75
G313	04/20/2021	611.46
G313	04/22/2021	611.41
G313	05/03/2021	611.68
G313	05/04/2021	611.66
G313	05/17/2021	611.62
G313	05/18/2021	611.66
G313	06/09/2021	611.57
G313	06/14/2021	611.55
G313	06/23/2021	611.29
G313	06/28/2021	611.58
G313	07/12/2021	611.70
G313	07/13/2021	611.81
G313	07/26/2021	611.71
G313	07/27/2021	611.73
G313	08/16/2021	611.90
G314	03/29/2021	596.40
G314	03/30/2021	597.11
G314	04/20/2021	603.16
G314	04/21/2021	603.48
G314	05/03/2021	604.66
G314	05/04/2021	604.64
G314	05/17/2021	605.61
G314	06/09/2021	607.54
G314	06/14/2021	608.16
G314	06/23/2021	605.19
G314	06/28/2021	606.45
G314	07/12/2021	605.32
G314	07/13/2021	605.60
G314	07/26/2021	606.66
G314	07/27/2021	606.84

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G314	08/16/2021	608.60
G314D	03/29/2021	572.75
G314D	03/30/2021	573.05
G314D	04/20/2021	571.76
G314D	04/21/2021	571.95
G314D	05/03/2021	568.77
G314D	05/04/2021	568.95
G314D	05/17/2021	566.84
G314D	05/19/2021	566.84
G314D	06/09/2021	567.45
G314D	06/14/2021	568.60
G314D	06/23/2021	566.77
G314D	07/12/2021	566.88
G314D	07/26/2021	566.65
G314D	07/28/2021	566.75
G314D	08/16/2021	567.28
G315	03/29/2021	621.24
G315	03/30/2021	621.20
G315	04/20/2021	621.05
G315	04/22/2021	621.12
G315	05/03/2021	621.13
G315	05/05/2021	621.25
G315	05/17/2021	621.14
G315	05/18/2021	621.34
G315	06/09/2021	620.24
G315	06/15/2021	619.70
G315	06/23/2021	619.17
G315	06/29/2021	621.04
G315	07/12/2021	620.91
G315	07/14/2021	621.13
G315	07/26/2021	620.42
G315	07/28/2021	620.44
G315	08/16/2021	620.29
G316	03/29/2021	591.63
G316	03/30/2021	591.55
G316	04/20/2021	591.23
G316	04/22/2021	591.31
G316	05/03/2021	591.39
G316	05/05/2021	591.63
G316	05/17/2021	591.28
G316	06/09/2021	581.54
G316	06/14/2021	590.61
G316	06/23/2021	590.06
G316	06/28/2021	591.40
G316	07/12/2021	591.16
G316	07/13/2021	591.50

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G316	07/26/2021	590.73
G316	07/27/2021	590.68
G316	08/16/2021	590.59
G317	03/29/2021	610.40
G317	03/30/2021	610.89
G317	04/20/2021	610.94
G317	04/22/2021	610.84
G317	05/03/2021	611.75
G317	05/05/2021	611.15
G317	05/17/2021	611.65
G317	05/18/2021	611.57
G317	06/09/2021	610.59
G317	06/15/2021	609.63
G317	06/23/2021	606.57
G317	06/28/2021	608.25
G317	07/12/2021	607.93
G317	07/13/2021	607.92
G317	07/26/2021	608.27
G317	07/28/2021	608.11
G317	08/16/2021	608.46
G401	11/16/2015	607.82
G401	02/08/2016	608.14
G401	05/09/2016	608.00
G401	07/25/2016	608.47
G401	11/12/2016	607.84
G401	02/04/2017	609.74
G401	05/13/2017	608.52
G401	07/08/2017	609.19
G401	05/08/2018	609.37
G401	08/02/2018	609.80
G401	10/23/2018	608.42
G401	01/15/2019	608.36
G401	08/05/2019	608.45
G401	01/20/2020	607.25
G401	05/06/2020	607.02
G401	08/10/2020	606.77
G401	01/29/2021	604.22
G401	04/20/2021	604.14
G401	07/26/2021	603.94
G401	08/16/2021	604.04
G402	11/16/2015	604.02
G402	02/08/2016	604.90
G402	05/09/2016	605.18
G402	07/25/2016	604.33
G402	11/12/2016	604.24
G402	02/04/2017	604.43

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G402	05/13/2017	604.83
G402	07/08/2017	604.42
G402	10/21/2017	600.77
G402	05/08/2018	605.36
G402	08/02/2018	603.82
G402	10/23/2018	602.25
G402	01/15/2019	602.37
G402	08/05/2019	603.82
G402	01/20/2020	605.12
G402	08/10/2020	602.09
G402	01/20/2021	603.01
G402	01/28/2021	603.01
G402	04/20/2021	603.78
G402	07/26/2021	602.83
G402	08/16/2021	603.29
G403	11/16/2015	621.81
G403	02/08/2016	621.78
G403	05/09/2016	621.76
G403	07/25/2016	622.16
G403	11/12/2016	621.80
G403	02/04/2017	622.45
G403	05/13/2017	622.26
G403	07/08/2017	622.16
G403	10/21/2017	618.36
G403	05/08/2018	621.66
G403	08/02/2018	622.38
G403	10/23/2018	619.48
G403	01/15/2019	620.51
G403	08/05/2019	621.64
G403	01/20/2020	621.63
G403	08/10/2020	621.14
G403	01/20/2021	619.88
G403	01/21/2021	619.88
G403	04/20/2021	619.41
G403	07/26/2021	619.56
G403	08/16/2021	619.27
G404	11/16/2015	611.67
G404	02/08/2016	611.58
G404	05/09/2016	611.46
G404	07/25/2016	611.67
G404	11/12/2016	610.58
G404	02/04/2017	610.57
G404	05/13/2017	610.87
G404	07/08/2017	611.75
G404	10/21/2017	607.58
G404	05/08/2018	611.42

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G404	08/02/2018	610.55
G404	10/23/2018	608.59
G404	01/15/2019	608.98
G404	08/05/2019	611.60
G404	01/20/2020	612.14
G404	08/10/2020	610.37
G404	01/20/2021	611.63
G404	01/21/2021	611.63
G404	04/20/2021	611.51
G404	07/26/2021	611.29
G404	08/16/2021	610.95
G405	11/16/2015	618.85
G405	02/08/2016	618.90
G405	05/09/2016	618.99
G405	07/25/2016	618.51
G405	11/12/2016	618.48
G405	02/04/2017	618.47
G405	05/13/2017	618.74
G405	07/08/2017	618.54
G405	10/21/2017	614.47
G405	05/08/2018	618.94
G405	08/02/2018	617.55
G405	10/23/2018	616.40
G405	01/15/2019	616.81
G405	08/05/2019	617.72
G405	01/20/2020	619.28
G405	08/10/2020	617.62
G405	01/20/2021	617.12
G405	01/21/2021	617.12
G405	04/20/2021	617.13
G405	07/26/2021	617.37
G405	08/16/2021	617.28
G406	11/12/2016	616.01
G406	02/04/2017	617.52
G406	05/13/2017	616.20
G406	07/08/2017	616.29
G406	10/21/2017	611.27
G406	05/08/2018	615.47
G406	08/02/2018	615.75
G406	10/23/2018	614.11
G406	01/15/2019	615.36
G406	08/05/2019	616.50
G406	01/20/2020	617.48
G406	08/10/2020	615.54
G406	01/20/2021	612.97
G406	04/20/2021	613.78

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
G406	07/26/2021	614.20
G406	08/16/2021	613.82
G407	11/12/2016	613.27
G407	02/04/2017	613.41
G407	05/13/2017	613.68
G407	07/08/2017	613.59
G407	10/21/2017	612.67
G407	05/08/2018	613.11
G407	08/02/2018	612.95
G407	10/23/2018	612.11
G407	01/15/2019	612.31
G407	08/05/2019	614.02
G407	01/20/2020	614.86
G407	08/10/2020	613.74
G407	01/20/2021	614.70
G407	04/20/2021	614.49
G407	07/26/2021	614.38
G407	08/16/2021	614.41
G410	10/23/2018	610.41
G410	01/15/2019	610.91
G410	08/05/2019	611.75
G410	01/20/2020	612.70
G410	08/10/2020	610.88
G410	01/20/2021	610.91
G410	04/20/2021	611.38
G410	07/26/2021	611.51
G410	08/16/2021	611.29
G411	10/23/2018	613.20
G411	01/15/2019	613.82
G411	08/05/2019	614.25
G411	01/20/2020	617.53
G411	08/10/2020	615.51
G411	01/20/2021	615.91
G411	04/20/2021	616.12
G411	07/26/2021	616.20
G411	08/16/2021	616.03
MW03D	04/20/2021	597.90
MW03D	05/03/2021	598.18
MW03D	05/17/2021	598.06
MW03D	06/09/2021	598.13
MW03D	06/23/2021	598.09
MW03D	07/12/2021	598.12
MW03D	07/26/2021	598.09
MW03D	08/16/2021	598.10
MW04S	02/08/2016	621.62
MW04S	05/09/2016	620.45

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW04S	07/25/2016	618.84
MW04S	11/12/2016	618.66
MW04S	02/04/2017	618.97
MW04S	05/13/2017	618.76
MW04S	07/08/2017	618.92
MW04S	10/21/2017	613.88
MW04S	05/08/2018	617.95
MW04S	08/02/2018	618.73
MW04S	10/23/2018	614.68
MW04S	01/15/2019	614.89
MW04S	08/05/2019	614.92
MW04S	01/20/2020	619.93
MW04S	08/10/2020	617.74
MW04S	01/20/2021	620.63
MW04S	04/20/2021	619.39
MW04S	07/26/2021	618.55
MW04S	08/16/2021	618.50
MW05S	02/08/2016	620.92
MW05S	05/09/2016	620.53
MW05S	07/25/2016	618.20
MW05S	11/12/2016	617.38
MW05S	02/04/2017	618.78
MW05S	05/13/2017	617.95
MW05S	07/08/2017	618.81
MW05S	10/21/2017	613.32
MW05S	05/08/2018	617.77
MW05S	08/02/2018	617.98
MW05S	10/23/2018	615.35
MW05S	01/15/2019	615.93
MW05S	08/05/2019	616.01
MW05S	01/20/2020	620.34
MW05S	08/10/2020	617.09
MW05S	01/20/2021	618.33
MW05S	04/20/2021	619.07
MW05S	07/26/2021	618.14
MW05S	08/16/2021	617.84
MW10S	02/08/2016	620.43
MW10S	05/09/2016	619.47
MW10S	07/25/2016	617.69
MW10S	11/12/2016	616.69
MW10S	02/04/2017	617.41
MW10S	05/13/2017	617.22
MW10S	07/08/2017	617.27
MW10S	10/21/2017	614.52
MW10S	05/08/2018	616.89
MW10S	08/02/2018	617.52

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW10S	10/23/2018	614.36
MW10S	01/15/2019	614.85
MW10S	08/05/2019	615.56
MW10S	08/10/2020	617.11
MW10S	01/20/2021	619.48
MW10S	04/20/2021	619.03
MW10S	07/26/2021	617.74
MW10S	08/16/2021	617.35
MW11S	02/08/2016	621.30
MW11S	05/09/2016	622.19
MW11S	07/25/2016	620.99
MW11S	11/12/2016	620.92
MW11S	02/04/2017	620.82
MW11S	05/13/2017	621.31
MW11S	07/08/2017	620.85
MW11S	10/21/2017	617.19
MW11S	05/08/2018	620.85
MW11S	08/02/2018	620.69
MW11S	10/23/2018	620.05
MW11S	01/15/2019	620.38
MW11S	08/05/2019	620.76
MW11S	01/20/2020	621.80
MW11S	08/10/2020	618.12
MW11S	01/20/2021	619.64
MW11S	04/20/2021	621.76
MW11S	05/03/2021	622.01
MW11S	05/17/2021	621.94
MW11S	06/09/2021	621.45
MW11S	06/23/2021	618.83
MW11S	07/12/2021	620.54
MW11S	07/26/2021	620.97
MW11S	08/16/2021	621.49
MW11D	04/20/2021	621.13
MW11D	05/03/2021	621.36
MW11D	05/17/2021	621.27
MW11D	06/09/2021	620.96
MW11D	06/23/2021	618.72
MW11D	07/12/2021	619.88
MW11D	07/26/2021	620.57
MW11D	08/16/2021	621.01
MW12S	02/08/2016	620.37
MW12S	05/09/2016	620.48
MW12S	07/25/2016	618.53
MW12S	11/12/2016	617.97
MW12S	02/04/2017	620.33
MW12S	05/13/2017	618.26

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW12S	07/08/2017	618.61
MW12S	10/21/2017	615.08
MW12S	05/08/2018	617.58
MW12S	08/02/2018	617.12
MW12S	10/23/2018	616.14
MW12S	01/15/2019	616.89
MW12S	08/05/2019	617.35
MW12S	01/20/2020	620.34
MW12S	08/10/2020	615.69
MW12S	01/20/2021	611.42
MW12S	04/20/2021	618.96
MW12S	05/03/2021	619.66
MW12S	05/17/2021	619.23
MW12S	06/09/2021	618.20
MW12S	06/23/2021	616.52
MW12S	07/12/2021	619.35
MW12S	07/26/2021	618.43
MW12S	08/16/2021	618.79
MW12D	04/20/2021	611.97
MW12D	05/03/2021	611.87
MW12D	05/17/2021	611.95
MW12D	06/09/2021	611.87
MW12D	06/23/2021	611.79
MW12D	07/12/2021	611.55
MW12D	07/26/2021	611.50
MW12D	08/16/2021	611.51
MW16S	02/08/2016	625.29
MW16S	05/09/2016	624.54
MW16S	07/25/2016	622.13
MW16S	11/12/2016	622.26
MW16S	02/04/2017	622.53
MW16S	05/13/2017	622.53
MW16S	07/08/2017	622.25
MW16S	10/21/2017	618.42
MW16S	05/08/2018	622.02
MW16S	08/02/2018	622.47
MW16S	10/23/2018	620.88
MW16S	01/15/2019	621.60
MW16S	08/05/2019	621.94
MW16S	01/20/2020	625.59
MW16S	08/10/2020	618.52
MW16S	01/20/2021	618.34
MW16S	04/20/2021	623.78
MW16S	05/03/2021	624.58
MW16S	05/17/2021	623.87
MW16S	06/09/2021	622.57

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
MW16S	06/23/2021	620.48
MW16S	07/12/2021	620.58
MW16S	07/26/2021	620.68
MW16S	08/16/2021	620.65
MW16D	04/20/2021	615.42
MW16D	05/03/2021	615.92
MW16D	05/17/2021	616.36
MW16D	06/09/2021	616.87
MW16D	06/23/2021	616.90
MW16D	07/12/2021	616.76
MW16D	07/26/2021	616.63
MW16D	08/16/2021	616.35
MW20S	02/08/2016	614.36
MW20S	05/09/2016	614.09
MW20S	07/25/2016	611.61
MW20S	11/12/2016	611.51
MW20S	02/04/2017	612.76
MW20S	05/13/2017	611.86
MW20S	07/08/2017	611.75
MW20S	10/21/2017	607.74
MW20S	05/08/2018	611.46
MW20S	08/02/2018	611.51
MW20S	10/23/2018	609.55
MW20S	01/15/2019	610.21
MW20S	08/05/2019	610.81
MW20S	01/20/2020	615.40
MW20S	08/10/2020	612.37
MW20S	01/20/2021	612.27
MW20S	04/20/2021	613.45
MW20S	07/26/2021	613.35
MW20S	08/16/2021	612.31
R104	01/20/2015	623.03
R104	04/08/2015	624.77
R104	10/06/2015	621.69
R104	11/16/2015	621.34
R104	02/08/2016	624.11
R104	05/09/2016	624.89
R104	07/25/2016	623.65
R104	11/12/2016	623.49
R104	02/04/2017	624.20
R104	05/13/2017	622.91
R104	07/08/2017	624.09
R104	10/21/2017	619.38
R104	05/08/2018	622.66
R104	08/02/2018	621.73
R104	10/23/2018	621.58

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
R104	01/15/2019	622.43
R104	08/05/2019	623.34
R104	01/20/2020	625.63
R104	08/10/2020	624.56
R104	10/15/2020	621.10
R104	01/20/2021	623.31
R104	01/28/2021	620.57
R104	04/20/2021	624.95
R104	07/26/2021	625.41
R104	08/16/2021	625.92
R201	10/05/2015	619.94
R201	11/16/2015	622.44
R201	02/08/2016	623.40
R201	05/09/2016	622.81
R201	07/25/2016	622.36
R201	11/12/2016	622.82
R201	02/04/2017	622.27
R201	05/13/2017	623.05
R201	07/08/2017	622.31
R201	10/21/2017	618.30
R201	01/25/2018	622.00
R201	05/08/2018	622.78
R201	08/02/2018	622.16
R201	10/23/2018	621.29
R201	01/15/2019	622.17
R201	08/05/2019	622.35
R201	01/20/2020	622.88
R201	08/10/2020	618.89
R201	10/13/2020	616.57
R201	01/20/2021	620.52
R201	01/29/2021	620.52
R201	03/29/2021	623.52
R201	04/20/2021	622.16
R201	04/21/2021	622.59
R201	05/03/2021	622.91
R201	05/06/2021	623.40
R201	05/17/2021	622.68
R201	06/09/2021	621.12
R201	06/14/2021	620.63
R201	06/23/2021	619.92
R201	06/29/2021	621.16
R201	07/12/2021	621.34
R201	07/13/2021	621.36
R201	07/26/2021	620.37
R201	07/28/2021	620.16
R201	08/16/2021	620.61

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
R205	05/08/2018	618.32
R205	08/02/2018	614.95
R205	10/23/2018	618.85
R205	01/15/2019	619.54
R205	08/05/2019	620.13
R205	01/20/2020	620.21
R205	08/10/2020	615.92
R205	10/14/2020	613.87
R205	01/20/2021	617.80
R205	01/28/2021	618.45
R205	04/20/2021	619.12
R205	07/26/2021	618.66
R205	08/16/2021	618.99
T127	01/19/2015	615.65
T127	04/08/2015	616.04
T127	07/23/2015	616.04
T127	10/06/2015	615.66
T127	11/16/2015	615.91
T127	02/08/2016	616.04
T127	05/09/2016	616.15
T127	07/25/2016	615.96
T127	11/12/2016	616.73
T127	02/04/2017	616.32
T127	05/13/2017	616.89
T127	07/08/2017	615.99
T127	10/21/2017	612.33
T127	01/27/2018	611.06
T127	05/08/2018	616.66
T127	08/02/2018	616.48
T127	10/23/2018	614.78
T127	01/15/2019	615.13
T127	05/03/2019	617.26
T127	08/05/2019	615.15
T127	01/20/2020	617.05
T127	05/05/2020	617.02
T127	08/10/2020	615.90
T127	10/14/2020	615.08
T127	01/20/2021	615.89
T127	01/29/2021	615.89
T127	04/20/2021	616.54
T127	06/29/2021	616.72
T127	07/26/2021	616.53
T127	08/16/2021	616.65
T128	01/19/2015	614.73
T128	04/08/2015	614.89
T128	07/23/2015	615.40

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
T128	10/06/2015	614.67
T128	02/08/2016	614.90
T128	05/09/2016	615.01
T128	07/25/2016	614.75
T128	11/12/2016	614.95
T128	02/04/2017	616.00
T128	05/13/2017	615.39
T128	07/08/2017	615.07
T128	10/21/2017	611.33
T128	05/08/2018	615.13
T128	08/02/2018	614.87
T128	10/23/2018	613.17
T128	01/15/2019	613.94
T128	08/05/2019	613.99
T128	01/20/2020	617.25
T128	08/10/2020	616.15
T128	10/14/2020	615.36
T128	01/20/2021	616.20
T128	01/28/2021	616.33
T128	04/20/2021	616.94
T128	07/26/2021	616.81
T128	08/16/2021	616.93
T202	02/08/2016	622.82
T202	05/09/2016	623.66
T202	07/25/2016	619.49
T202	11/12/2016	619.88
T202	02/04/2017	619.73
T202	05/13/2017	620.07
T202	07/08/2017	619.75
T202	10/21/2017	615.31
T202	05/08/2018	619.52
T202	08/02/2018	620.53
T202	10/23/2018	618.36
T202	01/15/2019	618.69
T202	08/05/2019	619.02
T202	01/20/2020	624.22
T202	08/10/2020	620.39
T202	01/20/2021	620.08
T202	04/20/2021	623.43
T202	07/26/2021	622.64
T202	08/16/2021	622.69
T408	11/12/2016	618.58
T408	02/04/2017	619.46
T408	05/13/2017	619.00
T408	07/08/2017	619.12
T408	10/21/2017	614.81

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
T408	05/08/2018	615.82
T408	08/02/2018	614.45
T408	10/23/2018	616.30
T408	01/15/2019	617.01
T408	08/05/2019	617.15
T408	01/20/2020	619.13
T408	08/10/2020	617.38
T408	01/20/2021	616.85
T408	04/20/2021	616.65
T408	07/26/2021	617.21
T408	08/16/2021	617.22
T409	11/12/2016	615.98
T409	02/04/2017	615.93
T409	05/13/2017	616.75
T409	07/08/2017	617.05
T409	10/21/2017	612.16
T409	05/08/2018	616.02
T409	08/02/2018	615.25
T409	10/23/2018	613.96
T409	01/15/2019	614.78
T409	08/05/2019	615.10
T409	01/20/2020	617.16
T409	08/10/2020	615.43
T409	01/20/2021	614.41
T409	04/20/2021	615.33
T409	07/26/2021	615.72
T409	08/16/2021	615.42
TA31	02/08/2016	621.56
TA31	05/09/2016	621.32
TA31	07/25/2016	620.63
TA31	11/12/2016	620.50
TA31	02/04/2017	621.55
TA31	05/13/2017	620.66
TA31	07/08/2017	620.94
TA31	10/21/2017	616.90
TA31	05/08/2018	619.80
TA31	08/02/2018	620.41
TA31	10/23/2018	618.32
TA31	01/15/2019	619.21
TA31	08/05/2019	619.37
TA31	01/20/2020	622.93
TA31	08/10/2020	614.89
TA31	01/20/2021	615.79
TA31	04/20/2021	622.14
TA31	07/26/2021	618.76
TA31	08/16/2021	619.17

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
TA32	02/08/2016	615.46
TA32	05/09/2016	616.02
TA32	07/25/2016	615.61
TA32	11/12/2016	615.53
TA32	02/04/2017	614.73
TA32	05/13/2017	615.82
TA32	07/08/2017	615.79
TA32	10/21/2017	612.42
TA32	08/05/2019	615.41
TA32	01/20/2020	616.30
TA33	02/08/2016	619.67
TA33	05/09/2016	619.75
TA33	07/25/2016	616.91
TA33	11/12/2016	616.81
TA33	02/04/2017	617.12
TA33	05/13/2017	617.22
TA33	07/08/2017	617.42
TA33	10/21/2017	612.91
TA33	05/08/2018	618.07
TA33	08/02/2018	616.68
TA33	10/23/2018	617.26
TA33	01/15/2019	617.66
TA33	08/05/2019	618.27
TA33	01/20/2020	620.35
TA33	08/10/2020	614.10
TA33	01/20/2021	614.34
TA33	04/20/2021	619.07
TA33	07/26/2021	616.82
TA33	08/16/2021	616.86
TA34	02/08/2016	619.29
TA34	05/09/2016	619.35
TA34	07/25/2016	617.37
TA34	11/12/2016	617.40
TA34	02/04/2017	617.45
TA34	05/13/2017	617.43
TA34	07/08/2017	617.44
TA34	10/21/2017	613.48
TA34	05/08/2018	617.06
TA34	08/02/2018	616.42
TA34	10/23/2018	614.92
TA34	01/15/2019	615.85
TA34	08/05/2019	616.54
TA34	01/20/2020	619.58
TA34	08/10/2020	615.68
TA34	01/20/2021	616.16
TA34	04/20/2021	618.74

TABLE E-1. GROUNDWATER ELEVATIONS
HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

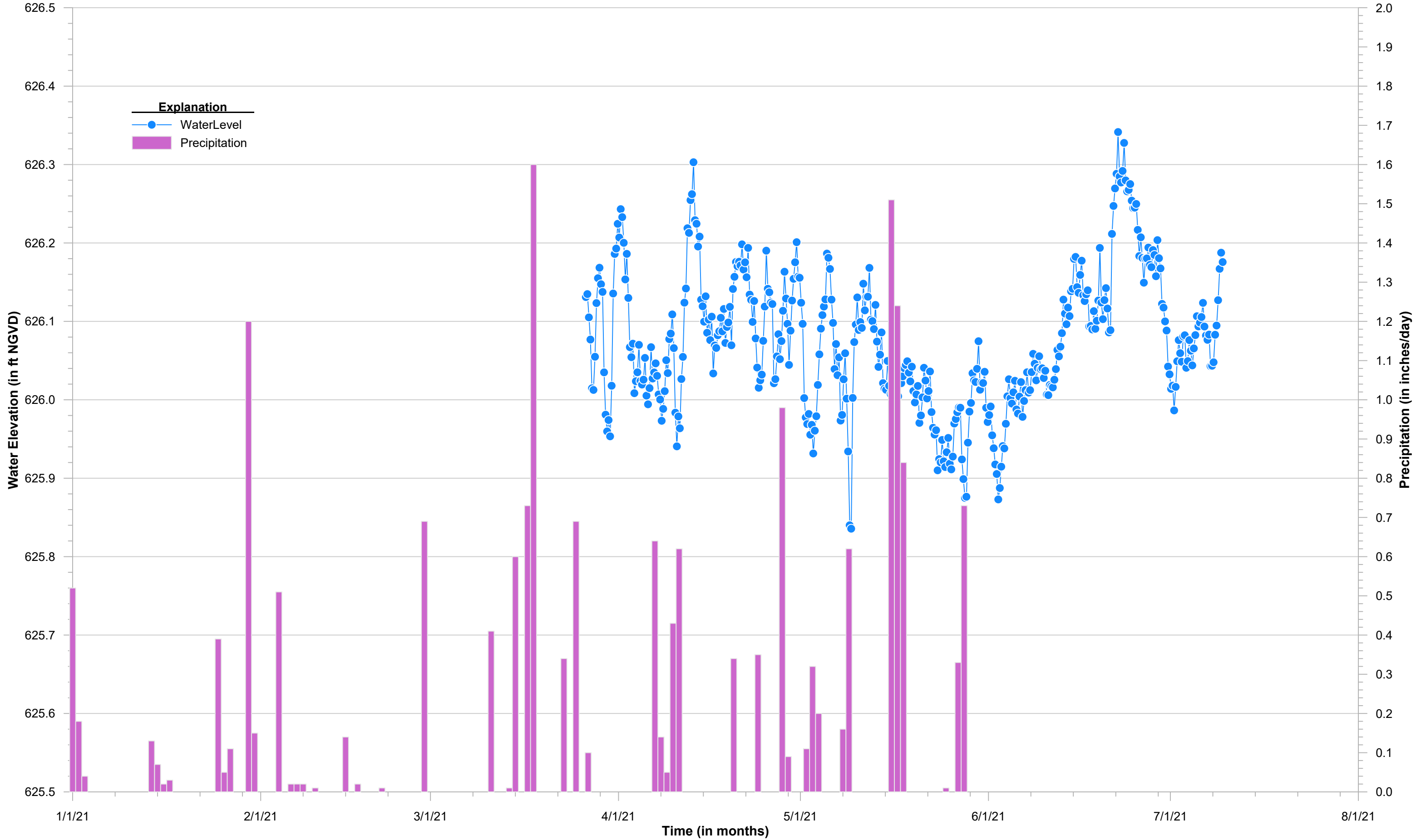
Sample Location	Sample Date	Groundwater Elevation (ft NAVD88)
TA34	07/26/2021	617.18
TA34	08/16/2021	617.42
TR32	08/05/2019	615.67
TR32	01/20/2020	616.56
TR32	08/10/2020	614.92
TR32	01/20/2021	614.50
TR32	04/20/2021	615.59
TR32	07/26/2021	616.09
TR32	08/16/2021	616.18
SG-02	03/29/2021	598.75
SG-02	04/20/2021	598.56
SG-02	05/03/2021	598.74
SG-02	05/17/2021	598.56
SG-02	06/09/2021	598.37
SG-02	06/23/2021	598.34
SG-02	07/12/2021	598.75
SG-02	07/26/2021	598.44
SG-02	08/16/2021	598.39
SG-03	04/20/2021	589.81
SG-03	05/03/2021	589.84
SG-03	05/17/2021	589.84
SG-03	06/09/2021	589.65
SG-03	06/23/2021	589.51
SG-03	07/12/2021	589.97
SG-03	07/26/2021	589.77
SG-03	08/16/2021	589.70
SG-04	04/20/2021	592.99
SG-04	05/03/2021	592.93
SG-04	05/17/2021	593.00
SG-04	06/09/2021	592.82
SG-04	06/23/2021	592.72
SG-04	07/12/2021	591.94
SG-04	07/26/2021	592.83
SG-04	08/16/2021	593.01

Notes:

ft NAVD88 = feet relative to the North American Vertical Datum 1988, GEOID 12A

generated 10/12/2021, 9:27:36 AM CDT

Coffeen Gypsum Pond - NE Riser Water Elevation and Precipitation versus Time



ADDITIONAL VERTICAL HYDRAULIC GRADIENTS

VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

ASH POND NO. 1

COFFEEN, ILLINOIS

Date	G405 Groundwater Elevation (ft NAVD88)	T408 Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
	UA	LCU (upper)				
2/4/2017	618.47	619.46	-0.99	12.00	-0.08	up
5/13/2017	618.74	619.00	-0.26	12.00	-0.02	up
7/8/2017	618.54	619.12	-0.58	12.00	-0.05	up
10/21/2017	614.47	614.81	-0.34	12.00	-0.03	up
5/8/2018	618.94	615.82	3.12	12.00	0.26	down
8/2/2018	617.55	614.45	3.10	12.00	0.26	down
10/23/2018	616.40	616.30	0.10	12.00	0.01	down
1/15/2019	616.81	617.01	-0.20	12.00	-0.02	up
8/5/2019	617.72	617.15	0.57	12.00	0.05	down
1/20/2020	619.28	619.13	0.15	12.00	0.01	down
8/10/2020	617.62	617.38	0.24	12.00	0.02	down
1/20/2021	617.12	616.85	0.27	12.00	0.02	down
4/20/2021	617.13	616.65	0.48	12.00	0.04	down
7/26/2021	617.37	617.21	0.16	12.00	0.01	down
Middle of screen elevation G405D					610.0	
Middle of screen elevation T408					598.0	

Date	G406 Groundwater Elevation (ft NAVD88)	T409 Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
	UA	LCU (upper)				
2/4/2017	617.52	615.93	1.59	8.23	0.19	down
5/13/2017	616.20	616.75	-0.55	8.23	-0.07	up
7/8/2017	616.29	617.05	-0.76	8.23	-0.09	up
10/21/2017	611.27	612.16	-0.89	8.23	-0.11	up
5/8/2018	615.47	616.02	-0.55	8.23	-0.07	up
8/2/2018	615.75	615.25	0.50	8.23	0.06	down
10/23/2018	614.11	613.96	0.15	8.23	0.02	down
1/15/2019	615.36	614.78	0.58	8.23	0.07	down
8/5/2019	616.50	615.10	1.40	8.23	0.17	down
1/20/2020	617.48	617.16	0.32	8.23	0.04	down
8/10/2020	615.54	615.43	0.11	8.23	0.01	down
1/20/2021	612.97	614.41	-1.44	8.23	-0.17	up
4/20/2021	613.78	615.33	-1.55	8.23	-0.19	up
7/26/2021	614.20	615.72	-1.52	8.23	-0.18	up
Middle of screen elevation G406					605.9	
Middle of screen elevation T409					597.7	

VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

ASH POND NO. 1

COFFEEN, ILLINOIS

Date	T408 Groundwater Elevation (ft NAVD88)	G45D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
	LCU (upper)	LCU (lower)				
2/4/2017	619.46	587.71	31.75	13.78	2.30	down
5/13/2017	619.00	586.19	32.81	13.78	2.38	down
7/8/2017	619.12	586.29	32.83	13.78	2.38	down
10/21/2017	614.81	584.69	30.12	13.78	2.19	down
5/8/2018	615.82	587.56	28.26	13.78	2.05	down
8/2/2018	614.45	585.81	28.64	13.78	2.08	down
10/23/2018	616.30	584.60	31.70	13.78	2.30	down
1/15/2019	617.01	586.96	30.05	13.78	2.18	down
8/5/2019	617.15	588.04	29.11	13.78	2.11	down
8/10/2020	617.38	614.21	3.17	13.78	0.23	down
1/20/2021	616.85	614.60	2.25	13.78	0.16	down
4/20/2021	616.65	614.32	2.33	13.78	0.17	down
7/26/2021	617.21	613.58	3.63	13.78	0.26	down
Middle of screen elevation T408					598.0	
Middle of screen elevation G45D					584.2	

Date	T409 Groundwater Elevation (ft NAVD88)	G46D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
	LCU (upper)	LCU (lower)				
2/4/2017	615.93	586.06	29.87	22.19	1.35	down
5/13/2017	616.75	584.87	31.88	22.19	1.44	down
7/8/2017	617.05	585.22	31.83	22.19	1.43	down
5/8/2018	616.02	585.86	30.16	22.19	1.36	down
8/2/2018	615.25	583.95	31.30	22.19	1.41	down
10/23/2018	613.96	582.05	31.91	22.19	1.44	down
1/15/2019	614.78	583.17	31.61	22.19	1.42	down
8/5/2019	615.10	583.68	31.42	22.19	1.42	down
8/10/2020	615.43	609.00	6.43	22.19	0.29	down
1/20/2021	614.41	610.49	3.92	22.19	0.18	down
4/20/2021	615.33	611.06	4.27	22.19	0.19	down
7/26/2021	615.72	607.21	8.51	22.19	0.38	down
Middle of screen elevation T409					597.7	
Middle of screen elevation G46D					575.5	

VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

ASH POND NO. 1

COFFEEN, ILLINOIS

Date	G307 Groundwater Elevation (ft NAVD88)	G307D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)		
	UA	LCU (lower)					
4/20/2021	624.50	622.48	2.02	38.06	0.05	down	
5/17/2021	624.45	622.44	2.01	38.06	0.05	down	
7/12/2021	624.45	622.59	1.86	38.06	0.05	down	
					Middle of screen elevation G307		606.7
					Middle of screen elevation G307D		568.6

Date	G311 Groundwater Elevation (ft NAVD88)	G311D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)		
	UA	LCU (lower)					
3/29/2021	616.54	575.42	41.12	43.41	0.95	down	
4/22/2021	613.68	575.74	37.94	43.41	0.87	down	
5/3/2021	614.01	573.09	40.92	43.41	0.94	down	
5/17/2021	613.86	572.40	41.46	43.41	0.96	down	
6/9/2021	613.13	573.85	39.28	43.41	0.90	down	
6/15/2021	612.78	575.25	37.53	43.41	0.86	down	
6/23/2021	612.45	571.74	40.71	43.41	0.94	down	
7/12/2021	613.75	571.63	42.12	43.41	0.97	down	
7/26/2021	613.05	569.74	43.31	43.41	1.00	down	
					Middle of screen elevation G311		606.7
					Middle of screen elevation G311D		563.3

VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

ASH POND NO. 1

COFFEEN, ILLINOIS

Date	G314 Groundwater Elevation (ft NAVD88)	G314D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
	LCU (upper)	DA (PMP)				
3/29/2021	596.40	572.75	23.65	29.76	0.79	down
4/20/2021	603.16	571.76	31.40	27.40	1.15	down
5/3/2021	604.66	568.77	35.89	27.40	1.31	down
5/17/2021	605.61	566.84	38.77	27.40	1.42	down
6/9/2021	607.54	567.45	40.09	27.40	1.46	down
6/14/2021	608.16	568.60	39.56	27.40	1.44	down
6/23/2021	605.19	566.77	38.42	27.40	1.40	down
7/12/2021	605.32	566.88	38.44	27.40	1.40	down
7/26/2021	606.66	566.65	40.01	27.40	1.46	down
Middle of screen elevation G314					594.0	
Middle of screen elevation G314D					566.6	

[O: KLT 6/4/21, C:YMD 6/7/21; U:KLT 8/25/21, C:EDP 8/31/21]

Notes:

¹ Distance change was calculated using the midpoint of the piezometer screen and water table surface. If the water table surface was above the top of the monitoring well screen, then distance change was calculated using the midpoint of both screens.

² Vertical gradients between ±0.0015 are considered flat, and typically have less than 0.02 foot difference in groundwater elevation between wells.

- - = no data collected on date / no vertical gradient calculated

DA = deep aquifer

dh = head change

dl = distance change

ft = foot/feet

LCU (lower) = lower confining unit (Smithboro)

LCU (upper) = lower confining unit (Vandalia)

NAVD88 = North American Vertical Datum of 1988

PMP = potential migration pathway

UA = uppermost aquifer

VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

GYPSUM MANAGEMENT FACILITY RECYCLE POND

COFFEEN, ILLINOIS

Date	G405 Groundwater Elevation (ft NAVD88)	T408 Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)		
	UA	LCU (upper)					
2/4/2017	618.47	619.46	-0.99	12.00	-0.08	up	
5/13/2017	618.74	619.00	-0.26	12.00	-0.02	up	
7/8/2017	618.54	619.12	-0.58	12.00	-0.05	up	
10/21/2017	614.47	614.81	-0.34	12.00	-0.03	up	
5/8/2018	618.94	615.82	3.12	12.00	0.26	down	
8/2/2018	617.55	614.45	3.10	12.00	0.26	down	
10/23/2018	616.40	616.30	0.10	12.00	0.01	down	
1/15/2019	616.81	617.01	-0.20	12.00	-0.02	up	
8/5/2019	617.72	617.15	0.57	12.00	0.05	down	
1/20/2020	619.28	619.13	0.15	12.00	0.01	down	
8/10/2020	617.62	617.38	0.24	12.00	0.02	down	
1/20/2021	617.12	616.85	0.27	12.00	0.02	down	
4/20/2021	617.13	616.65	0.48	12.00	0.04	down	
7/26/2021	617.37	617.21	0.16	12.00	0.01	down	
					Middle of screen elevation G405D		610.0
					Middle of screen elevation T408		598.0

Date	G275 Groundwater Elevation (ft NAVD88)	G275D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)		
	UA	DA (PMP)					
4/20/21-4/21/20	605.00	568.33	36.67	42.14	0.87	down	
7/12/21-7/13/21	605.63	570.43	35.20	42.77	0.82	down	
7/26/2021	605.05	570.35	34.70	42.18	0.82	down	
					Middle of screen elevation G275		605.7
					Middle of screen elevation G275D		562.9

VERTICAL HYDRAULIC GRADIENTS

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT
 COFFEEN POWER PLANT
 GYPSUM MANAGEMENT FACILITY RECYCLE POND
 COFFEEN, ILLINOIS

Date	T408 Groundwater Elevation (ft NAVD88)	G45D Groundwater Elevation (ft NAVD88)	Head Change (ft)	Distance Change ¹ (ft)	Vertical Hydraulic Gradient ² (dh/dl)	
	LCU (upper)	LCU (lower)				
2/4/2017	619.46	587.71	31.75	13.78	2.30	down
5/13/2017	619.00	586.19	32.81	13.78	2.38	down
7/8/2017	619.12	586.29	32.83	13.78	2.38	down
10/21/2017	614.81	584.69	30.12	13.78	2.19	down
5/8/2018	615.82	587.56	28.26	13.78	2.05	down
8/2/2018	614.45	585.81	28.64	13.78	2.08	down
10/23/2018	616.30	584.60	31.70	13.78	2.30	down
1/15/2019	617.01	586.96	30.05	13.78	2.18	down
8/5/2019	617.15	588.04	29.11	13.78	2.11	down
8/10/2020	617.38	614.21	3.17	13.78	0.23	down
1/20/2021	616.85	614.60	2.25	13.78	0.16	down
4/20/2021	616.65	614.32	2.33	13.78	0.17	down
7/26/2021	617.21	613.58	3.63	13.78	0.26	down
Middle of screen elevation T408					598.0	
Middle of screen elevation G45D					584.2	

[O: KLT 6/4/21, C:YMD 6/7/21][U:KLT 8/25/21, C:EDP 8/31/21]

Notes:

¹ Distance change was calculated using the midpoint of the piezometer screen and water table surface. If the water table surface was above the top of the monitoring well screen, then distance change was calculated using the midpoint of both screens.

² Vertical gradients between ±0.0015 are considered flat, and typically have less than 0.02 foot difference in groundwater elevation between wells.

-- = no data collected on date / no vertical gradient calculated

DA = deep aquifer

dh = head change

dl = distance change

ft = foot/feet

LCU (lower) = lower confining unit (Smithboro)

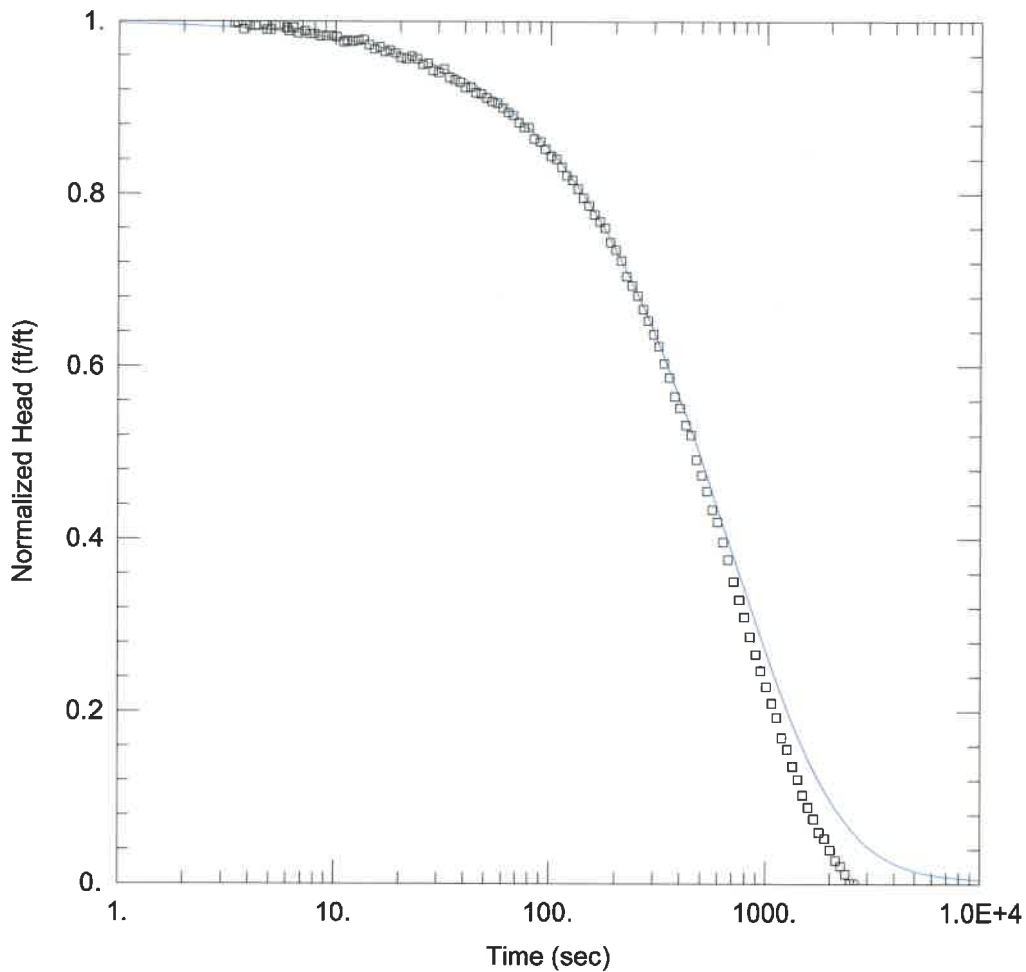
LCU (upper) = lower confining unit (Vandalia)

NAVD88 = North American Vertical Datum of 1988

PMP = potential migration pathway

UA = uppermost aquifer

APPENDIX F
HYDRAULIC CONDUCTIVITY TEST DATA



G206 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G206-fh.aqt
 Date: 06/02/21 Time: 15:40:13

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G206
 Test Date: 17 March 2021

AQUIFER DATA

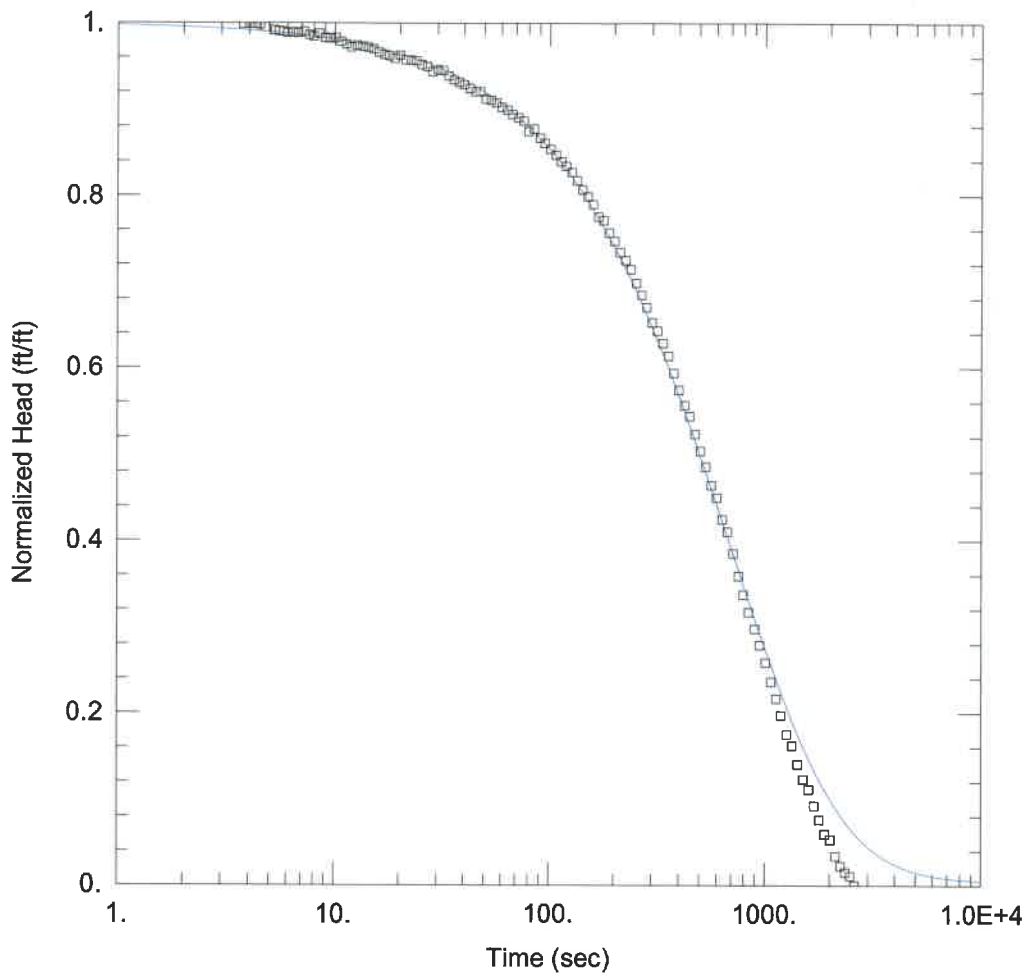
Saturated Thickness: 2.3 ft

WELL DATA (G206)

Initial Displacement: 1.5 ft Static Water Column Height: 14.47 ft
 Total Well Penetration Depth: 13.97 ft Screen Length: 4.41 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 $K_r = 0.0005 \text{ cm/sec}$ $S_s = 1.0E-6 \text{ ft}^{-1}$
 $K_z/K_r = 1.$



G206 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G206-rh.aqt
 Date: 06/02/21 Time: 15:40:14

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G206
 Test Date: 17 March 2021

AQUIFER DATA

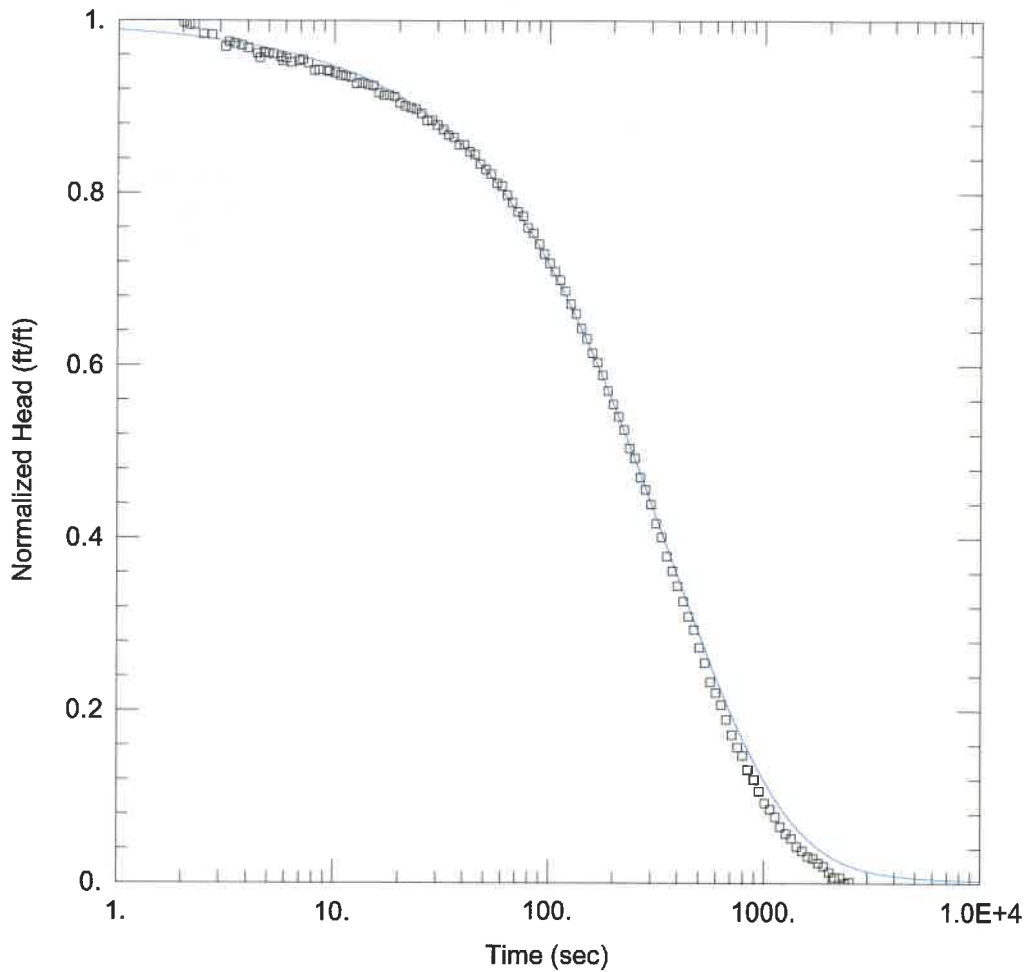
Saturated Thickness: 2.3 ft

WELL DATA (G206)

Initial Displacement: <u>1.385 ft</u>	Static Water Column Height: <u>14.47 ft</u>
Total Well Penetration Depth: <u>13.97 ft</u>	Screen Length: <u>4.41 ft</u>
Casing Radius: <u>0.08333 ft</u>	Well Radius: <u>0.08333 ft</u>

SOLUTION

Aquifer Model: <u>Confined</u>	Solution Method: <u>KGS Model</u>
Kr = <u>0.00049 cm/sec</u>	Ss = <u>1.0E-6 ft⁻¹</u>
Kz/Kr = <u>1.</u>	



G209 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G209-rh.aqt
 Date: 06/02/21 Time: 15:40:16

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G209
 Test Date: 17 March 2021

AQUIFER DATA

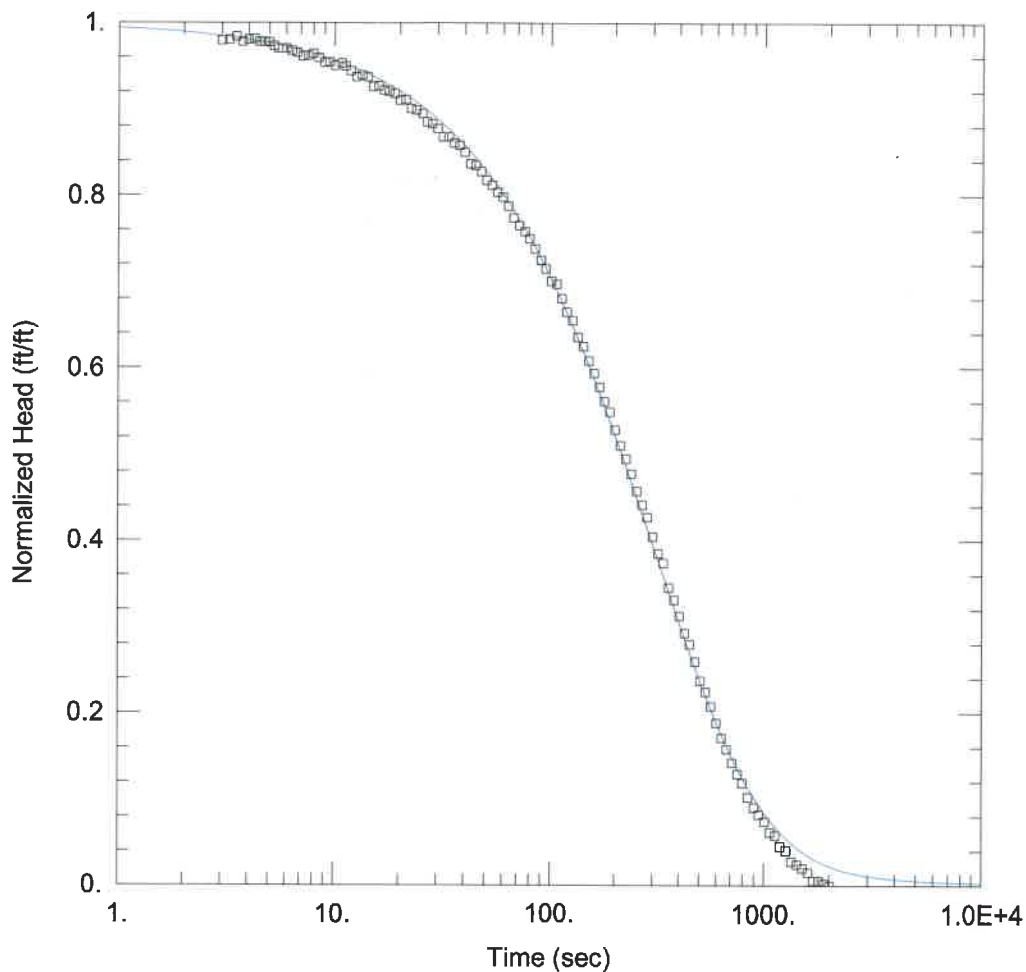
Saturated Thickness: 8. ft

WELL DATA (G209)

Initial Displacement: 1.575 ft Static Water Column Height: 14.76 ft
 Total Well Penetration Depth: 11.89 ft Screen Length: 4.54 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 $K_r = \underline{0.00025 \text{ cm/sec}}$ $S_s = \underline{0.0007 \text{ ft}^{-1}}$
 $K_z/K_r = \underline{1.}$



G212 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G212-fh.aqt
 Date: 06/02/21 Time: 15:40:17

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G212
 Test Date: 16 March 2021

AQUIFER DATA

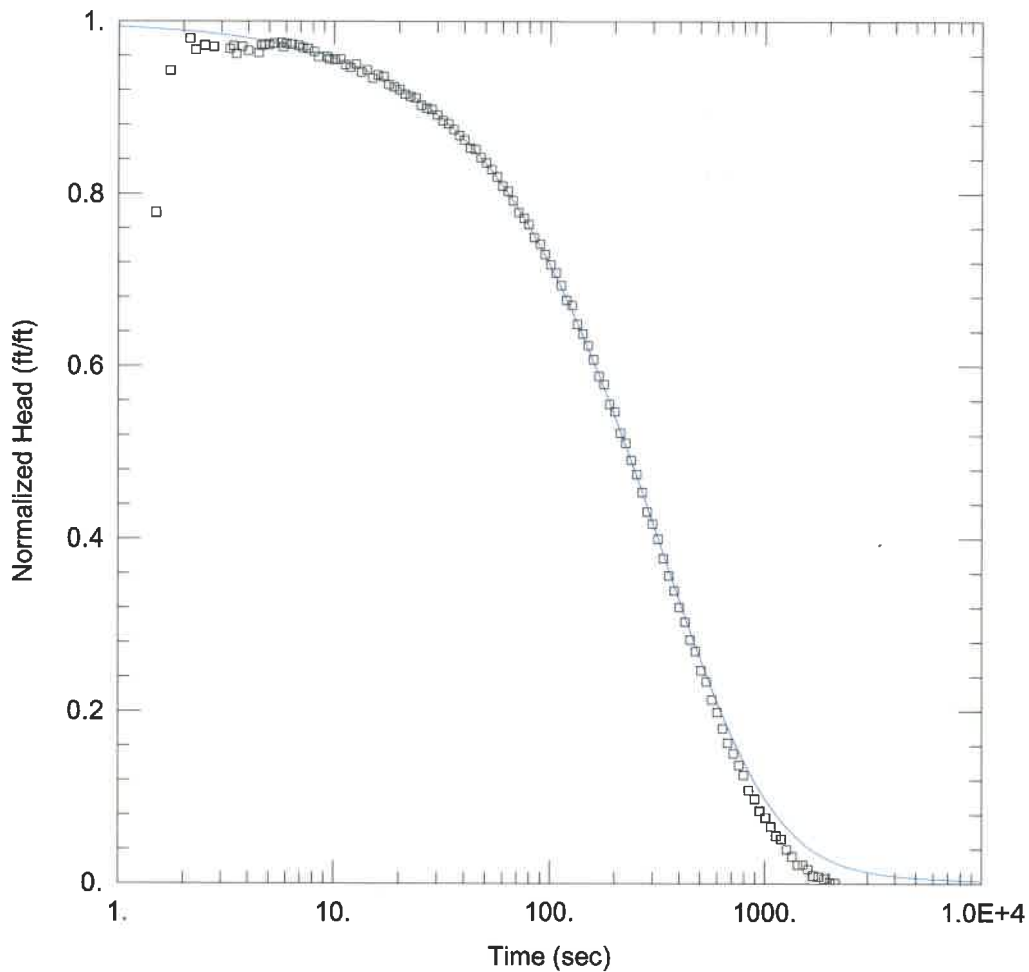
Saturated Thickness: 1. ft

WELL DATA (G212)

Initial Displacement: <u>1.45 ft</u>	Static Water Column Height: <u>13.14 ft</u>
Total Well Penetration Depth: <u>12.62 ft</u>	Screen Length: <u>4.55 ft</u>
Casing Radius: <u>0.08333 ft</u>	Well Radius: <u>0.08333 ft</u>

SOLUTION

Aquifer Model: <u>Confined</u>	Solution Method: <u>KGS Model</u>
Kr = <u>0.0021 cm/sec</u>	Ss = <u>1.5E-5 ft⁻¹</u>
Kz/Kr = <u>1.</u>	



G212 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G212-rh.aqt
 Date: 06/02/21 Time: 15:40:19

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Ciffeen, IL
 Test Well: G212
 Test Date: 16 March 2021

AQUIFER DATA

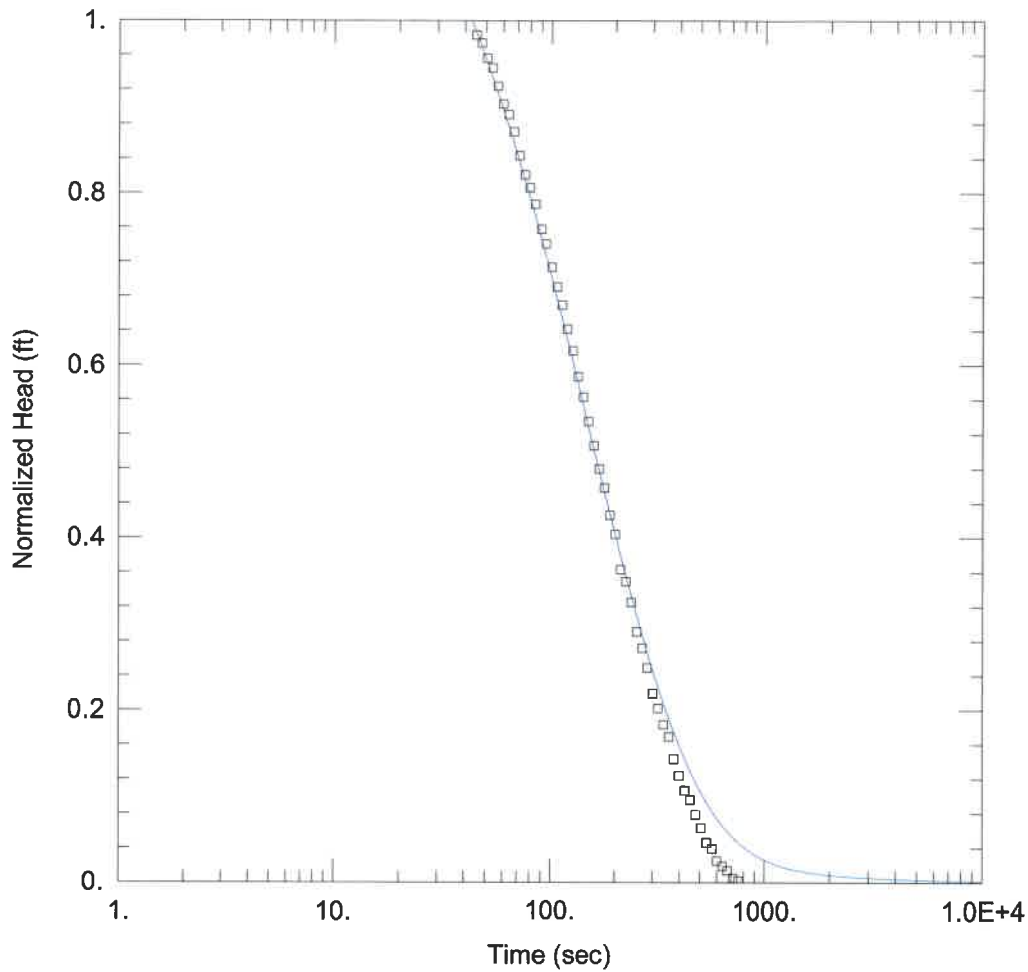
Saturated Thickness: 1. ft

WELL DATA (G212)

Initial Displacement: 1.525 ft	Static Water Column Height: 13.14 ft
Total Well Penetration Depth: 12.62 ft	Screen Length: 4.55 ft
Casing Radius: 0.08333 ft	Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined	Solution Method: KGS Model
Kr = 0.0018 cm/sec	Ss = 4.0E-5 ft ⁻¹
Kz/Kr = 1.	



G215 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G215-fh.aqt
 Date: 06/02/21 Time: 15:40:20

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G215
 Test Date: 16 March 2021

AQUIFER DATA

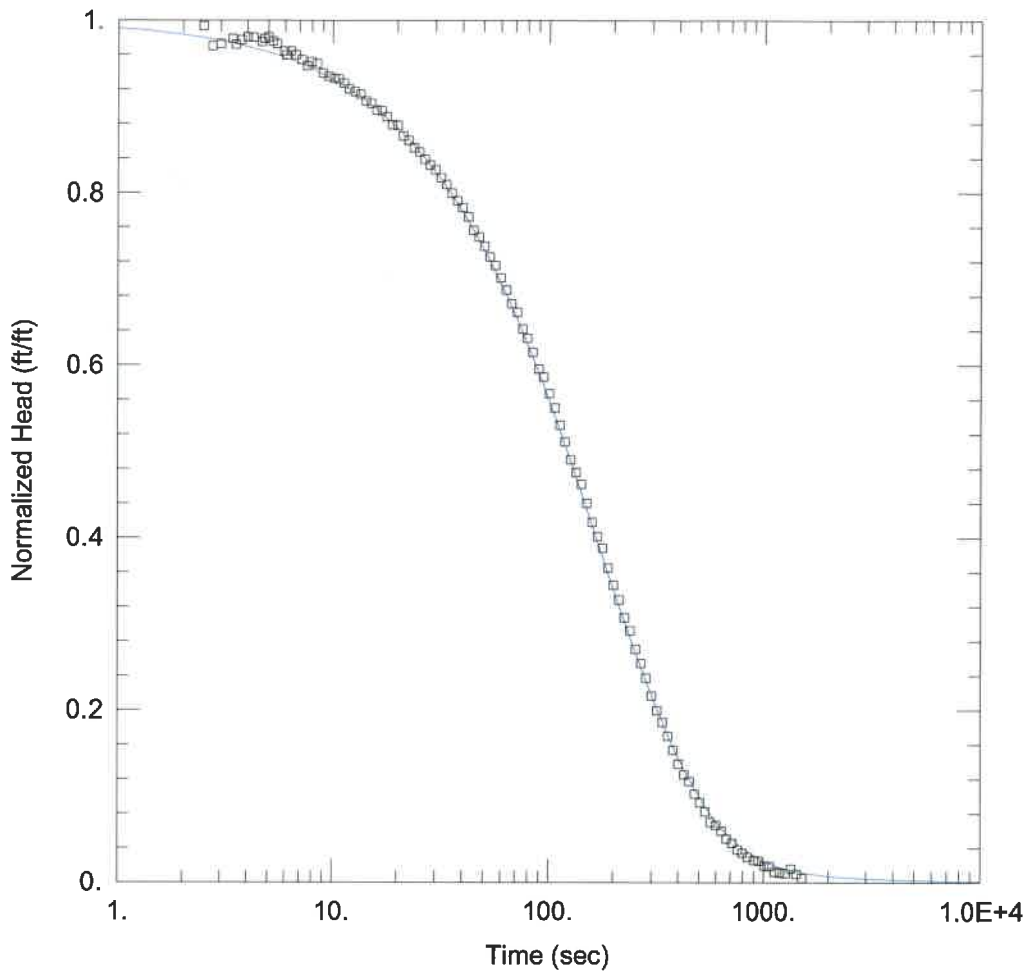
Saturated Thickness: 1.2 ft

WELL DATA (G215)

Initial Displacement: <u>1.35 ft</u>	Static Water Column Height: <u>12.4 ft</u>
Total Well Penetration Depth: <u>12.4 ft</u>	Screen Length: <u>4.9 ft</u>
Casing Radius: <u>0.08333 ft</u>	Well Radius: <u>0.08333 ft</u>

SOLUTION

Aquifer Model: <u>Confined</u>	Solution Method: <u>KGS Model</u>
Kr = <u>0.004 cm/sec</u>	Ss = <u>1.0E-5 ft⁻¹</u>
Kz/Kr = <u>1.</u>	



G215 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G215-rh.aqt
 Date: 06/02/21 Time: 15:40:21

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G215
 Test Date: 16 March 2021

AQUIFER DATA

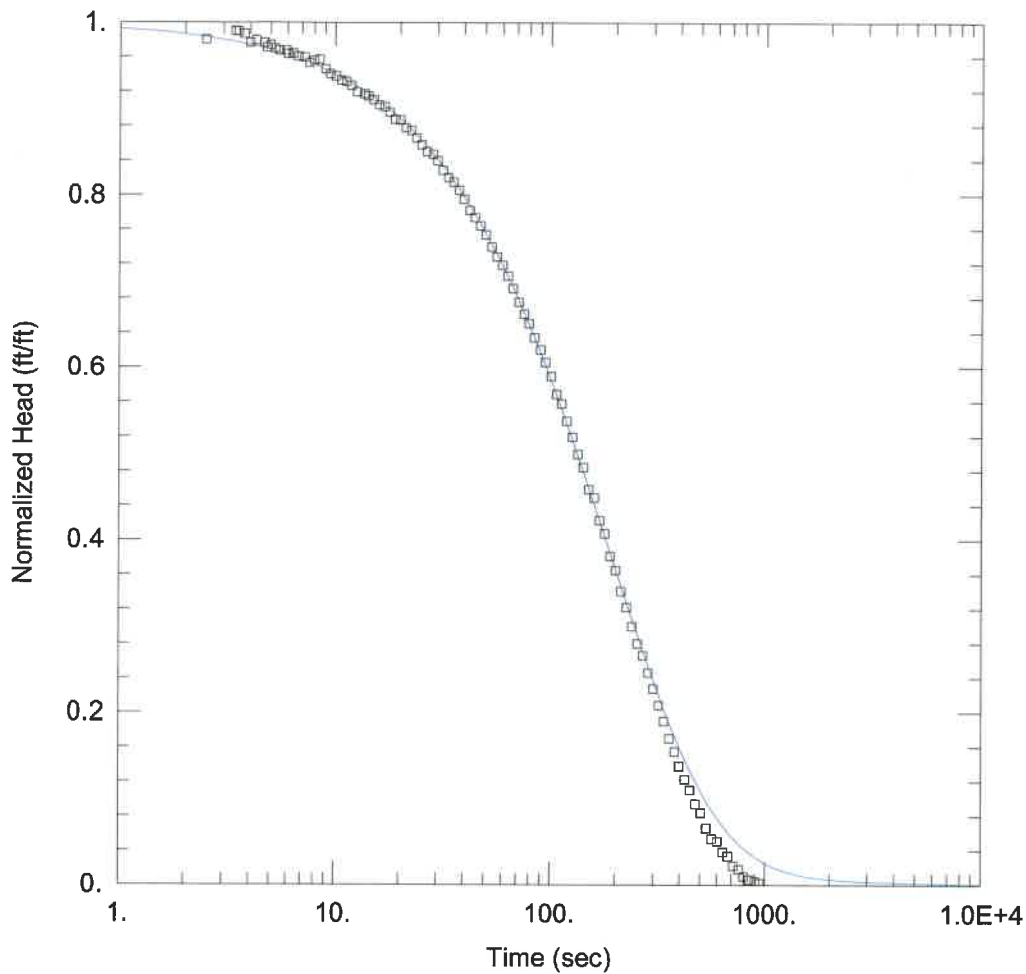
Saturated Thickness: 1.2 ft

WELL DATA (G215)

Initial Displacement: 1.4 ft Static Water Column Height: 12.4 ft
 Total Well Penetration Depth: 12.4 ft Screen Length: 4.9 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 $K_r = 0.0035 \text{ cm/sec}$ $S_s = 1.0E-5 \text{ ft}^{-1}$
 $K_z/K_r = 1.$



G218 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G218-fh.aqt
 Date: 06/02/21 Time: 15:40:23

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G218
 Test Date: 16 March 2021

AQUIFER DATA

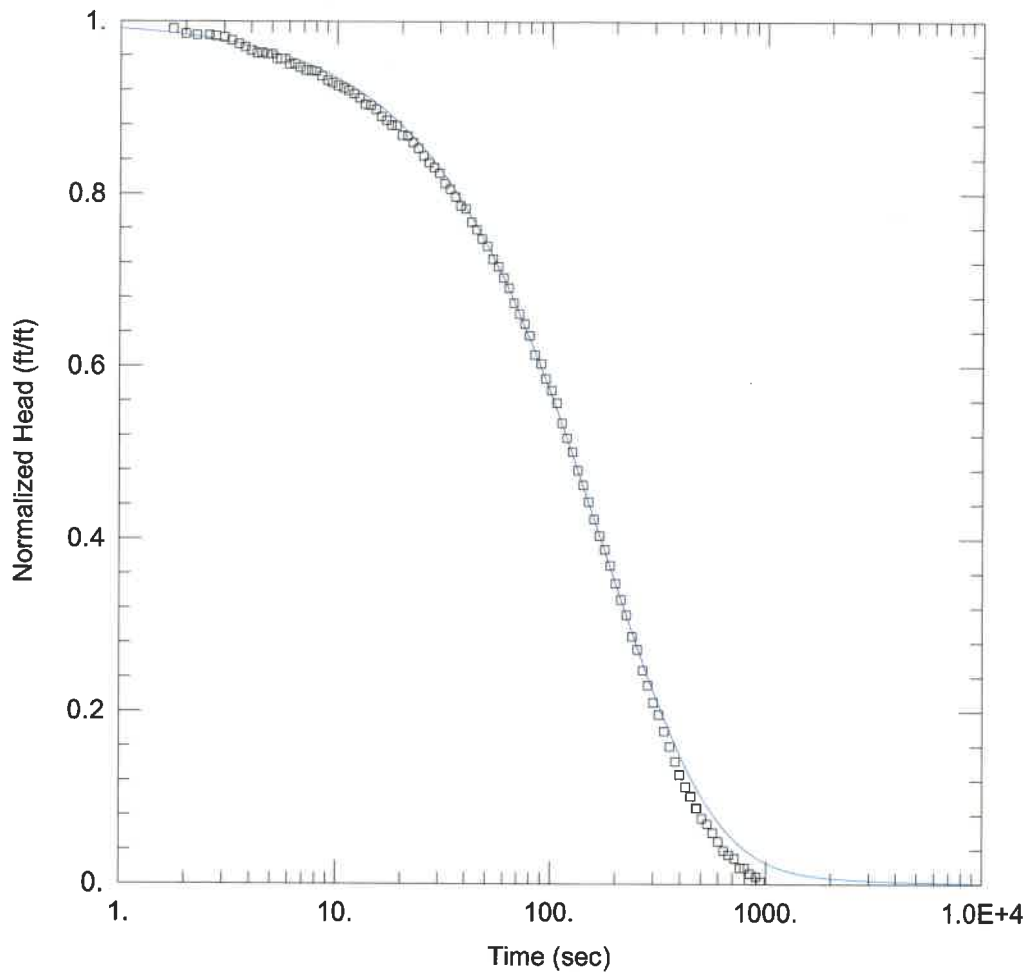
Saturated Thickness: 1.9 ft

WELL DATA (G218)

Initial Displacement: 1.45 ft Static Water Column Height: 20.86 ft
 Total Well Penetration Depth: 12.86 ft Screen Length: 4.44 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 $K_r = 0.0026 \text{ cm/sec}$ $S_s = 1.0E-6 \text{ ft}^{-1}$
 $K_z/K_r = 1.$



G218 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G218-rh.aqt
 Date: 06/02/21 Time: 15:40:24

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G218
 Test Date: 16 March 2021

AQUIFER DATA

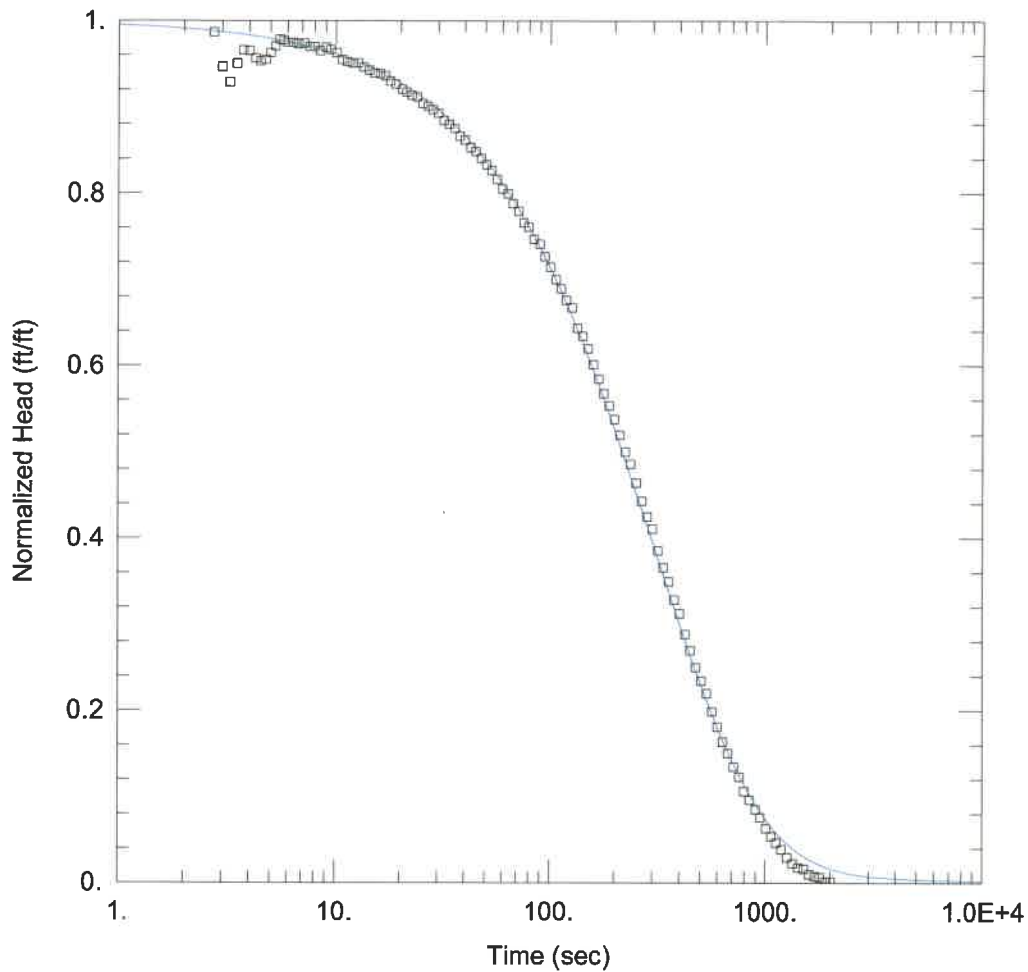
Saturated Thickness: 1.9 ft

WELL DATA (G218)

Initial Displacement: <u>1.5 ft</u>	Static Water Column Height: <u>20.86 ft</u>
Total Well Penetration Depth: <u>12.86 ft</u>	Screen Length: <u>4.44 ft</u>
Casing Radius: <u>0.08333 ft</u>	Well Radius: <u>0.08333 ft</u>

SOLUTION

Aquifer Model: <u>Confined</u>	Solution Method: <u>KGS Model</u>
Kr = <u>0.0024 cm/sec</u>	Ss = <u>1.0E-5 ft⁻¹</u>
Kz/Kr = <u>1.</u>	



G272 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G272-fh.aqt
 Date: 06/02/21 Time: 15:40:26

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G272
 Test Date: 16 March 2021

AQUIFER DATA

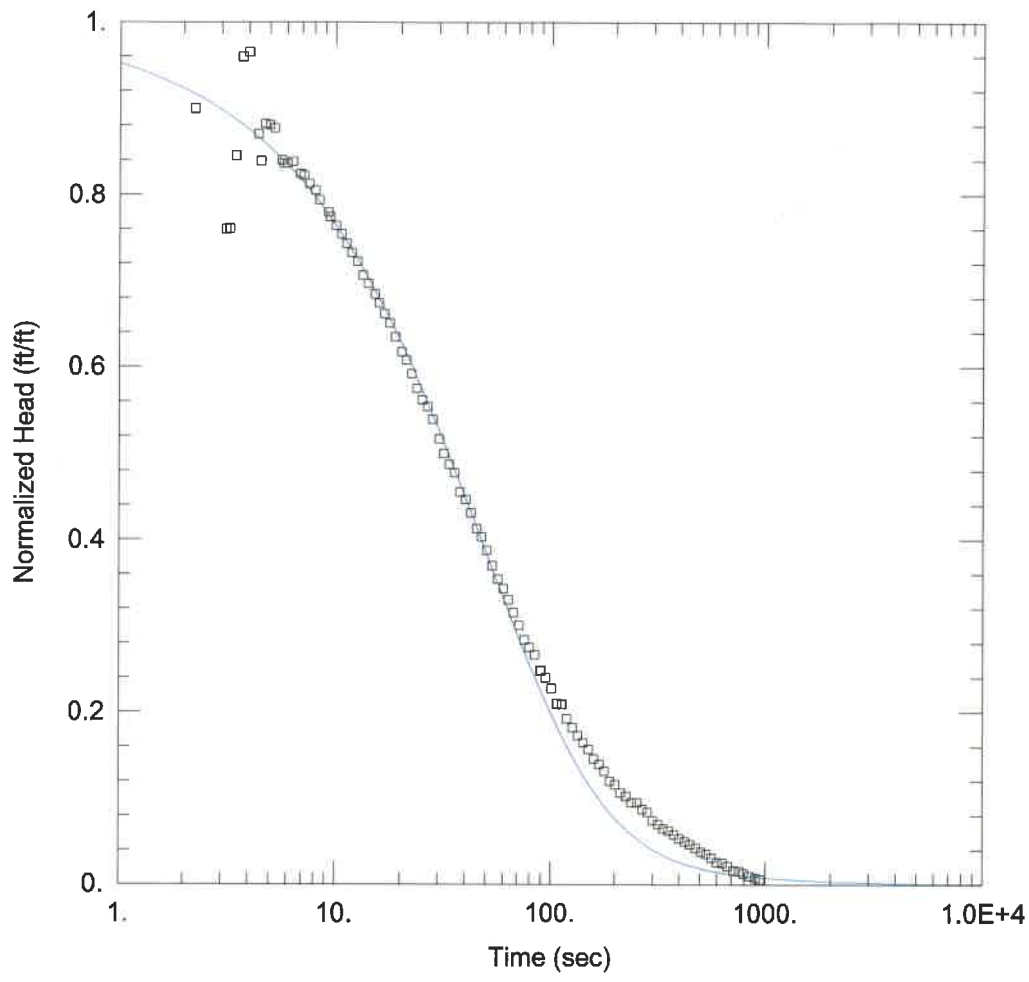
Saturated Thickness: 1.7 ft

WELL DATA (G272)

Initial Displacement: <u>1.425 ft</u>	Static Water Column Height: <u>7.22 ft</u>
Total Well Penetration Depth: <u>7.7 ft</u>	Screen Length: <u>4.87 ft</u>
Casing Radius: <u>0.08333 ft</u>	Well Radius: <u>0.08333 ft</u>

SOLUTION

Aquifer Model: <u>Confined</u>	Solution Method: <u>KGS Model</u>
Kr = <u>0.0017 cm/sec</u>	Ss = <u>1.0E-6 ft⁻¹</u>
Kz/Kr = <u>1.</u>	



G283 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G283-fh.aqt
 Date: 06/02/21 Time: 15:41:16

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G283
 Test Date: 16 March 2021

AQUIFER DATA

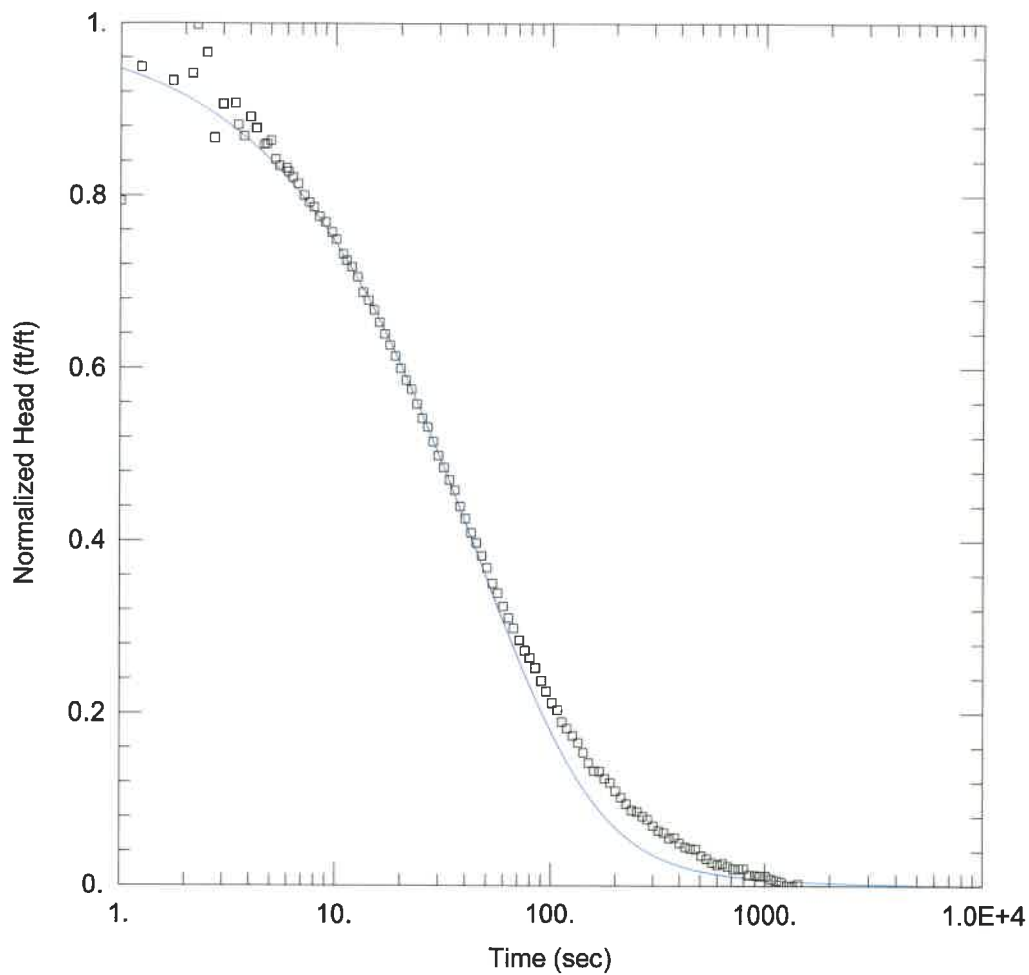
Saturated Thickness: 1.8 ft

WELL DATA (G283)

Initial Displacement: 1.55 ft Static Water Column Height: 16.77 ft
 Total Well Penetration Depth: 16.58 ft Screen Length: 9.78 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 Kr = 0.0042 cm/sec Ss = 0.005 ft⁻¹
 Kz/Kr = 1.



G283 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G283-rh.aqt
 Date: 06/02/21 Time: 15:41:17

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G283
 Test Date: 16 March 2021

AQUIFER DATA

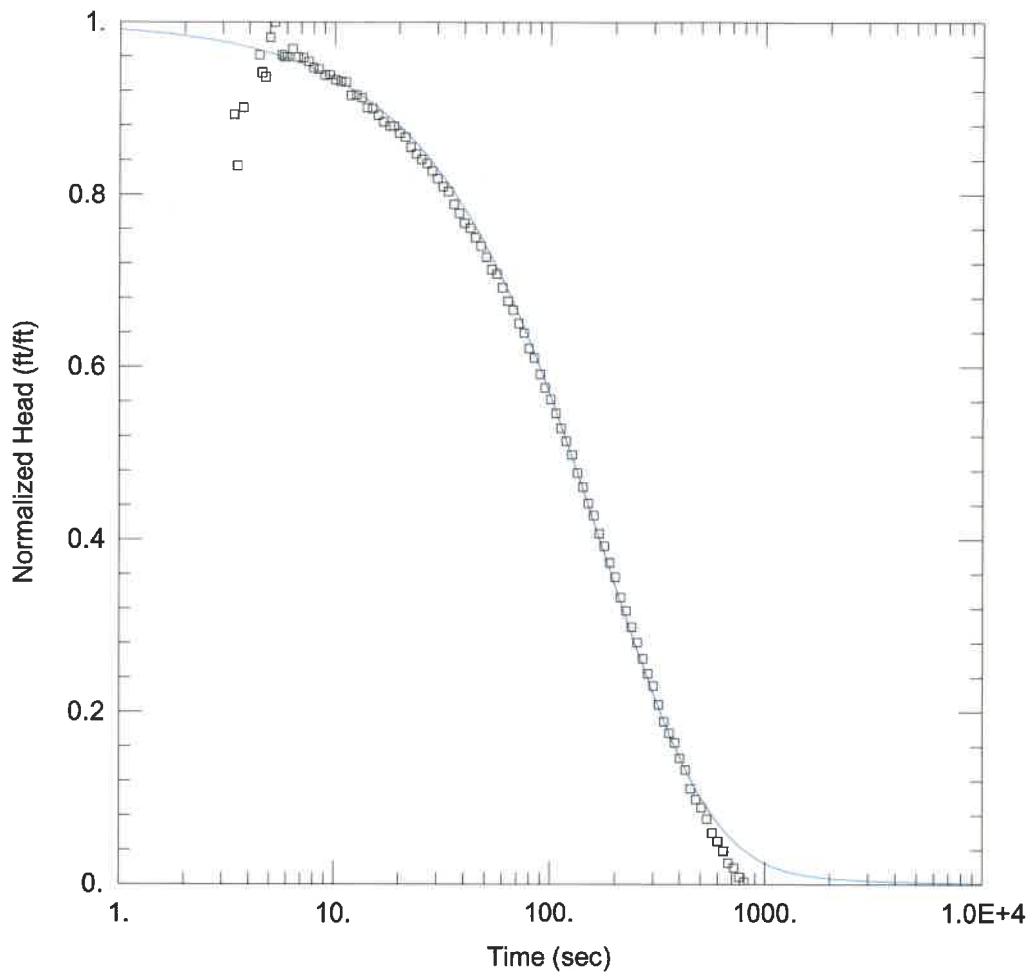
Saturated Thickness: 1.8 ft

WELL DATA (G283)

Initial Displacement: <u>1.6 ft</u>	Static Water Column Height: <u>16.77 ft</u>
Total Well Penetration Depth: <u>16.58 ft</u>	Screen Length: <u>9.78 ft</u>
Casing Radius: <u>0.08333 ft</u>	Well Radius: <u>0.08333 ft</u>

SOLUTION

Aquifer Model: <u>Confined</u>	Solution Method: <u>KGS Model</u>
Kr = <u>0.0045 cm/sec</u>	Ss = <u>0.006 ft⁻¹</u>
Kz/Kr = <u>1.</u>	



G284 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G284-fh.aqt
 Date: 06/02/21 Time: 15:41:19

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G284
 Test Date: 12 March 2021

AQUIFER DATA

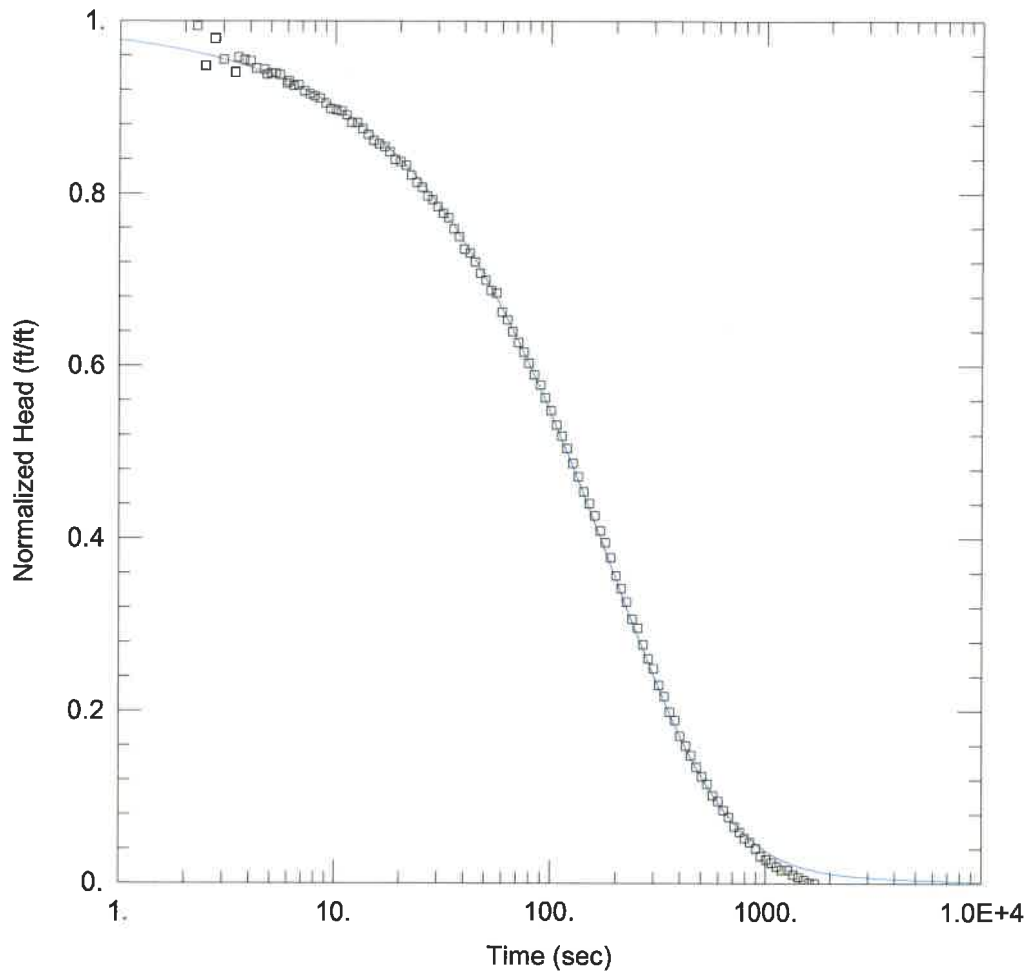
Saturated Thickness: 3.8 ft

WELL DATA (G284)

Initial Displacement: 1.425 ft	Static Water Column Height: 7.05 ft
Total Well Penetration Depth: 6.67 ft	Screen Length: 4.77 ft
Casing Radius: 0.08333 ft	Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined	Solution Method: KGS Model
Kr = 0.0012 cm/sec	Ss = 2.5E-5 ft ⁻¹
Kz/Kr = 1.	



G284 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G284-rh.aqt
 Date: 06/02/21 Time: 15:41:20

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G284
 Test Date: 12 March 2021

AQUIFER DATA

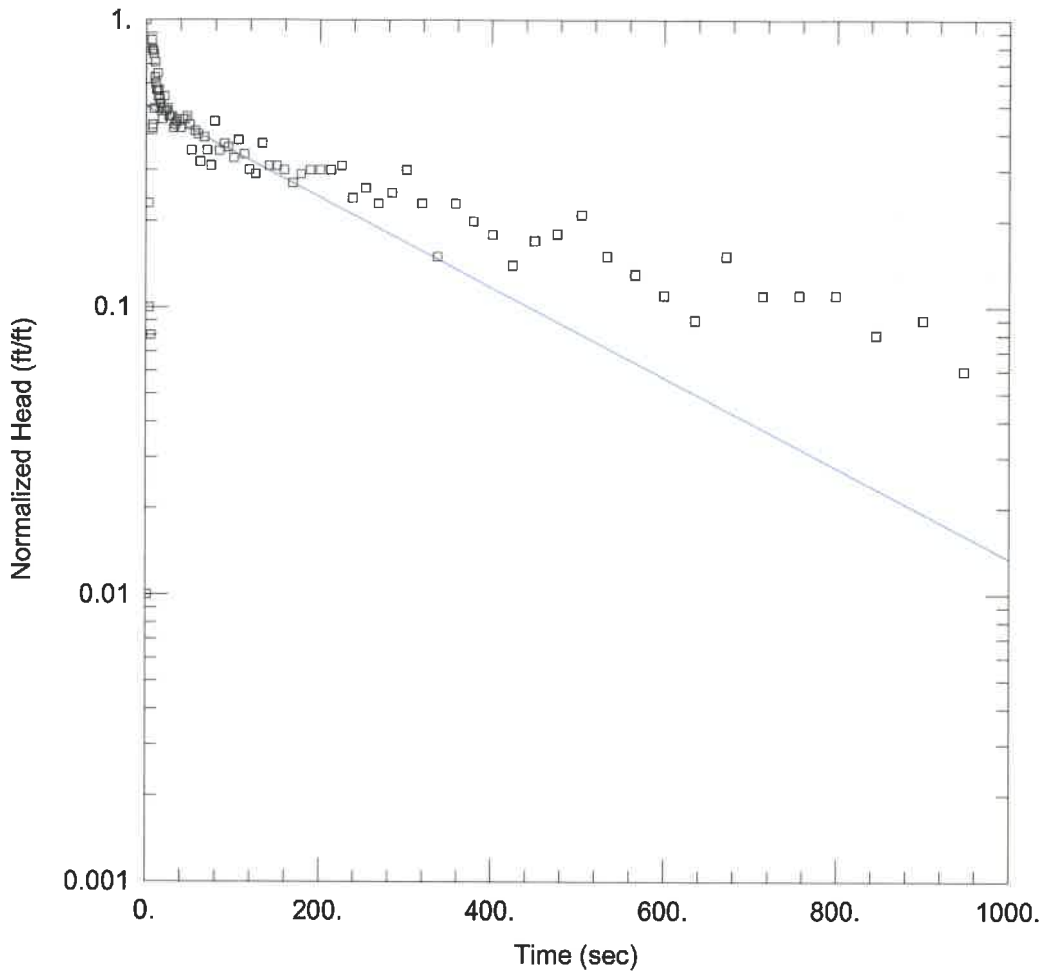
Saturated Thickness: 3.8 ft

WELL DATA (G284)

Initial Displacement: 1.55 ft Static Water Column Height: 7.05 ft
 Total Well Penetration Depth: 6.67 ft Screen Length: 4.77 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 $K_r = 0.00078 \text{ cm/sec}$ $S_s = 0.0037 \text{ ft}^{-1}$
 $K_z/K_r = 1.$



G285 FALLING HEAD TEST

Data Set: I:\20\jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G285-fh.aqt
 Date: 06/02/21 Time: 15:41:22

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G285
 Test Date: 12 March 2021

AQUIFER DATA

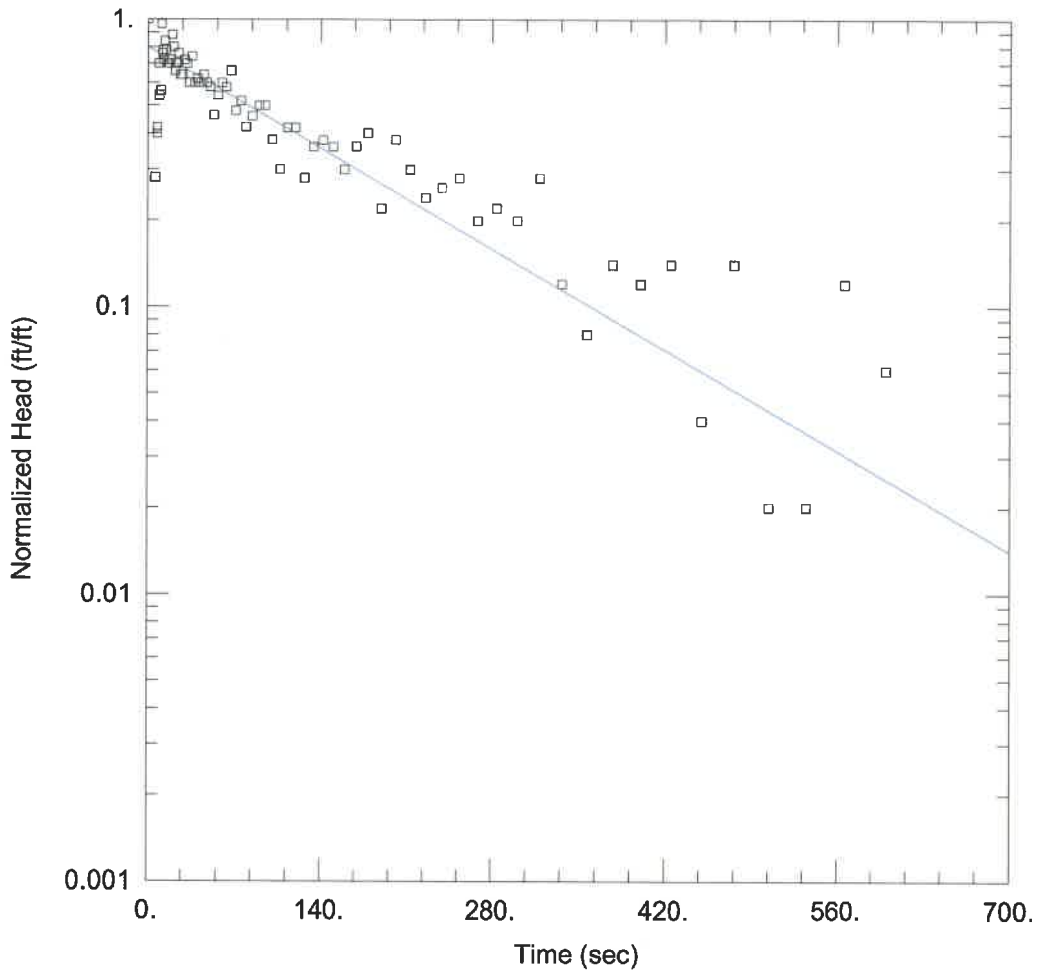
Saturated Thickness: 5.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (G285)

Initial Displacement: 0.1 ft Static Water Column Height: 18.11 ft
 Total Well Penetration Depth: 17.73 ft Screen Length: 9.77 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: Bower-Rice
 K = 0.00027 cm/sec y0 = 0.05 ft



G285 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G285-rh.aqt
 Date: 06/02/21 Time: 15:41:23

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G285
 Test Date: 12 March 2021

AQUIFER DATA

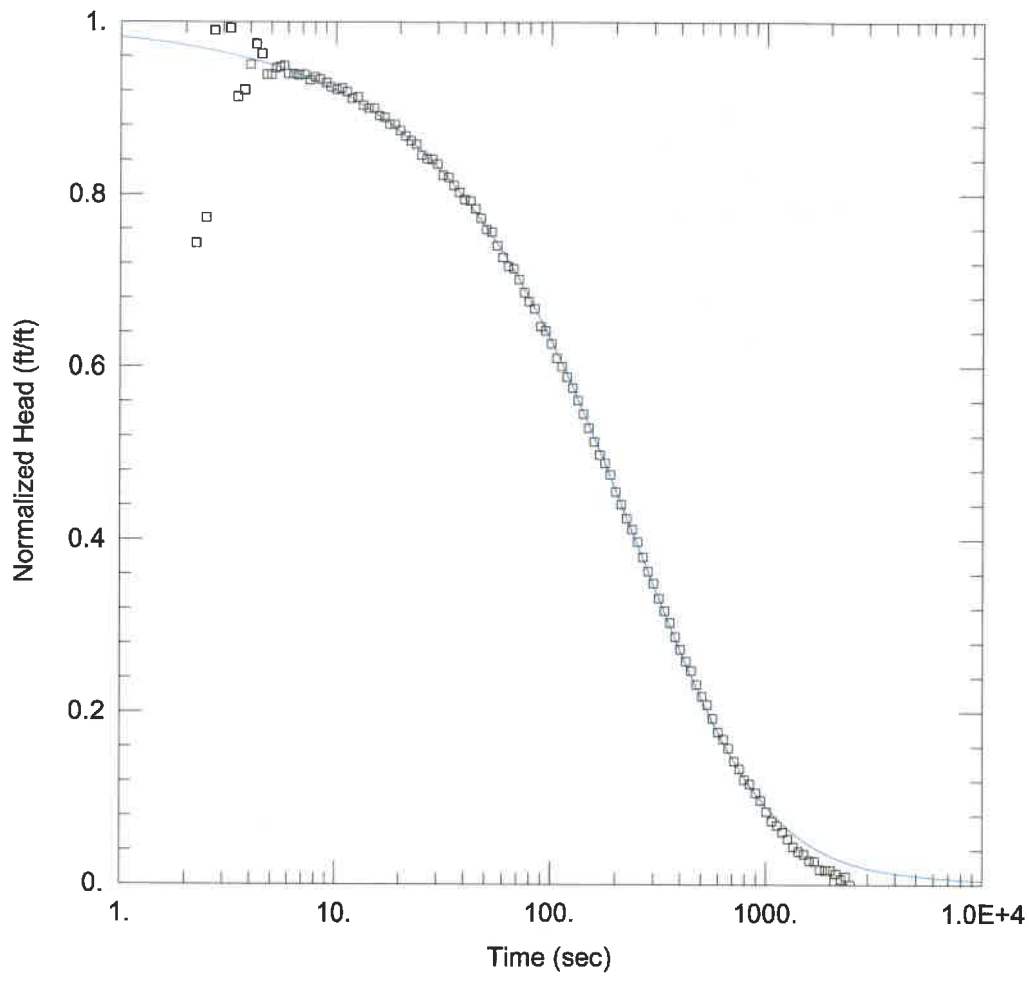
Saturated Thickness: 5.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (G285)

Initial Displacement: 0.05 ft Static Water Column Height: 18.11 ft
 Total Well Penetration Depth: 17.73 ft Screen Length: 9.77 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: Bouwer-Rice
 K = 0.00043 cm/sec y0 = 0.04 ft



G286 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G286-fh.aqt
 Date: 06/02/21 Time: 15:41:25

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G286
 Test Date: 12 March 2021

AQUIFER DATA

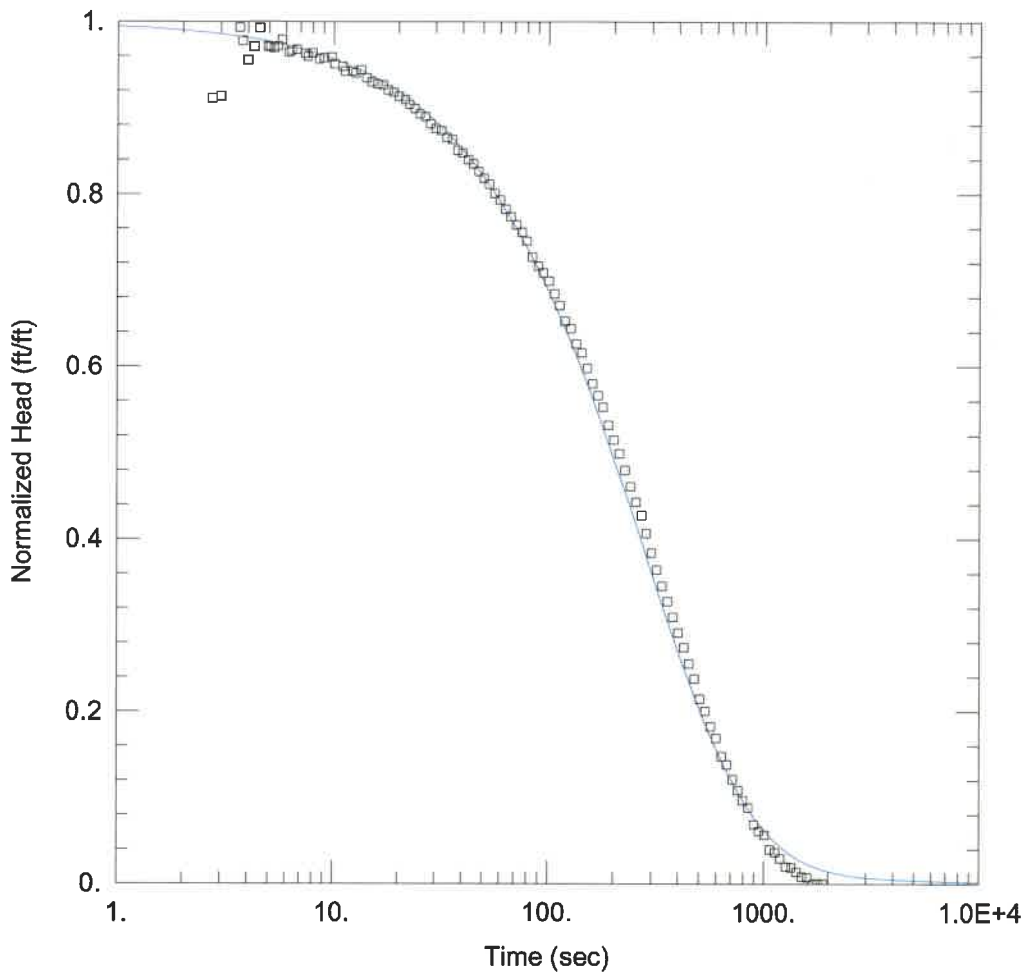
Saturated Thickness: 1. ft

WELL DATA (G286)

Initial Displacement: 1.25 ft	Static Water Column Height: 6.26 ft
Total Well Penetration Depth: 5.92 ft	Screen Length: 4.79 ft
Casing Radius: 0.08333 ft	Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined	Solution Method: KGS Model
Kr = 0.0012 cm/sec	Ss = 0.01 ft ⁻¹
Kz/Kr = 1.	



G287 FALLING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G287-fh.aqt
 Date: 06/02/21 Time: 15:41:26

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G287
 Test Date: 17 March 2021

AQUIFER DATA

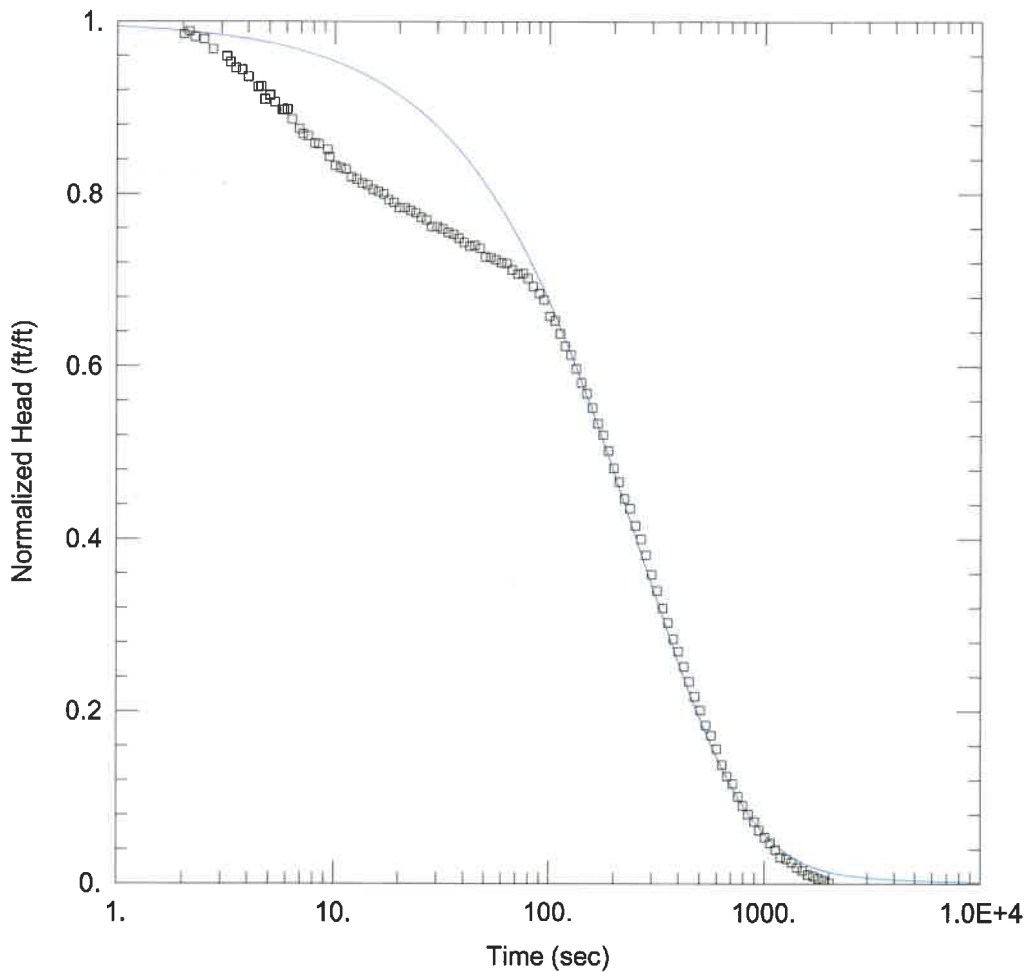
Saturated Thickness: 2.7 ft

WELL DATA (G287)

Initial Displacement: 1.425 ft Static Water Column Height: 6.05 ft
 Total Well Penetration Depth: 5.71 ft Screen Length: 4.82 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 $K_r = \underline{0.0011}$ cm/sec $S_s = \underline{3.5E-6}$ ft⁻¹
 $K_z/K_r = \underline{1}$



G287 RISING HEAD TEST

Data Set: I:\20jobs\20E0111A\Admin\13-Calculations\Slug Tests\Coffeen\G287-rh.aqt
 Date: 06/02/21 Time: 15:41:28

PROJECT INFORMATION

Client: Coffeen Power Station
 Project: 20E0111A
 Location: Coffeen, IL
 Test Well: G287
 Test Date: 17 March 2021

AQUIFER DATA

Saturated Thickness: 2.7 ft

WELL DATA (G287)

Initial Displacement: 1.5 ft Static Water Column Height: 6.05 ft
 Total Well Penetration Depth: 5.71 ft Screen Length: 4.82 ft
 Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Confined Solution Method: KGS Model
 $K_r = 0.0011 \text{ cm/sec}$ $S_s = 7.0E-6 \text{ ft}^{-1}$
 $K_z/K_r = 1.$

ADDITIONAL FIELD HYDRAULIC CONDUCTIVITIES

FIELD HYDRAULIC CONDUCTIVITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

ASH POND NO. 1

COFFEEN, ILLINOIS

Well ID	Gradient Position	Bottom of Screen Elevation (ft NAVD88)	Screen Length ¹ (ft)	Field Identified Screened Material	Slug Type	Analysis Method	Falling Head (Slug In) Hydraulic Conductivity (cm/s)	Rising Head (Slug Out) Hydraulic Conductivity (cm/s)	Minimum Hydraulic Conductivity (cm/s)	Maximum Hydraulic Conductivity (cm/s)	Hydraulic Conductivity Geometric Mean (cm/s)
Uppermost Aquifer											
G301	D	604.31	4.65	(ML)s	solid	Kansas Geological Survey	1.1E-03	1.2E-03	2.6E-04	9.1E-03	2.0E-03
G303	D	599.07	10	CL	solid	Kansas Geological Survey	2.8E-04	2.6E-04			
G308	D	606.70	4.79	s(ML), s(CL), (CL)s	solid	Kansas Geological Survey	5.5E-03	1.6E-03			
G309	D	605.02	4.78	SP, s(CL), (ML)s	solid	Kansas Geological Survey	9.1E-03	8.8E-04			
G310	D	604.86	4.79	SM, s(ML)	solid	Kansas Geological Survey	7.5E-03	5.9E-03			
G311	D	604.28	4.77	s(ML), s(CL)	solid	Bouwer-Rice	1.5E-03	--			
G312	D	602.34	4.79	s(ML), s(CL)	solid	Kansas Geological Survey	1.1E-03	1.1E-03			
G313	D	600.40	4.81	SP, s(ML), (CL)s	solid	Kansas Geological Survey	2.7E-03	3.5E-03			
G315	D	606.46	4.79	s(CL)	solid	Kansas Geological Survey	6.6E-03	5.8E-03			
Lower Confining Unit											
G307D	D	563.76	9.77	(CL)s	solid	Kansas Geological Survey	3.2E-04	1.2E-04	1.2E-04	2.3E-03	5.0E-04
G311D	D	558.29	9.94	CL	solid	Kansas Geological Survey	3.8E-04	2.1E-04			
G316	D	584.82	4.80	SP, s(ML), (CL)s	solid	Kansas Geological Survey	2.3E-03	2.3E-03			
Deep Aquifer (PMP)											
G314D	D	561.76	9.77	SP, s(CL)	solid	Bouwer-Rice	3.3E-04	2.3E-05	2.3E-05	3.3E-04	8.7E-05

[O: KLT 07/09/21; C:EDP 8/31/21]

Notes:

- 1. All wells are constructed from 2 inch PVC with 0.01 inch slotted screens.
- = Test not analyzed/performed
- cm/s = centimeters per second
- D = downgradient
- ft = foot/feet
- NAVD88 = North American Vertical Datum of 1988
- PMP= potential migration pathway
- PVC = polyvinyl chloride

USCS = Unified Soil Classification System

- CL = Lean Clay
- s(CL) = Sandy Lean Clay
- (CL)s = Lean Clay with Sand
- s(ML) = Sandy Silt
- (ML)s = Silt with Sand
- SP = Poorly-Graded Sand

FIELD HYDRAULIC CONDUCTIVITIES

HYDROGEOLOGIC SITE CHARACTERIZATION REPORT

COFFEEN POWER PLANT

GMF RECYCLE POND

COFFEEN, ILLINOIS

Well ID	Gradient Position	Bottom of Screen Elevation (ft NAVD88)	Screen Length ¹ (ft)	Field Identified Screened Material	Slug Type	Analysis Method	Falling Head (Slug In) Hydraulic Conductivity (cm/s)	Rising Head (Slug Out) Hydraulic Conductivity (cm/s)	Minimum Hydraulic Conductivity (cm/s)	Maximum Hydraulic Conductivity (cm/s)	Hydraulic Conductivity Geometric Mean (cm/s)
Uppermost Aquifer											
G272	D	606.74	4.87	SP to ML, (CL)s	solid	Kansas Geological Survey	1.7E-03	- -	7.8E-04	1.7E-03	1.1E-03
G284	D	602.48	4.77	ML	solid	Kansas Geological Survey	1.2E-03	7.8E-04			
G286	D	601.81	4.79	SP, ML, CL	solid	Kansas Geological Survey	1.2E-03	- -			
G287	D	604.09	4.82	SP, ML, CL	solid	Kansas Geological Survey	1.1E-03	1.1E-03			
Lower Confining Unit (PMP)											
G283	D	590.13	9.78	SP, ML	solid	Kansas Geological Survey	4.2E-03	4.5E-03	2.7E-04	4.5E-03	1.2E-03
G285	D	587.09	9.77	CL	solid	Bouwer-Rice	2.7E-04	4.3E-04			

[O: KLT, C:EDP 8/31/21]

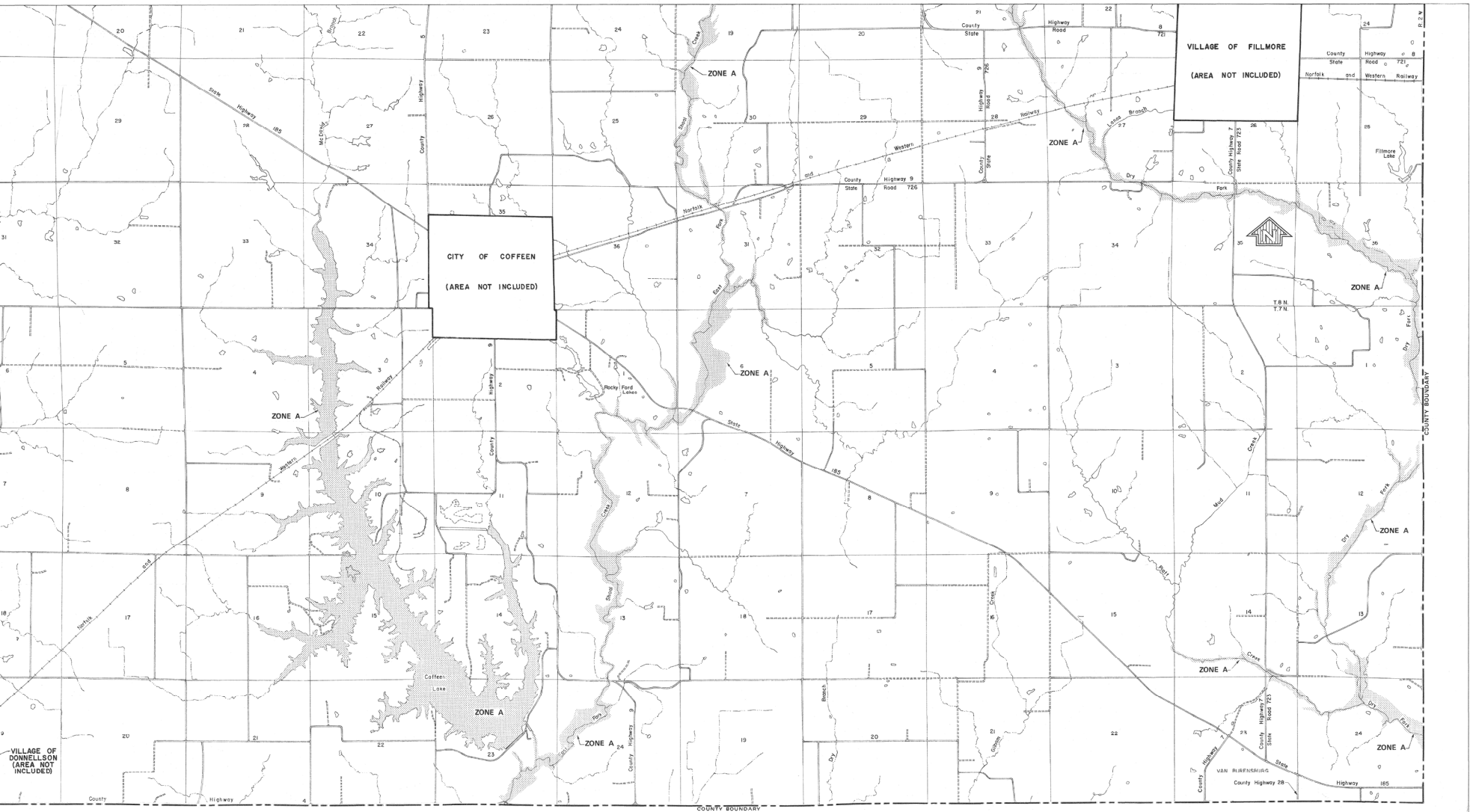
Notes:

- 1. All wells are constructed from 2 inch PVC with 0.01 inch slotted screens.
- - = Test not analyzed/performed
- cm/s = centimeters per second
- D = downgradient
- ft = foot/feet
- NAVD88 = North American Vertical Datum of 1988
- PVC = polyvinyl chloride
- PMP = potential migration pathway

USCS = Unified Soil Classification System

- CL = Lean Clay
- (CL)s = Lean Clay with Sand
- ML = Silt
- SP = Poorly-Graded Sand

APPENDIX G
FEMA FLOOD HAZARD MAP



NATIONAL FLOOD INSURANCE PROGRAM

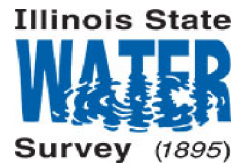
FHBM
FLOOD HAZARD MAP

MONTGOMERY COUNTY, ILLINOIS
UNINCORPORATED AREAS

PANEL 9 OF 10
(SEE MAP INDEX)

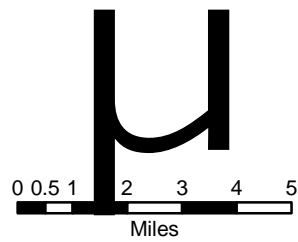
COMMUNITY

 federal emergency management agency
federal insurance



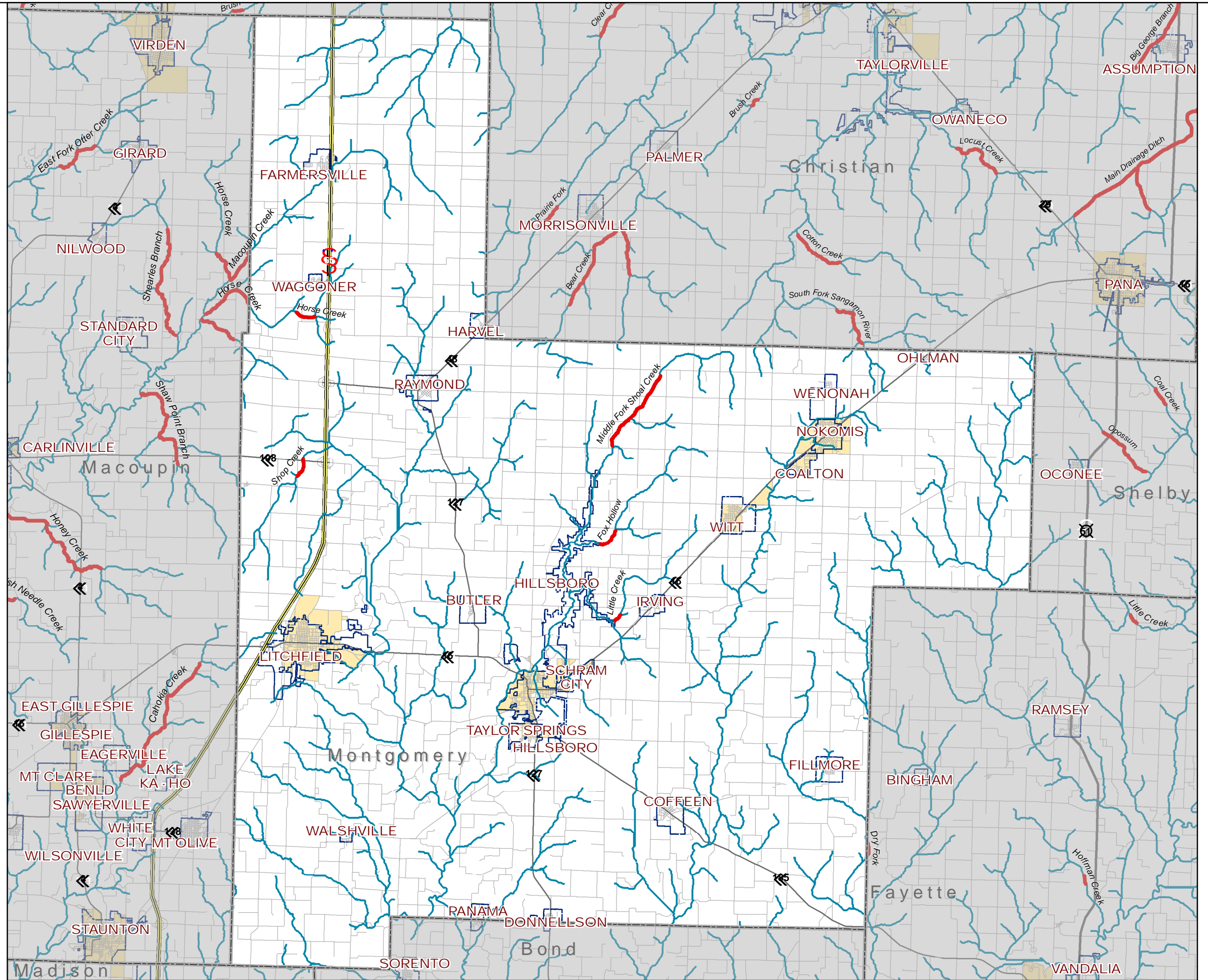
This map illustrates stream reaches at risk for potential flooding that were not designated as Special Flood Hazard Areas (SFHAs) and were not shown on Federal Emergency Management Agency (FEMA) Flood Insurance Rate Maps as of July 1, 2007. Also shown are areas of Risk Class A (high), B (medium), and C (low) adopted by FEMA Region 5. Streams lacking a SFHA designation and draining more than one square mile in a Risk Class A area or draining more than 10 square miles in a Risk Class B or C area are shown with red lines.

A table associated with this map lists streams with unmapped potential flood risk, their names, and the number of miles not within a Special Flood Hazard Area.



Legend

- Streams without SFHAs
 - Streams
 - Municipal Boundaries
 - County Boundaries
 - Interstate Hwy
 - US Highways
 - State Routes
 - Roads
- Risk Classes**
- A - High Risk
 - B - Medium Risk
 - C - Low Risk



Streams Classified by Risk: Montgomery County, Illinois

APPENDIX E

Closure Priority Categorization



Phil Morris
Illinois Power Generating Company
Luminant
1500 Eastport Plaza Drive
Collinsville, IL 62234

May 19, 2021

Mr. Darin LeCrone, P.E.
Manager, Industrial Unit
Bureau of Water, Division of Water Pollution Control, Permits Section
Illinois Environmental Protection Agency
1021 North Grand Avenue, East
Springfield, IL 62794-9276

Re: CCR Surface Impoundment Category Designation and Justification for Illinois Power Generating Company

Dear Mr. LeCrone:

Pursuant to 35 I.A.C. 845.700(c), Illinois Power Generating Company submits the information necessary to categorize the CCR surface impoundments located at the Newton Power Plant and the now retired Coffeen Power Plant. The following parameters were used in assessing and justifying each assigned category.

- **Category 1 – Impacts to existing potable water supply well or impacts to groundwater quality within the setback of an existing potable water supply well.**
 - This review includes an assessment of potable water wells within 2,500 feet of CCR surface impoundments to determine whether any potential impacts are occurring within the setback zone of any community water supply well established under the Illinois Groundwater Protection Act.
 - This information was developed during the Part 845 rulemaking and is summarized in Attachment 1, Table 2: Impacts to Potable Water Supply.
- **Category 2 – Imminent threat to human health or the environment or have been designated by IEPA under (g)(5)**
 - The surface impoundments at Newton and Coffeen Power Plants do not pose an imminent threat to human health or the environment. There are no known conditions at or around the facility where someone or something may be exposed to contaminant concentrations reasonably expected to cause harm
- **Category 3 – Located in areas of environmental justice (“EJ”) concern**
 - EJ areas were evaluated using the EJ mapping link from IEPA’s webpage located at <https://www2.illinois.gov/epa/topics/environmental-justice>. Per the IEPA mapping tool, the EJ Status thresholds were determined as twice the state averages for Minority and Low Income consistent with 35 IAC 845.700(g)(6).
 - An EJ map denoting the facilities with impoundments is located in Attachment 2.

- **Category 4-7**
 - Category 4 - Inactive CCR surface impoundments that have an exceedance of the groundwater protection standards in Section 845.600
 - Category 5 - Existing CCR surface impoundments that have exceedances of the groundwater protection standards in Section 845.600
 - Category 6 - Inactive CCR surface impoundments that are in compliance with the groundwater protection standards in Section 845.600.
 - Category 7 – Existing CCR surface impoundments that are in compliance with the groundwater protection standards in Section 845.600

Based on the information above, category designations have been assigned. The category designations for each CCR impoundment are shown in Attachment 1, Table 1: Category Designations.

If you have any questions regarding this submittal, please contact Phil Morris at 618-343-7794 or phil.morris@vistracorp.com.

Sincerely,

A handwritten signature in black ink, appearing to read 'Phil Morris', written in a cursive style.

Phil Morris
Senior Environmental Director

Attachments

Attachment 1

Table 1: Category Designation

Facility	Pond Description	Classifications	Potable Water Supply Impacts (Category 1)	Human Health or Environment Threat (Category 2)	Located within Environmental Justice Areas ¹ (Category 3)	Standards Exceedances ² (Categories 4,5,6,7)	Impoundment Category 845.700(g)
Coffeen	Ash Pond 1	Inactive	No	No	No	Yes	5
	GMF Pond	Inactive	No	No	No	Yes	5
	GMF Recycle Pond	Inactive	No	No	No	Yes	5
Newton	Primary Ash Pond	Existing	No	No	No	Yes	5

¹ See Attachment 2 Environmental Justice Area Map

² Ground water analyses for purposes of categories 4-7, assumptions have been made based on current groundwater data. However, since sampling and analysis is ongoing and subject to IEPA review and approval, IPGC reserves the right to update its category designations for Categories 4-7.

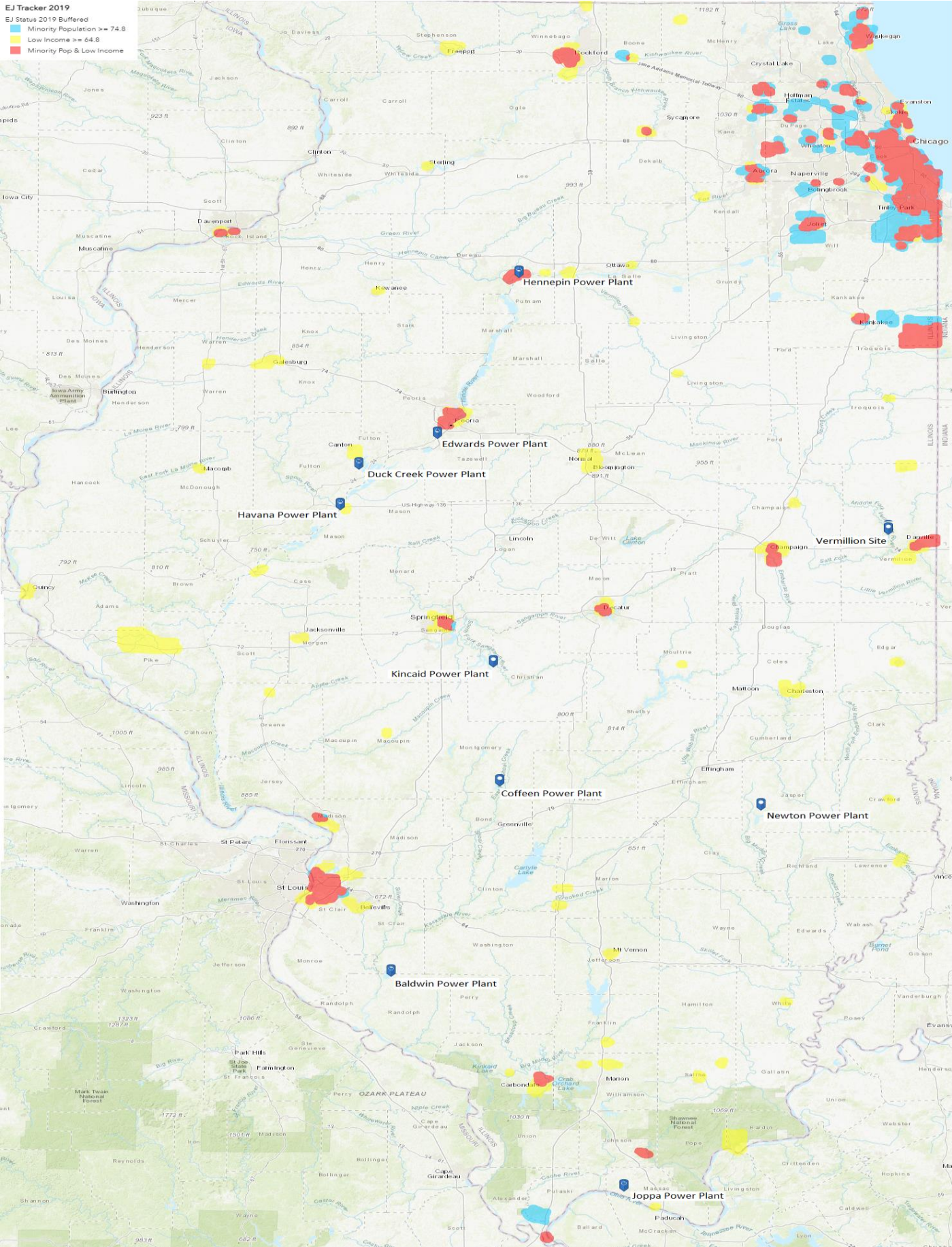
Table 2: Impacts to Potable Water Supply¹

Site Name	Private and Semi-Private Wells	Non-Community Water Supply (CWS) Wells	Non-CWS Surface Water Intakes	Community Water Supply Wells	CWS Surface Water Intakes
Coffeen	Present, but not at risk Thirty-four (34) water wells were identified; however, they are unlikely to be at risk because of their hydrogeologic location relative to the power plant, they are abandoned, or they do not appear to be used for potable purposes. None of the off-site wells are located in a downgradient direction.	Present, but not at risk Three (3) non-CWS wells were identified; however, they are unlikely to be at risk because of their hydrogeologic location relative to the power plant and/or their inactive status.	Absent	Absent	Absent
Newton	Present, but not at risk Twenty-four (24) water wells were identified; however, they are unlikely to be at risk because of their hydrogeologic location relative to the power plant, they are abandoned, and/or they are unlikely to be present based on the mapped location. None of the offsite wells are located in a downgradient direction.	Absent	Absent	Absent	Absent

¹ Ramboll, WELL/WATER SUPPLY SURVEY AND EVALUATION COAL-FIRED POWER PLANTS IN ILLINOIS (September 24, 2020), filed with the Illinois Pollution Control Board in R2020-019.

Attachment 2: EJ Mapping Denoting Facilities with Impoundments

EJ Tracker 2019
EJ Status 2019 Buffered
Minority Population ≥ 74.8
Low Income ≥ 64.8
Minority Pop & Low Income



APPENDIX F

Groundwater Modeling

Intended for
Illinois Power Generating Company

Date
July 28, 2022

Project No.
1940101010-008

GROUNDWATER MODELING REPORT

GMF GYPSUM STACK POND AND GMF RECYCLE POND

COFFEEN POWER PLANT

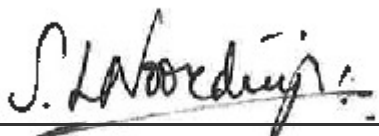
COFFEEN, ILLINOIS

GROUNDWATER MODELING REPORT COFFEEN POWER PLANT GMF GYPSUM STACK POND AND GMF RECYCLE POND

Project name **Coffeen Power Plant GMF Gypsum Stack Pond and GMF Recycle Pond**
Project no. **1940101010-008**
Recipient **Illinois Power Generating Company**
Document type **Groundwater Model Report**
Revision **FINAL**
Date **July 28, 2022**

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Consultant



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APPENDICES

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Appendix C	HELP Model Output Files

ACRONYMS AND ABBREVIATIONS

§	Section
35 I.A.C.	Title 35 of the Illinois Administrative Code
AP1	Ash Pond No. 1
AP2	Ash Pond No. 2
bgs	below ground surface
CBR	closure by removal
CCR	coal combustion residual(s)
CIP	closure in place
cm/s	centimeter per second
CPP	Coffeen Power Plant
CSM	conceptual site model
DA	deep aquifer
DCU	deep confining unit
DEM	Digital Elevation Model
ft ²	square feet
ft/d	feet per day
ft/ft	feet per foot
Geosyntec	Geosyntec Consultants, Inc.
GHB	general head boundary conditions
GMF GSP	Gypsum Management Facility Gypsum Stack Pond
GMF RP	Gypsum Management Facility Recycle Pond
GMP	Groundwater Monitoring Plan
GMR	Groundwater Modeling Report
Golder	Golder Associates
GWPS	groundwater protection standard(s)
Hanson	Hanson Professional Services, Inc.
HCR	Hydrogeologic Site Characterization Report
HDPE	high density polyethylene
HELP	Hydrologic Evaluation of Landfill Performance
HFB	hydraulic flow boundary
HUC	Hydrologic Unit Code
ID	identification
IEPA	Illinois Environmental Protection Agency
in/yr	inches per year
IPGC	Illinois Power Generating Company - IPGC
ISGS	Illinois State Geological Survey
Kd	distribution coefficient
Kh/Kv	anisotropy ratio
LCU	lower confining unit
LF	Landfill
m	meter
mg/L	milligrams per liter
mil	One thousandth of an inch
mL/g	milliliters per gram
NAVD88	North American Vertical Datum of 1988

NID	National Inventory of Dams
No.	number
NPDES	National Pollutant Discharge Elimination System
NRT	Natural Resource Technology, Inc.
Part 845	35 I.A.C. § 845: Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments
R2	correlation coefficient
Ramboll	Ramboll Americas Engineering Solutions, Inc.
SI	surface impoundment(s)
SSR	sum of squared residuals
TDS	total dissolved solids
TR	transient model
TVD	total-variation-diminishing
UA	uppermost aquifer
UCU	upper confining unit
USDA/NRCS	United States Department of Agriculture/Natural Resources Conservation Service
USEPA	United States Environmental Protection Agency
USGS	United States Geological Survey

EXECUTIVE SUMMARY

Ramboll Americas Engineering Solutions, Inc. (Ramboll) has prepared this Groundwater Modeling Report (GMR) on behalf of the Coffeen Power Plant (CPP), operated by Illinois Power Generating Company - IPGC (IPGC), in accordance with requirements of Title 35 of the Illinois Administrative Code (35 I.A.C.) Section (§) 845: Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments (Part 845) (Illinois Environmental Protection Agency [IEPA], 2021). This document presents the results of predictive groundwater modeling simulations for proposed closure scenarios for the coal combustion residuals (CCR) management units Gypsum Management Facility Gypsum Stack Pond (GMF GSP [Vistra Identification [ID] Number [No.] 103, IEPA ID No. W1350150004-03, and National Inventory of Dams [NID] No. IL50579]) and Gypsum Management Facility Recycle Pond (GMF RP [(Vistra ID No. 104, IEPA ID No. W1350150004-04, and NID No. IL50578])). The GMF GSP is a 77-acre, lined surface impoundment (SI), and the GMF RP is a 17-acre, lined SI, both of which are used to manage CCR waste streams at the CPP.

The CPP is located in Montgomery County, in central Illinois between the two lobes of Coffeen Lake (**Figure 1-1**), which was formed in 1963 by damming the McDavid Branch of the East Fork of Shoal Creek. Coffeen Lake encompasses approximately 1,100 acres and was created to provide a source of cooling water for the CPP. Coffeen Lake borders the CPP to the west, east, and south, and agricultural land is located to the north. Historically coal mines were operated at depth below the site. Mine shafts, processing facilities, and historic coal storage were located on the southern extent of the CPP, south of Coffeen Ash Pond Number No. 1 (AP1). The CPP operated as a coal-fired power plant from 1964 until November 2019 and has five CCR management units, with the GMF GSP and GMF RP being the subject of this GMR. Unlithified material present above the bedrock in the vicinity of the CPP was categorized into hydrostratigraphic units as part of the 2021 Hydrogeologic Site Characterization Reports (HCRs; Ramboll, 2021a; Ramboll, 2021b). In addition to the CCR, the hydrostratigraphic units occur in the following order (from ground surface downward) and include:

- **Upper Confining Unit (UCU):** Consists of the Loess Unit and the upper clayey portion of the Hagarstown Member which has generally lower vertical permeability. Construction of the GMF GSP and GMF RP required the excavation and removal of this layer within each unit's footprint and the UCU has been eroded east of the GMF GSP and GMF RP, near the Unnamed Tributary.
- **Uppermost Aquifer (UA):** The UA is the sandy portion of the Hagarstown Member which is classified as primarily sandy to gravelly silts and clays with thin beds of sands. Similar to the Loess Unit, the Hagarstown Member was excavated in some areas to facilitate construction of the GMF GSP and GMF RP and the Hagarstown Member is also absent in some locations near the Unnamed Tributary.
- **Lower Confining Unit (LCU):** Comprised of the Vandalia Member, Mulberry Grove Member, and Smithboro Member. These units include a sandy to silty till with thin, discontinuous sand lenses, a discontinuous and limited extent sandy silt which has infilled prior erosional features, and silty to clayey diamicton, respectively.
- **Deep Aquifer (DA):** Sand and sandy silt/clay units of the Yarmouth Soil, which include accretionary deposits of fine sediment and organic materials, typically less than five feet thick and discontinuous across the CPP.

- **Deep Confining Unit (DCU):** Comprised of the Banner Formation and generally consists of clays, silts, and sands. The Lierle Clay Member is the upper layer of the Banner Formation which was encountered at the CPP.

Flow of groundwater from central portions of the CPP to Coffeen Lake or the Unnamed Tributary through the UA are the primary pathways for contaminant migration. Groundwater elevations are primarily controlled by surface topography, geologic unit topography, and water levels within Coffeen Lake and the Unnamed Tributary. A groundwater divide trending north-south is observed running through the approximate center of the CPP. Phreatic surfaces or water elevations within the SIs are generally consistent and have not been observed to fluctuate with groundwater elevations, indicating limited hydraulic connection with the SIs.

The conceptual site model (CSM) for modeling the groundwater at the CPP is as follows:

- Most hydrostratigraphic layers are laterally continuous across the area. The flat to gently rolling uplands are dissected by deeply incised streams (into the materials of the UCU, UA, and LCU) that are tributaries to river systems in the area. Coffeen Lake was created by damming one of these tributary streams for use by the CPP.
- The GMF GSP and GMF RP are lined SIs which sit within the UCU and UA. The low permeability liner acts as a barrier to groundwater flow and transport.
- Surface recharge and groundwater migrate vertically through the low permeability sediments of the UCU. Groundwater migrates horizontally through the higher permeability sediments of the UA.
- Groundwater elevations and lake elevations indicates groundwater flows into Coffeen Lake from the UA.

A review and summary of data collected from 2015 through 2021 for parameters with groundwater protection standards (GWPS) listed in 35 I.A.C. § 845.600 is provided in the HCRs (Ramboll, 2021a; Ramboll, 2021b). Concentration results presented in the HCRs and summarized in the History of Potential Exceedances (Ramboll, 2021c; Ramboll, 2021d) are considered potential exceedances because the methodology used to determine them is proposed in the Statistical Analysis Plan (Appendix A to the Groundwater Monitoring Plan [GMP], Ramboll, 2021e; Ramboll, 2021f), which has not been reviewed or approved by IEPA at the time of submittal of the Part 845 operating permit application. The following constituents with potential exceedances of the GWPS listed in 35 I.A.C. § 845.600 were identified: boron, sulfate, and total dissolved solids (TDS) (Ramboll, 2021d) at GMF RP; none were identified at the GMF GSP.

Statistically significant correlations between sulfate concentrations and concentrations of boron and TDS identified as potential exceedances of the GWPS indicate sulfate is an acceptable surrogate for these parameters in the groundwater model. Concentrations of these parameters are expected to change along with model predicted sulfate concentrations.

It was assumed that sulfate would not significantly sorb or chemically react with aquifer solids (distribution coefficient [Kd] was set to 0 milliliters per gram [mL/g]) which is a conservative estimate for predicting contaminant transport times in the model. Boron, sulfate, and TDS transport is likely to be affected by both chemical and physical attenuation mechanisms (*i.e.*, adsorption and/or precipitation reactions as well as dilution and dispersion).

All available hydrological information were used to construct a CSM and numerical model of the CPP. A steady state, 5-layer numerical model, based on a previous groundwater model of the area, was constructed to characterize the long-term groundwater flow conditions at the site. The hydrostratigraphic units included in the model were the UCU, UA, and LCU. The DA and DCU were not included in the model. Calibration of the model focused on simulating mean groundwater elevations for 95 wells at the site by modifying hydraulic parameters for the different hydrostratigraphic units, alongside river and general head boundary conductance. The calibrated model represents a reasonable match to the observed head and sulfate concentration data.

The calibrated model was used to predict the sulfate concentration for two closure scenarios using information provided in the Final Closure Plans (Golder Associates [Golder], 2022a and 2022b) including:

- **Scenario 1:** closure in place (CIP) including removal of CCR from the GMF RP and the southern portion of the GSP, consolidation into the northern portion of the GSP, and construction of a cover system over the remaining CCR, and;
- **Scenario 2:** closure by removal (CBR) including removal of all CCR and SI liner and regrading of the removal area for both GMF GSP and GMF RP.

Prior to the simulation of these scenarios, a dewatering simulation was included which simulated the removal of free liquids from the GMF GSP and GMF RP prior to the implementation of the two closure scenarios.

The existing liner system (modeled with hydraulic conductivity of 1.02×10^{-11} cm/sec) physically isolates the CCR stored in the GMF GSP consolidation area from the surrounding soils and groundwater, minimizing water flux through the CCR material. Removal of free liquids, consolidation, and placement of the final cover system will minimize the volume of water within the facility, further reducing the potential for water flux through the CCR material.

There are limited differences in the timeframes to reach the GWPS for most monitoring wells at the GMF GSP and GMF RP between CIP and CBR. In general, the simulated groundwater concentrations in the monitoring wells within the UA will achieve the GWPS in 7 years for both the CIP and CBR closure scenarios at the GMF GSP. For the GMF RP, the simulated groundwater concentrations in the monitoring wells within the UA will achieve the GWPS in 4.6 years for both the CIP and CBR closure scenarios.

Results of groundwater fate and transport modeling conservatively estimate that groundwater concentrations will attain the GWPS for all constituents identified as potential exceedances of the GWPS in the UA monitoring wells within 7 years of closure implementation for both CIP and CBR. The residual sulfate plumes from the calibrated model associated with both the GMF GSP and GMF RP remain in close proximity to the CCR units and are simulated to decline below the GWPS (400 mg/L) in all layers of the model 13 and 14 years after closure, respectively, for CIP and CBR.

1. INTRODUCTION

1.1 Overview

In accordance with the requirements of Part 845 (IEPA, 2021), Ramboll has prepared this GMR on behalf of the CPP, operated by IPGC. This report will apply specifically to the CCR units referred to as the GMF GSP and the GMF RP (**Figure 1-1**). However, information gathered to evaluate other CCR units at CPP regarding geology, hydrogeology, and groundwater quality is included, where appropriate. The GMF GSP is a 77-acre, lined SI, and the GMF RP is a 17-acre, lined SI, both of which are used to manage CCR waste streams at the CPP. This GMR presents and evaluates the results of predictive groundwater modeling simulations for two proposed closure scenarios, including CCR consolidation and CIP, and CBR scenarios summarized below:

- **Scenario 1:** CIP including removal of CCR from the GMF RP and the southern portion of the GMF GSP, consolidation into the northern portion of the GMF GSP, and construction of a cover system over the remaining CCR.
- **Scenario 2:** CBR including removal of all CCR and SI liner system and regrading of the removal area.

1.2 Previous Groundwater Modeling Reports

Several reports containing groundwater modeling have been completed at the CPP. The information presented in this GMR includes data collected in support of the previous groundwater models as well as data collected as part of 2021 field investigations to support development of an HCR (Ramboll, 2021a; Ramboll, 2021b). The HCRs were provided as an attachment to the initial operating permit application required by 35 I.A.C. § 845.230. Previous groundwater modeling reports completed for the various CCR units located at the CPP include, but are not limited to, the following:

- **Natural Resources Technology, Inc. (NRT), January 24, 2017. Hydrostatic Modeling Report. Coffeen Power Station, Coffeen, Illinois.**
Utilized the Hydrologic Evaluation of Landfill Performance (HELP) model to predict percolation from Coffeen Ash Pond No. 2 (AP2) and evaluate AP2 hydrostatic conditions in response to the proposed cover system as described in the Revised 30% Closure Design Package.
- **NRT, January 24, 2017. Groundwater Modeling Report. Coffeen Power Station, Coffeen, Illinois.**
Included simulations of the site hydrology, the extent of CCR leachate impacts on groundwater, and the effect of pond closure on groundwater quality.

1.3 Site Location and Background

The CPP is located in Montgomery County, in central Illinois, within Section 11 Township 7 North and Range 7 East (**Figure 1-1**). The CPP is approximately two miles south of the city of Coffeen and about eight miles southeast of the city of Hillsboro, Illinois. The GMF GSP and GMF RP are located between the two lobes of Coffeen Lake (identified as "Coffeen Lake" and "Unnamed Tributary" on **Figure 1-1** and **Figure 1-2**) to the west, east, and south, and is bordered by agricultural land to the north. The approximately 1,100-acre Coffeen Lake was built by damming the McDavid Branch of the East Fork of Shoal Creek in 1963 for use as an artificial cooling lake for the CPP. Historically, several coal mines were operated at depth in the vicinity of the CPP as

well as the US Minerals processing facility located to the north. **Figure 1-2** is a site map showing the location of AP1, AP2, GMF GSP and GMF RP (Part 845 regulated CCR Units and subject of this GMR), and Coffeen Landfill (LF). A surface water pond southwest of the LF collects overflow from the LF, this feature does not contain CCR. The areas near the GMF GSP and GMF RP will hereinafter be referred to as the Site.

1.4 Site History and CCR Units

The CPP was a coal-fired electrical generating plant that began operation in 1964. The plant initially burned bituminous coal from Illinois and CCR from the coal fired units was disposed of in AP1. AP2 was also utilized in the early 1970's and AP1 was reconstructed in 1978. Both of these units were used until the mid-1980's. Beginning in 2010 CCR material was placed in the LF and GMF Units (*i.e.*, GMF GSP and GMF RP). All approximate dates of construction of each successive stage of the CCR Units at the CPP are included in the groundwater model and described here.

AP1: This SI (also known as the Bottom Ash/Recycle Pond) is a reclaimed ash pond that was reconstructed utilizing the existing earthen berms with reinforcement, as provided by Water Pollution Control Permit 1978-EA-389 issued by the IEPA on May 26, 1978. AP1 (existing unlined SI) covers an area of approximately 23 acres, has berms up to 41 feet above the surrounding land surface, and a volume of 300 acre-feet. It primarily received bottom ash and low volume wastes from floor drains in the main power block building. Several years ago, air heater wash and boiler chemical cleaning wastes were directed to AP1, but this practice was discontinued. The bottom ash was periodically removed for beneficial uses by a third-party contractor. Sluicing of waste to AP1 ceased prior to November 4, 2019.

AP2: AP2 is a closed (IEPA approved) SI with a surface area of approximately 60 acres and berms 47 feet higher than the surrounding land surface. AP2 was originally removed from service and capped in the mid 1980's. A clay and soil cap was placed on the surface of the pond with contouring and drainage provided to direct storm water to four engineered revetment down drain structures. Prior to capping, this pond was identified as Outfall 004 in the facility National Pollutant Discharge Elimination System (NPDES) operating permit, IL0000108. Additional closure activities include the construction of a geomembrane cover system that began in July 2019 and was completed on November 17, 2020. The construction was completed in accordance with the Closure and Post Closure Care Plan approved by the IEPA on January 30, 2018.

GMF GSP: The 77-acre GMF GSP received blowdown from the air emission scrubbers and was put into operation in 2010. Construction of the GMF GSP was in accordance with Water Pollution Control Permit 2008-EA-4661 and features a composite 60-one thousandths of an inch (mil) high-density polyethylene (HDPE) liner with 3 feet of recompacted soil with a hydraulic conductivity of 1×10^{-7} centimeters per second (cm/s) with internal piping and drains to collect contact water. Construction of the unit required excavation to approximately 603 feet North American Vertical Datum of 1988 (NAVD88), removal of the sands and silts of the UA prior to construction of the liner, and installation of a groundwater underdrain system to eliminate inward pressure on the liner prior to placement of CCR. The GMF GSP underdrain was actively pumped during construction but is no longer actively pumped. IPGC ceased receipt of waste to the GMF GSP prior to April 11, 2021.

GMF RP: The 17-acre GMF RP received blowdown from the air emission scrubbers and was put into operation in 2010. Construction of the GMF RP was in accordance with Water Pollution Control Permit 2008-EA-4661 and features a composite 60-mil HDPE liner with 3 feet of

recompacted soil with a hydraulic conductivity of 1×10^{-7} cm/s with internal piping and drains to collect contact water. Construction of the unit required excavation to approximately 601 feet NAVD88, removal of the sands and silts of the UA prior to construction of the liner, and installation of a groundwater underdrain system to eliminate inward pressure on the liner prior to placement of CCR. The GMF RP underdrain is a passive, gravity drained system. IPGC ceased receipt of waste to the GMF RP prior to April 11, 2021.

LF: Fly ash was managed in a permitted composite lined landfill constructed in 2010. The LF has an active groundwater underdrain system that is currently being pumped. Additionally, the ash landfill leachate collection system is restricted by rule to no more than one foot of leachate on the composite liner. An IEPA groundwater monitoring program is in effect for the GMF GSP and GMF RP (under Bureau of Water), and LF (under Bureau of Land).

2. SITE GEOLOGY AND HYDROGEOLOGY

2.1 Stratigraphy

The geology and hydrogeology of the GMF GSP and GMF RP are described in detail in the HCRs (Ramboll, 2021a; Ramboll, 2021b) and summarized below.

The unlithified stratigraphy within and immediately surrounding the GMF GSP and GMF RP consists of the following in descending order: fill material and CCR; clays and silts (Loess Unit); gravelly clay till and sandy materials, absent in some locations (Hagarstown Member); a weathered till zone and sandy, silt, or clay till (Vandalia Member); silt and sandy silt/clay unit (Mulberry Grove Member); silty clay diamicton (Smithboro Member); sand and sandy silt/clay, absent in some locations (Yarmouth Soil); and clay and silt with some sand (Lierle Clay Member). The unlithified units overlay Pennsylvanian-age limestone, sandstone, and minor coal beds (Bond Formation). The Bond Formation bedrock was not encountered in any borings advanced at the CPP, so site-specific information is not available.

CCR consisting of gypsum, gypsum scrubber waste, and other non-CCR wastes are present within the lined GMF GSP and GMF RP. Borings were not advanced during the 2021 investigation in the GMF GSP or GMF RP due to safety concerns. Fill and CCR are estimated to be a maximum of 17 feet thick at the northern extent of the GMF GSP and a maximum of 13 feet thick in the western extent of the GMF RP as estimated from topography and the elevation of the base of the liner from available construction details (Ramboll, 2021a; Ramboll, 2021b; Hanson Professional Services, Inc. [Hanson], 2009). Non-CCR fill material consisting of silt, clay, and sand comprises the berms surrounding the GMF GSP and GMF RP.

The Loess Unit is the uppermost unlithified unit identified at the CPP. This unit is comprised of the combined Roxana and Peoria Silt and extends from beneath the topsoil, derived from the loess, to the top of the Hagarstown Member. The loess has been classified as silt or clayey silt, with minor amounts of sand. The Loess Unit ranges in thickness from 0 feet (absent) to 16 feet across the CPP, and was generally 3 to 14 feet thick, where present near the GMF GSP and GMF RP. The Loess Unit is generally considered unsaturated, and the UA is recharged by precipitation that percolates through this unit.

The Hagarstown Member (also referred to as Hagarstown Beds) exhibits two units: the first unit consisting of the gravelly clay till and the second consisting of sandy material overlying the Vandalia Member. The clay till portion had varying thicknesses ranging from approximately 2 to 6 feet as observed adjacent to the GMF GSP and GMF RP (Ramboll, 2021a; Ramboll, 2021b). The sandy portion of the Hagarstown, where present, was typically encountered between 6 and 25 feet below ground surface (bgs) near the GMF GSP and GMF RP, and is generally 1 to 4 feet thick, although thicknesses up to 7 feet have been observed north of the LF. The composition of the sandy portion of the Hagarstown unit varies across the CPP and has been classified as gravelly till, poorly sorted gravel, well sorted gravel, sand, and silty sand. Based on historic topography, the Hagarstown Member is not present in former drainage features present along the banks of Coffeen Lake and the Unnamed Tributary. During construction of the LF, GMF GSP, and the GMF RP, the Loess Unit and portions of the Hagarstown Member were excavated to facilitate construction.

The Vandalia (*i.e.*, till) Member is a sandy/silty till with thin, discontinuous lenses of silt, sand, and gravel. The Vandalia Member was encountered between 1.5 and 34 feet bgs in all borings advanced at the CPP. The Vandalia Member typically ranged in thickness from 11.7 feet in the northern portion of the CPP, to 31.0 feet between the GMF GSP and the GMF RP. Similar to the observed top elevation of the Hagarstown Member, the top of the Vandalia Member declines in elevation near Coffeen Lake and topographic drainage features. This unit is relatively thick throughout the CPP, with an average thickness of over 15 feet (Hanson, 2009).

The Mulberry Grove (*i.e.*, silt) Member typically consists of a thin, lenticular unit of gray sandy silt (Willman et al., 1975). It represents the interval between the retreat of the glacier that deposited the Smithboro Member and the advance of the glacier that deposited the Vandalia Member. At the CPP, the Mulberry Grove Member is represented by gray sandy silt layers deposited in depressions found in the surface of the underlying Smithboro Member. This unit was absent in many borings through the central portion of the CPP from south to north, and is generally less than 2 feet thick, but was measured at up to 4.9 feet thick near the GMF GSP (Hanson, 2009).

The Smithboro (*i.e.*, till) Member is described as a gray, compact, silty, clayey diamicton. The Smithboro Member ranges in thickness from 6.7 to 21.2 feet northwest of the LF.

The Yarmouth Soil is described as the weathered zone on the Kansan drift, but in some places, it consists of accretionary deposits of fine sediment and organic material that accumulated in poorly drained areas on the surface of the Kansan deposits. Historical borings in the northern portion of the CPP which encountered the Yarmouth were summarized previously by Hanson (2009) as ranging in thickness from 0 feet (absent) to 5.1 feet.

The Lierle Clay Member is the uppermost member of the Kansan Stage Banner Formation. It is described as an accretion gley with clay, silt, and some sand. It was encountered by Hanson (2009) in all but a few borings on site. During the 2021 investigation, the top of the Lierle Clay was observed between 54 and 57 feet bgs. No borings advanced at the CPP penetrated the full thickness of the Banner Formation.

Pennsylvanian-age Bond Formation bedrock was not encountered in any borings advanced at the CPP, so site-specific information is not available.

2.2 Hydrogeology

Regionally, the water table conforms to the topographic features of the land surface. Recharge occurs in the uplands and flows towards drainage features. Moderate thicknesses of unconsolidated materials fill shallow valleys or are present on the uplands bordering the main valleys. These materials contain thin and discontinuous deposits of sand and gravel.

2.2.1 Groundwater Flow

Monitoring well locations are illustrated in **Figure 2-1**. Monitoring well locations and construction details are summarized in **Table 2-1**. Overall groundwater flow within the UA is divided towards the two lobes of Coffeen Lake. Groundwater generally flows from the center of the CPP west towards Coffeen Lake, and east towards the Unnamed Tributary, the eastern lobe of Coffeen Lake, and the discharge flume, resulting in a groundwater divide (high) running through the middle of the CPP (**Figure 2-2** and **Figure 2-3**). Groundwater near the GMF GSP flows east and south from the groundwater divide present between the LF and the GMF GSP ultimately flowing

toward the Unnamed Tributary. Groundwater near the GMF RP flows from the flow divide east toward the Unnamed Tributary. Although elevations vary seasonally, the groundwater flow direction in the UA is consistent and likely controlled by the proximity and hydraulic connection to Coffeen Lake.

2.2.2 Hydraulic Properties

Over 100 monitoring wells have been installed since 2006 to monitor groundwater conditions around the five CCR units at the CPP for both State and Federal groundwater compliance programs. Six hydrostratigraphic units were described in detail in the HCRs (Ramboll, 2021a; Ramboll, 2021b) and are summarized as follows:

- **CCR:** These units are composed of CCR, consisting primarily of gypsum scrubber waste. This also includes earthen fill deposits of predominantly silt and clay materials from on-site excavations that were used to construct berms and roads surrounding the various impoundments across the CPP. Laboratory testing of one CCR (ash) sample from the GMF GSP had a vertical hydraulic conductivity of 8.9×10^{-4} cm/s. No CCR samples were collected from within the GMF RP.
- **UCU:** Consists of the Loess Unit and the upper clayey portion of the Hagarstown Member which has generally lower vertical permeability and generally greater than 60 percent fines (Ramboll, 2021a; Ramboll, 2021b). This unit was encountered across most of the CPP, with the exception of near the Unnamed Tributary where the unit was eroded following deposition or locations where it has been excavated for construction. Vertical hydraulic conductivities based on laboratory testing ranged from 1.3×10^{-8} to 5.0×10^{-7} cm/s.
- **UA:** This unit consists primarily of sand and sandy silts and clays at the base of the Hagarstown Member and, in some locations, the uppermost weathered sandy clay portion of the Vandalia Member. This unit is absent in several locations due to weathering and in others due to excavation during construction of CCR Units. Field hydraulic conductivity tests indicated hydraulic conductivities ranged from 1.7×10^{-5} to 9.1×10^{-3} cm/s near AP1 (Ramboll, 2021g). Laboratory testing of one UA sample, collected near the GMF RP, had a vertical hydraulic conductivity of 1.6×10^{-4} cm/s. No samples were collected near the GMF GSP.
- **LCU:** This unit is composed of the sandy clay till of the Vandalia Member, the silt of the Mulberry Grove Formation, and the compacted clay till of the Smithboro Member. The unit underlies the UA and was encountered in all boring locations on the CPP. Results from laboratory tests completed for vertical hydraulic conductivity indicate the Vandalia Member has a very low vertical hydraulic conductivity. Field hydraulic conductivity tests indicated hydraulic conductivities from 4.0×10^{-8} to 3.4×10^{-5} cm/s; however, these likely reflect the isolated and discontinuous sandy lenses. Vertical hydraulic conductivities based on laboratory testing were from 1.3×10^{-8} to 5.0×10^{-7} cm/s.
- **DA:** This unit consists primarily of sandy silt and sands of the Yarmouth Soil, which are thin (less than 5 feet) and discontinuous across the CPP. Field hydraulic conductivity tests indicated hydraulic conductivities from 8.7×10^{-5} to 1.7×10^{-3} cm/s within the DA.
- **DCU:** This unit underlies the DA and is composed of the Banner Formation, of which the thick Lierle Clay is the first encountered unit. No boring penetrated the full thickness of this formation.

2.2.3 Groundwater Elevation Data

During the 2021 Part 845 investigation, groundwater elevations in the UA ranged from approximately 591 to 625 feet NAVD88 across the CPP. Groundwater elevations were typically highest towards the northern extent of the CPP, near the GMF GSP and GMF RP, except monitoring well G307 south of AP1, which consistently had the highest groundwater elevation. Groundwater elevations were lowest near the Unnamed Tributary and east of AP1 towards Coffeen Lake. Groundwater elevations in the vicinity of the GMF GSP were typically from 617 to 622 feet NAVD88, and between 601 and 623 feet in the vicinity of the GMF RP (**Figure 2-2** and **Figure 2-3**).

No seasonal variation has been observed in the UA monitoring wells, and any seasonal responses may be muted by the proximity and hydraulic connection to Coffeen Lake.

2.2.4 Mining Activity

Several coal mines, both strip and underground types, previously operated in Montgomery County, Illinois. Three mines - the Hillsboro Mine (Illinois State Geological Survey [ISGS] Mine No. 871), the Clover Leaf No. 4 Mine (ISGS Mine No. 442), and the Clover Leaf No. 1 Mine (ISGS Mine No. 3001) – were operated as room and pillar mines in the vicinity of the site beginning as early as 1889. The mines extracted coal from the Herrin (No. 6) Coal at depths of approximately 500 to 535 feet bgs (ISGS, 2019). All nearby mining operations ceased in 1983.

The Hillsboro Mine showed indications of small-scale faulting, roof stability issues and floor heaving. Mine shafts, processing facilities, and some historic coal storage associated with these historic mines were located south of AP1. The southernmost portion of the GMF GSP and GMF RP fall within the buffer zone of the Hillsboro Mine. The GMF GSP directly overlies the southernmost portion of the Clover Leaf No. 4 Mine and the GMF RP lie within the buffer zone. The GMF GSP and GMF RP are outside of the buffer zone of the Clover Leaf No. 1 mine (Ramboll, 2021a; Ramboll, 2021b).

3. GROUNDWATER QUALITY

3.1 Groundwater Classification

Per 35 I.A.C. § 620.210, groundwater within the UA at the GMF GSP and GMF RP meet the definition of a Class I - Potable Resource Groundwater based on the following criteria:

- Groundwater in the UA is located 10 feet or more below the land surface and
- Within a geologic material which is capable of a hydraulic conductivity of 1×10^{-4} cm/s or greater using a slug test.

Field hydraulic conductivity tests performed in the UA near the GMF GSP and GMF RP in 2021 had geometric means of 1.4×10^{-3} and 1.2×10^{-3} cm/s, respectively (Ramboll, 2021a; Ramboll, 2021b). Based on this information groundwater is classified as Class I – Potable Resource Groundwater.

3.2 Potential Groundwater Exceedances

A review and summary of data collected from 2015 through 2021 for parameters with GWPSs listed in 35 I.A.C. § 845.600 is provided in the HCRs (Ramboll, 2021a; Ramboll, 2021b). Concentration results presented in the HCRs were compared directly to 35 I.A.C. § 845.600 GWPSs to determine potential exceedances. The results are considered potential exceedances because the results were compared directly to the standard and did not include an evaluation of background groundwater quality or utilize the statistical methodologies proposed in the GMPs (Ramboll, 2021e; Ramboll, 2021f) attached to the operating permit application.

Groundwater concentrations from 2015 to 2021 are summarized in the History of Potential Exceedances (Ramboll, 2021c; Ramboll, 2021d) (attached to the operating permit application) and are considered potential exceedances because the methodology used to determine them is proposed in the Statistical Analysis Plan (Appendix A to the GMP, Ramboll, 2021c; Ramboll, 2021d), which has not been reviewed or approved by IEPA at the time of submittal of the Part 845 operating permit application.

The History of Potential Exceedances attached to the operating permit application summarizes all potential groundwater exceedances following the proposed Statistical Analysis Plan. No potential exceedances were present at the GMF GSP. The following potential exceedances were identified for the GMF RP:

- Boron - determined at well G275.
- Sulfate - determined at wells G273, G275, and G285.
- TDS - determined at wells G275 and G285.

Note that monitoring well G285 is located east of the Unnamed Tributary and screened within the LCU. Consequently, exceedances at G285 are not associated with the GMF GSP or GMF RP and are not discussed further in this GMR.

4. GROUNDWATER MODEL

4.1 Overview

Data collected at the Site from 2015 to the 2021 field investigation were used to update an existing groundwater model of the CPP (Natural Resource Technology [NRT], 2017b). The updated model was then used to evaluate the results of predictive groundwater modeling simulations for two proposed closure scenarios, including CCR consolidation and CIP, and CBR. The modeling results are summarized and evaluated in this GMR. The associated model files are included as **Appendix A**.

4.2 Description of Existing Model

The NRT (2017b) contaminant fate and transport model simulated boron and was performed to support closure of AP2 using MODFLOW and MT3DMS. AP1, GMF GSP, GMF RP, and LF were present within the previous model domain.

The NRT (2017b) modeling consisted of the following:

- Steady-state MODFLOW model was developed to represent site conditions for 2016. This model was calibrated to a set of groundwater elevation data collected during November 2016.
- The hydraulic properties from the steady-state model were used in the calibration of the transient MODFLOW and MT3DMS models which simulated groundwater flow and transport at the AP2 from 1970 to 2017. Boron concentrations collected in August 2016 were used to calibrate the transport model.
- Predictive simulations to estimate future boron concentrations for a baseline (no action) and capping closure scenario for AP2 were completed. Closure action was modeled over a period of 1500 years, beginning in January 2018.
- Predicted boron concentrations were simulated to reach compliance for CIP at AP2 after 101 years (NRT, 2017b). These modeling results were part of the closure plan IEPA approved on January 30, 2018.

4.3 Conceptual Model

The HCRs (Ramboll, 2021a; Ramboll, 2021b) form the foundation of the GMF GSP and GMF RP hydrogeological setting. The GMF GSP and GMF RP overlies the recharge area for the underlying geologic media, which are composed of unlithified deposits.

4.3.1 Hydrogeology

As discussed in **Section 2.2**, groundwater flow direction in the UA at the CPP is divided and flows towards the two lobes of Coffeen Lake. The loess of the UCU and sands of the UA are hydraulically connected. The groundwater flow in the silts and clays of the UCU and LCU are expected to be primarily vertical. The Hagarstown member is where the majority of the horizontal migration is expected to occur. The hydrogeological CSM consists of the following layers:

- Hagarstown Loess Unit (*i.e.*, UCU) – Loess unit and the upper clayey portion of the Hagarstown Member.

- Hagarstown Member (*i.e.*, UA) – sand and sandy silts and clays at the base of the Hagarstown Member and, in some locations, the uppermost weathered sandy clay portion of the Vandalia Member.
- Vandalia Member/Mulberry Grove Member (*i.e.*, LCU) – unweathered sandy clay till and discontinuous silts.
- Smithboro Till (*i.e.*, LCU) – compacted clay till of the Smithboro Member.

The hydrostratigraphic units included in the model were the UCU, UA, and LCU. The DA and DCU were not included in the model, which includes consistency with the original model (NRT, 2017b). No potential GWPS exceedances have been observed in the DA. This, coupled with the limited groundwater data available for the DA and DCU, meant that these layers were not included in the model. Therefore, the Smithboro Till (*i.e.*, LCU) represents the lower boundary of the CSM.

Surfaces for each of the three major geological units (Loess Unit, Hagarstown Member, Vandalia/Mulberry Grove Member and Smithboro Till Member) were taken from the NRT model (2017b). The NRT model (2017b) used available information from well logs to interpolate the top and base of the UA.

4.3.2 Extent and Boundaries

The United States Geological Survey (USGS) National Map places the CPP within the East Fork Shoal Creek watershed subbasin (Hydrologic Unit Code [HUC] 071402030303).

The CPP CSM extent is bounded by a hydrological catchment (watershed) divide to the east based on watershed data from USGS. The north, south, and east model boundary has been placed along known waterbodies as much as possible. As such, it is assumed groundwater inflow from adjacent watersheds is negligible through both the UA and LCU.

The Coffeen Lake water levels are managed at an average elevation 591.0 feet NAVD88. Coffeen Lake and Unnamed Tributary are the receiving body of water for surface water in the area encompassed by the CSM.

Infiltration of precipitation to the groundwater table is applied as recharge at the site. Groundwater in the UCU migrates downward into the Hagarstown Formation. As discussed in **Section 2.2.1**, the Hagarstown Formation is considered the UA for groundwater adjacent to the GMF GSP and GMF RP.

4.3.3 GMF Gypsum Stack Pond and GMF Recycle Pond

The GMF GSP and GMF RP were both constructed with an earthen berm and have a liner system which acts as a low permeability interface between the CCR contained within the GMF GSP and GMF RP and the ambient groundwater system. The liner system was installed along the inner faces of the GMF GSP and GMF RP (sides and base of the excavated area). The GMF RP has a passive gravity-driven underdrain system which was used to eliminate inward pressure on the liner prior to placement of CCR.

Findings from the HCRs (Ramboll, 2021a; Ramboll, 2021b) indicate that the GMF GSP and GMF RP do not appear to impact groundwater flow directions via recharge to groundwater. Given the low permeability of the liner system and the removal of the Hagarstown member (UA) below the units, it is more likely that the GMF GSP and GMF RP are barriers to groundwater flow within the UA, directing flow from upgradient areas through the underdrains beneath the units and/or

around the perimeter of the GMF GSP and GMF RP toward the Unnamed Tributary and eastern lobe of Coffeen Lake.

Sulfate was selected for transport modeling. Sulfate is commonly used as an indicator parameter for contaminant transport modeling for CCR because: (i) it is commonly present in coal ash leachate; and (ii) it is mobile and typically not very reactive but conservative (*i.e.*, low rates of sorption or degradation) in groundwater.

4.5 Model Approach

4.5.1 Potential Groundwater Exceedances

Comparisons of observed boron and TDS concentrations to sulfate (**Figure A** and **Figure B**, respectively, below) indicate statistically significant correlations between these parameters in UA wells where these potential exceedances were observed. Observed concentrations were transformed into Log10 concentrations for evaluation. The correlation coefficient (R^2) and p values (indicator of statistical significance) are also provided on **Figure A** and **Figure B**. Higher R^2 values (*i.e.*, closer to 1) indicate stronger correlation between parameters. A correlation is considered statistically significant when the p value is lower than 0.05. Both correlations have p values less than the target of 0.05, indicating correlations are statistically significant. The correlations are strongest between sulfate and TDS. The statistically significant correlations associated with sulfate concentrations indicate sulfate is an acceptable surrogate for boron and TDS in the groundwater model, and concentrations of these parameters are expected to change along with model predicted sulfate concentrations.

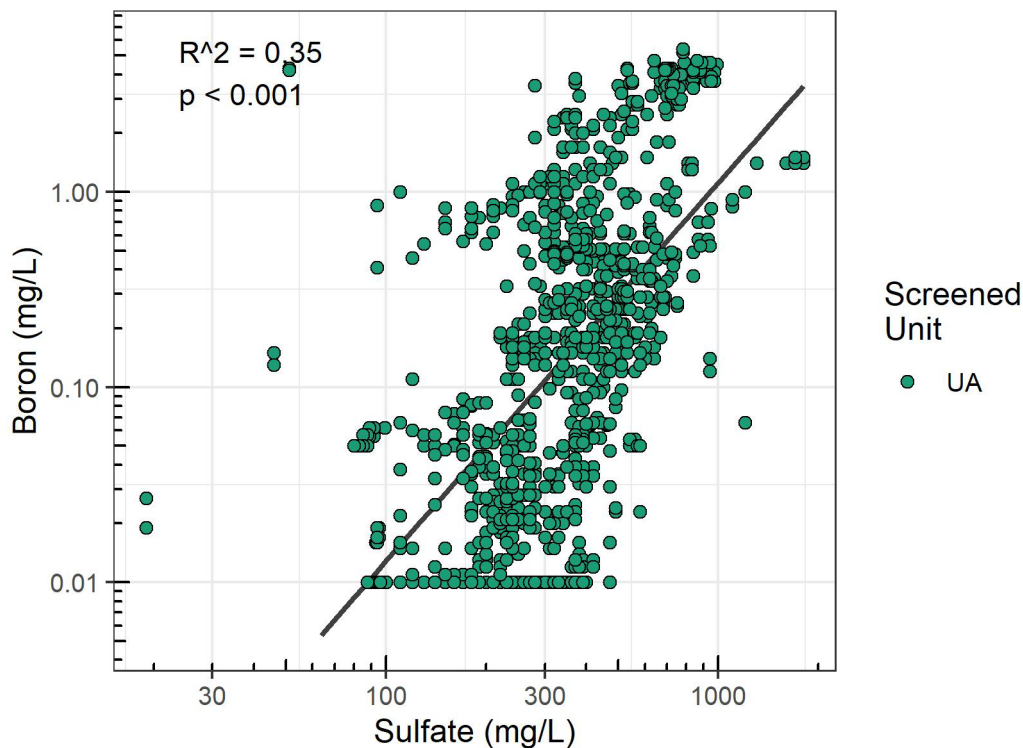


Figure A. Sulfate Correlation with Boron in UA Wells

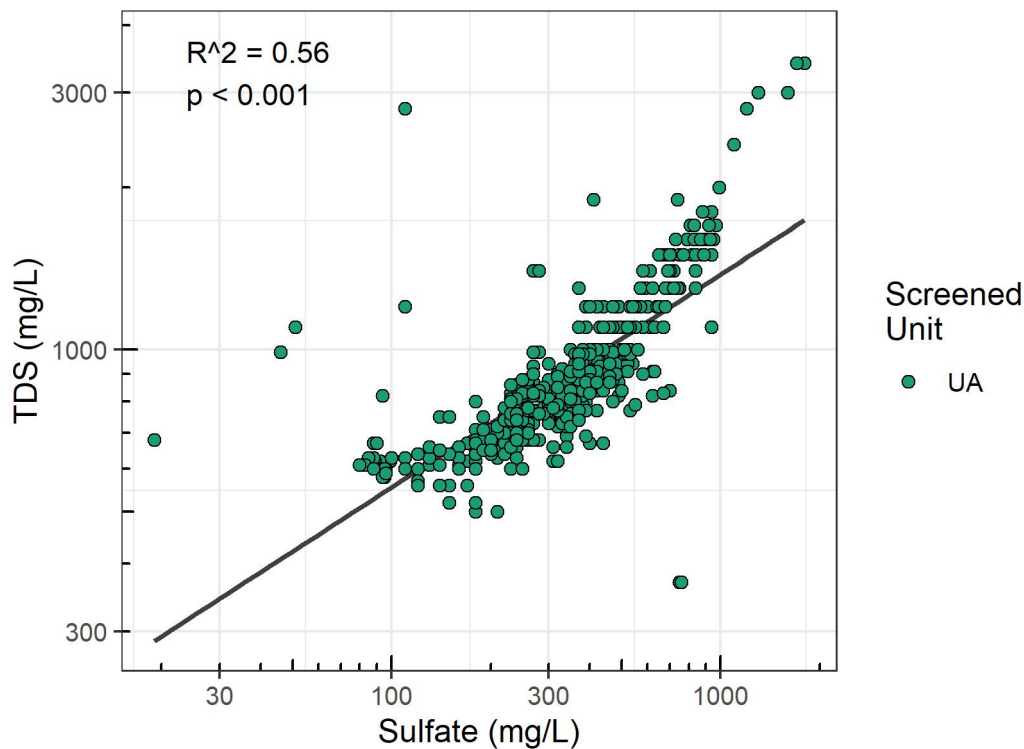


Figure B. Sulfate Correlation with TDS in UA Wells

4.5.2 Summary of Modeling Activities

A three-dimensional groundwater flow and transport model was calibrated to represent the conceptual flow system described above. Prediction simulations were performed to evaluate the effects of closure (source control) measures (CCR consolidation and CIP, and CBR scenarios) for the CCR units on groundwater quality following closure, which includes removal of free liquids (dewatering). **Figure 4-1** illustrates the calibration and predictive modeling timelines.

Three model codes were used to simulate groundwater flow and contaminant transport:

- Groundwater flow was modeled in three dimensions using MODFLOW 2005.
- Contaminant transport was modeled in three dimensions using MT3DMS.
- Percolation (recharge) was modeled using the results of the HELP model.

Modeling steps are summarized below:

- A steady state model was created in MODFLOW 2005 and used to simulate the mean groundwater flow conditions at the site. The model was calibrated to match mean groundwater elevations observed between 2015 to 2021 (**Table 4-1**).
- Transient flow models based off of the calibrated steady state model were used to simulate groundwater flow and transport for 42 years using MODFLOW 2005 and MT3DMS to simulate

changes in site conditions through time and match currently observed concentrations of sulfate in groundwater (**Table 4-1**).

- Prediction simulations began with a 2-year dewatering period simulated in MODFLOW 2005 and MT3DMS where heads were reduced within the CCR unit and concentrations were removed from CCR removal areas.
- Prediction simulations resumed for CIP and CBR following the 2-year dewatering period using the results of HELP modeling as input values for recharge rates in the construction areas.
- The prediction simulations were run using MODFLOW 2005 and MT3DMS to estimate the time for sulfate concentrations to meet the GWPS in the compliance wells; and, to evaluate the differences between the two closure scenarios.

5. MODEL SETUP AND CALIBRATION

5.1 Model Descriptions

For the construction and calibration of the numerical groundwater flow model for the site, Ramboll selected the model code MODFLOW, a publicly-available groundwater flow simulation program developed by the USGS (McDonald and Harbaugh, 1988). MODFLOW is thoroughly documented, widely used by consultants, government agencies and researchers, and is consistently accepted in regulatory and litigation proceedings. MODFLOW uses a finite difference approximation to solve a three-dimensional head distribution in a transient, multi-layer, heterogeneous, anisotropic, variable-gradient, variable-thickness, confined or unconfined flow system—given user-supplied inputs of hydraulic conductivity, aquifer/layer thickness, recharge, wells, and boundary conditions. The program also calculates water balance at wells, rivers, and drains.

MODFLOW was developed by USGS (McDonald and Harbaugh, 1988) and has been updated several times since. Major assumptions of the code are: (i) groundwater flow is governed by Darcy's law; (ii) the formation behaves as a continuous porous medium; (iii) flow is not affected by chemical, temperature, or density gradients; and (iv) hydraulic properties are constant within a grid cell. Other assumptions concerning the finite difference equation can be found in McDonald and Harbaugh (1988). MODFLOW 2005 was used for these simulations with Groundwater Vistas 7 software for model pre- and post- processing tasks (Environmental Simulations, Inc., 2017).

MT3DMS (Zheng and Wang, 1998) is an update of MT3D. It calculates concentration distribution for a single dissolved solute as a function of time and space. Concentration is distributed over a three-dimensional, non-uniform, transient flow field. Solute mass may be input at discrete points (wells, drains, river nodes, constant head cells), or distributed evenly or unevenly over the land surface (recharge).

MT3DMS accounts for advection, dispersion, diffusion, first-order decay, and sorption. Sorption can be calculated using linear, Freundlich, or Langmuir isotherms. First-order decay terms may be differentiated for the adsorbed and dissolved phases.

The program uses the standard finite difference method, the particle-tracking-based Eulerian-Lagrangian methods and the higher-order finite-volume total-variation-diminishing (TVD) method for the solution schemes. The finite difference solution has numerical dispersion for low-dispersivity transport scenarios but conserves good mass balance. The particle-tracking method avoids numerical dispersion but was not accurate in conserving mass. The TVD solution is not subject to significant numerical distribution and adequately conserves mass, but is numerically intensive, particularly for long-term models such as developed for this model. The finite difference solution was used for this simulation.

Major assumptions of MT3DMS are: (i) changes in the concentration field do not affect the flow field; (ii) changes in the concentration of one solute do not affect the concentration of another solute; (iii) chemical and hydraulic properties are constant within a grid cell; and (iv) sorption is instantaneous and fully reversible, while decay is not reversible.

The HELP model was developed by the United States Environmental Protection Agency (USEPA). HELP is a one-dimensional hydrologic model of water movement across, into, through, and out of

a landfill or soil column based on precipitation, evapotranspiration, runoff, and the geometry and hydrogeologic properties of a layered soil and waste profile. For this modeling, results of the HELP model, HELP Version 4.0 (Tolaymat and Krause, 2020) completed for the groundwater model were used to estimate the hydraulic flux from closure construction.

5.2 Flow and Transport Model Setup

The 2017 flow and transport models were retained and revised as appropriate to perform simulations for the GMF GSP and GMF RP.

The modeled area was approximately 10,000 feet by 15,025 feet (150,250,000 square feet [ft²]) centered on the CPP (**Figure 5-1**). The model boundaries along the northern and eastern edges of the model were selected to maintain sufficient distance from the CPP to reduce boundary interference with model calculations, while not extending too far past the extent of available calibration data. The eastern edge of the model also approximates topographic highs, surface water divides, and watershed boundaries.

The steady state MODFLOW model was calibrated to mean groundwater elevations collected from 2015 to 2021 and are presented in **Table 4-1**. MT3DMS was run on the transient flow model and model-simulated concentrations were calibrated to observed sulfate concentration values at the monitoring wells from January 2015 to July 2021 and are presented in **Table 4-1**. Multiple iterations of MODFLOW and MT3DMS calibration were performed to achieve an acceptable match to observed flow and transport data. For GMF GSP and GMF RP, the calibrated flow and transport models were used in predictive modeling to evaluate the CIP and CBR closure scenarios. Prior to simulation of CIP and CBR, a dewatering phase, which simulated the removal of free liquid from the CCR material in the GMF GSP and GMF RP was completed. Closure scenarios were simulated by removing saturated ash cells from removal areas and using HELP modeled recharge values to simulate changes proposed in the closure scenarios.

5.2.1 Grid and Boundary Conditions

A five-layer, 326 x 211 node grid was established with a variable grid spacing between 25 and 100 feet (**Figures 5-2 through Figure 5-6**), with a total number of 284,575 active cells.

The main body of Coffeen Lake is immediately adjacent to CPP on the west and south and the Unnamed Tributary borders CPP to the east. These surface water features form the southern, eastern, and western boundaries of the model. The northern boundary of the model domain is a general head boundary. Vertically, the model domain extends from the top of the saturated zone to the base of the Smithboro Member. The thick clays of the Banner Formation are relatively impermeable compared to the overlying unconsolidated sediments and provides a base for the model.

The northern boundaries for layers 3, 4, and 5 are general head boundaries placed to simulate flow in the sandier soils of the Hagarstown Member, Vandalia Member, and Smithboro Till composing the UA (layer 3) and LCU (layer 4 and 5). The northern boundary represents the regional flow conditions within these units. The eastern edge is a no-flow boundary in all model layers.

Coffeen Lake is represented as a constant head boundary based on an average surface water elevation of 591.0 feet NAVD88. The constant head boundary was simulated with an elevation equal to 591.0 feet. The lake is in hydraulic connection with multiple layers within the model.

The bottom of the model was also a no-flow boundary. The top of the model was a time-dependent specified flux boundary, with specified flux rates equal to the recharge rate. A specified mass flux boundary was used to simulate downward percolation of solute mass from the GMF GSP and GMF RP. This boundary condition assigns a specified concentration to recharge water entering the cells within the GMF GSP and GMF RP, and the resulting concentration in the GMF GSP and GMF RP cells is a function of the relative rate and concentration of recharge water (water percolating from the impoundments) compared to the rate and concentration of other water entering the node.

5.2.2 Flow Model Input Values and Sensitivity

Evaluation of monitoring well data for the CPP has not identified statistically significant seasonal trends in groundwater flow or quality which could affect model applicability for prediction of transport. The MODFLOW model was calibrated to mean groundwater elevations from 2015 to 2021. Multiple iterations of MODFLOW calibration were performed to achieve an acceptable match to observed flow data.

Sensitivity analysis was conducted by changing input values and observing changes in the sum of squared residuals (SSR). Horizontal conductivity, vertical conductivity, and river and general head conductance terms were all varied by one order of magnitude (*i.e.*, between one-tenth and ten times) of the calibrated values. Recharge terms were varied between one-half and two times calibrated values. River stage was obtained from the 10 meter (m) Digital Elevation Model (DEM) from the United States Department of Agriculture/Natural Resources Conservation Service (USDA/NRCS) National Geospatial Center of Excellence (USDA/NRCS, 2022). The vertical error of the 10 m DEM is 0.82 m (2.7 feet); therefore, the stream stage was varied by adding and subtracting 2.7 feet. Where appropriate, drain stage was modified based on the DEM error, where this was inappropriate drain stage increased and decreased by 2 feet. General head boundary head terms were varied between 90 and 110 percent of calibrated values. The hydraulic flow boundary was varied by increasing the hydraulic conductivity by a factor of 100 and 1000. When the calibrated model was tested, the SSR was 351. Sensitivity test results were categorized into negligible, low, moderate, moderately high, and high sensitivity based on the change in the SSR as summarized in the notes in **Table 5-1**.

5.2.2.1 Layer Top/Bottom

The top of the saturated zone was used as the top of the model. The elevations for the base of each hydrostratigraphic layer were obtained from the NRT model (2017b) and were imported as grid data into MODFLOW. The upper Loess Unit of the Hagarstown Member (UCU) was divided into two layers to accommodate the explicit inclusion of the CCR in AP1 and AP2. The sand and silts of the Hagarstown Member which form the UA were represented using a single layer. The LCU was represented by two layers, the upper LCU (layer 4) represents the unweathered Vandalia/Mulberry Grove Member and the lower LCU (layer 5) represents the Smithboro Member.

The UCU layer was split into two layers (layers 1 and 2) to simulate the construction of AP1 and AP2. Within AP1 and AP2, layer 1 represents ash fill and layer 2 represents the UCU present below the ash and above the UA. Outside of AP1 and AP2, both layers 1 and 2 represent the UCU. Layer 3 represents the UA and the LCU is present in layers 4 and 5. **Figures 5-7 through 5-11** show the bottom elevations of the five model layers. The resulting model layers represent the distribution and change in thickness of each water-bearing unit across the model domain.

Table A below provides elevation and thickness information for the model layers and hydrostratigraphic units used in the model.

Table A. Flow Model Layer Descriptions

Layer	Hydrostratigraphic Unit Name	Hydrostratigraphic Unit Used to Determine Layer Thickness	Top Elevation ¹	Bottom Elevation ¹	Thickness (feet)
			Mean (Minimum – Maximum)		
1&2	UCU and CCR	Loess Unit of Hagarstown Member and CCR	640 (-)	607.73 (604.0-614.15)	27.1 (26.0-29.85)
3	UA	Hagarstown Member	607.73 (604.0-614.15)	600.9 (580.0-612.0)	5.2 (2.0-34.0)
4	LCU	Vandalia/Mulberry Grove Member	600.9 (580.0-612.0)	588.5 (578.0-594.0)	18.83 (2.0-30.0)
5	LCU	Base of Coffeen Lake	588.5 (578.0-594.0)	540.0 (-)	48.4 (38.0-51.1)

Notes:

¹ Elevation is measured in feet, referenced to NAVD88.

5.2.2.2 Hydraulic Conductivity

Hydraulic conductivity values and sensitivity results are summarized in **Table 5-1**. The spatial distribution of the hydraulic conductivities within the UCU, UA and LCU were considered homogenous. **Figures 5-12 through 5-16** show the spatial distribution of the hydraulic conductivity zones, GMF GSP and GMF RP, and other units on site for each of the five model layers. Construction of the GMF units (*i.e.*, GMF GSP and GMF RP) removed the sands and silts of the UA prior to construction of the liner; therefore, the UA is absent beneath these units and liner hydraulic properties are assigned. Conductivity zones that did not have representative site data (*i.e.*, zones 19 and 21, representing the cells above the river cells and the disturbed sediments between the LF and GMF GSP, respectively) were determined through model calibration.

Where available, hydraulic conductivity values were derived from field measured or laboratory tested values reported in the HCRs (Ramboll, 2021a; Ramboll, 2021b) (**Section 2.2.2**). No horizontal anisotropy was assumed. Vertical anisotropy was applied to conductivity zones to simulate preferential flow in the horizontal direction in these materials, and are presented as anisotropy ratio (Kh/Kv) in **Table 5-1**.

The model was highly sensitive to changes in horizontal and vertical hydraulic conductivity in zones 1 (UCU), 2 (UA), and 3 (LCU - unweathered Vandalia), and moderately sensitive to changes in horizontal and vertical hydraulic conductivity in zones 10 (CCR fill-AP1) and 19 (UCU-fill). The model exhibited a negligible to low sensitivity in the remaining zones for both horizontal and vertical conductivity.

5.2.2.3 Recharge

Recharge rates were determined through calibration of the model to observed groundwater elevations. For the calibration model, recharge was applied to the uppermost active layer and the rates varied based on different units, namely the AP1, AP2, GMF GSP, GMF RP, LF, Surface Water Pond, and Cooling Pond. Model inputs are summarized in **Table 5-1**. The distribution of recharge

is shown in **Figure 5-17**. Changes in operational history, such as the addition of AP1 to the site in 1977 and the GMF units in 2010 as illustrated in **Figures 5-18 through 5-21**, have been incorporated into the transient model simulation (**Table 5-2**), see **Section 5.2.3.1** for additional discussion of time discretization.

The model had a high sensitivity to changes in recharge in zones 1 (UCU) and 7 (CCR fill-AP1). The model had negligible to low sensitivity to changes in recharge in the remaining zones, with the exception of zone 6 (CCR fill-AP2), where the sensitivity was moderate.

5.2.2.1 Storage and Specific Yield

The flow calibration model did not use these terms because it was run at steady state. For the transport model, which was run as a transient simulation, no field data defining these terms were available so published values were used consistent with Fetter (1988). Specific yield was set to equal effective porosity values described in **Section 5.2.3.5**. The spatial distribution of the storage and specific yield zones were consistent with those of the hydraulic conductivity zones. The sensitivity of these parameters was tested by evaluating their effect on the transport model as described in **Section 5.2.3.6**.

5.2.2.2 River Parameters

Five river reaches were included in the model as head dependent flux boundaries that required inputs for elevation of the surface water, bottom of the stream, width, bed thickness, and bed hydraulic conductivity (**Table 5-1**). The five river reaches were the Unnamed Tributary east of the CPP (reach 0 and reach 5), the Unnamed Tributary west of the CPP (reach 1), ponded surface water west of the LF (reach 2), and the condenser cooling water discharge flume (reach 3). The river and drain information is summarized in **Table B** below.

Table B. River and Drain Information

Name	Boundary Type	Length (feet)	Slope (ft/ft)
Unnamed Tributary East	River	8959.0	-0.0031
Unnamed Tributary East – downstream reach	River	1438.3	-0.0026
Unnamed Tributary West	River	3436.5	-0.0098
Ponded Surface Water West	River	-	-
Condenser Cooling Flume	River	-	-
Active Landfill Underdrain	Drain	2147.0	-
Gravity Drain Recycle Pond	Drain	2181.8	-
North Drain	Drain	3032.0	-

Notes:
 ft/ft = feet per foot

In the absence of river geometry information, the DEM was used to estimate stream stage at the upstream and downstream limits of the Unnamed Tributary east of the CPP and the Unnamed Tributary west of the CPP. The surface water stages for the ponded surface water west of the LF and the Condenser Cooling Flume were constant (not sloped) and were also obtained from the DEM. For both Unnamed Tributaries (east and west), the slope of the river was then linearly interpolated along the reaches, providing an estimation of stream stage along the length of each

reach for each model grid cell through which the river flows. Bed thickness was set at 2 foot and river width was set at 10 feet. The river bottom is set 3 feet below the stage for both the Unnamed Tributaries. The downstream reach (reach 5) of the Unnamed Tributary is located in layer 5 of the model adjacent to the SI unit AP2, this layer represents the LCU-Smithboro till and has a low hydraulic conductivity. To increase connectivity of the tributary to the overlying layers the hydraulic conductivity of the streambed was modified during calibration.

The Condenser Cooling flume stage is maintained at 604.0 feet and the ponded surface water west of the LF was maintained at 617.5 feet, and bed thicknesses for these reaches were set to 1 foot. The width of the Cooling Flume (approximately 52 feet) and ponded surface water west of the LF are larger than the grid cell dimensions (25 feet by 25 feet), therefore the conductance term for both were based on the area of the cells which coincide with the flume and ponded water.

The model had low to moderate sensitivity to changes in river stage. The model had low to moderately high sensitivity to changes in river conductance, with the exceptions of reach 0 (Unnamed Tributary East) and reach 3 (Condenser Cooling Flume) which had high sensitivity.

5.2.2.3 Drain Parameters

The LF has an active underdrain, which is actively pumped to prevent more than 1-foot of groundwater head above the liner. This was estimated to be 603.5 feet. The GMF RP has a passive drain beneath the liner which discharges water towards the Unnamed Tributary east of the unit. This was estimated to be 600.5 feet. Both the active LF drain and passive GMF RP drain were placed in layer 4 (LCU) below the low hydraulic conductivity zones which represent the base of the lined units. A surface water drain in the north of the model was also included; the placement of this northern drain was determined using google earth imagery. The Northern drain appears to be a man-made feature and no hydrological data are available as to its flow conditions. Therefore, its implementation in the model as a drain makes the fewest assumptions of its interaction with the aquifer. This surface water drain is located in layer 1 and has an elevation of 622.0 feet.

The model had low sensitivity to changes in drain stage. The model had negligible to moderate sensitivity to changes in drain conductance, with the exception of reach 0 (Active LF Underdrain) where the model had moderately high sensitivity to changes in drain conductance.

5.2.2.4 GMF Unit Parameters

All GMF units (GMF GSP, GMF RP, and LF) have a similar liner construction (**Table C** below); they were all implemented into the model using a hydraulic flow barrier to present the liner system on the sides of the units. The bottom of the liner is implemented by assigning the liner system hydraulic conductance to model layer 3 within the footprint of the pond. The base elevation of layer 3 within the footprint of the GMF units simulates the base elevation of the liner. The thickness of model layer 3 within the footprint of the pond was set to three feet. Removal of the sands and silts below the GMF units (as described in **Sections 1.4** and **2.1**) means that the liner is in direct contact with the Vandalia Member. The groundwater flow dynamics beneath/around the Ash Landfill and GMF Units is affected by several factors, including removal of the Hagarstown Member from beneath the Units; presence of the construction dewatering systems around the units; and the lateral variability of lithology within the Hagarstown Member (Hanson

2016). Drains discussed above were used to represent the underdrains associated with the GMF units. The hydraulic properties within the GMF units were set to represent the CCR.

Estimates of the hydraulic properties of each of the components within the liner system were derived using values from the HELP model; see **Section 5-1** for more information about HELP. For flow perpendicular to the layer orientation, as is the case in the liner where the hydraulic gradient is vertical for the base and horizontal for the sides of the pond, the harmonic mean was used to obtain the effective hydraulic conductivity (K_{eff}) (Fetter, 1988). The harmonic mean was determined by:

$$K_{eff} = \frac{\sum b}{\sum \frac{b}{K}}$$

Where b is the layer thickness and K is the horizontal hydraulic conductivity.

Hydraulic flow boundary input parameters are presented in **Table 5-1**. The model had low to moderate sensitivity to changes in the hydraulic conductivity in the horizontal flow barrier (HFB).

Table C. Liner System Properties From Top to Bottom for the GMF GSP, GMF RP, and LF

Liner Component	Thickness (feet)	Hydraulic Conductivity (cm/s)	Hydraulic Conductivity (feet/day)
HDPE geomembrane (60 mm)	0.06	2.0×10^{-13}	5.7×10^{-10}
Recompacted Soil	3.0	1.0×10^{-7}	2.8×10^{-4}
Vertical Harmonic Mean of Liner System	NA	NA	2.89×10^{-8}

* Estimated based on available information
 NA = not applicable

5.2.2.5 General Head Boundary

General head boundary conditions (GHB) were used along the northern boundary of the model for layer 3 through 5 (**Figures 5-4 through 5-6**). The GHB at the northern limit of the model represents groundwater entering the model domain from upgradient areas, the GHB is present in layers 3 through 5 was used to simulate groundwater flow into the model via the UA and LCU. The groundwater levels used for the northern boundary of the model in layers 3 through 5 were estimated using the Dupuit equation for steady state flow in an unconfined aquifer with recharge.

The DEM of the site provided estimates of the surface water levels for Coffeen Lake on the west boundary of the model (591 feet), and Rocky Ford Sportsman Club North Lake (604 feet) on the east of the model domain (**Figure 5-1**). The calibrated ambient recharge to the UCU was used in the calculation of the groundwater level distribution at the northern boundary. The hydraulic conductivity value used in the Dupuit equation was estimated during model calibration.

This GHB was only applied to cells along the northern boundary where the base of the cell was below the calculated groundwater head for a given distance from the constant head boundaries, the head was determined by the Dupuit equation. Cell conductance was then calculated using the cells' saturated thickness and cell width, and hydraulic conductivity were based on cell hydraulic conductivities and adjusted if appropriate during calibration.

The GHB elevation for northern boundary in the UA was established during calibration (**Table 5-1**). The distance to the GHB head was set to 1, and the GHB conductivity was

calculated using the cell width, cell thickness, and calibrated hydraulic conductivity from the model.

The sensitivity to changes in specified head was low to moderate, with the exception of reach 3 (Northern Model Boundary in LCU Layer 4) where the model sensitivity was high. The flow calibration model had negligible sensitivity to changes in conductance.

5.2.3 Transport Model

MT3DMS input values are listed in **Table 5-2** and described below. Sensitivity of the transport model is summarized in **Table 5-3**.

Groundwater transport was calibrated to groundwater sulfate concentration ranges at each well as measured from the monitoring wells between 2015 (where available) and 2021. The transport model calibration targets are summarized in **Table 4-1**.

Sensitivity analysis was conducted by changing input values and observing percent change in sulfate concentration at each well from the calibrated model sulfate concentration. Effective porosity was varied by decreasing and increasing calibrated model values by 0.05. Storage values were multiplied and divided by a factor of 10, and specific yield by a factor of 2. The dispersivity values in the calibrated model were increased by a factor of 5 and 10. The sensitivity of the transport model to changes in the liner conductance was also investigated by increasing and decreasing the hydraulic conductivity of the liner by one order of magnitude (*i.e.*, between one-tenth and ten times).

The transport model generally had a low to moderately high sensitivity to changes in storage and specific yield (**Table 5-3**) as discussed in **Section 5.2.3.6**, not including monitoring locations where the calibration concentration was less than 10.0 mg/L. The transport model generally ranged from low to moderate sensitivity to effective porosity and low to high sensitivity to dispersivity as discussed in **Sections 5.2.3.5** and **5.2.3.7**, respectively. The transport model generally had a low to high sensitivity to changes in the liner conductivity as discussed in **Section 5.2.3.2**.

5.2.3.1 Time Discretization and Stress Periods

The evolution of the CPP required changes to the hydraulic properties within the model; this is not possible in a single model where hydraulic properties are assumed to remain constant. As a result, the changes in the site (*e.g.*, inclusion of the GMF units) are simulated in three consecutive numerical models, as summarized in **Table D** on the following page. The simulation length was revised from the existing model to extend to the current time (2022).

Table D. Transient Model Setup and Time Discretization

Date	Model	Stress Period	Operational Change	Previous model
Pre-1970	Steady-State	NA	No CCR units present	Not applicable
1970-2010	Transient (TR-1)	1:1970-1985	AP2 only	Steady State Pre-1970 flow
		2:1985-2010	AP2 and AP1 in operation	
2010-2018	Transient (TR-2)	1:2010-2018.	AP1, GMF GSP and GMF RP in operation.	TR-1 as initial flow and concentrations
2018-2022	Transient (TR-3)	1:2018-2022	Modification to lined units GMF GSP and GMF RP, AP2 capped	TR-2 as initial flow and concentrations

Notes:
 TR = transient model

5.2.3.2 GMF Units

Groundwater chemistry data from wells G215 (located adjacent to the GMF GSP), and wells G275 and G279 (located adjacent to the GMF RP), indicate an increase in sulfate concentrations post 2018 when compared with sulfate concentrations in adjacent wells. Sulfate concentrations in G215 have experienced further increases since 2021. Sulfate concentrations around the GMF RP tend to be higher than those around the GMF GSP, with elevated sulfate concentrations observed since 2015 (the earliest sampling date). Elevated sulfate concentrations along the southern boundary of the GMF RP are associated with historic groundwater impacts from AP2. However, wells G275 and G279 are located along the eastern boundary of the pond and have elevated sulfate concentrations. To simulate observed sulfate concentrations at these isolated wells (GMF GSP well G215 and GMF RP wells G275 and G279), the hydraulic conductivity of the liner (simulated using hydraulic flow boundary [HFB] cells) was increased to allow sulfate migration from the CCR unit in the transient model TR-3, as shown in **Figure C** below and **Table D** above.

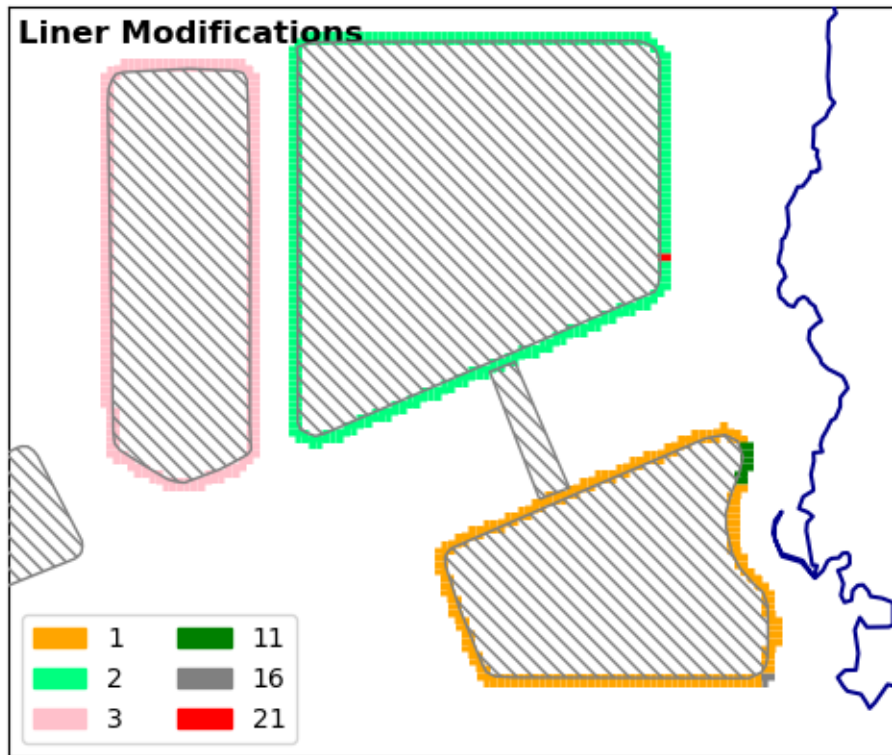


Figure C. Liner Modification Zones

As part of the transport calibration process, the hydraulic conductivity of HFB reaches 11, 16, and 21 were modified to simulate the observed rises in sulfate. The changes are summarized in **Table 5-2**.

The model had a negligible to high sensitivity to changes in the HFB values, not including monitoring locations where the calibration concentration was less than 10.0 mg/L (*i.e.*, G102, G103, G105, G106, G206, G207, G208, G210, G211, G212, G216, G217, G218, G270, and G280) (**Table 5-3**). An increase in the liner conductance produces the greatest sensitivity in monitoring wells G215, G275, G276 and G279. This high sensitivity is anticipated given that the liner properties were modified to match observed sulfate concentrations in the vicinity of these wells (**Table 5-2** and **Table 5-3**).

5.2.3.3 Initial Concentration

No initial concentrations were placed in the steady state flow calibration model. The flow model was run as transient and concentration was added to the model through recharge starting at the same time as the transient flow simulation. Modeling was performed for a sufficient period (42 years) to allow modeled concentrations in the primary transport layer (*i.e.*, UA) to reach recently observed levels.

Modeling was performed over three numerical models which mirror the operational developments at the CPP. **Table 5-2** provides an overview of how the source concentrations and recharge rates change through time.

5.2.3.4 Source Concentration

Five sources in the form of vertical percolation (recharge) and constant concentration cells were simulated in the CCR material for calibration (**Table 5-2**) (in chronological order): (i) percolation through CCR in AP2 (1970-2022), (ii) percolation through CCR in AP1 (1978-2022), (iii) percolation through CCR in GMF RP (2010-2022), (iv) percolation through CCR in GMF GSP (2010-2022), and (v) percolation through CCR in GMF LF (2010-2022). All five sources were simulated by assigning concentration to the recharge input. The CCR sources were also simulated with constant concentration cells placed where CCR was present (**Figures 5-18 through 5-21**) to simulate saturated CCR conditions. From the model perspective, this means that when the simulated water level is above the base of these cells, water that passes through the cell will take on the assigned concentration. All source concentrations were calibrated in the transport model to the sulfate concentration data collected from November 2015 to August 2021. The source concentrations applied to the recharge zones and saturated ash cells immediately below the recharge zones have the same concentration values. **Table 4-1** indicates that the background sulfate concentrations (identified with a "B" for background in the "CCR unit" column) at CPP show considerable variability across the site, from 11 mg/L (G286) to 770.0 mg/L (G288). No background sulfate concentration was applied to recharge beyond the source areas in the model.

Because these are the sources of concentration in the model, the model will be highly sensitive to changes in the input values. For that reason, sensitivity testing was not completed for the source values.

5.2.3.5 Effective Porosity

Effective porosity for each modeled hydraulic conductivity zones were based on the NRT model (2017b), data from the HCRs (Ramboll, 2021a; Ramboll, 2021b), and literature values (Fetter, 2001) and are presented in **Table 5-2**.

The model had a negligible to moderate sensitivity to changes in porosity values, not including monitoring locations where the calibration concentration was less than 10.0 mg/L (*i.e.*, G102, G103, G105, G106, G206, G207, G208, G210, G211, G212, G216, G217, G218, G270, and G280) (**Table 5-3**). For wells with calibration concentrations greater than 10.0 mg/L, the greatest sensitivity for porosity was moderate for both the low and high porosity sensitivity tests at monitoring locations G213, G214, G215, G271, G272, and G276.

5.2.3.6 Storage and Specific Yield

The transport model had a negligible to high sensitivity to changes in storage and specific yield, not including monitoring locations where the calibration concentration was less than 10.0 mg/L (*i.e.*, G102, G103, G105, G106, G206, G207, G208, G210, G211, G212, G216, G217, G218, G270, and G280). Monitoring wells G213, G214, G215, and G271 had moderately high to high sensitivity to changes in storage and specific yield. Of these wells only G215 had simulated sulfate concentrations in both the calibrated model and sensitivity models which exceed the GWPS of 400 mg/L (**Table 5-3**).

5.2.3.7 Dispersivity and Diffusion

Physical attenuation (dilution and dispersion) of contaminants is simulated in MT3DMS. Dispersion in porous media refers to the spreading of contaminants over a greater region than would be predicted solely from the average groundwater velocity vectors (Anderson, 1979;

Anderson, 1984). Dispersion is caused by both mechanical dispersion, a result of deviations of actual velocity at a microscale from the average groundwater velocity, and molecular diffusion driven by concentration gradients. Molecular diffusion is generally secondary and negligible compared to the effects of mechanical dispersion and only becomes important when groundwater velocity is very low. The sum of mechanical dispersion and molecular diffusion is termed hydrodynamic dispersion, or simply dispersion (Zheng and Wang, 1998).

Longitudinal dispersivity was 10 feet in the UA and 1 foot in the UCU and LCU, with transverse and vertical dispersion coefficients assuming a ratio of 1/10 and 1/100.

The model had a low to high sensitivity to changes in porosity values, not including monitoring locations where the calibration concentration was less than 10.0 mg/L (*i.e.*, G102, G103, G105, G106, G206, G207, G208, G210, G211, G212, G216, G217, G218, G270, and G280) (**Table 5-3**). For wells with calibration concentrations greater than 10.0 mg/L, the greatest sensitivity for dispersivity was high sensitivity at monitoring locations G213, G214, and G271.

5.2.3.8 Retardation and Decay

It was assumed that sulfate would not significantly sorb or chemically react with aquifer solids (distribution coefficient [Kd] was set to 0 mL/g) which is a conservative estimate for estimating contaminant transport times. Boron, sulfate, and TDS transport is likely to be affected by both chemical and physical attenuation mechanisms (*i.e.*, adsorption and/or precipitation reactions as well as dilution and dispersion). Batch adsorption testing was conducted to generate site specific partition coefficient results for boron and sulfate (Geosyntec, 2022a, **Appendix B**) for location G215. Results of the testing are summarized below:

- Boron: A boron partition coefficient was not determined for any isotherm for the boron amended with microcosms. Both the linear and linearized Langmuir isotherms yielded negative partition coefficients, and the linearized Freundlich could not be calculated as the data were not conducive to log transformation. Other studies have reported low partition coefficients for boron ranging from 0.19 to 1.3 L/kg, depending on pH conditions and the amount of sorbent present (EPRI, 2005; Strenge & Peterson, 1989).
- Sulfate: A sulfate partition coefficient was not determined for any isotherm for the sulfate amended microcosms. The linear isotherm yielded a partition coefficient of 0.1 L/kg but had a very poor goodness-of-fit, and the Langmuir isotherm yielded a negative coefficient. As in the boron-amended microcosms, the Freundlich isotherm could not be calculated because the data were not conducive to log transformation. These results are consistent with the findings of Strenge & Peterson (1989), who found that partition coefficients for sulfate are 0.0 L/kg, regardless of pH conditions and the amount of sorbent present.

The results from site samples did not provide representative isotherms which supports modeling sulfate without retardation. The potential exceedances identified in groundwater (boron, sulfate, and TDS) are affected by natural attenuation processes in multiple ways and to varying degrees. Further assessment of these processes and how they may be applied as a potential groundwater remedy will be completed as part of future remedy selection evaluations, as necessary. For the purposes of this GMR, and as mentioned at the beginning of this section, no retardation was applied to sulfate transport in the model (*i.e.*, Kd was set to 0). Sensitivity tests were not run for retardation.

5.3 Flow and Transport Model Assumptions and Limitations

Simplifying assumptions were made while developing this model:

- Leading up to 2022, the groundwater flow system cannot be simulated as steady state.
- Natural recharge is constant over the long term.
- Fluctuations in lake stage do not affect groundwater flow and transport over the long term.
- Hydraulic conductivity is consistent within hydrostratigraphic units.
- The approximate base of ash surface in the AP1, GMF GSP, GMF RP, and LF were developed with Golder using soil borings and historic topographic maps.
- Source concentrations are assumed to remain constant over time.
- Sulfate is not adsorbed and does not decay and mixing and dispersion are the only attenuation mechanisms.

The model is limited by the data used for calibration, which adequately define the local groundwater flow system and the source and extent of the plume. Since data used for calibration are located near the units on site, model predictions of transport distant spatially and temporally from the calibrated conditions at the CCR units will not be as reliable as predictions closer to the CCR units and concentrations observed in 2021.

5.4 Calibration Flow Model

The groundwater model was manually calibrated to best approximate the mean groundwater elevations in 95 wells at the site. The mean elevations used for calibration and locations of wells within the flow model are summarized in **Table 4-1**. Well locations are shown in **Figure 2-1**. This involved modifying the hydraulic conductivities of the different hydrostratigraphic units, recharge rate, and conductance of the drains, rivers, and general head boundaries within the model to minimize the difference between the mean observed groundwater elevation and simulated groundwater elevation. Where possible, the range of the parameter values used during calibration were based on observed values (*i.e.*, for the range in hydraulic conductivity estimates from the HCRs). Where this was not possible, such as for the drain and general head boundary conductance, the range of parameter values were based on other site information or inferred from knowledge from similar sites. Where data were limited, the parameter values were less constrained during calibration (*e.g.*, parameter values had wider ranges). The SSR was used as a metric to identify the optimal values for the different parameters.

5.5 Calibration Flow and Transport Model Results

Results of the MODFLOW modeling are presented below. The model files accompany this report (**Appendix A**). **Table 5-1** shows the calibrated hydraulic conductivity for the different units shown in **Figures 5-12 through 5-16**.

Groundwater model calibration results are presented in **Figure 5-22** and **Figure 5-23**, which shows the observed and simulated groundwater elevations and the observed groundwater elevation versus residuals. The near-linear relationship between observed and simulated values presented on **Figure 5-22** indicates that the model adequately represents the calibration dataset. The root mean squared error of the simulated groundwater elevation across all wells was

1.92 feet. The mass balance error for the flow model was 0.00 percent and the ratio of the residual standard deviation to the range of heads was 9.0 percent, which is below the desired target value of 10 percent. Another flow model calibration goal is that residuals are evenly distributed such that there is no bias affecting modeled flow. The observed heads are plotted versus the simulated heads in **Figure 5-23** and simulated values are evenly distributed above and below observed values. The mean residual was also near zero with a value of 0.10 feet, indicating a small bias towards underestimating the groundwater elevation in the calibrated model; this is also illustrated in the observed versus residuals plot in **Figure 5-23**.

The simulated groundwater elevations within the UA (layer 3) for the entire site are shown in **Figure 5-24**. **Figure 5-25** shows the simulated groundwater elevations in proximity to the GMF GSP and GMF RP. In general, the model is able to simulate the groundwater flow patterns in the UA (**Figure 2-2** and **Figure 2-3**) at the GMF GSP and GMF RP interpreted from the site well data for April and July 2021, respectively.

In general, the model is able to simulate the groundwater flow patterns in the UA around the GMF units (**Figure 2-2** and **Figure 2-3**). The removal of the UA sands beneath the units and presence of a low hydraulic conductivity of the liner and underdrain beneath the GMF RP does appear to increase flow towards the Unnamed Tributary, particularly in the southeast corner of the GMF RP.

Twelve wells provided calibration targets for the simulated groundwater levels around the GMF RP (**Figure 5-25**). The simulated groundwater levels are within 2 feet for all wells except G274 at the southeast corner, which is underpredicted by 2.5 feet. The simulated groundwater levels are within 1 foot for nine of the wells and within 2 feet for two wells. There is a tendency for the model to overestimate the groundwater levels to the east of the GMF RP close to the Unnamed Tributary, and underestimate the groundwater levels to the south of the GMF RP.

Twenty-two wells provided calibration targets for the simulated groundwater levels around the GMF GSP (**Figure 5-25**). The simulated groundwater levels to the south of the GMF GSP are generally underestimated. Of the ten wells located along this boundary, the simulated groundwater levels in two wells are within 1 foot of the observed groundwater levels, five wells are within 2 feet, and two wells are within 3 feet. One well at the southeast corner is underestimated by 3.34 feet; however, this is directly adjacent to a well whose simulated groundwater level is within 2 feet of the observed groundwater level. On the eastern boundary of the GMF GSP, the simulated groundwater levels are underestimated in the southeast and overestimated in the northeast. Of the five wells located along this boundary, the simulated groundwater level for two wells is within 1 foot, and the remaining three wells are within 2 feet of the observed groundwater elevation. There are five wells located along the western boundary of the GMF GSP. The simulated groundwater levels for two wells are within 1 foot (G106 and G102), and one well is within 2 feet (G103). The remaining two wells, G105 and R105, are simulated within 2.61 feet and 3.89 feet, respectively. Two wells, MW11S and T202, provide calibration targets to the north of the GMF GSP with the simulated groundwater levels which are overestimated by 0.55 and 3.39 feet, respectively.

Construction of the lined GMF units, combined with partial to complete removal of the UA beneath the unit footprints along with installation of undrain systems, created a significant disturbance to the subsurface flow pattern. Capturing this change in a groundwater flow model is challenging. The changes in subsurface hydraulic properties in proximity to the units may be

considerable. For example, the inclusion of a zone which represents the area between the GMF GSP and LF was created to capture the unique conditions between these two CCR units. The zonation currently applied to disturbed materials is a simple representation based on available soil borings near the GMF units. The general flow pattern in and around the GMF units is in good agreement with the observed flow patterns in the area (**Figure 2-2** and **Figure 2-3**).

The range of observed sulfate concentrations for transport calibration locations are summarized in **Table 4-1**. The goals of the transport model calibration were to have predicted concentrations fall within the range of observed concentrations and to have predicted concentrations above and below the GWPS for sulfate (400 mg/L) match observed concentrations above or below the standard at each well. For the GMF RP, one or both of these goals were achieved at all of the transport calibration location wells, except G276 where concentrations were overpredicted (**Figure 5-26**). Deviations from the observed ranges are discussed below. The distribution of sulfate concentrations in the calibrated model are presented on **Figure 5-27**. The elevation of the basal liner system influences the distribution of sulfate concentration within the footprint of the GMF RP. **Figure 5-8** illustrates the bottom elevation of model layer 2 which represents the top of the liner system. Within the footprints of both the GMF GSP and GMF RP, the base of the liner incorporates part of the bounding berm system for each of the units. As a result of the increase in elevation at the edges of the SIs, high sulfate concentrations are simulated. This is most notable at the south end of the GMF GSP in **Figure 5-27**.

G276 has a maximum observed sulfate concentration of 310 mg/L, and the simulated sulfate concentration is 543 mg/L. G276 is located 140 feet downgradient of G275 where sulfate concentrations ranged from 650 to 940 mg/L. The observed and simulated flow direction around the southeastern corner of the GMF RP is northeasterly. This flow direction leads to sulfate transport in a northeasterly direction from G275 towards G276. The lower observed sulfate concentration at G276 as compared to G275 may be the result of subsurface heterogeneity within the UA that is not captured in the model.

The model under predicts concentrations in G271, G272, G273, and G274. The observed sulfate concentrations in these wells ranges from 260 and 690 mg/L. For all wells, modeled and minimum observed sulfate concentrations are both below 400 mg/L, so one of the two calibration goals was satisfied.

Modification to the liner conductivity for the GMF RP enabled the simulated sulfate concentrations to reasonably match the observed concentrations in calibration wells G275 and G279. **Table 5-2** provides the changes to the hydraulic conductivity for the discrete reaches within the liner. During calibration the hydraulic conductivity was increased from 2.89×10^{-8} feet per day (ft/d) to 3.0×10^{-4} ft/d for reach 11 and 6.5×10^{-4} ft/d for reach 16.

For the GMF GSP, one or both of these goals were achieved at all of the transport calibration location wells (**Figure 5-26**). The model tends to underestimate the sulfate concentrations in the GMF GSP wells, excluding G215 where the simulated sulfate concentration is within the observed range. The variability in observed sulfate concentrations in the GMF GSP wells, excluding G215, is within the range of the sulfate concentrations in the background wells, as presented in **Section 5.2.3.4**. No background sulfate concentration was applied to the model, which results in general underestimation of observed sulfate concentrations.

Only calibration well G215 has observed sulfate concentration above the GWPS (400 mg/L), the remaining calibration wells are below the GWPS. Modifications to the liner conductivity of the GMF GSP have enabled the simulated sulfate concentrations to match the observed concentrations in these wells (**Table 5-2**). During calibration, the hydraulic conductivity was increased from 2.89×10^{-8} ft/d to 6.0×10^{-4} ft/d for reach 21.

In general, the calibrated transport model was able to simulate the sulfate concentrations in the remaining wells with observations above the standard GWPS for sulfate (400 mg/L) from January 2015 to October 2021 which had calibrated concentrations above the GWPS.

6. PREDICTIVE SIMULATIONS

6.1 Overview and Prediction Model Development

Prediction simulations were performed to evaluate the effects of closure (source control measures) for the GMF GSP and GMF RP on groundwater quality. The prediction simulations evaluated changes in groundwater sulfate concentrations from Scenario 1: CIP (removal of CCR from the GMF RP and southern portion of the GMF GSP and consolidation in the northern portion of the GMF GSP) and Scenario 2: CBR (removal of all CCR material from both the GMF GSP and GMF RP). As discussed in **Section 5.2.3.7**, physical attenuation (dilution and dispersion) of contaminants in groundwater is simulated in MT3DMS, which captures the physical process of natural attenuation as part of corrective actions for both closure scenarios simulated. No retardation was applied to sulfate transport in the model (*i.e.*, K_d was set to 0) as discussed in **Section 5.2.3.8**.

Closure scenarios were simulated by initially removing free liquids from the CCR material over the course of 2 years by placing drain cells within GMF GSP and GMF RP with an elevation of 610 feet and applying zero recharge to simulate dewatering of the CCR units.

HELP-calculated percolation rates, based on removal and final soil backfill grading designs provided in the Final Closure Plans for GMF GSP (Golder, 2022a), GMF RP (Golder, 2022b), and AP1 (Golder, 2022c), were applied for the different closure scenarios. HELP modeling input and output values are summarized in **Table 6-1** and described in detail below.

The CIP and CBR scenarios were simulated for a 100-year period. The following simplifying assumptions were made during the simulations:

- Removal of free liquids from CCR takes place prior to the CIP and CBR closure scenarios. Drain cells were placed within the units to simulate the removal of free water within the ponds; and recharge was set to zero.
- In the CIP and CBR closure scenarios, HELP-calculated average annual percolation rates were developed from a 30-year HELP model run. This 30-year HELP-calculated percolation rate remained constant over the duration of the closure scenario prediction model runs following CCR dewatering period.
- Changes in recharge resulting from removal of free liquids (decrease calibration model recharge rates to zero) and CCR fill removal/final soil backfill grading (recharge rates are based on HELP-calculated average annual percolation rates) have an instantaneous effect on recharge and percolation through surface materials.
- Sulfate source concentrations were assumed to be negligible (0 mg/L) in CCR removal areas in both the CIP and CBR scenarios. The spatial distribution of CCR concentrations within the consolidation area for the CIP scenario were maintained from the initial transport simulation.
- Cap construction in CIP scenario was assumed to be completed with a cover system consisting of the following (listed from ground surface down): a vegetative cover (6 inches thick), rooting zone (18 inches thick), a 200-mil geocomposite drainage layer and a 40-mil linear HDPE geomembrane.
- The start of each closure prediction simulation was initiated at the end of the calibration model period of 42 years plus 2 years to complete removal of free liquids. For example, the

simulation of Scenario 1: CIP begins at 44 years (42 years for calibration plus 2 years). The prediction modeling timeline for each scenario is illustrated in **Figure 4-1**.

- CCR consolidation/removal areas were assumed to be graded and include proper drainage controls to remove excess water from the surface using the design drawings provided (Golder, 2022a and 2022b).
- The CIP scenario includes the placement of a stormwater pond within the removal area. The outflow elevation for both the GMF GSP and GMF RP are 615 feet, which will discharge into the Unnamed Tributary adjacent to the GMF GSP and GMF RP. This is represented as a drain in the model whose elevation is equal to the outflow elevation. All saturated CCR (constant concentration cells) in the transport calibration model were removed instantaneously in all CCR removal areas for all prediction models.
- Local fill materials applied to the prediction models have similar hydraulic properties as the UCU materials used in the transport calibration models. However, the local fill materials were assumed to have reduced vertical anisotropy ratios, approaching isotropic, due to reworking of the material as it is placed as backfill (Kh/Kv decreased from measured values of 10 to 1 for reworked material).

6.2 HELP Model Setup and Results

HELP (Version 4.0; Tolaymat and Krause, 2020) was used to estimate percolation through the GMF GSP areas of CCR consolidation with final cover system and expected LF cover system. HELP input and output files are included electronically and attached to this report.

HELP input data and results are provided in **Table 6-1**. All scenarios were modeled for a period of 30 years. Climatic inputs were synthetically generated using default equations developed for Belleville Scott Air Force Base in Belleville, Illinois (the closest weather station included in the HELP database). Precipitation, temperature, and solar radiation was simulated based on the latitude of CPP. Thickness of soil backfill and soil runoff input parameters were developed for the ash fill removal scenarios using data provided in the Preliminary Closure Concepts for Coffeen AP1, GMF GSP, and GMF RP (Golder, 2022a, 2022b and 2022c).

HELP model results (**Table 6-1**) indicated 0.00019 inches of percolation per year for GMF GSP through the CCR and final cover system for the CIP scenario. No recharge rate was calculated for removal areas in both the CIP and CBR scenario as removal areas are subject to stormwater controls. HELP model results (**Table 6-1**) indicated 0.000012 inches of percolation per year for the LF through the CCR and final cover system. The differences in HELP model runs for each area included the following parameters: area, soil backfill thickness, slopes, and soil runoff slope length; all other HELP model input parameters were the same for each simulated area. HELP input data and results are provided in **Appendix A**.

Two additional HELP model simulations were completed to support the *Proposed Alternative Final Protective Layer Equivalency Demonstration*, (Geosyntec, 2022b) which is an appendix to the Construction Permit Application to which this report is also attached. Results of these two HELP simulations were not incorporated in the MODFLOW simulations for closure. Simulation inputs and output results are presented in **Appendix C**.

6.3 Simulation of Closure Scenarios

The calibrated model was used to evaluate the effectiveness of the two closure scenarios by defining CCR removal and consolidation areas, reducing head to simulate removal of free liquids, removing source concentrations from the removal areas, adding drain cells and removing recharge to simulate stormwater management within the removal areas, and applying reduced recharge in the CCR consolidation areas to simulate the effects of the cover system on flow and transport. Removal of source inputs from the ash removal areas was simulated by reducing the sulfate concentrations associated with recharge in the areas to 0 mg/L and removing constant concentration cells.

Each prediction scenario was simulated as a continuation of the GMF GSP and GMF RP dewatering simulation which followed the transient calibrated model. The prediction model input values are summarized in **Table 6-2**, and the modifications to the recharge zones and drain placement for the CIP scenario are illustrated in **Figure 6-1**. **Figure 6-2** illustrates the CCR removal area for the CBR at the GMF GSP and GMF RP. The two closure scenarios are discussed in this report based on predicted changes in sulfate concentrations as described below and results are presented in **Figure 6-3 to Figure 6-6**.

6.3.1 Closure in Place Model Results

The design for Scenario 1: CIP includes an initial 2-year dewatering period to remove free liquids followed by CCR removal from the GMF GSP and GMF RP, consolidation of CCR from both the GMF GSP and GMF RP in the northern area of GMF GSP, and construction of a cover system over the remaining CCR (**Figure 6-1**). No CCR material remains in the GMF RP (Golder, 2022b).

A general decline in sulfate concentration occurs where CCR is removed and saturated ash cells (constant concentration cells) are reduced in the area of the highest modeled source concentrations. Following removal of CCR in the southern area of the GMF GSP and entire footprint of the GMF RP, sulfate is no longer entering the model domain from recharge or from saturated ash cells (constant concentration cells). Dewatering also reduces the head within the GMF GSP and GMF RP. These low heads are maintained following completion of closure by the drain cells that simulate storm water management designs within the removal areas in both the GMF GSP and GMF RP, and limited infiltration rates (recharge) from the ash consolidation area. As a result of the reduced heads and recharge, downward percolation of solute mass from the GMF GSP is reduced and no future downward percolation of solute mass is simulated for GMF RP, which decreases the sulfate concentration entering the model domain.

6.3.1.1 GMF GSP

The predicted concentrations at the GMF GSP show a brief period of concentration increases post closure, after which there is a rapid decline in sulfate concentration. The sulfate concentrations at monitoring well G215 drive groundwater compliance, which indicate sulfate concentrations rapidly decline once the impact of the closure actions are established within the prediction model (**Figure 6-3**). Fluctuations in simulated concentration are caused by the removal of concentration during the dewatering phase and the subsequent reestablishment of groundwater flow patterns after the liner is removed from the model and the stormwater drain is established. Similar initial fluctuations in concentration are also apparent in some of the GMF RP wells, namely G279 and G275 (**Figure 6-5**).

Of the GMF GSP wells, only G215 has observations above the GWPS for sulfate (400 mg/L) at the end of the transport calibration model. The prediction model indicates that G215 will decline and reach the GWPS (400 mg/L) in approximately 5.6 years (**Figure 6-3**) after closure. The maximum extent of the plume in all layers of the model at 5.6 years is also illustrated in **Figure 6-4**. The results illustrate how sulfate concentrations above the GWPS remain within the liner of the GMF GSP consolidation area. These concentrations remain confined to the lined and capped area of the GMF GSP (zone 16 in model layer 3) throughout the simulation period of 100 years and decrease with time. The reduced recharge rate, and therefore significantly lower addition of sulfate mass into the model, leads to gradual reduction in sulfate concentration in the base of the liner over time. The residual sulfate plume from the calibrated model remains in close proximity to the GMF GSP and declines below the GWPS approximately 13 years after closure.

After closure, the CCR stored in the GMF GSP consolidation area will be completely encapsulated by the final cover and liner systems, physically isolating it from contact with surrounding soils, groundwater, surface water, and the atmosphere, and minimizing the potential release of CCR or leachate (Golder, 2022a). The existing liner system (modeled with hydraulic conductivity of 1.02×10^{-11} cm/sec, refer to **Table C** and **Table 5-1**) physically isolates the CCR stored in the GMF GSP consolidation area from the surrounding soils and groundwater, minimizing water flux through the CCR material. Removal of free liquids, consolidation, and placement of the final cover system will minimize the volume of water within the facility, further reducing the potential for water flux through the CCR material.

6.3.1.2 GMF RP

The predictive model indicates that GMF RP wells within the UA will reach the GWPS (400 mg/L) in approximately 4.6 years (**Figure 6-5**) after closure. The maximum extent of the plume in all layers of the model at this time is illustrated in **Figure 6-6**. The predicted concentrations in G275 and G279 with the greatest observed sulfate concentrations are both below the GWPS within 4.6 years. Similar to the GMF GSP, the prediction model indicates the residual sulfate plume in all layers of the model remains in close proximity to the GMF RP and declines below the GWPS approximately 14 years after closure.

6.3.2 Closure by Removal

The design for Scenario 2: CBR includes an initial 2-year dewatering period followed by CCR removal from the GMF GSP and GMF RP (**Figure 6-2**).

The prediction model shows a general decline in sulfate concentration as all CCR is removed from the GMF GSP and GMF RP, and saturated ash cells (constant concentration cells) are removed. Following removal of CCR and the liner system in both the GMF GSP and GMF RP, sulfate is no longer entering the model domain from recharge or from saturated ash cells (constant concentration cells); all source concentrations are removed. Dewatering through removal of free liquids also reduces the head within the GMF GSP and GMF RP. These low heads are maintained following completion of closure by the drain cells that simulate stormwater management designs within the GMF GSP and GMF RP.

6.3.2.1 GMF GSP

Of the GMF GSP wells, the prediction model indicates that G215 will reach the GWPS (400 mg/L) in approximately 6.6 years (**Figure 6-3**) after closure. This result is 1 year longer than the

estimate for CIP, which predicted 5.6 years to reach the GWPS. This is attributed to the minor differences in the predicted groundwater flow patterns associated with each scenario. **Figure 6-3** and **Figure 6-4** illustrate very little difference in the extent of the plume in the UA and the maximum extent of the plume in all model layers after 5.6 years, respectively. Similar to CIP, the residual sulfate plume from the calibrated model remains in close proximity to the GMF GSP and declines below the GWPS in all layers of the model approximately 13 years after closure.

6.3.2.2 GMF RP

The implementation of the CIP scenario and CBR scenario are identical with regard to the GMF RP. All CCR materials and liner system are removed from the GMF RP in both the CIP and CBR scenarios. Therefore, simulation results are very similar to those discussed in **Section 6.3.1**. In both scenarios, the time for wells in the UA to reach the GWPS is 4.6 years (**Figure 6-5**) after closure. The maximum extent of the plume in all layers of the model at this time is illustrated in **Figure 6-6**. All monitoring wells with observations above the standard GWPS for sulfate (400 mg/L) are predicted to be below the GWPS 4.6 years for the GMF RP, after closure implementation.

The residual sulfate plumes associated with the GMF RP in the CBR prediction model behave similarly to the plume in the CIP prediction model. The prediction model indicates the residual sulfate plume from the calibration model remains in close proximity to the GMF RP and declines below the GWPS in all layers of the model approximately 14 years after closure.

7. CONCLUSIONS

This GMR has been prepared to evaluate how proposed CIP and CBR scenarios will achieve compliance with the applicable groundwater standards at the CPP for the lined CCR units GMF GSP and GMF RP. An existing groundwater model was updated to include data collected from the recent 2021 field investigations and used to predict the impacts of the closure scenarios on groundwater quality at the CPP. Statistically significant correlations between sulfate concentrations and concentrations of boron and TDS identified as potential exceedances of the GWPS indicate sulfate is an acceptable surrogate for these parameters in the groundwater model. It was assumed that sulfate would not significantly sorb or chemically react with aquifer solids (K_d was set to 0 mL/g) which is a conservative estimate for predicting contaminant transport times in the model. The MODFLOW and MT3DMS models were used to evaluate two scenarios including information provided in the Final Closure Plans (Golder, 2022a and 2022b):

- **Scenario 1:** CIP including removal of CCR from the GMF RP and the southern portion of the GMF GSP, consolidation into the northern portion of the GMF GSP, and construction of a cover system over the remaining CCR.
- **Scenario 2:** CBR including removal of all CCR and SI liner system and regrading of the removal area.

The existing liner system (modeled with hydraulic conductivity of 1.02×10^{-11} cm/sec) physically isolates the CCR stored in the GMF GSP consolidation area from the surrounding soils and groundwater minimizing water flux through the CCR material. Removal of free liquids, consolidation, and placement of the final cover system will minimize the volume of water within the facility, further reducing the potential for water flux through the CCR material.

There are limited differences in the timeframes for groundwater to reach the GWPS for most monitoring wells at the GMF GSP and GMF RP between CIP and CBR.

- In general, the simulated groundwater concentrations in the monitoring wells within the UA will achieve the GWPS in approximately 7 years for both the CIP and CBR closure scenarios at the GMF GSP.
 - A minor difference exists in the predicted timeframes for the GMF GSP, such that the timeframes to reach the GWPS differs by 1 year, with the CIP predicting 5.6 years to reach the GWPS and the CBR predicting 6.6 years. This difference in timeframe is not significant and can be attributed to the minor differences in the predicted groundwater flow patterns associated with the scenarios.
- For the GMF RP, the simulated groundwater concentrations in the monitoring wells within the UA will achieve the GWPS in approximately 4.6 years for both the CIP and CBR closure scenarios.
- The residual sulfate plumes from the calibrated model associated with both the GMF GSP and GMF RP remain in close proximity to the CCR units and are simulated to decline below the GWPS (400 mg/L) in all layers of the model 13 and 14 years after closure, respectively, for CIP and CBR.

Results of groundwater fate and transport modeling conservatively estimate that groundwater concentrations will attain the GWPS for all constituents identified as potential exceedances of the GWPS in the UA monitoring wells at the GMF GSP and GMF RP within 7 years of closure implementation for both CIP and CBR.

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TABLES

TABLE 2-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS

GROUNDWATER MODELING REPORT

COFFEEN POWER PLANT

GMF GYPSUM STACK POND AND GMF RECYCLE POND

COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (feet bgs)	Screen Bottom Depth (feet bgs)	Screen Top Elevation (feet)	Screen Bottom Elevation (feet)	Well Depth (feet bgs)	Bottom of Boring Elevation (feet)	Screen Length (feet)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G045D	LCU	08/17/2016	623.81	623.81	Top of PVC	620.94	31.88	41.52	589.06	579.42	41.92	578.90	9.6	2	39.064349	-89.396281
G046D	LCU	08/19/2017	625.24	625.24	Top of PVC	621.91	41.61	51.26	580.30	570.65	51.65	569.90	9.7	2	39.060305	-89.398524
G101	UA	02/02/2010	--	627.60	Top of Disk	625.27	15.68	20.32	609.59	604.95	20.89	603.40	4.6	2	39.071386	-89.400107
G102	UA	04/28/2006	--	629.04	Top of Disk	626.18	12.02	16.78	614.16	609.40	17.15	609.00	4.8	2	39.071387	-89.398991
G103	UA	02/15/2010	--	633.80	Top of Disk	627.94	15.88	20.67	612.06	607.27	21.09	606.90	4.8	2	39.070412	-89.399107
G104	UA	02/15/2010	--	632.94	Top of Disk	627.96	14.91	19.61	613.05	608.35	20.08	605.80	4.7	2	39.069451	-89.399104
G105	UA	02/16/2010	--	632.08	Top of Disk	626.86	16.11	20.90	610.75	605.96	21.37	604.40	4.8	2	39.068491	-89.3991
G106	UA	02/16/2010	--	631.15	Top of Disk	625.96	14.37	18.96	611.59	607.00	19.44	605.50	4.6	2	39.06753	-89.399097
G107	UA	02/17/2010	630.22	630.22	Top of Disk	628.20	13.87	18.50	614.33	609.70	19.00	607.50	4.6	2	39.067106	-89.399646
G108	UA	02/12/2010	--	630.22	Top of Disk	625.58	16.82	21.50	608.76	604.08	22.00	603.60	4.7	2	39.066984	-89.400035
G109	UA	02/11/2010	--	629.76	Top of Disk	624.79	15.39	19.93	609.40	604.86	20.50	604.30	4.5	2	39.067045	-89.400423
G110	UA	02/11/2010	--	629.65	Top of Disk	624.81	15.05	19.59	609.76	605.22	20.16	604.70	4.5	2	39.067172	-89.400704
G111	UA	02/11/2010	--	629.90	Top of Disk	625.28	14.61	19.15	610.67	606.13	19.72	605.60	4.5	2	39.067292	-89.40097
G119	UA	02/09/2010	--	631.55	Top of Disk	626.57	17.29	21.83	609.28	604.74	22.38	604.20	4.5	2	39.068986	-89.401213
G120	UA	02/08/2010	--	631.87	Top of Disk	627.21	15.10	19.62	612.11	607.59	20.21	605.10	4.5	2	39.069479	-89.401214
G121	UA	02/04/2010	--	632.83	Top of Disk	627.94	16.79	21.47	611.15	606.47	21.95	603.80	4.7	2	39.069781	-89.401216
G122	UA	02/04/2010	--	632.69	Top of Disk	628.05	16.51	21.05	611.54	607.00	21.66	606.20	4.5	2	39.070098	-89.401218
G123	UA	02/04/2010	--	632.96	Top of Disk	628.12	20.94	25.46	607.18	602.66	26.07	602.10	4.5	2	39.070399	-89.401219
G124	UA	02/03/2010	--	633.39	Top of Disk	628.70	15.98	20.51	612.72	608.19	21.06	606.70	4.5	2	39.070715	-89.40122
G125	UA	02/03/2010	--	633.51	Top of Disk	628.85	17.03	21.56	611.82	607.29	22.04	606.80	4.5	2	39.071003	-89.401221
G126	UA	02/10/2010	--	625.39	Top of Disk	622.96	12.89	17.43	610.07	605.53	18.00	605.00	4.5	2	39.067304	-89.401274
G151	UA	12/19/2011	--	625.93	Top of Disk	622.82	15.34	19.84	607.48	602.98	20.46	602.40	4.5	2	39.0672	-89.40159
G152	UA	12/20/2011	--	626.52	Top of Disk	623.06	13.59	18.09	609.47	604.97	18.57	604.50	4.5	2	39.066275	-89.401289
G153	UA	12/15/2011	626.35	626.40	Top of Disk	623.23	15.90	20.34	607.33	602.89	20.80	602.50	4.4	2	39.065857	-89.402567
G154	UA	12/16/2011	--	626.35	Top of Disk	623.52	14.26	18.76	609.26	604.76	19.10	603.50	4.5	2	39.067089	-89.403574
G155	UA	12/19/2011	--	625.86	Top of Disk	622.89	15.09	19.58	607.80	603.31	23.23	599.70	4.5	2	39.067493	-89.402659
G200	UA	02/25/2008	--	625.94	Top of Disk	623.27	12.19	16.98	611.08	606.29	17.36	605.30	4.8	2	39.075139	-89.395009
G201	UA	02/25/2008	627.15	627.15	Top of Riser	624.19	13.01	17.80	611.18	606.39	18.15	606.00	4.8	2	39.075141	-89.397829
G205	UA	02/21/2008	--	624.34	Top of Disk	622.10	10.04	14.53	612.06	607.57	15.07	606.10	4.5	2	39.068596	-89.394147
G206	UA	10/14/2010	--	632.82	Top of Disk	630.53	17.51	21.92	613.02	608.61	22.42	606.50	4.4	2	39.067399	-89.398548
G206D	DA	01/25/2021	634.14	634.14	Top of PVC	631.41	49.20	59.00	582.21	572.41	59.39	571.41	9.8	2	39.067428	-89.398493
G207	UA	10/08/2010	--	633.21	Top of Disk	630.61	18.24	22.77	612.37	607.84	23.30	606.60	4.5	2	39.067568	-89.397952
G208	UA	10/07/2010	--	633.16	Top of Disk	630.57	17.53	22.06	613.04	608.51	22.60	606.60	4.5	2	39.067743	-89.397402
G209	UA	10/07/2010	--	632.91	Top of Disk	630.57	17.74	22.28	612.83	608.29	22.81	606.60	4.5	2	39.067923	-89.39685
G210	UA	10/06/2010	--	632.99	Top of Disk	630.48	19.39	23.93	611.09	606.55	24.46	605.50	4.5	2	39.068088	-89.396322
G211	UA	10/11/2010	--	632.64	Top of Disk	630.31	17.34	21.88	612.97	608.43	22.41	606.30	4.5	2	39.068263	-89.395792
G212	UA	10/11/2010	--	632.89	Top of Disk	630.59	16.74	21.29	613.85	609.30	21.81	606.60	4.6	2	39.06843	-89.395318
G213	UA	10/12/2010	--	632.81	Top of Disk	630.34	16.75	21.29	613.59	609.05	21.82	606.30	4.5	2	39.068585	-89.394822
G214	UA	10/14/2010	--	632.85	Top of Disk	630.39	17.75	22.14	612.64	608.25	22.65	606.40	4.4	2	39.068919	-89.393982

TABLE 2-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (feet bgs)	Screen Bottom Depth (feet bgs)	Screen Top Elevation (feet)	Screen Bottom Elevation (feet)	Well Depth (feet bgs)	Bottom of Boring Elevation (feet)	Screen Length (feet)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G215	UA	10/13/2010	--	633.06	Top of Disk	630.48	19.41	23.80	611.07	606.68	24.31	606.20	4.4	2	39.069309	-89.39394
G216	UA	10/13/2010	--	632.76	Top of Disk	630.28	20.04	24.42	610.24	605.86	24.93	604.30	4.4	2	39.069765	-89.393946
G217	UA	10/12/2010	--	633.10	Top of Disk	630.67	20.49	24.88	610.18	605.79	25.38	604.70	4.4	2	39.07034	-89.393959
G218	UA	10/12/2010	--	633.11	Top of Disk	630.64	20.33	24.77	610.31	605.87	25.27	604.60	4.4	2	39.070876	-89.393956
G270	UA	02/26/2008	--	625.86	Top of Disk	623.73	13.13	17.92	610.60	605.81	18.27	605.50	4.8	2	39.066564	-89.397403
G271	UA	09/10/2009	--	625.57	Top of Disk	622.89	9.96	14.31	612.93	608.58	14.79	606.90	4.4	2	39.065007	-89.395587
G272	UA	09/10/2009	--	623.81	Top of Disk	620.72	9.11	13.98	611.61	606.74	14.32	606.40	4.9	2	39.064989	-89.394785
G273	UA	09/10/2009	--	623.02	Top of Disk	620.17	9.08	14.56	611.09	605.61	15.10	604.20	5.5	2	39.064985	-89.393973
G274	UA	09/16/2009	--	624.04	Top of Disk	621.67	12.90	17.67	608.77	604.00	18.06	603.60	4.8	2	39.064991	-89.393198
G275	UA	09/16/2009	--	618.26	Top of Disk	616.14	8.22	12.62	607.92	603.52	13.19	603.00	4.4	2	39.065151	-89.392561
G275D	DA	01/14/2021	620.31	620.31	Top of PVC	617.52	49.76	59.55	567.76	557.97	59.89	517.80	9.8	2	39.065121	-89.392595
G276	UA	09/16/2009	--	632.00	Top of Disk	629.14	22.41	27.22	606.73	601.92	27.65	601.10	4.8	2	39.065534	-89.392617
G277	UA	09/14/2009	--	623.08	Top of Disk	620.79	14.29	18.77	606.50	602.02	19.24	600.80	4.5	2	39.065927	-89.392572
G278	UA	09/11/2009	631.19	631.17	Top of Disk	628.85	18.93	23.70	609.92	605.15	24.06	604.80	4.8	2	39.066737	-89.393161
G279	UA	09/10/2009	--	632.04	Top of Disk	629.19	22.40	26.79	606.79	602.40	27.30	601.20	4.4	2	39.067156	-89.392998
G280	UA	02/26/2008	625.35	625.35	Top of Riser	623.11	12.79	17.63	610.32	605.48	17.98	605.10	4.8	2	39.067216	-89.394992
G281	UA	09/08/2015	--	626.36	Top of Disk	623.82	15.51	20.16	608.31	603.66	20.30	603.50	4.7	2	39.065405	-89.399322
G283	LCU	01/14/2021	610.75	610.75	Top of PVC	608.30	8.39	18.17	599.91	590.13	18.36	589.90	9.8	2	39.064645	-89.392119
G284	UA	02/03/2021	618.42	618.42	Top of PVC	615.33	8.08	12.85	607.25	602.48	13.23	601.30	4.8	2	39.065487	-89.390631
G285	LCU	01/25/2021	613.52	613.52	Top of PVC	610.54	13.68	23.45	596.86	587.09	23.83	584.50	9.8	2	39.066513	-89.391474
G286	UA	01/18/2021	613.13	613.13	Top of PVC	609.97	3.37	8.16	606.60	601.81	8.50	600.00	4.8	2	39.067277	-89.391883
G287	UA	01/20/2021	617.45	617.45	Top of PVC	614.34	5.43	10.25	608.91	604.09	10.59	602.50	4.8	2	39.068297	-89.392388
G288	UA	01/19/2021	620.07	620.07	Top of PVC	617.08	7.59	12.26	609.49	604.82	12.75	603.10	4.7	2	39.067834	-89.390082
G301	UA	09/04/2015	--	622.65	Top of Disk	620.88	11.31	15.96	608.96	604.31	16.21	604.10	4.7	2	39.05951	-89.395415
G302	UA	09/04/2015	--	620.04	Top of Disk	618.52	13.21	17.86	604.74	600.09	18.39	599.60	4.7	2	39.059544	-89.393192
G303	UA	08/26/2010	--	622.02	Top of Disk	619.33	10.00	20.00	609.07	599.07	20.40	598.70	10	2	39.057144	-89.391721
G304	UA	08/26/2010	--	626.72	Top of Disk	623.32	10.00	20.00	613.32	603.32	20.40	602.90	10	2	39.057205	-89.395663
G305	UA	05/03/2016	625.67	625.67	Top of PVC	623.23	13.44	18.27	609.10	604.27	18.50	604.10	4.8	2	39.056558	-89.396798
G306	UA	05/03/2016	625.91	625.91	Top of PVC	623.57	13.07	17.68	609.77	605.16	17.90	604.80	4.6	2	39.056494	-89.393556
G307	UA	07/27/2016	624.60	624.60	Top of PVC	624.73	12.96	17.80	609.12	604.28	18.22	603.90	4.8	2	39.057214	-89.395545
G307D	LCU	01/19/2021	624.88	624.88	Top of PVC	622.51	48.98	58.75	573.53	563.76	59.60	562.50	9.8	2	39.05721	-89.39552
G308	UA	01/18/2021	624.59	624.59	Top of PVC	621.59	10.10	14.89	611.49	606.70	15.24	605.80	4.8	2	39.057379	-89.397134
G309	UA	01/21/2021	625.88	625.88	Top of PVC	622.77	12.97	17.75	609.80	605.02	18.10	604.70	4.8	2	39.058508	-89.397243
G310	UA	02/09/2021	622.87	622.87	Top of PVC	619.89	10.24	15.03	609.65	604.86	15.38	604.00	4.8	2	39.059532	-89.396907
G311	UA	01/13/2021	621.04	621.04	Top of PVC	618.32	9.27	14.04	609.05	604.28	14.40	603.90	4.8	2	39.059513	-89.394363
G311D	LCU	01/12/2021	621.24	621.24	Top of PVC	618.39	50.16	60.10	568.23	558.29	60.58	557.80	9.9	2	39.059513	-89.394312
G312	UA	01/15/2021	619.78	619.78	Top of PVC	616.92	9.79	14.58	607.13	602.34	14.93	601.70	4.8	2	39.059558	-89.391983
G313	UA	02/05/2021	614.30	614.30	Top of PVC	611.51	6.30	11.11	605.21	600.40	11.46	599.50	4.8	2	39.058773	-89.391124
G314	LCU	02/05/2021	613.88	613.88	Top of PVC	611.11	14.56	19.58	596.55	591.53	20.02	591.10	5	2	39.05782	-89.390964

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GROUNDWATER MODELING REPORT

COFFEEN POWER PLANT

GMF GYPSUM STACK POND AND GMF RECYCLE POND

COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (feet bgs)	Screen Bottom Depth (feet bgs)	Screen Top Elevation (feet)	Screen Bottom Elevation (feet)	Well Depth (feet bgs)	Bottom of Boring Elevation (feet)	Screen Length (feet)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G314D	DA	02/04/2021	613.70	613.70	Top of PVC	610.87	39.34	49.11	571.53	561.76	49.47	510.60	9.8	2	39.057852	-89.390958
G315	UA	01/14/2021	623.52	623.52	Top of PVC	620.94	9.69	14.48	611.25	606.46	14.85	605.00	4.8	2	39.057165	-89.393667
G316	LCU	02/26/2021	602.59	602.59	Top of PVC	599.64	10.02	14.82	589.62	584.82	15.16	583.90	4.8	2	39.057847	-89.389698
G317	UA	02/12/2021	641.93	641.93	Top of PVC	638.85	30.14	34.93	608.71	603.92	35.28	602.90	4.8	2	39.056727	-89.390148
G401	UA	09/14/2015	--	625.57	Top of Disk	623.03	14.36	18.79	608.67	604.24	19.29	603.70	4.4	2	39.060259	-89.395295
G402	UA	08/27/2010	--	613.37	Top of Disk	610.36	10.00	20.00	600.36	590.36	20.40	590.00	10	2	39.060207	-89.391712
G403	UA	09/11/2015	--	626.47	Top of Disk	623.81	13.11	17.78	610.70	606.03	18.15	605.70	4.7	2	39.063167	-89.398779
G404	UA	05/01/2007	--	615.67	Top of Disk	613.57	6.42	11.17	607.15	602.40	11.62	601.60	4.8	2	39.064329	-89.392493
G405	UA	05/01/2007	--	623.63	Top of Disk	621.40	9.01	13.76	612.39	607.64	14.21	607.20	4.8	2	39.064345	-89.396234
G406	UA	08/19/2016	625.36	625.36	Top of PVC	621.86	13.56	18.37	608.30	603.49	18.75	603.10	4.8	2	39.060309	-89.398508
G407	UA	08/16/2016	621.32	621.32	Top of PVC	618.35	13.78	18.61	604.57	599.74	19.04	598.40	4.8	2	39.061574	-89.402004
G410	UA	02/23/2018	--	619.79	Top of Disk	617.21	8.89	13.68	608.32	603.53	14.09	603.10	4.8	2	39.061572	-89.403763
G411	UA	02/22/2018	--	623.25	Top of Disk	620.49	11.21	16.07	609.28	604.42	16.47	604.00	4.9	2	39.063979	-89.404033
MW01D	DA	05/03/2006	609.02	609.02	Top of PVC	607.08	33.29	38.05	573.79	569.03	38.41	567.10	4.8	2	39.067068	-89.402747
MW02S	UA	05/05/2006	627.12	627.12	Top of PVC	624.16	10.34	15.12	613.82	609.04	15.51	608.70	4.8	2	39.071017	-89.403648
MW02D	LCU	05/05/2006	626.99	626.99	Top of PVC	624.14	22.03	26.83	602.11	597.31	27.22	596.90	4.8	2	39.071031	-89.403649
MW03D	DA	04/27/2006	629.01	629.01	Top of PVC	625.86	52.29	57.06	573.57	568.80	57.40	567.90	4.8	2	39.071386	-89.398976
MW04S	UA	05/11/2006	625.89	625.89	Top of PVC	622.63	9.83	14.26	612.80	608.37	14.77	607.90	4.4	2	39.075356	-89.399232
MW05S	UA	05/17/2006	625.95	625.95	Top of PVC	622.65	12.66	17.41	609.99	605.24	17.71	604.90	4.8	2	39.075866	-89.40333
MW05D	DA	05/17/2006	625.91	625.91	Top of PVC	622.65	45.57	50.33	577.08	572.32	50.72	568.70	4.8	2	39.075863	-89.403313
MW06S	UA	05/04/2006	626.15	626.15	Top of PVC	623.37	11.04	15.62	612.33	607.75	16.08	607.30	4.6	2	39.078189	-89.403644
MW07S	UA	05/09/2006	627.60	627.60	Top of PVC	624.90	9.91	13.79	614.99	611.11	14.39	610.50	3.9	2	39.0786	-89.399383
MW08S	UA	05/10/2006	628.01	628.01	Top of PVC	625.09	11.51	16.00	613.58	609.09	16.60	608.00	4.5	2	39.080234	-89.399079
MW09S	UA	05/03/2006	627.62	627.62	Top of PVC	624.70	11.21	15.62	613.49	609.08	16.20	608.50	4.4	2	39.079954	-89.394899
MW09D	LCU	05/03/2006	627.61	627.61	Top of PVC	624.68	45.81	50.57	578.87	574.11	51.00	570.70	4.8	2	39.07994	-89.394899
MW10S	UA	05/02/2006	624.45	624.45	Top of PVC	621.43	11.28	15.76	610.15	605.67	16.30	605.10	4.5	2	39.07601	-89.394068
MW10D	LCU	05/01/2006	624.47	624.47	Top of PVC	621.33	41.74	46.57	579.59	574.76	47.02	572.60	4.8	2	39.075995	-89.39407
MW11S	UA	04/28/2006	625.27	625.27	Top of PVC	622.04	8.89	13.63	613.15	608.41	14.08	608.00	4.7	2	39.071888	-89.393913
MW11D	LCU	04/28/2006	625.52	625.52	Top of PVC	622.19	28.31	33.04	593.88	589.15	33.50	585.90	4.7	2	39.071888	-89.393894
MW12S	UA	05/10/2006	625.31	625.31	Top of PVC	622.24	10.61	15.18	611.63	607.06	15.61	606.60	4.6	2	39.068514	-89.394199
MW12D	DA	05/10/2006	625.21	625.21	Top of PVC	622.24	42.46	46.99	579.78	575.25	47.47	572.20	4.5	2	39.068501	-89.394199
MW13S	UA	05/09/2006	625.96	625.96	Top of PVC	622.80	11.43	16.23	611.37	606.57	16.62	606.20	4.8	2	39.066297	-89.40118
MW13D	DA	05/09/2006	625.86	625.86	Top of PVC	622.85	49.81	54.60	573.04	568.25	55.00	567.90	4.8	2	39.066293	-89.401163
MW14S	UA	05/02/2006	626.88	626.88	Top of PVC	624.62	12.26	17.02	612.36	607.60	17.38	607.20	4.8	2	39.069153	-89.400442
MW15S	UA	04/25/2006	626.66	626.66	Top of PVC	623.83	14.41	19.16	609.42	604.67	19.62	604.20	4.8	2	39.069772	-89.397088
MW15D	LCU	04/25/2006	626.44	626.44	Top of PVC	623.83	33.68	38.45	590.15	585.38	38.80	585.00	4.8	2	39.06977	-89.397073
MW16S	UA	04/25/2006	629.47	629.47	Top of PVC	626.32	14.59	19.41	611.73	606.91	19.76	606.40	4.8	2	39.073571	-89.397006
MW16D	DA	04/25/2006	629.38	629.38	Top of PVC	626.37	45.90	50.34	580.47	576.03	50.78	575.40	4.4	2	39.073571	-89.397036
MW17S	UA	05/04/2006	630.56	630.56	Top of PVC	627.28	14.02	23.56	613.26	603.72	24.11	603.20	9.5	2	39.07715	-89.396978

TABLE 2-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Well Number	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (feet bgs)	Screen Bottom Depth (feet bgs)	Screen Top Elevation (feet)	Screen Bottom Elevation (feet)	Well Depth (feet bgs)	Bottom of Boring Elevation (feet)	Screen Length (feet)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
MW17D	DA	05/04/2006	630.29	630.29	Top of PVC	627.47	48.82	53.32	578.65	574.15	53.87	573.60	4.5	2	39.077151	-89.396958
MW18S	UA	05/11/2006	628.66	628.66	Top of PVC	625.69	11.31	15.79	614.38	609.90	16.40	609.30	4.5	2	39.077033	-89.401698
MW20S	UA	05/01/2007	622.90	622.90	Top of PVC	620.26	8.41	13.22	611.85	607.04	13.67	604.30	4.8	2	39.064968	-89.394322
R104	UA	10/08/2010	--	632.84	Top of Disk	629.03	14.59	19.32	614.44	609.71	19.85	609.20	4.7	2	39.069474	-89.399109
R201	UA	10/08/2010	--	626.34	Top of Disk	624.02	14.59	19.32	609.43	604.70	19.85	604.20	4.7	2	39.075142	-89.397855
R205	UA	03/20/2017	--	624.52	Top of Disk	621.91	11.32	16.01	610.59	605.90	16.42	605.50	4.7	2	39.068593	-89.394164
T127	UA	02/10/2010	--	630.96	Top of Disk	625.53	17.53	22.07	608.00	603.46	22.64	602.90	4.5	2	39.068119	-89.40121
T128	UA	02/09/2010	631.03	630.93	Top of Disk	626.27	16.53	21.04	609.74	605.23	21.64	602.20	4.5	2	39.068532	-89.401211
T202	UA	10/15/2010	--	628.63	Top of Disk	626.22	12.27	16.65	613.95	609.57	17.21	608.20	4.4	2	39.071776	-89.397705
T408	LCU	08/17/2016	624.08	624.08	Top of PVC	621.09	20.66	25.49	600.43	595.60	25.92	595.20	4.8	2	39.064353	-89.396307
T409	LCU	08/19/2016	625.01	625.01	Top of PVC	621.85	21.79	26.59	600.06	595.26	26.99	594.90	4.8	2	39.0603	-89.398538
TA31	UA	10/28/2014	626.55	626.55	Top of PVC	623.89	15.09	19.57	608.80	604.32	20.19	603.70	4.5	2	39.071368	-89.401366
TA32	UA	10/27/2014	621.42	621.42	Top of PVC	618.93	11.31	15.68	607.62	603.25	16.47	602.50	4.4	2	39.074093	-89.402223
TA33	UA	06/02/2015	625.27	625.27	Top of PVC	622.51	12.23	16.89	610.28	605.62	17.44	605.10	4.7	2	39.071556	-89.403506
TA34	UA	06/03/2015	626.52	626.52	Top of PVC	624.10	10.92	15.41	613.18	608.69	16.10	608.00	4.5	2	39.069631	-89.402759
TR32	UA	07/02/2021	621.68	621.68	Top of PVC	619.28	11.00	15.68	608.28	603.60	16.17	603.11	4.68	2	39.074064	-89.397758
NE Riser	S	--	--	626.13	--	--	--	--	--	--	--	--	--	--	39.071111	-89.393889
SG-02	SW	--	--	605.87	Top of Protective Casing	605.87	--	--	--	--	--	--	--	--	39.059695	-89.391429
SG-03	SW	--	--	594.94	Top of Protective Casing	594.94	--	--	--	--	--	--	--	--	39.059092	-89.390342
SG-04	SW	--	--	599.52	Top of Protective Casing	599.52	--	--	--	--	--	--	--	--	39.064146	-89.390504

Notes:
 All elevation data are presented relative to the North American Vertical Datum 1988 (NAVD88), GEOID 12A
 -- = data not available
 bgs = below ground surface
 DA = deep aquifer
 ft = foot or feet
 HSU = hydrostratigraphic Unit
 LCU = lower confining unit
 PVC = polyvinyl chloride
 S = source water
 SW = surface water
 UA = uppermost aquifer
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TABLE 4-1. FLOW AND TRANSPORT MODEL CALIBRATION TARGETS

GROUNDWATER MODELING REPORT

COFFEEN POWER PLANT

GMF GYPSUM STACK POND AND GMF RECYCLE POND

COFFEEN, ILLINOIS

Well Name	Easting	Northing	HSU	CCR Unit	Flow Targets								Transport Targets							
					Number of Samples	mean GWL ¹ (feet)	std GWL ¹ (feet)	min GWL ¹ (feet)	max GWL ¹ (feet)	Earliest Sample Date	Latest Sample Date	Flow Calibration Wells	Number of Samples	mean Sulfate (mg/L)	std Sulfate (mg/L)	min Sulfate (mg/L)	max Sulfate (mg/L)	Earliest Sample Date	Latest Sample Date	Transport Calibration Well
G101	2514214.26	876551.76	UA	LF	20	617.989	2.504194166	612.95	623.65	15/01/2019	16/11/2015	Yes	-	-	-	-	-	-	-	-
G102	2514531.1	876554.8	UA	GSP	25	622.8612	1.751842649	618.96	627.12	15/01/2019	16/11/2015	Yes	19	90.6	29.7	49	140	04/08/2015	01/26/2021	Yes
G103	2514501.17	876199.41	UA	GSP	19	622.0884211	1.754825927	617.95	624.93	15/01/2019	11/12/2016	Yes	3	66.3	11.2	54	76	04/08/2015	10/06/2015	Yes
G105	2514509.06	875499.78	UA	GSP	19	622.0884211	2.178504235	613.96	624	15/01/2019	11/12/2016	Yes	3	116.7	11.5	110	130	04/08/2015	10/06/2015	Yes
G106	2514512.87	875149.77	UA	GSP	20	620.763	1.194844628	617.46	622.6	15/01/2019	16/11/2015	Yes	19	66.1	23.3	36	140	04/08/2015	01/26/2021	Yes
G107	2514358.3	874994.03	UA	LF	19	619.1036842	1.658802147	615.46	622.33	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G108	2514248.22	874948.67	UA	LF	19	619.4994737	1.31911786	616.24	622.22	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G109	2514137.87	874969.96	UA	LF	19	618.7294737	1.25543031	615.7	620.84	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G110	2514057.7	875015.54	UA	LF	20	618.104	1.590105591	613.27	620.65	15/01/2019	16/11/2015	Yes	-	-	-	-	-	-	-	-
G111	2513981.81	875058.61	UA	LF	19	616.9310526	1.267626368	613.16	618.53	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G119	2513907.62	875675	UA	LF	19	615.9689474	1.16332328	612.24	617.45	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G120	2513905.82	875854.56	UA	LF	19	614.3242105	1.834418817	612.13	617.69	15/01/2019	16/11/2015	Yes	-	-	-	-	-	-	-	-
G121	2513904.33	875964.54	UA	LF	18	614.6861111	2.034979806	611.93	618.73	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G122	2513902.79	876080	UA	LF	18	615.3283333	2.095957594	612.94	620.41	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G123	2513901.58	876189.62	UA	LF	18	614.5494444	3.842648401	610.31	622.79	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G124	2513900.33	876304.71	UA	LF	19	617.8857895	2.128430083	615.09	622.86	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G125	2513899.16	876409.6	UA	LF	20	619.676	2.365809976	614.6	622.96	15/01/2019	16/11/2015	Yes	-	-	-	-	-	-	-	-
G126	2513895.46	875062.25	UA	LF	19	614.87	1.340053896	612.28	616.87	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G151	2513806.06	875023.62	UA	LF	16	614.468125	0.894980214	612.13	615.49	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G152	2513894.35	874687.44	UA	SW	16	615.421875	1.122949799	612.77	617.44	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G153	2513532.77	874532.15	UA	SW	16	614.5425	1.204416871	612.37	616.3	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G154	2513243.08	874978.46	UA	SW	16	614.16	1.731546515	610.33	618.28	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G155	2513501.64	875127.78	UA	SW	16	613.686875	1.278998143	609.91	615.99	15/01/2019	11/12/2016	Yes	-	-	-	-	-	-	-	-
G200	2515650.03	877930.9	UA	B	26	621.4965385	1.461968378	618.16	623.29	15/01/2019	16/11/2015	Yes	25	101.2	8.3	87	120	01/20/2015	07/28/2021	-
G205	2515915	875549.93	UA	GSP	8	619.71	1.482912193	616.33	621.45	02/04/2017	11/12/2016	Yes	-	-	-	-	-	-	-	-
G206	2514669.15	875103.38	UA	GSP	25	621.286	1.444036588	616.61	622.76	15/01/2019	16/11/2015	Yes	20	119.4	24.7	32	150	01/21/2015	01/27/2021	Yes
G207	2514837.85	875166.36	UA	GSP	19	621.9526316	1.135658605	619.41	623.39	15/01/2019	11/12/2016	Yes	4	44.5	30.1	16	72	01/21/2015	10/07/2015	Yes
G208	2514993.46	875231.42	UA	GSP	19	622.0989474	1.175154339	618.97	624.07	15/01/2019	11/12/2016	Yes	4	53.5	37.7	33	110	01/21/2015	10/07/2015	Yes
G209	2515149.64	875298.3	UA	GSP	25	621.6212	1.211081885	617.76	623.18	15/01/2019	16/11/2015	Yes	20	248.8	51.6	95	310	01/21/2015	01/27/2021	Yes
G210	2515299.04	875359.67	UA	GSP	19	620.8747368	1.372254303	616.82	622.5	15/01/2019	11/12/2016	Yes	4	90.3	6.5	84	99	01/21/2015	10/07/2015	Yes
G211	2515448.98	875424.68	UA	GSP	19	621.1094737	1.148145721	618.14	622.45	15/01/2019	11/12/2016	Yes	4	79.8	5.4	74	87	01/21/2015	10/07/2015	Yes
G212	2515583.04	875486.65	UA	GSP	25	620.7644	1.197814259	617.19	622.12	15/01/2019	16/11/2015	Yes	20	55.9	4.2	49	66	01/21/2015	01/26/2021	Yes
G213	2515723.38	875544.3	UA	GSP	19	620.6210526	0.889262458	618.62	621.72	15/01/2019	11/12/2016	Yes	4	53.3	3.3	50	57	01/21/2015	10/07/2015	Yes
G214	2515960.85	875667.97	UA	GSP	19	617.8473684	1.193332598	614.52	619.39	15/01/2019	11/12/2016	Yes	4	71.3	3.9	68	76	01/21/2015	10/07/2015	Yes
G215	2515971.56	875810.11	UA	GSP	25	617.9504	1.033285537	615.48	619.51	15/01/2019	16/11/2015	Yes	21	167.1	109.9	100	490	01/21/2015	06/29/2021	Yes
G216	2515968.45	875976.18	UA	GSP	19	617.8368421	1.365349172	614.37	619.86	15/01/2019	11/12/2016	Yes	4	217.5	9.6	210	230	01/21/2015	10/07/2015	Yes
G217	2515962.98	876185.57	UA	GSP	19	617.5063158	1.127668246	614.32	619.13	15/01/2019	11/12/2016	Yes	4	132.5	5.0	130	140	01/21/2015	10/07/2015	Yes
G218	2515962.17	876380.8	UA	GSP	25	618.3172	1.25211328	614.46	620.1	01/15/2019	11/16/2015	Yes	20	135.8	34.0	94	220	01/21/2015	01/26/2021	Yes
G270	2514996.81	874802.01	UA	RP	26	620.3503846	2.547542315	614.45	623.38	01/15/2019	11/16/2015	Yes	21	69.8	25.8	49	140	01/20/2015	03/30/2021	Yes
G271	2515517.24	874239.3	UA	RP	25	615.7952	1.212807075	613.31	617.95	01/15/2019	11/16/2015	Yes	6	455.0	89.6	340	610	08/10/2018	02/01/2021	Yes

TABLE 4-1. FLOW AND TRANSPORT MODEL CALIBRATION TARGETS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Well Name	Easting	Northing	HSU	CCR Unit	Flow Targets								Transport Targets							
					Number of Samples	mean GWL ¹ (feet)	std GWL ¹ (feet)	min GWL ¹ (feet)	max GWL ¹ (feet)	Earliest Sample Date	Latest Sample Date	Flow Calibration Wells	Number of Samples	mean Sulfate (mg/L)	std Sulfate (mg/L)	min Sulfate (mg/L)	max Sulfate (mg/L)	Earliest Sample Date	Latest Sample Date	Transport Calibration Well
G272	2515745.01	874234.68	UA	RP	19	614.3836842	1.271854335	611.45	616.88	01/15/2019	12/11/2016	Yes	4	332.5	45.7	270	380	01/21/2015	10/08/2015	Yes
G273	2515975.58	874235.18	UA	RP	25	611.5884	1.339299195	608.82	614.2	01/15/2019	11/16/2015	Yes	20	475.0	89.5	360	690	01/21/2015	02/01/2021	Yes
G274	2516195.61	874239.23	UA	RP	19	610.4968421	1.009549144	607.79	612	01/15/2019	12/11/2016	Yes	4	322.5	53.2	260	390	01/21/2015	10/08/2015	Yes
G275	2516375.98	874299.05	UA	RP	19	604.7021053	0.833210517	602.97	605.97	01/15/2019	12/11/2016	Yes	3	780.0	147.3	650	940	01/21/2015	07/23/2015	Yes
G276	2516358.89	874438.41	UA	RP	24	604.3108333	0.781508667	603.11	606.6	01/15/2019	11/16/2015	Yes	19	223.6	59.6	19	310	01/21/2015	06/28/2021	Yes
G277	2516370.45	874581.65	UA	RP	15	602.6546667	0.949126415	601.23	603.79	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
G278	2516200.7	874875.24	UA	RP	19	605.7357895	1.268819731	604.29	608.15	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
G279	2516245.69	875028.24	UA	RP	24	607.4420833	2.205378759	599.69	611.08	01/15/2019	11/16/2015	Yes	20	569.0	336.3	170	1600	01/21/2015	01/28/2021	Yes
G280	2515679.35	875045.28	UA	RP	26	618.8873077	1.884508546	614.47	622.33	01/15/2019	11/16/2015	Yes	27	78.1	12.2	52	94	01/21/2015	07/27/2021	Yes
G281	2514455.52	874375.28	UA	B	27	619.6537037	1.162395233	616.41	621.68	01/15/2019	11/16/2015	Yes	24	296.3	34.2	250	380	11/20/2015	07/27/2021	-
G283	2516503.05	874115.82	LCU	AP2	9	605.86	1.027898341	604.56	607.8	03/29/2021	08/16/2021	Yes	8	242.5	7.1	230	250	03/31/2021	07/27/2021	Yes
G284	2516922.93	874426.1	UA	B	9	607.9777778	1.492646792	606.17	611.14	03/29/2021	08/16/2021	Yes	8	69.5	10.8	60	95	03/30/2021	07/27/2021	-
G285	2516680.39	874797.74	LCU	B	9	606.5866667	1.509014579	604.33	608.62	03/29/2021	08/16/2021	Yes	8	570.0	40.0	490	620	03/30/2021	07/27/2021	-
G286	2516561.89	875075	UA	B	6	606.6166667	1.448346183	604.68	609.08	03/29/2021	12/07/2021	Yes	8	13.5	2.1	11	16	03/31/2021	07/27/2021	-
G287	2516415.34	875445.28	UA	B	7	608.9657143	1.217249045	607.59	610.83	03/29/2021	08/16/2021	Yes	8	44.4	2.7	41	50	03/29/2021	07/27/2021	-
G288	2517071.51	875282.23	UA	B	9	613.6466667	1.259801572	611.9	616.32	03/29/2021	08/16/2021	Yes	8	200.5	302.5	29	770	03/30/2021	07/27/2021	-
G301	2515583.06	872237.64	UA	AP1	25	615.0272	1.602722995	610.39	618.07	01/15/2019	11/16/2015	Yes	16	742.5	79.8	570	860	11/20/2015	01/27/2021	Yes
G302	2516214.19	872255.38	UA	AP1	25	609.8508	2.621329052	604.64	615.41	01/15/2019	11/16/2015	Yes	16	414.4	86.0	260	530	11/20/2015	01/27/2021	Yes
G303	2516639.34	871384.83	UA	AP1	25	615.7748	1.750197894	611.18	618.05	01/15/2019	11/16/2015	Yes	16	770.0	76.2	600	870	11/20/2015	01/26/2021	Yes
G304	2515519.76	871397.53	UA	AP1	2	623.99	0.113137085	623.91	624.07	08/02/2016	09/05/2016	Yes	3	1033.3	57.7	1000	1100	11/20/2015	05/20/2016	-
G305	2515199.45	871159.15	UA	AP1	23	618.0413043	1.084004798	615.3	620.49	01/15/2019	12/11/2016	Yes	5	864.0	87.6	710	930	05/19/2016	11/17/2016	Yes
G306	2516120.28	871143.66	UA	AP1	26	618.9373077	1.290400117	616.12	621.73	01/15/2019	12/11/2016	Yes	24	284.0	113.3	5.9	700	05/19/2016	07/27/2021	Yes
G307	2515553.24	871401.09	UA	AP1	17	624.0317647	1.239890294	619.33	624.6	01/15/2019	12/11/2016	Yes	13	1029.2	113.1	850	1300	08/16/2016	01/27/2021	Yes
G308	2515101.51	871457.36	UA	AP1	11	619.7218182	0.671190259	618.54	621.03	03/29/2021	08/16/2021	Yes	8	1125.0	46.3	1100	1200	03/29/2021	07/27/2021	Yes
G309	2515067.07	871868.3	UA	AP1	11	618.9445455	0.814350829	617.89	621.09	03/29/2021	08/16/2021	Yes	8	787.5	38.8	740	840	03/29/2021	07/27/2021	Yes
G310	2515159.33	872242.06	UA	AP1	11	614.4509091	1.049528032	613.2	617.27	03/29/2021	08/16/2021	Yes	8	990.0	552.5	420	2300	03/29/2021	07/28/2021	Yes
G311	2515881.77	872241.27	UA	AP1	11	613.6636364	1.07212194	612.45	616.54	03/29/2021	08/16/2021	Yes	8	811.3	35.6	750	860	03/30/2021	07/27/2021	Yes
G312	2516557.45	872263.4	UA	AP1	11	608.9363636	1.307511168	606.99	612.19	03/29/2021	08/16/2021	Yes	8	838.8	143.6	600	1000	03/30/2021	07/27/2021	Yes
G314	2516852.2	871632.87	UA	AP1	10	605.13	3.49532386	596.4	608.6	03/29/2021	08/16/2021	Yes	8	1953.8	473.9	830	2400	03/30/2021	07/27/2021	Yes
G315	2516086.68	871387.77	UA	AP1	10	620.529	0.69468538	619.17	621.24	03/29/2021	08/16/2021	Yes	8	908.8	81.1	850	1100	03/30/2021	07/28/2021	Yes
G316	2517211.62	871645.773	UA	AP1	10	590.022	3.016792999	581.54	591.63	03/29/2021	08/16/2021	-	8	691.3	156.1	330	840	03/30/2021	07/27/2021	Yes
G317	2517087.32	871236.763	UA	AP1	10	609.619	1.740890258	606.57	611.75	03/29/2021	08/16/2021	-	8	952.5	93.6	780	1100	03/30/2021	07/28/2021	Yes
G401	2515614.82	872510.72	UA	AP2	18	607.6811111	1.846264556	603.94	609.8	01/15/2019	11/16/2015	Yes	-	-	-	-	-	-	-	-
G402	2516632.39	872500.43	UA	AP2	20	603.743	1.213286533	600.77	605.36	01/15/2019	11/16/2015	Yes	-	-	-	-	-	-	-	-
G403	2514616.58	873561.48	UA	AP2	20	621.055	1.263622612	618.36	622.45	01/15/2019	11/16/2015	Yes	-	-	-	-	-	-	-	-
G404	2516397.84	873999.83	UA	AP2	20	610.838	1.183783408	607.58	612.14	01/15/2019	11/16/2015	Yes	-	-	-	-	-	-	-	-
G405	2515335.58	873996.63	UA	AP2	20	617.8585	1.158348529	614.47	619.28	01/15/2019	11/16/2015	Yes	-	-	-	-	-	-	-	-
G406	2514702.32	872521.21	UA	AP2	16	615.141875	1.675395351	611.27	617.52	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
G407	2513705.74	872973.57	UA	B	16	613.60625	0.84114109	612.11	614.86	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
MW04S	2514450.47	877999.78	UA	B	19	618.2110526	2.142835335	613.88	621.62	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-

TABLE 4-1. FLOW AND TRANSPORT MODEL CALIBRATION TARGETS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Well Name	Easting	Northing	HSU	CCR Unit	Flow Targets							Transport Targets								
					Number of Samples	mean GWL ¹ (feet)	std GWL ¹ (feet)	min GWL ¹ (feet)	max GWL ¹ (feet)	Earliest Sample Date	Latest Sample Date	Flow Calibration Wells	Number of Samples	mean Sulfate (mg/L)	std Sulfate (mg/L)	min Sulfate (mg/L)	max Sulfate (mg/L)	Earliest Sample Date	Latest Sample Date	Transport Calibration Well
MW05S	2513285.52	878175.73	UA	B	19	617.8810526	1.843543975	613.32	620.92	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
MW10S	2515914.48	878250.4	UA	B	18	617.255	1.690963004	614.36	620.43	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
MW11S	2515971.24	876749.49	UA	GSP	24	620.7020833	1.218373753	617.19	622.19	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
MW12S	2515900.49	875519.94	UA	GSP	24	617.9708333	2.049907562	611.42	620.48	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
MW16S	2515087.93	877355.01	UA	B	24	622.0208333	2.003932908	618.34	625.59	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
MW20S	2515876.54	874228.14	UA	B	19	612.0194737	1.76501959	607.74	615.4	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
R104	2514503.48	875857.78	UA	B	20	623.479	1.640654234	619.38	625.92	01/15/2019	11/16/2015	Yes	7	74.4	2.2	72	77	04/08/2015	08/03/2016	-
R201	2514842.05	877925.14	UA	B	26	621.8242308	1.348306117	618.3	623.52	01/15/2019	11/16/2015	Yes	28	211.0	55.8	89	370	01/20/2015	07/28/2021	-
T127	2513911.13	875359.24	UA	B	20	615.954	1.042297058	612.33	617.05	01/15/2019	11/16/2015	Yes	-	-	-	-	-	-	-	-
T128	2513909.58	875509.65	UA	B	19	615.1989474	1.45420805	611.33	617.25	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
T202	2514895.01	876699.56	UA	GSP	19	620.5410526	2.211231167	615.31	624.22	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
T408	2515314.82	873999.37	UA	B	16	617.25875	1.507615667	614.45	619.46	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
T409	2514693.83	872517.86	UA	B	16	615.403125	1.232908316	612.16	617.16	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
TA31	2513856.87	876542.19	UA	B	19	619.7289474	2.10867756	614.89	622.93	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
TA32	2513605.22	877532.63	UA	B	10	615.309	1.097172629	612.42	616.3	01/20/2020	12/11/2016	Yes	-	-	-	-	-	-	-	-
TA33	2513248.73	876605.56	UA	B	19	617.2257895	1.90237663	612.91	620.35	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-
TA34	2513466.7	875906.23	UA	B	19	617.0926316	1.535020239	613.48	619.58	01/15/2019	12/11/2016	Yes	-	-	-	-	-	-	-	-

Notes:

¹ Groundwater Elevation
 AP1 = Ash Pond No. 1
 AP2 = Ash Pond No. 2
 B = Background
 GSP = Gypsum Management Facility Gypsum Stack Pond
 GWL = groundwater elevation
 LF = Landfill
 max = maximum
 mg/L = milligrams per liter
 min = minimum
 RP = Gypsum Management Facility Gypsum Recycle Pond
 std = standard deviation from the mean

HSU = Hydrostratigraphic Unit
 CCR = coal combustion residuals
 UA = uppermost aquifer
 LCU = lower confining unit

[O: SLN 04/20/22; C: EGP 4/29/22]

TABLE 5-1. FLOW MODEL INPUT AND SENSITIVITY ANALYSIS RESULTS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Zone	Hydrostratigraphic Unit	Materials	ft/d	cm/s	Kh/Kv	Value Source	Sensitivity ¹
Horizontal Hydraulic Conductivity			Calibration Model				
1	UCU	loess and clay	0.51	1.80E-04	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	High
2	UA	sand and sandy silt	4.04	1.43E-03	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	High
3	LCU (unweathered Vandalia)	sand clay till	0.83	2.93E-04	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	High
4	LCU (Smithboro Formation)	sand clay till	0.0014	4.94E-07	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Low
5	SW Pond	NA	2.89E-09	1.02E-12	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
6	LF-CCR	CCR	13.6	4.80E-03	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
7	GSP-CCR	CCR	13.6	4.80E-03	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
8	RP-CCR	CCR	13.6	4.80E-03	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
9	AP2	CCR	13.6	4.80E-03	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
10	AP1	CCR	13.6	4.80E-03	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Moderate
11	Cooling Pond	clay and silt	0.51	1.80E-04	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Low
12	GSP-RP connector	lined channel within UCU	0.51	1.80E-04	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
13	AP2 -berm	loess and clay	0.51	1.80E-04	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
14	AP1-berm	loess and clay	0.51	1.80E-04	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
15	Pond (west)	loess and clay	0.51	1.80E-04	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
16	GSP-liner	liner	2.89E-08	1.02E-11	NA	Harmonic mean of liner layers	Negligible
17	RP-liner	liner	2.89E-08	1.02E-11	NA	Harmonic mean of liner layers	Negligible
18	LF-liner	liner	2.89E-08	1.02E-11	NA	Harmonic mean of liner layers	Negligible
19	UCU- fill (drain/river)	NA	10	3.53E-03	NA	Calibrated - Conductivity Value to Allow Groundwater Flow from UCU to River and Drain Boundary Conditions	Moderate
21	LF-GSP shared embankment	reworked silts and clays	0.01	3.53E-06	NA	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
Vertical Hydraulic Conductivity			Calibration Model				
1	UCU	loess and clay	0.0510	1.80E-05	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	High
2	UA	sand and sany silt	0.4040	1.43E-04	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	High
3	LCU (unweathered Vandalia)	sand clay till	0.0830	2.93E-05	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	High
4	LCU (Smithboro Formation)	sand clay till	0.0001	4.94E-08	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Low
5	SW Pond	lined	2.89E-09	1.02E-12	1	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
6	LF-CCR	CCR	0.2500	8.82E-05	54	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
7	GSP-CCR	CCR	0.2500	8.82E-05	54	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
8	RP-CCR	CCR	0.2500	8.82E-05	54	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
9	AP2	CCR	0.2500	8.82E-05	54	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
10	AP1	CCR	0.2500	8.82E-05	54	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Moderate
11	Cooling Pond	clay and silt	0.0510	1.80E-05	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Low
12	GSP-RP connector	lined channel within UCU	0.0510	1.80E-05	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
13	AP2 -berm	loess and clay	0.0510	1.80E-05	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
14	AP1-berm	loess and clay	0.0510	1.80E-05	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
15	Pond (west)	loess and clay	0.0510	1.80E-05	10	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
16	GSP-liner	liner	2.89E-08	1.02E-11	1	Harmonic mean of liner layers	Negligible
17	RP-liner	liner	2.89E-08	1.02E-11	1	Harmonic mean of liner layers	Negligible

TABLE 5-1. FLOW MODEL INPUT AND SENSITIVITY ANALYSIS RESULTS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Zone	Hydrostratigraphic Unit	Materials	ft/d	cm/s	Kh/Kv	Value Source	Sensitivity ¹
Vertical Hydraulic Conductivity (Continued)			Calibration Model				
18	LF-liner	liner	2.89E-08	1.02E-11	1	Harmonic mean of liner layers	Negligible
19	UCU- fill (drain/river)	NA	10.0000	3.53E-03	1	Calibrated - Conductivity Value to Allow Groundwater Flow from UCU to River and Drain Boundary Conditions	Moderate
21	LF-GSP shared embankment	reworked silts and clays	0.0100	3.53E-06	1	Calibrated - Within Range of Field Test Results (Ramboll, 2021a)	Negligible
Zone	Hydrostratigraphic Unit	Materials	ft/d	in/yr	Kh/Kv	Value Source	Sensitivity ¹
Recharge			Calibration Model				
1	UCU	clay and silt	0.00055	2.41	NA	Calibrated	High
2	SW Pond	clay and silt	1.50E-08	6.57E-05	NA	Calibrated	Negligible
3	LF	CCR	8.00E-08	3.50E-04	NA	Calibrated	Negligible
4	GSP	CCR	8.00E-08	3.50E-04	NA	Calibrated	Negligible
5	RP	CCR	8.00E-08	3.50E-04	NA	Calibrated	Negligible
6	AP2	CCR	0.0005	2.19	NA	Calibrated	Moderate
7	AP1	CCR	0.0024	10.51	NA	Calibrated	High
8	Cooling pond	clay and silt	1.40E-05	0.06	NA	Calibrated	Negligible
9	GSP-RP connector	clay and silt	0.00055	2.41	NA	Calibrated	Low
10	AP2-Berm	clay and silt	0.00055	2.41	NA	Calibrated	Negligible
11	AP1-Berm	clay and silt	0.00055	2.41	NA	Calibrated	Negligible
12	Pond (west)	clay and silt	5.50E-04	2.41	NA	Calibrated	Negligible
Storage			<i>Not used in steady-state calibration model</i>				
1	UCU	loess and clay					
2	UA	sand and sandy silt					
3	LCU (unweathered Vandalia)	sand clay till					
4	LCU (Smithboro Formation)	sand clay till					
5	SW Pond	lined					
6	LF-CCR	CCR					
7	GSP-CCR	CCR					
8	RP-CCR	CCR					
9	AP2	CCR					
10	AP1	CCR					
11	Cooling Pond	clay and silt					
12	GSP-RP connector	lined channel within UCU					
13	AP2 -berm	loess and clay					
14	AP1-berm	loess and clay					
15	Pond (west)	loess and clay					
16	GSP-liner	liner					
17	RP-liner	liner					
18	LF-liner	liner					
19	UCU- fill (drain/river)	NA					
21	LF-GSP shared embankment	reworked silts and clays					

TABLE 5-1. FLOW MODEL INPUT AND SENSITIVITY ANALYSIS RESULTS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

River Parameters							
	Relative Location	River Width (feet)	River depth (feet)	Bed Thickness (feet)	Hydraulic Conductivity (ft/d)	Head (feet)	River Boundary Conductance (ft²/d)
Reach 0	Unnamed Tributary East Coffeen Lake	10	3	2	4.00E-02	594.7-621.84	0.08-20.4
Sensitivity ¹	NA	---	---	---	---	Moderate	High
Reach 5	Unnamed Tributary East Coffeen Lake - downstream in layer 5	10	3	2	4.00E-01	591.0-594.7	1.5-109.2
Sensitivity ¹	---	---	---	---	---	Moderate	Low
Reach 1	Unnamed Tributary West Coffeen Lake	10	3	2	4.80E-02	591.0-622.45	0.04-12.3
Sensitivity ¹	NA	---	---	---	---	Low	Moderately High
Reach 2	Pond (west)	cell dimensions	3	1	3.20E-03	617.50	4.0
Sensitivity ¹	NA	---	---	---	---	Low	Low
Reach 3	Condenser Cooling Flume	cell dimensions	4	1	5.00	604.00	5.00
Sensitivity ¹	NA	---	---	---	---	Moderate	High
Value Source	NA	Calibrated	Calibrated	Calibrated	Calibrated	Estimated based on DEM	Calibrated
Drain Parameters							
	Name	Drain Width (feet)	Drain depth (feet)	Bed Thickness (feet)	Hydraulic Conductivity (ft/d)	Stage (feet)	Drain Conductance (ft²/d)
Reach 0	Active LF Underdrain	2	2	1.5	2.40E-02	603.5	6.6e-5-0.47
Sensitivity ¹	NA	---	---	---	---	Low	Moderately High
Reach 1	Gravity Driven RP Drain	cell dimensions	2	1.5	2.50E-02	600.5	9.7e-5-0.51
Sensitivity ¹	NA	---	---	---	---	Low	Moderate
Reach 2	Northern Drain	cell dimensions	2	1.5	2.00E+00	622	5.1-135.46
Sensitivity ¹	NA	---	---	---	---	Low	Negligible
Value Source	NA	Calibrated	Calibrated	Calibrated	Calibrated	Estimated based on DEM	Calibrated
General Head Parameters							
	Relative Location	Width of General Head Boundary Cell (feet)	Distance to General Head Boundary Head (feet)	Saturated Thickness of Cell (feet)	Hydraulic Conductivity (ft/d)	Head (feet)	General Head Boundary Conductance (ft²/d)
Reach 2	Northern Model Boundary in UA	<i>variable</i>	1	variable	4.54	591-610.66	1.4-7032.9
Sensitivity ¹	NA	---	---	---	---	Moderate	Negligible
Reach 3	Northern Model Boundary in LCU Layer 4	<i>variable</i>	1	variable	0.83	591-610.66	166-1812.6
Sensitivity ¹	NA	---	---	---	---	High	Negligible
Reach 4	Northern Model Boundary in LCU Layer 5	<i>variable</i>	1	variable	0.0014	591-610.66	1.61-6.0
Sensitivity ¹	NA	---	---	---	---	Low	Negligible
Value Source	NA	Calibrated	Calibrated	Calibrated	Calibrated	Estimated based on Groundwater Elevation Targets in UA around the GSP/GRP/LF	Calibrated

TABLE 5-1. FLOW MODEL INPUT AND SENSITIVITY ANALYSIS RESULTS

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Hydraulic Flow Boundary Parameters			
	Relative Location	Width of HFB (feet) ²	Hydraulic Conductivity (feet)
Reach 1	GSP	1	2.89E-08
Sensitivity ¹	NA	- - -	
Reach 2	RP	1	2.89E-08
Sensitivity ¹	NA	- - -	
Reach 3	LF	1	2.89E-08
Sensitivity ¹	NA	- - -	Harmonic mean of construction material
Value Source	NA	Calibrated	Harmonic mean of construction material

[O: SLN 04/01/22; C: 4/29/22]

Notes:

¹ Sensitivity Explanation:

- Negligible - SSR changed by less than 1%
- Low - SSR change between 1% and 10%
- Moderate - SSR change between 10% and 50%
- Moderately High - SSR change between 50% and 100%
- High - SSR change greater than 100%

² Liner thickness accounted for in harmonic mean calculation
 - - - = not tested

- AP1 = Ash Pond No. 1
- AP2 = Ash Pond No. 2
- CCR = coal combustion residuals
- cm/s = centimeters per second
- ft/d = feet per day
- ft²/d = feet squared per day
- GSP = Gypsum Management Facility Gypsum Stack Pond
- in/yr = inches per year
- Kh/Kv = anisotropy ratio
- LF = Landfill
- NA = not applicable
- RP = Gypsum Management Facility Gypsum Recycle Pond
- SSR = sum of squared residuals
- SW = surface water

HSU = Hydrostratigraphic Unit

- UCU = upper confining unit
- UA = uppermost aquifer
- LCU = lower confining unit

TABLE 5-2. TRANSPORT MODEL INPUT VALUES (CALIBRATION)

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Zone	Hydrostratigraphic Unit	Materials	Calibration Model								Value Source	Sensitivity
			Recharge (ft/d)				Sulfate Concentration (mg/L)					
Initial Concentration												
Entire Domain	NA	NA	0.00055				0				NA	---
Source Concentration (recharge and constant concentration cells)												
	Model Name and Stress Period	Materials	Pre-GMF		Post-GMF		Pre-GMF		Post-GMF			
			TR1 - STP 1	TR1 - STP 2	TR2 - STP 1	TR3 - STP 1	TR1 - STP 1	TR1 - STP 2	TR2 - STP 1	TR3 - STP 1		
			1970-1984	1985-2009	2010-2017	2018-2022	1970-1984	1985-2009	2010-2017	2018-2022		
6	AP2	CCR	0.0005	0.0005	0.0005	0.00027	1,600	1,600	1,600	0	Leachate sulfate concentrations	---
13	AP2 Northwest seep area	-	0.002	0.002	0.002	0.00055	1,600	1,600	1,600	0	Based on previous model	---
14	AP2 East and Southwest seep area	-	0.01	0.01	0.01	0.00055	300	300	300	0	Based on previous model	---
13	AP2 closure structures	-									Based on previous model	---
7	AP1	CCR	0.00055	0.00240	0.00240	0.00240	0	1,000	1,000	1,000	Calibrated	---
5	RP	CCR	NA	NA	8.00E-08	8.00E-08	NA	NA	15,000	15,000	Leachate sulfate concentrations	---
4	GSP	CCR	NA	NA	8.00E-08	8.00E-08	NA	NA	11,000	11,000	Leachate sulfate concentrations	---
3	LF	CCR	NA	NA	8.00E-08	8.00E-08	NA	NA	7,500	7,500	Leachate sulfate concentrations	---
GMF Units liner modification (HFB)												
	Model Name and Stress Period	Well Data	Hydraulic Conductivity (ft/d)									
			Pre-GMF		Post-GMF							
			TR1 - STP 1	TR1 - STP 2	TR2 - STP 1	TR3 - STP 1						
	Time Period		1970-1984	1985-2009	2010-2017	2018-2022						
1	RP		NA	NA	2.89E-08	2.89E-08		Harmonic Mean	see Table 5-3			
11	RP-northeast	G279	NA	NA	2.89E-08	3.00E-04		Calibrated	see Table 5-3			
16	RP-southeast	G275	NA	NA	2.89E-08	6.54E-04		Calibrated	see Table 5-3			
2	GSP		NA	NA	2.89E-08	2.89E-08		Harmonic Mean	see Table 5-3			
21	GSP-seep 1 east	G215	NA	NA	2.89E-08	6.00E-04		Calibrated	see Table 5-3			
3	LF		NA	NA	2.89E-08	2.89E-08		Harmonic Mean	see Table 5-3			

TABLE 5-2. TRANSPORT MODEL INPUT VALUES (CALIBRATION)

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Storage, Specific Yield and Effective Porosity						Calibration Model		
Zone	Hydrostratigraphic Unit	Materials	Storage	Specific Yield	Effective Porosity	NA	Value Source	Sensitivity
1	UCU	Loess and clay	0.0034	0.35	0.35		Ramboll (2021a) HCR	see Table 5-3
2	UA	sand and sandy silt	0.0034	0.16	0.16		Ramboll (2021a) HCR	see Table 5-3
3	LCU (unweathered Vandalia)	sand clay till	0.0034	0.19	0.19		Ramboll (2021a) HCR	see Table 5-3
4	LCU (Smithboro Formation)	sand clay till	0.0034	0.28	0.28		Ramboll (2021a) HCR	see Table 5-3
5	SW Pond	NA	0.0034	0.35	0.35		Ramboll (2021a) HCR	see Table 5-3
6	Landfill-CCR	CCR	0.0034	0.19	0.19		Ramboll (2021a) HCR	see Table 5-3
7	GSP-CCR	CCR	0.0034	0.19	0.19		Ramboll (2021a) HCR	see Table 5-3
8	RP-CCR	CCR	0.0034	0.19	0.19		Ramboll (2021a) HCR	see Table 5-3
9	AP2	CCR	0.0034	0.19	0.19		Ramboll (2021a) HCR	see Table 5-3
10	AP1	CCR	0.0034	0.19	0.19		Ramboll (2021a) HCR	see Table 5-3
11	Cooling Pond	clay and silt	0.0034	0.35	0.35		Ramboll (2021a) HCR	see Table 5-3
12	GSP-RP connector	lined channel within UD	0.0034	0.35	0.35		Ramboll (2021a) HCR	see Table 5-3
13	AP2 -berm	Loess and clay	0.0034	0.35	0.35		Ramboll (2021a) HCR	see Table 5-3
14	AP1-berm	Loess and clay	0.0034	0.35	0.35		Ramboll (2021a) HCR	see Table 5-3
15	Pond (west)	Loess and clay	0.0034	0.35	0.35		Ramboll (2021a) HCR	see Table 5-3
16	GSP-liner	liner	0.0034	0.16	0.16		Ramboll (2021a) HCR	see Table 5-3
17	RP-liner	liner	0.0034	0.16	0.16		Ramboll (2021a) HCR	see Table 5-3
18	Landfill-liner	liner	0.0034	0.16	0.16		Ramboll (2021a) HCR	see Table 5-3
19	UCU- fill (drain/river)	NA	0.0034	0.5	0.5		Calibrated	see Table 5-3
21	Landfill-GSP shared embankment	reworked silts and clays	0.0034	0.16	0.16		Calibrated	see Table 5-3
Dispersivity								
Applicable Region	Hydrostratigraphic Unit	Materials	Longitudinal (feet)	Transverse (feet)	Vertical (feet)	NA	Value Source	Sensitivity
1	UCU	Loess and clay	1	0.1	0.01		calibrated	see Table 5-3
2	UA	sand and sandy silt	10	1	0.1		calibrated	see Table 5-3
3	LCU (unweathered Vandalia)	sand clay till	1	0.1	0.01		calibrated	see Table 5-3
4	LCU (Smithboro Formation)	sand clay till	1	0.1	0.01		calibrated	see Table 5-3

[O: SLN 04/01/22; C: 4/29/22]

Notes:

¹ The concentrations from the end of the calibrated transport model were imported as initial concentrations for the prediction model runs.

- - - = not tested
- AP1 = Ash Pond No. 1
- AP2 = Ash Pond No. 2
- CCR = coal combustion residuals
- ft/day = feet per day
- GMF = Gypsum Management Facility
- GSP = Gypsum Management Facility Gypsum Stack Pond
- LF = Landfill
- mg/L = milligrams per liter
- NA = not applicable
- RP = Gypsum Management Facility Gypsum Recycle Pond
- SS = steady state model
- STP = Stress Period
- SW = surface water
- TR = Transient model

Hydrostratigraphic Unit
 UCU = upper confining unit
 UA = upper aquifer
 LCU = lower confining unit

TABLE 5-3. TRANSPORT MODEL INPUT SENSITIVITY (CALIBRATION)

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Well ID	SI	Calibration on Sulfate Concentration (mg/L)	Storage and Specific Yield				Effective Porosity			
			Sulfate Concentration (mg/L)	Sensitivity ¹	Sulfate Concentration (mg/L)	Sensitivity ¹	Sulfate Concentration (mg/L)	Sensitivity ¹	Sulfate Concentration (mg/L)	Sensitivity ¹
G102	GSP	0.0*	0.0*	Negligible	0.2	Negligible	0.0*	Negligible	0.0*	Negligible
G103	GSP	0.7	0.3	Moderately High	1.6	High	1.4	Moderately High	0.4	Moderate
G105	GSP	0.4	0.1	Moderately High	0.8	Moderately High	0.9	High	0.2	Moderately High
G106	GSP	0.0*	0.0*	Negligible	0.6	Negligible	0.0*	Negligible	0.0*	Negligible
G206	GSP	0.0*	0.0*	Negligible	0.1	Negligible	0.0*	Negligible	0.0*	Negligible
G207	GSP	0.0*	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible
G208	GSP	0.0*	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible
G209	GSP	0.0*	0.0*	Negligible	0.2	Negligible	1.3	Negligible	0.0*	Negligible
G210	GSP	0.0*	0.0*	Negligible	6.8	Negligible	0.0*	Negligible	0.0*	Negligible
G211	GSP	2.9	2.6	Low	27.6	High	6.0	High	0.8	Moderately High
G212	GSP	8.9	7.0	Moderate	33.9	High	12.4	Moderate	6.3	Moderate
G213	GSP	24.3	20.6	Moderate	51.2	High	28.9	Moderate	20.5	Moderate
G214	GSP	19.3	16.2	Moderate	34.6	Moderately High	25.9	Moderate	15.2	Moderate
G215	GSP	477.8	825.2	Moderately High	182.0	Moderately High	607.8	Moderate	381.7	Moderate
G216	GSP	9.3	10.9	Moderate	20.9	High	15.2	Moderately High	5.8	Moderate
G217	GSP	0.0*	6.1	Negligible	0.0*	Negligible	3.8	Negligible	0.0*	Negligible
G218	GSP	0.0*	3.3	Negligible	0.0*	Negligible	1.0	Negligible	0.0*	Negligible
G270	RP	0.0*	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible
G271	RP	28.2	54.1	Moderately High	16.0	Moderate	40.0	Moderate	21.1	Moderate
G272	RP	123.7	148.8	Moderate	99.9	Moderate	140.0	Moderate	108.4	Moderate
G273	RP	164.4	175.6	Low	159.6	Low	164.9	Negligible	161.0	Low
G274	RP	196.5	197.8	Negligible	183.8	Low	194.5	Negligible	196.8	Negligible
G275	RP	859.1	1224.8	Moderate	636.2	Moderate	918.8	Low	801.1	Low
G276	RP	543.0	513.5	Low	521.7	Low	636.4	Moderate	468.6	Moderate
G279	RP	1,561.1	2,019.9	Moderate	1,175.2	Moderate	1,727.5	Moderate	1,412.5	Low
G280	RP	1.9	1.1	Moderate	3.6	Moderately High	2.0	Low	1.7	Moderate
G283	RP	385.7	329.6	Moderate	384.6	Negligible	409.8	Low	373.8	Low
			S*0.1 Sy*0.5		S*10 Sy*2		Porosity-0.05		Porosity+0.05	

TABLE 5-3. TRANSPORT MODEL INPUT SENSITIVITY (CALIBRATION)

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN, ILLINOIS

Well ID	SI	Dispersivity				HFB (GMF GSP and GMF RP Liner)			
		Sulfate Concentration (mg/L)	Sensitivity ¹	Sulfate Concentration (mg/L)	Sensitivity ¹	Sulfate Concentration (mg/L)	Sensitivity ¹	Sulfate Concentration (mg/L)	Sensitivity ¹
G102	GSP	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible
G103	GSP	14.0	High	30.0	High	0.7	Negligible	0.8	Moderate
G105	GSP	9.0	High	20.2	High	0.4	Negligible	0.5	Moderate
G106	GSP	1.5	Negligible	4.2	Negligible	0.0*	Negligible	0.0*	Negligible
G206	GSP	0.0*	Negligible	1.3	Negligible	0.0*	Negligible	0.0*	Negligible
G207	GSP	7.6	Negligible	29.8	Negligible	0.0*	Negligible	0.0*	Negligible
G208	GSP	9.5	Negligible	62.5	Negligible	0.0*	Negligible	0.0*	Negligible
G209	GSP	79.6	Negligible	171.1	Negligible	0.0*	Negligible	0.0*	Negligible
G210	GSP	16.5	Negligible	48.6	Negligible	0.0*	Negligible	0.0*	Negligible
G211	GSP	45.8	High	91.3	High	2.8	Low	3.8	Moderate
G212	GSP	50.1	High	102.5	High	8.7	Low	10.2	Moderate
G213	GSP	97.0	High	180.3	High	24.0	Low	27.5	Moderate
G214	GSP	100.9	High	196.8	High	20.1	Low	24.5	Moderate
G215	GSP	665.3	Moderate	752.7	Moderately High	30.2	Moderately High	7,063.8	High
G216	GSP	85.4	High	146.4	High	9.2	Negligible	14.1	Moderately High
G217	GSP	67.1	Negligible	142.7	Negligible	0.0*	Negligible	1.5	Negligible
G218	GSP	56.6	Negligible	132.9	Negligible	0.0*	Negligible	0.0*	Negligible
G270	RP	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible	0.0*	Negligible
G271	RP	109.4	High	178.5	High	26.2	Low	45.2	Moderately High
G272	RP	186.7	Moderately High	231.2	Moderately High	123.5	Negligible	124.5	Negligible
G273	RP	200.4	Moderate	230.2	Moderate	164.4	Negligible	165.4	Negligible
G274	RP	238.7	Moderate	303.2	Moderately High	195.9	Negligible	201.0	Low
G275	RP	805.1	Low	777.2	Low	252.3	Moderately High	5,918.0	High
G276	RP	590.0	Low	708.5	Moderate	219.4	Moderately High	3,701.6	High
G279	RP	1,767.5	Moderate	1778.0	Moderate	134.2	Moderately High	11,990.8	High
G280	RP	24.9	High	54.1	High	1.9	Low	2.5	Moderate
G283	RP	363.4	Low	381.1	Low	387.0	Negligible	376.9	Low
		Disp*5		Disp*10		HFB*0.1		HFB*10	

Notes: [O: SLN 04/10/22; C: EGP 5/5/22]

* corrected to zero due to numerical errors producing simulated negative concentrations

¹ Sensitivity Explanation:
 Negligible = concentration changed by less than 1%
 Low = concentration change between 1% and 10%
 Moderate = concentration change between 10% and 50%
 Moderately High = concentration change between 50% and 100%
 High = concentration change greater than 100%

² sensitivity test used transient transport

AP1 = Ash Pond No. 1
 AP2 = Ash Pond No. 2
 Disp = dispersivity
 GSP = Gypsum Management Facility Gypsum Stack Pond
 HFB = Horizontal Flow Boundary
 ID = identification
 LF = Landfill
 mg/L = milligrams per liter
 RP = Gypsum Management Facility Gypsum Recycle Pond
 S = storativity
 SI = surface impoundment
 Sy = specific yield

TABLE 6-1. HELP MODEL INPUT AND OUTPUT VALUES

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND LANDFILL
 COFFEEN, ILLINOIS

Closure Scenario Number (Drainage Length)	GMF Gypsum Stack Pond - CIP Consolidation Area	Landfill Closure In Place	Notes
Input Parameter			
Climate-General			
City	Coffeen, Illinois	Coffeen, Illinois	Nearby city to the Site within HELP database
Latitude	39.06	39.06	Site latitude
Evaporative Zone Depth	18	18	Estimated based on geographic location (Illinois) and uppermost soil type (Tolaymat, T. and Krause, M 2020)
Maximum Leaf Area Index	4.5	4.5	Maximum for geographic location (Illinois) (Tolaymat, T. and Krause, M, 2020)
Growing Season Period, Average Wind Speed, and Quarterly Relative Humidity	Belleville Scott Air Force Base Belleville, Illinois	Belleville Scott Air Force Base Belleville, Illinois	Nearby city to the Coffeen Power Plant within HELP database
Number of Years for Synthetic Data Generation	30	30	
Temperature, Evapotranspiration, and Precipitation	Precipitation, temperature, and solar radiation was simulated based on HELP V4 weather simulation for: Lat/Long: 39.06/-89.39	Precipitation, temperature, and solar radiation was simulated based on HELP V4 weather simulation for: Lat/Long: 39.06/-89.39	
Soils-General			
% where runoff possible	100	100	
Area (acres)	12.4	13.5	CBR - Removal Area based on HCR (Ramboll, 2021); CIP - Consolidation and Cover System Area based on construction drawing for GMF GSP; Landfill Consolidation Area based on HDR drawings
Specify Initial Moisture Content	No	No	
Surface Water/Snow	Model Calculated	Model Calculated	
Soils-Layers			
1	Vegetative Soil Layer (HELP Final Cover Soil [topmost layer])	Protective Cover Layer (HELP Final Cover Soil [topmost layer])	Layers details for CIP and Landfill areas based on grading plans, construction drawings, and cover system design for GMF GSP and Landfill
2	Protective Cover Layer (HELP Vertical Percolation Layer)	Protective Cover Layer (HELP Vertical Percolation Layer)	
3	Geocomposite Drainage Layer (HELP Geosynthetic Drainage Net)	Geocomposite Drainage Layer (HELP Geosynthetic Drainage Net)	
4	Geomembrane Liner	Geomembrane Liner	
5	Unsaturated CCR Material (HELP Waste)	Unsaturated CCR Material (HELP Waste)	
6	Geomembrane Liner	Geomembrane Liner	
7	Clay Liner	Clay Liner	
Soil Parameters--Layer 1			
Type	1	1	Vertical Percolation Layer (Cover Soil)
Thickness (in)	6	6	For CIP removal areas, layer 1 thickness is the average thickness of unsaturated backfill material placed after removal
Texture	12	10	Defaults used
Description	Silty Clay Loam	Silty Clay Loam	
Saturated Hydraulic Conductivity (cm/s)	4.20E-05	4.20E-05	Defaults used
Soil Parameters--Layer 2			
Type	1	1	Vertical Percolation Layer
Thickness (in)	18	18	design thickness
Texture	14	14	Defaults used
Description	Silty Clay	Silty Clay	
Saturated Hydraulic Conductivity (cm/s)	2.50E-05	2.50E-05	Defaults used
Soil Parameters--Layer 3			
Type	2	2	Lateral Drainage Layer
Thickness (in)	0.2	0.2	design thickness
Texture	20	20	Defaults used
Description	Drainage Net (0.5cm)	Drainage Net (0.5cm)	
Saturated Hydraulic Conductivity (cm/s)	1.00E+01	1.00E+01	Defaults used

TABLE 6-1. HELP MODEL INPUT AND OUTPUT VALUES

GROUNDWATER MODELING REPORT
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND AND LANDFILL
 COFFEEN, ILLINOIS

Closure Scenario Number (Drainage Length)	GMF Gypsum Stack Pond - CIP Consolidation Area	Landfill Closure In Place	Notes
Soil Parameters--Layer 4			
Type	4	4	Flexible Membrane Liner
Thickness (in)	0.04	0.04	design thickness
Texture	36	36	Defaults used
Description	LDPE Membrane	LDPE Membrane	
Saturated Hydraulic Conductivity (cm/s)	4.00E-13	4.00E-13	Defaults used
Soil Parameters--Layer 5			
Type	1	1	Vertical Percolation Layer (Waste)
Thickness (in)	144	720	design thickness
Texture	83	83	Defaults used
Description	Gypsum Waste Material (Sandy Loam)	Landfill CCR Material	
Saturated Hydraulic Conductivity (cm/s)	6.70E-04	2.69E-04	defaults used
Soil Parameters--Layer 6			
Type	4	4	Flexible Membrane Liner (GSP and Landfill)
Thickness (in)	0.06	0.06	Background clay thickness (Ash Pond No. 1) design thickness (GSP and Landfill)
Texture	36	36	Defaults used (GSP and Landfill)
Description	LDPE Membrane	LDPE Membrane	
Saturated Hydraulic Conductivity (cm/s)	4.00E-13	4.00E-13	Defaults used (GSP and Landfill)
Soil Parameters--Layer 7			
Type	3	3	Drainage Liner
Thickness (in)	36	36	design thickness
Texture	16	16	Defaults used
Description	Liner Soil (High)	Liner Soil (High)	
Saturated Hydraulic Conductivity (cm/s)	6.80E-07	6.80E-07	Defaults used
Soils--Runoff			
Runoff Curve Number	85.4	86.7	HELP-computed curve number
Slope	4.00%	25.00%	Estimated from construction design drawings
Length (ft)	650	250	estimated maximum flow path
Vegetation	fair	fair	fair indicating fair stand of grass on surface of soil backfill
Execution Parameters			
Years	30	30	
Report Daily	No	No	
Report Monthly	No	No	
Report Annual	Yes	Yes	
Output Parameter			
Unsaturated Percolation Rate (in/yr)	0.00017	0.000014	

Notes:

- % = percent
- CBR = closure by removal
- CCR = coal combustion residuals
- CIP = closure in place
- cm = centimeters
- cm/s = centimeters per second
- ft = feet
- MGMF = Gypsum Management Facility
- GSP = Gypsum Stack Pond
- HDPE = high density polyethylene
- HELP = Hydrologic Evaluation of Landfill Performance
- in = inches
- in/yr = inches per year

References:

- Tolaymat, T. and Krause, M, 2020. Hydrologic Evaluation of Landfill Performance: HELP 4.0 User Manual . United States Environmental Protection Agency, Washington, DC, EPA/600/B 20/219
- Ramboll Americas Engineering Solutions, Inc. (Ramboll), 2021. Hydrogeologic Site Characterization Report. AP1, GMF GSP, Coffeen Power Plant. Coffeen, Illinois.

TABLE 6-2. PREDICTION MODEL INPUT VALUES
GROUNDWATER MODELING REPORT
COFFEEN POWER PLANT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN, ILLINOIS

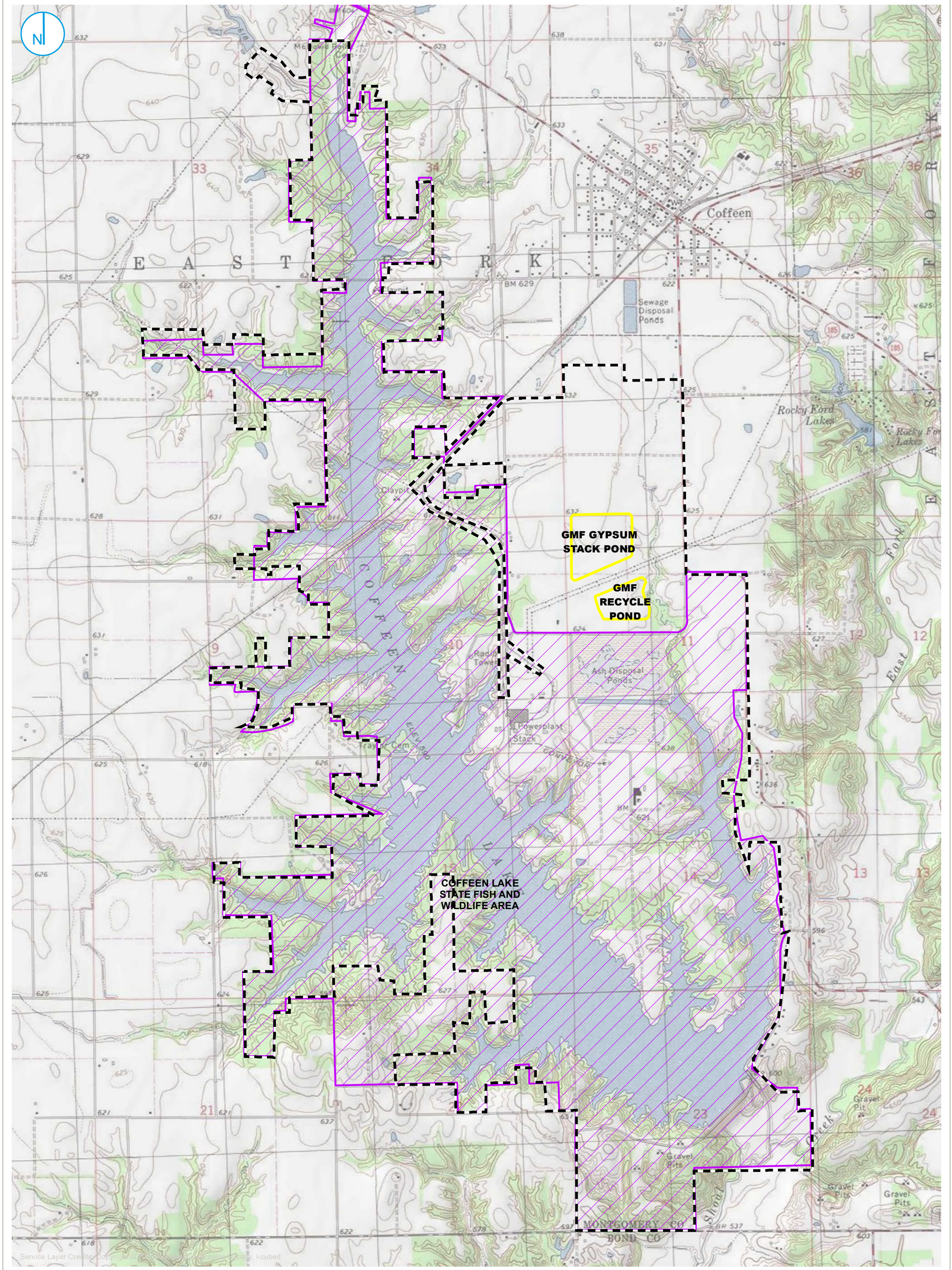
Hydrostratigraphic Unit/Recharge Area	Notes	Recharge Zone	Sulfate Concentration (mg/L)	Recharge (ft/day)	Recharge (inches/yr)
Scenario 1: CIP					
GMF RP - removal area	FILL	5	- - -	- - -	- - -
GMF GSP - removal area	CCR	3	- - -	- - -	- - -
GMF GSP - consolidation area	CCR	4	11,000	4.34E-08	1.90E-04
LF - consolidation area	CCR	7	7,500	3.20E-09	1.20E-05
Scenario 2: CBR					
GMF RP - removal area	FILL	5	- - -	- - -	- - -
GMF GSP - removal area	FILL	4	- - -	- - -	- - -
LF - consolidation area	CCR	3	7,500	3.20E-09	1.20E-05

[O: SLN 04/01/22; C: EGP 4/29/22]

Notes:

- - - = not included
- CBR = closure by removal
- CCR = coal combustion residuals
- CIP = closure in place
- ft/day = feet per day
- GMF GSP = Gypsum Management Facility Gypsum Stack Pond
- GMF RP = Gypsum Management Facility Gypsum Recycle Pond
- inches/yr = inches per year
- LF = Landfill
- mg/L = milligrams per liter

FIGURES



- PART 845 REGULATED UNIT (SUBJECT UNIT)
- PROPERTY BOUNDARY
- COFFEEN LAKE STATE FISH AND WILDLIFE AREA

SITE LOCATION MAP

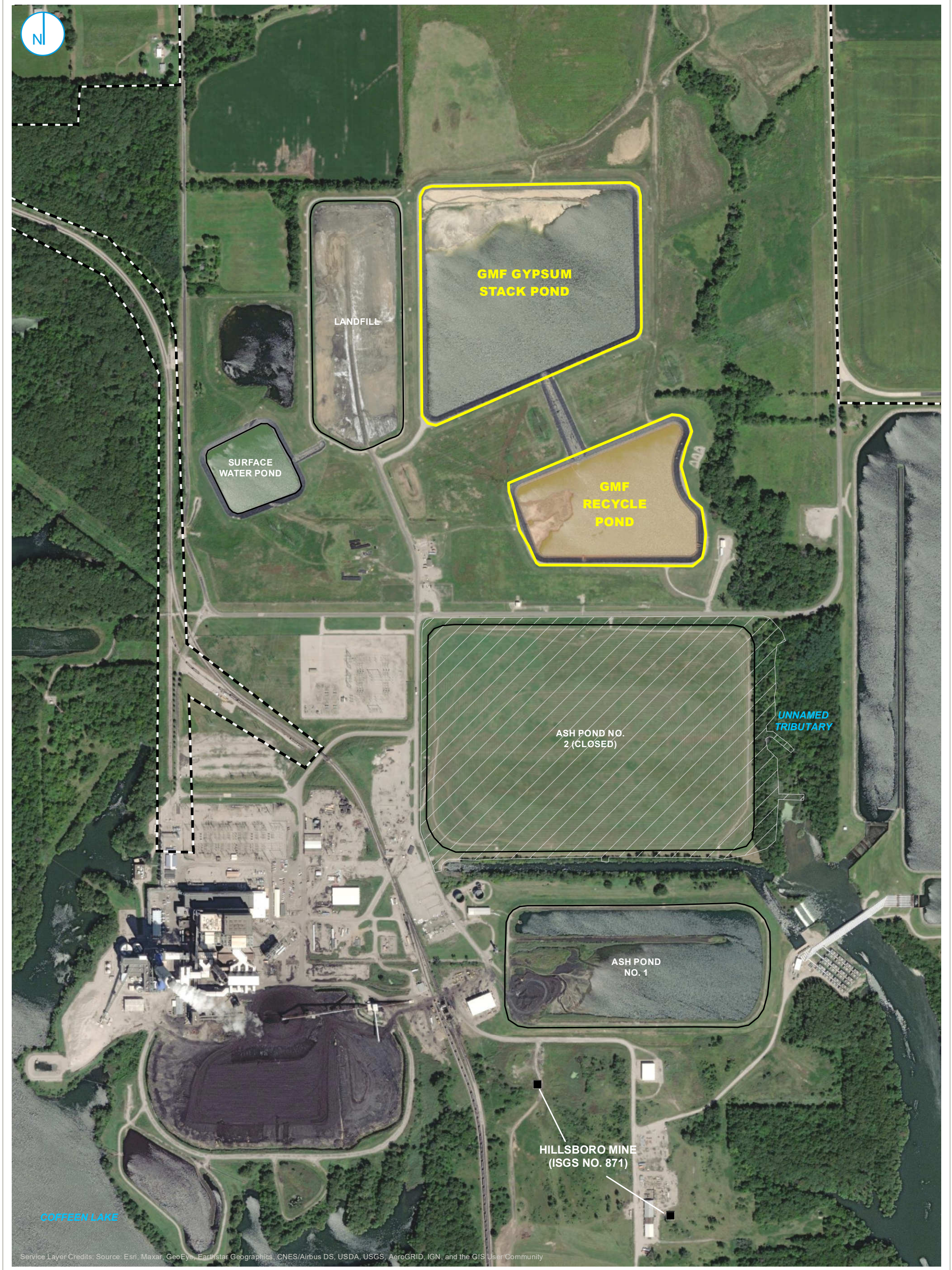
FIGURE 1-1

0 1,000 2,000 Feet

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- COAL MINE SHAFT
- ▭ PART 845 REGULATED UNIT (SUBJECT UNIT)
- ▭ SITE FEATURE
- ▭ LIMITS OF FINAL COVER
- ▭ PROPERTY BOUNDARY

SITE MAP

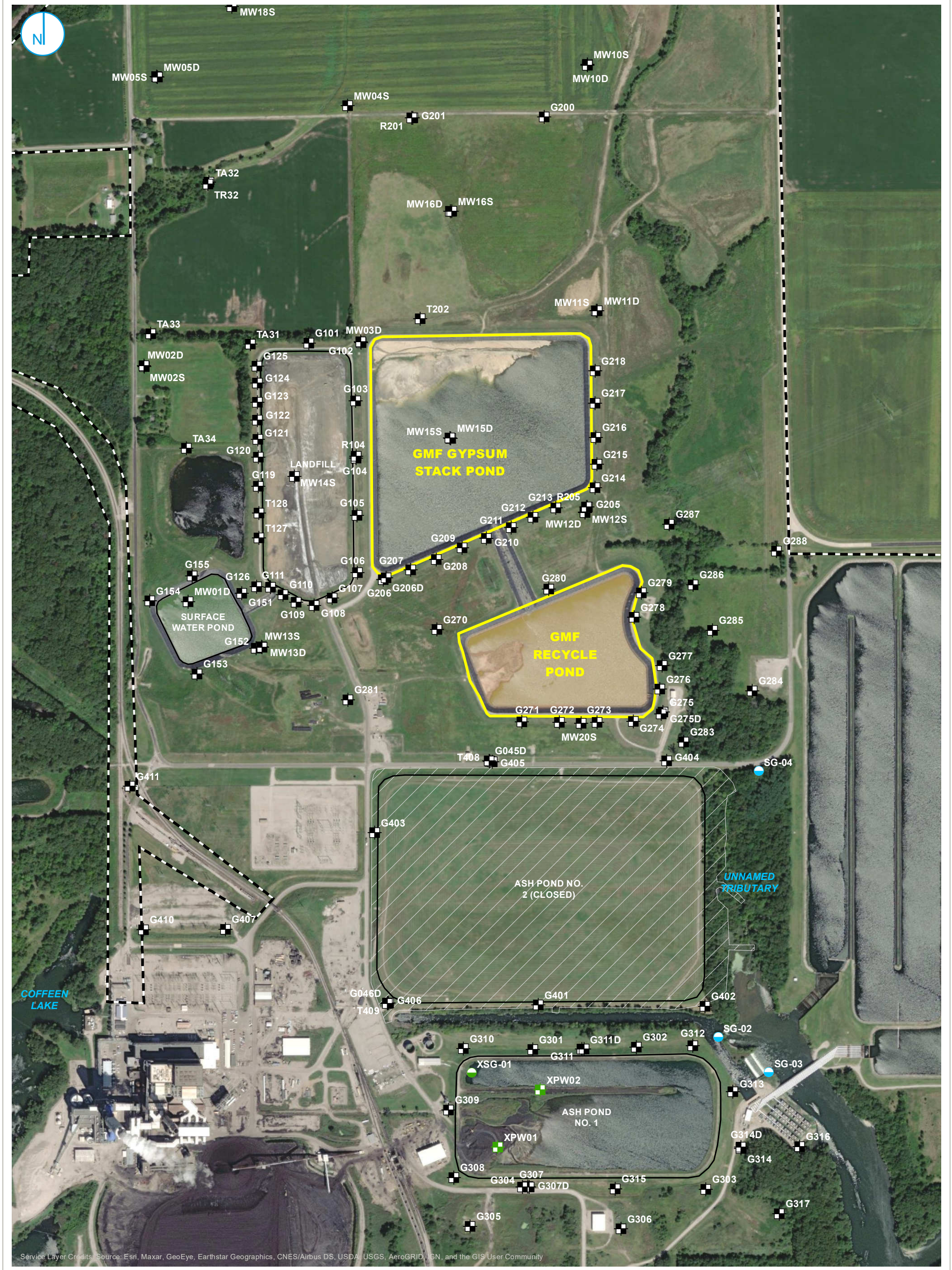
FIGURE 1-2

0 275 550
Feet

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- MONITORING WELL
 - PORE WATER WELL
 - STAFF GAGE, RIVER
 - STAFF GAGE, CCR UNIT
 - PART 845 REGULATED UNIT (SUBJECT UNIT)
 - SITE FEATURE
 - LIMITS OF FINAL COVER
 - PROPERTY BOUNDARY
- 0 275 550 Feet

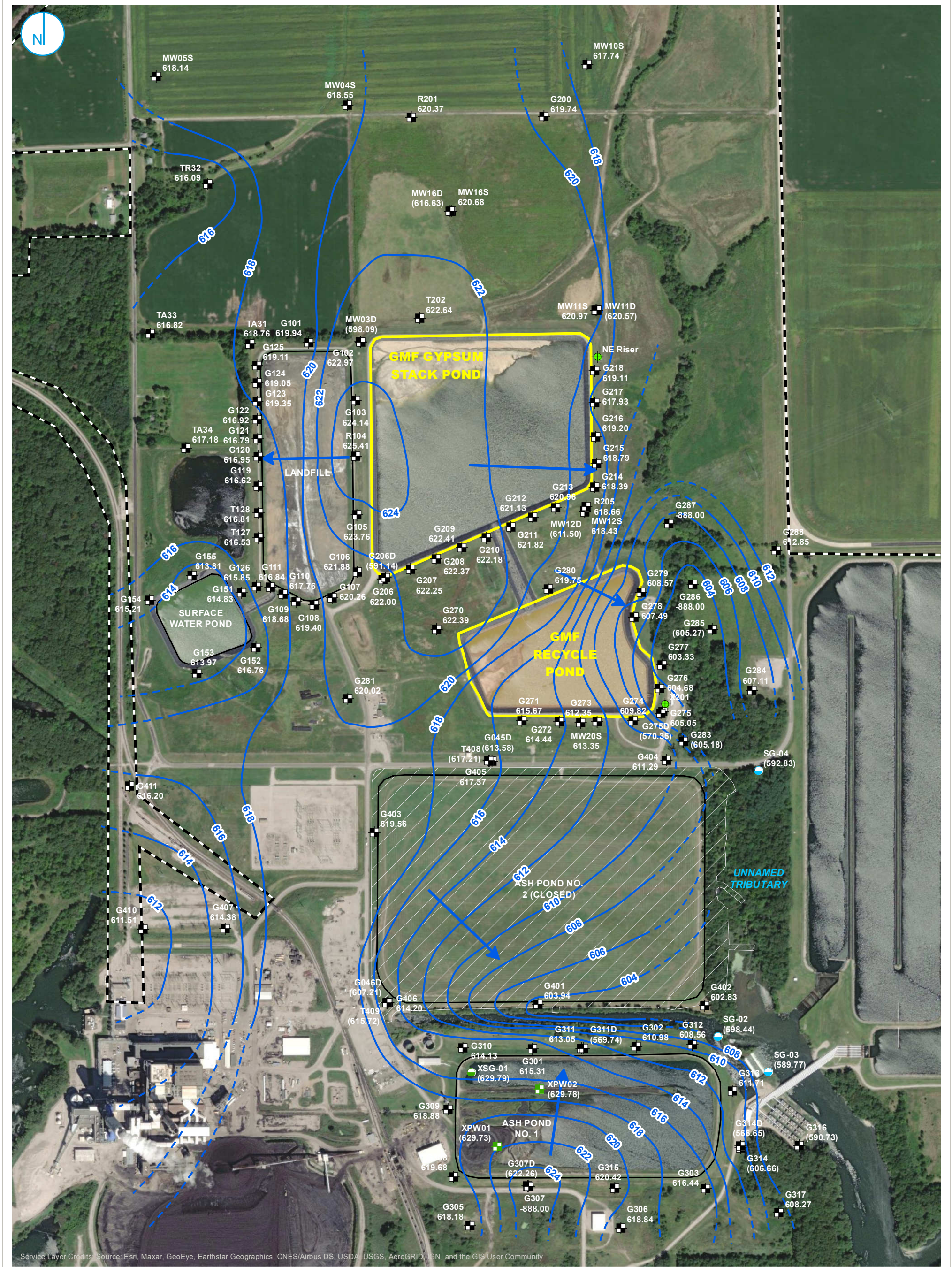
MONITORING WELL LOCATION MAP

FIGURE 2-1

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
 ENGINEERING SOLUTIONS, INC.





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- MONITORING WELL
- PORE WATER WELL
- LEACHATE WELL
- STAFF GAGE, RIVER
- STAFF GAGE, CCR UNIT
- PART 845 REGULATED UNIT (SUBJECT UNIT)
- LIMITS OF FINAL COVER
- PROPERTY BOUNDARY
- GROUNDWATER ELEVATION CONTOUR (2-FT CONTOUR INTERVAL, NAVD88)
- INFERRED GROUNDWATER ELEVATION CONTOUR
- GROUNDWATER FLOW DIRECTION

NOTE:
ELEVATIONS IN PARENTHESES WERE NOT USED FOR CONTOURING.

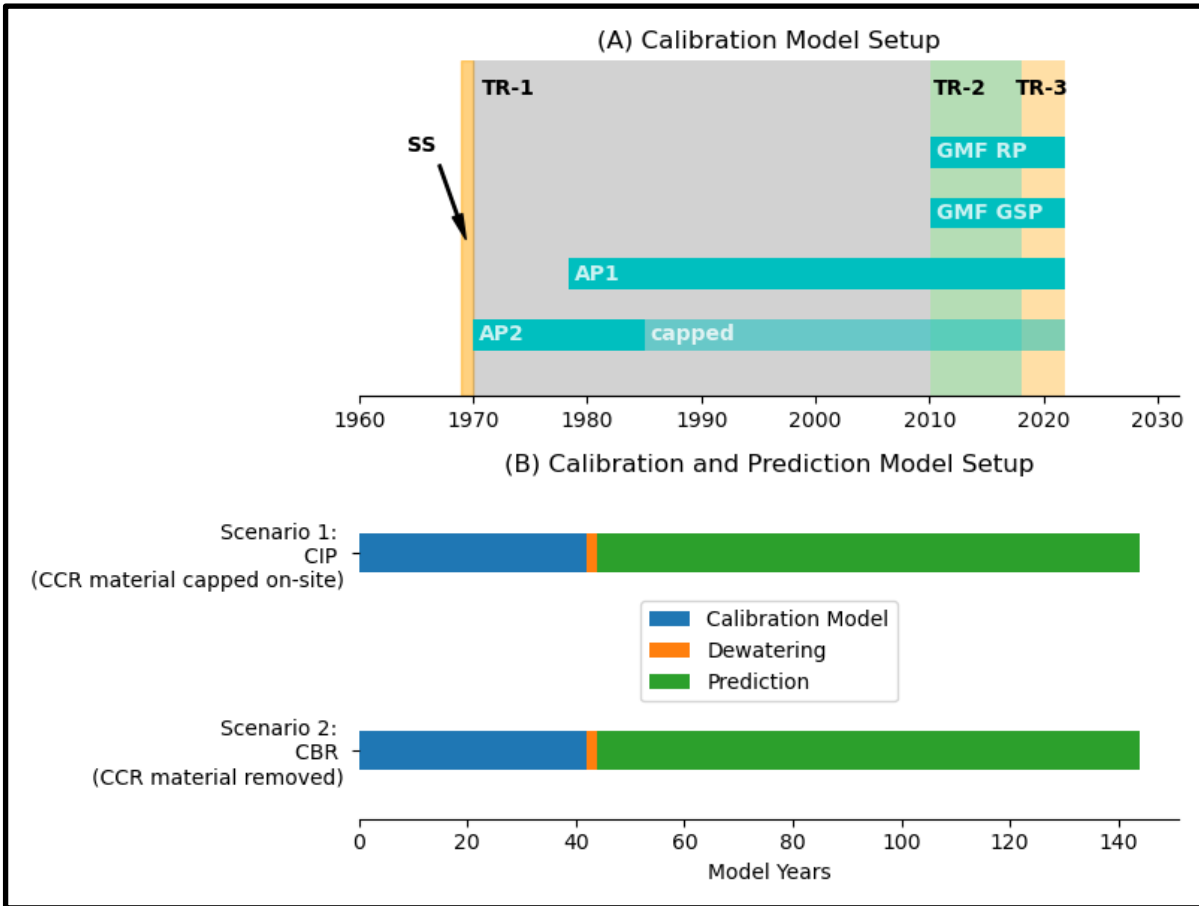
UPPERMOST AQUIFER POTENTIOMETRIC SURFACE MAP
JULY 26, 2021

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

FIGURE 2-3

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





CALIBRATION AND PREDICTIVE TIMELINE (SS = STEADY STATE MODEL AND TR = TRANSIENT MODEL).

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS





Service Layer Credits: Source: Esri, Maxar, GeoEye, Earthstar Geographics, CNES/Airbus DS, USDA, USGS, AeroGRID, IGN, and the GIS User Community

- DRAIN
- STREAM
- LAKE
- COOLING CONDENSER FLUME
- SI UNIT
- POND
- MODEL GRID
- PART 845 REGULATED UNIT (SUBJECT UNIT)

0 625 1,250
Feet

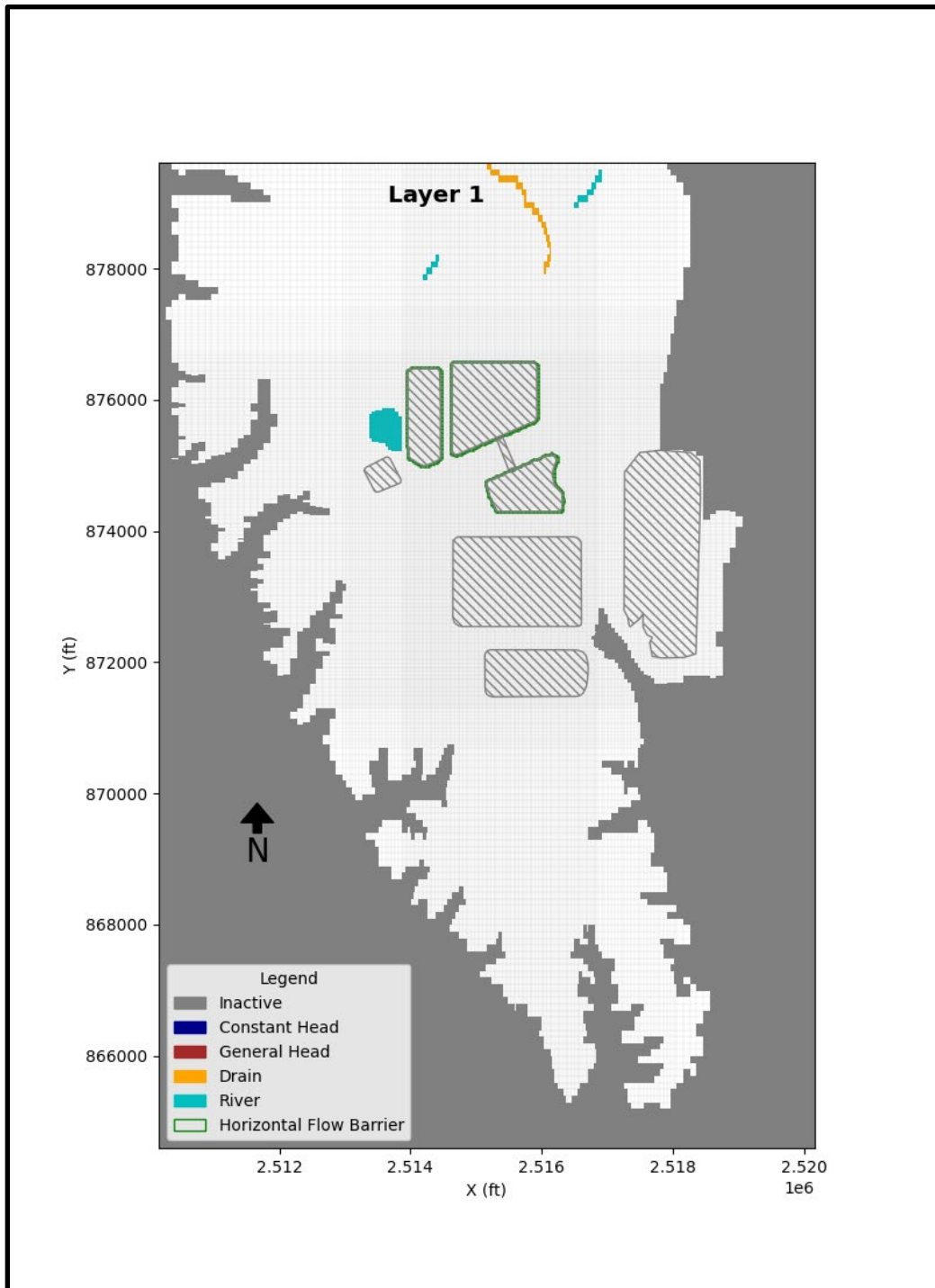
MODEL AREA MAP

FIGURE 5-1

**GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS**

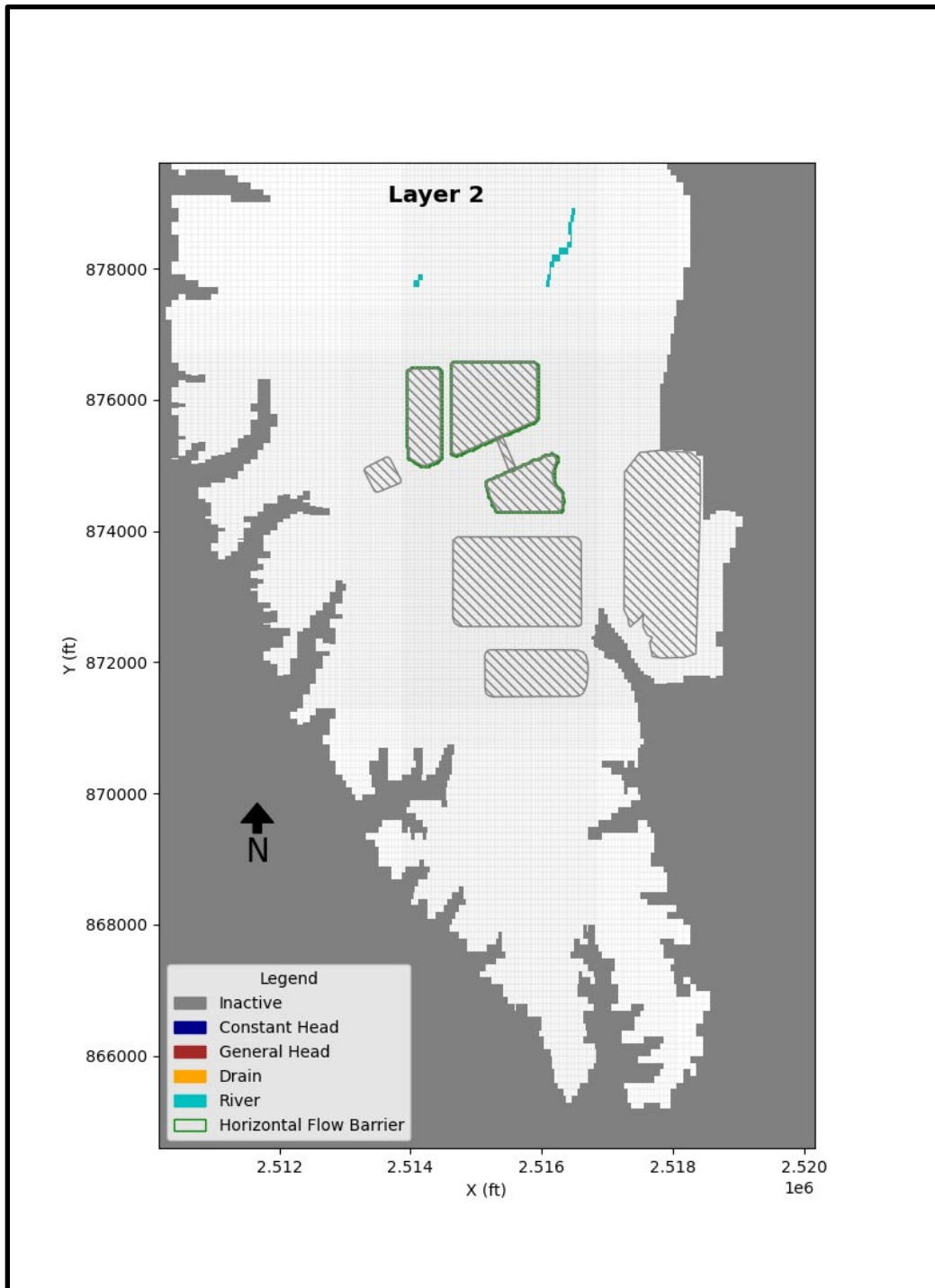
RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





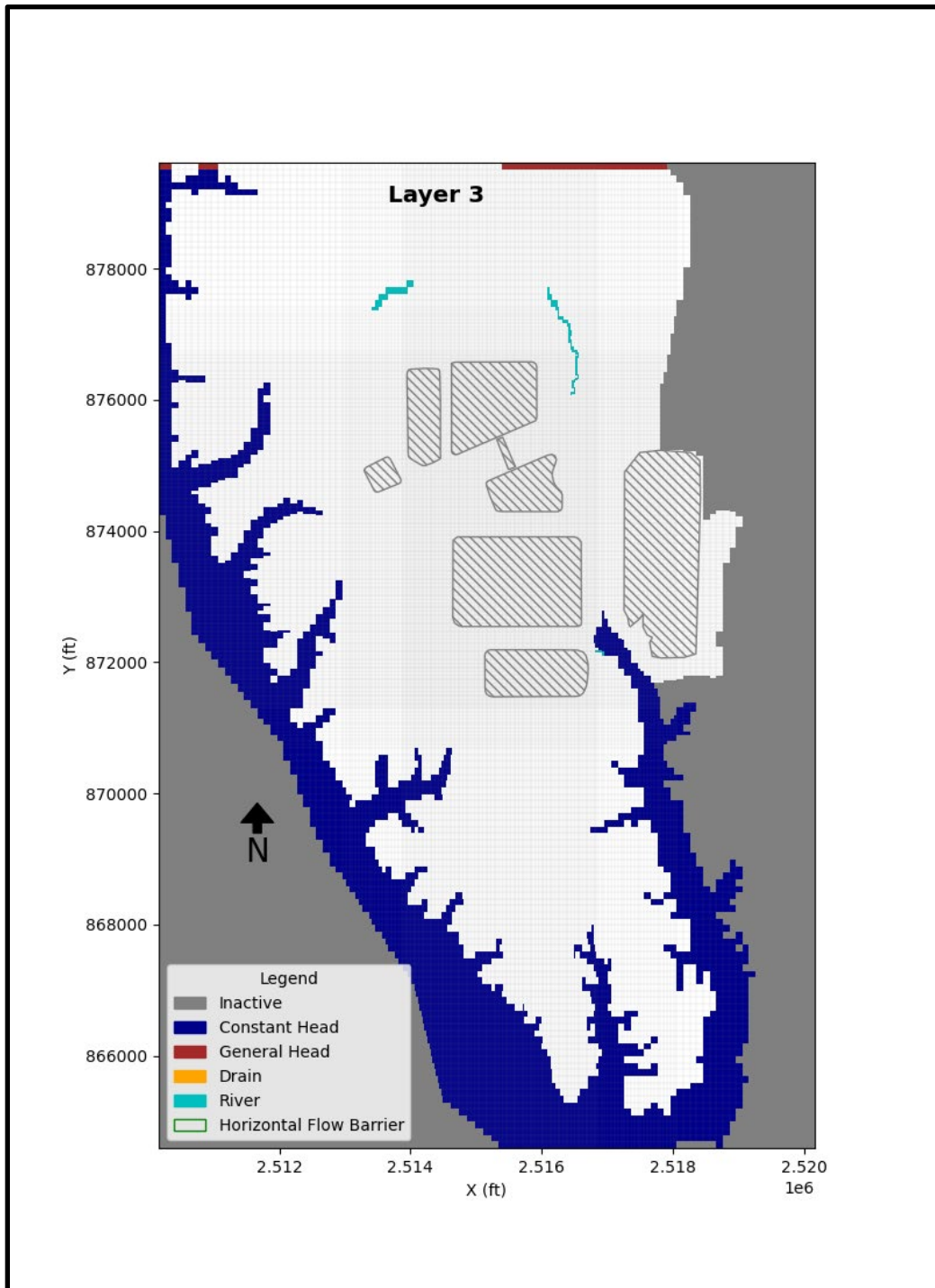
BOUNDARY CONDITIONS FOR LAYER 1

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



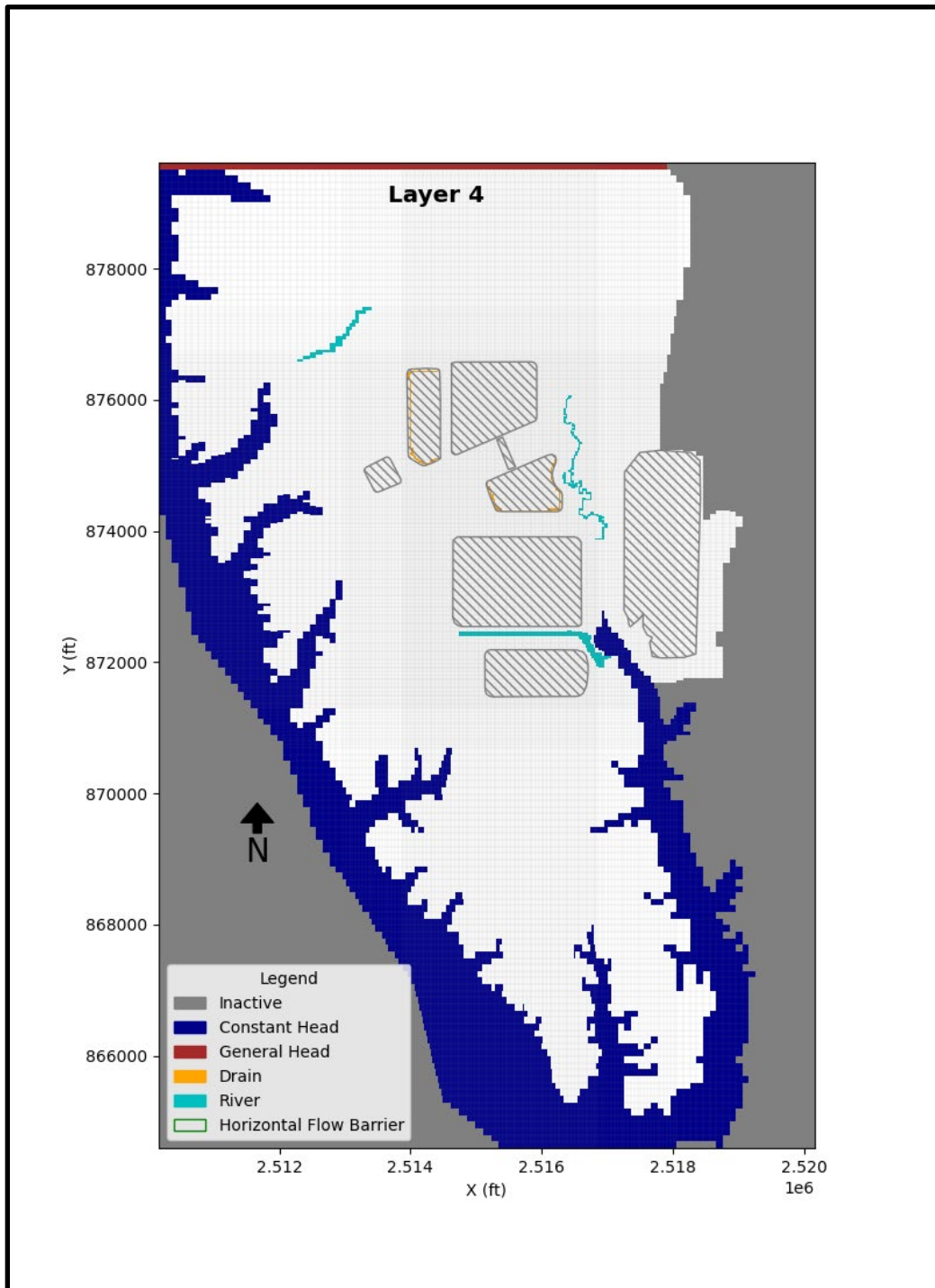
BOUNDARY CONDITIONS FOR LAYER 2

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



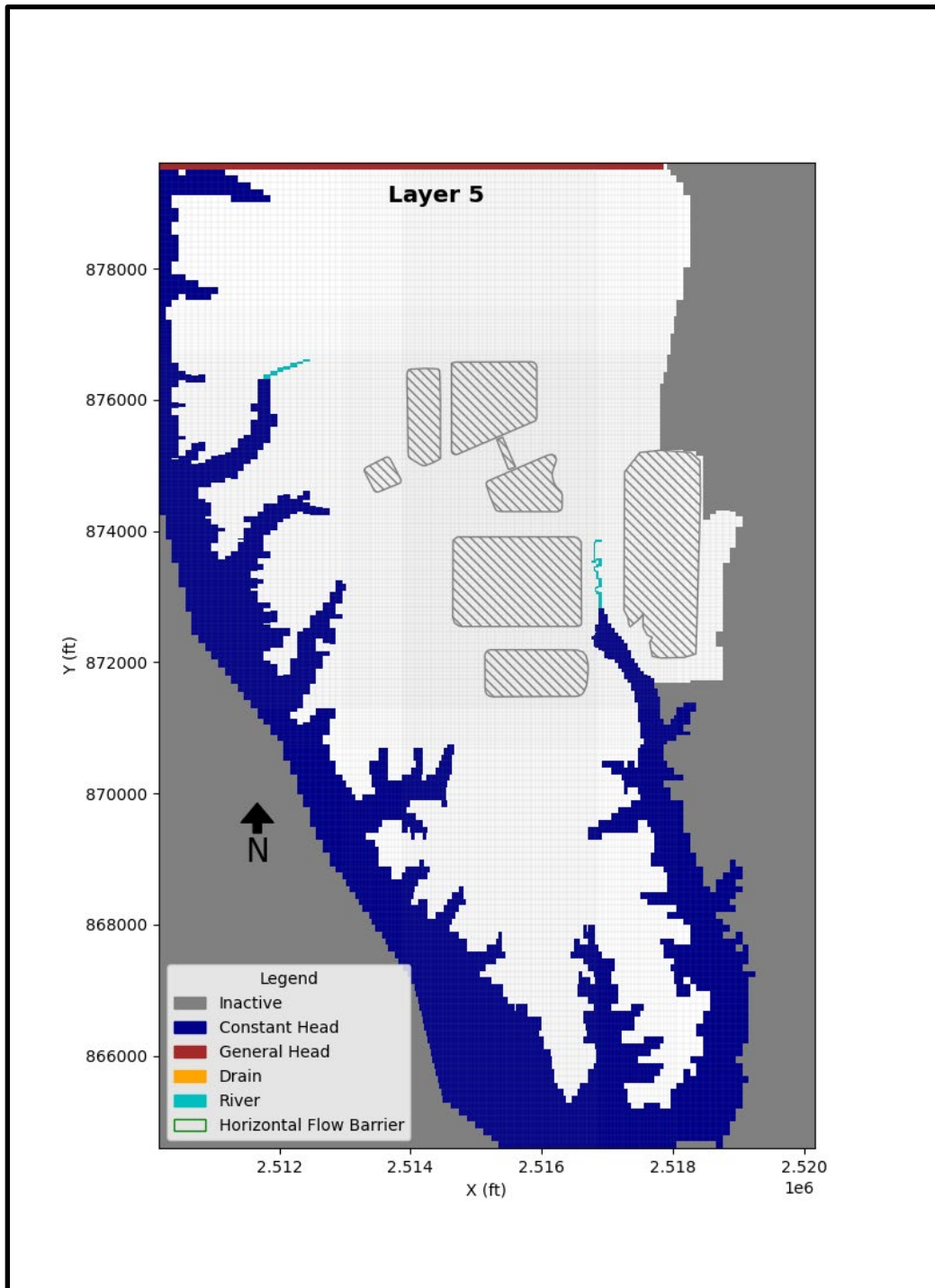
BOUNDARY CONDITIONS FOR LAYER 3

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



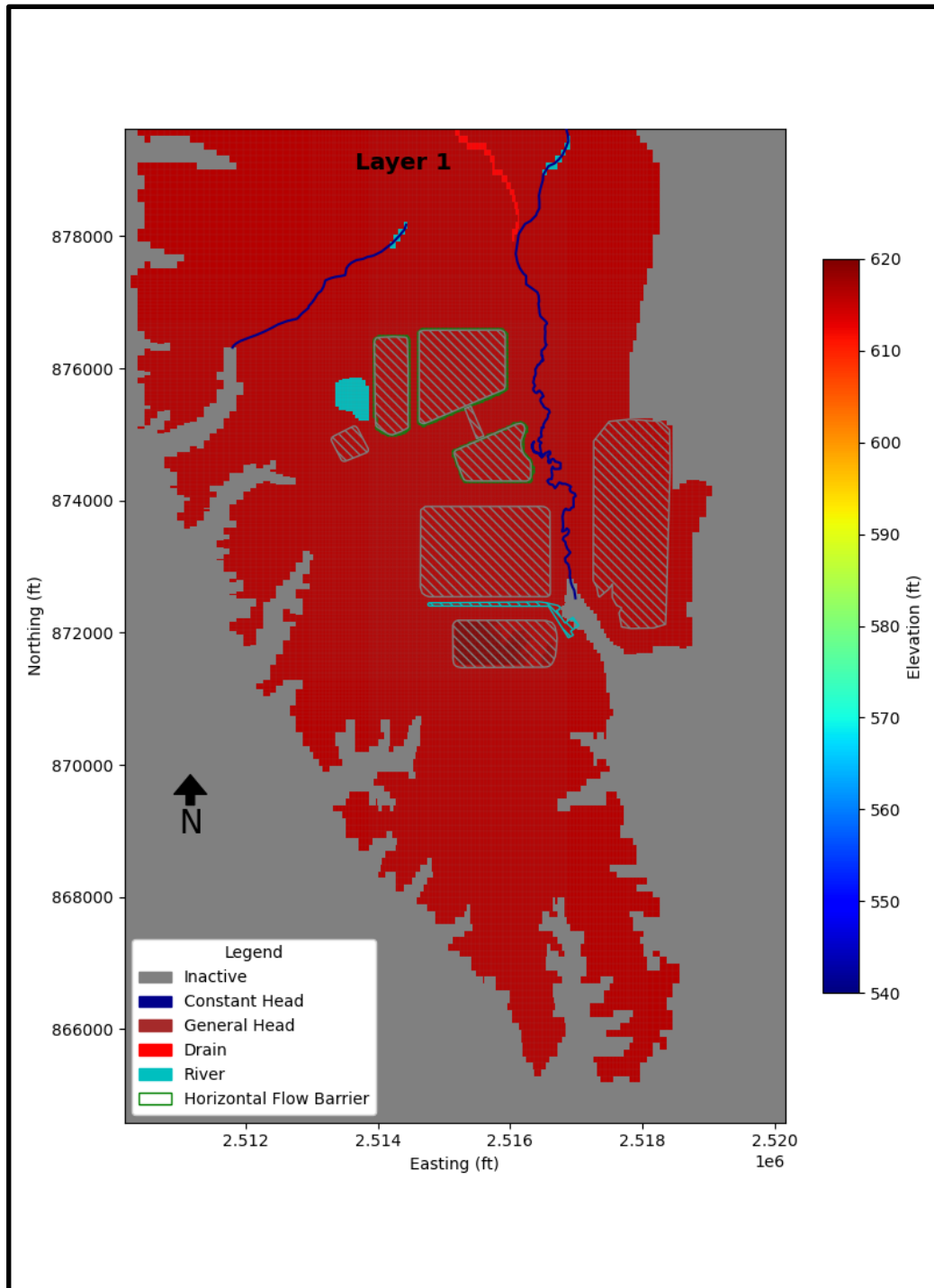
BOUNDARY CONDITIONS FOR LAYER 4

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



BOUNDARY CONDITIONS FOR LAYER 5

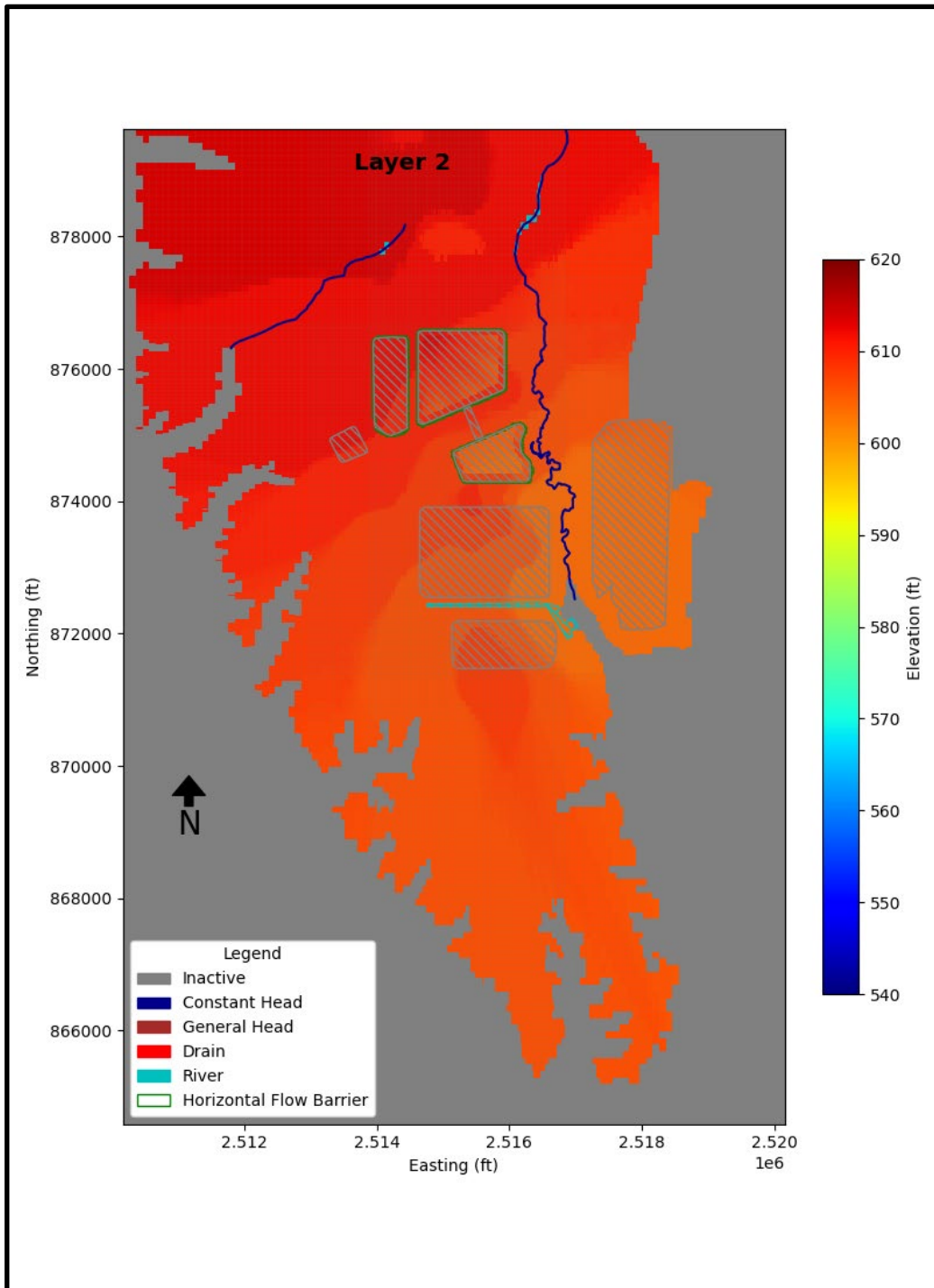
GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



BASE OF MODEL FOR LAYER 1

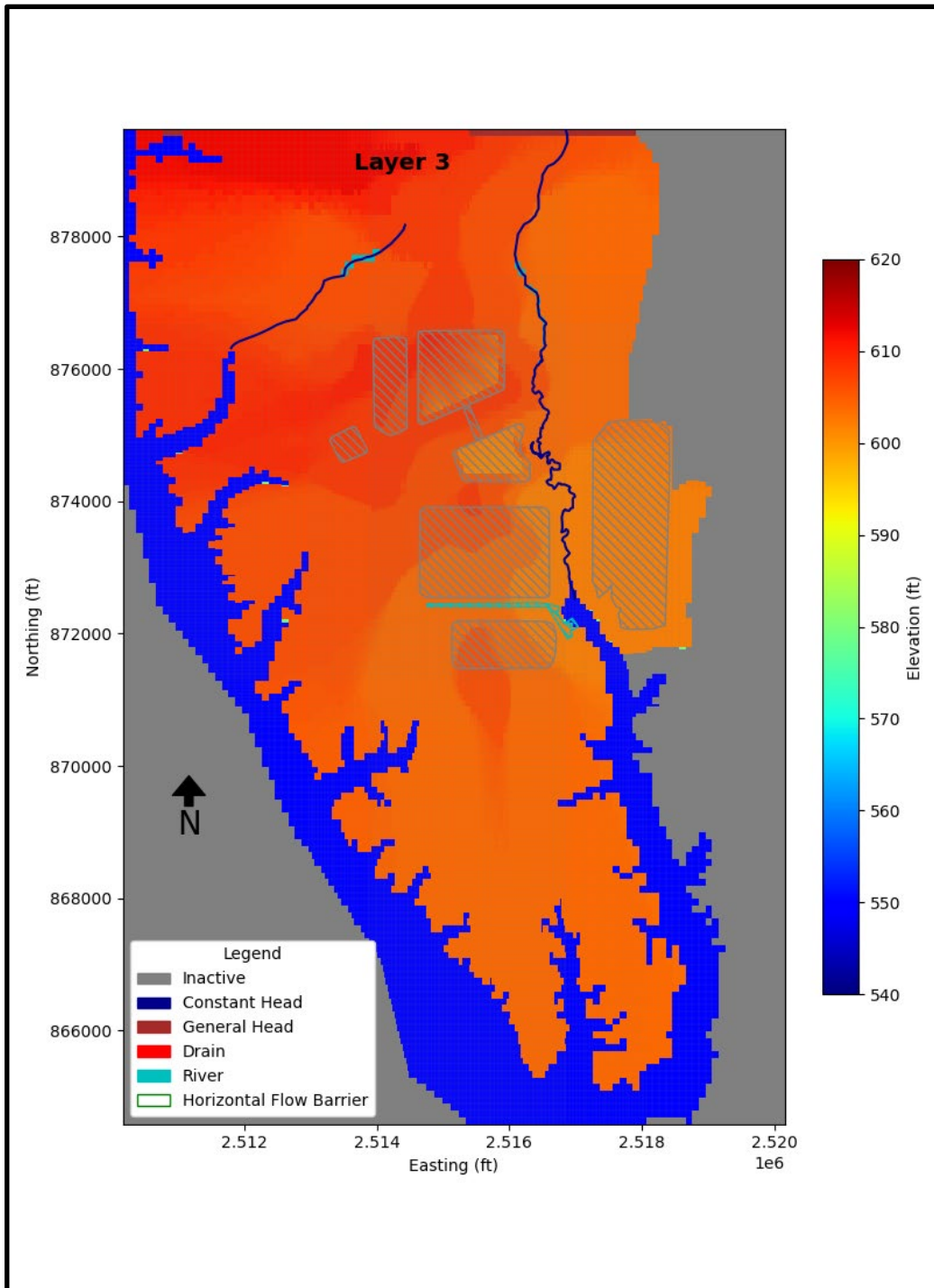
GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS





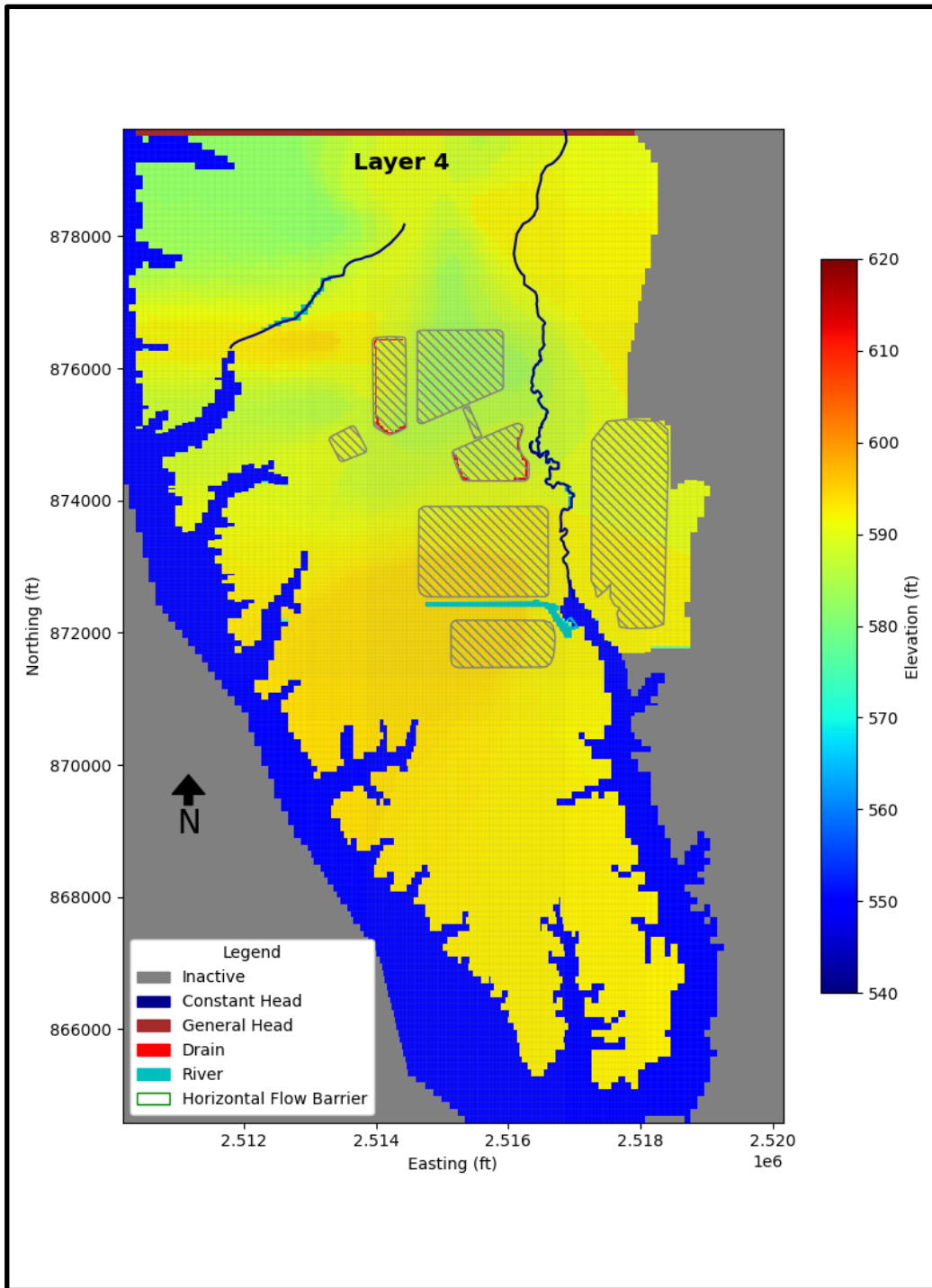
BASE OF MODEL FOR LAYER 2

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



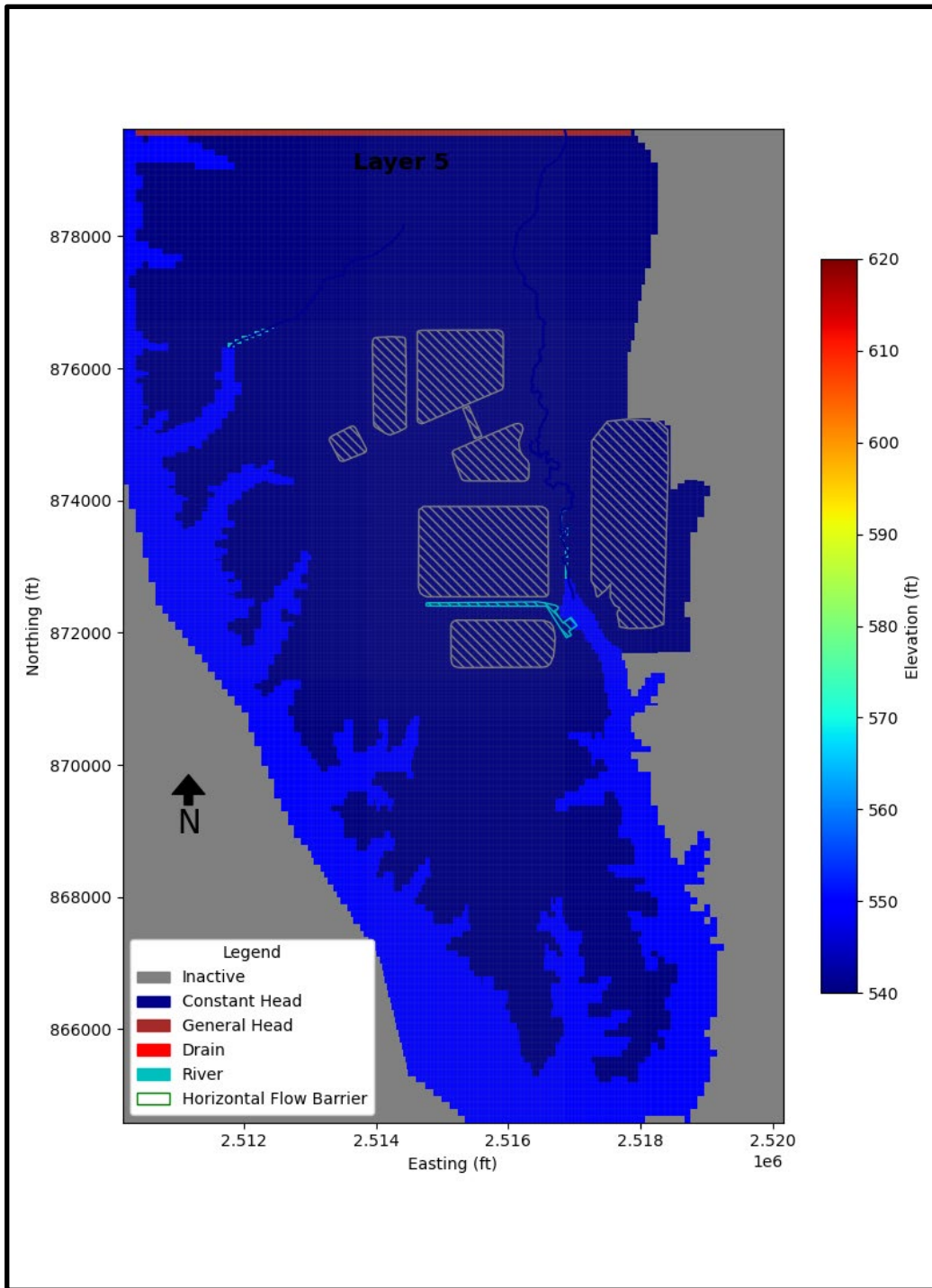
BASE OF MODEL FOR LAYER 3

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



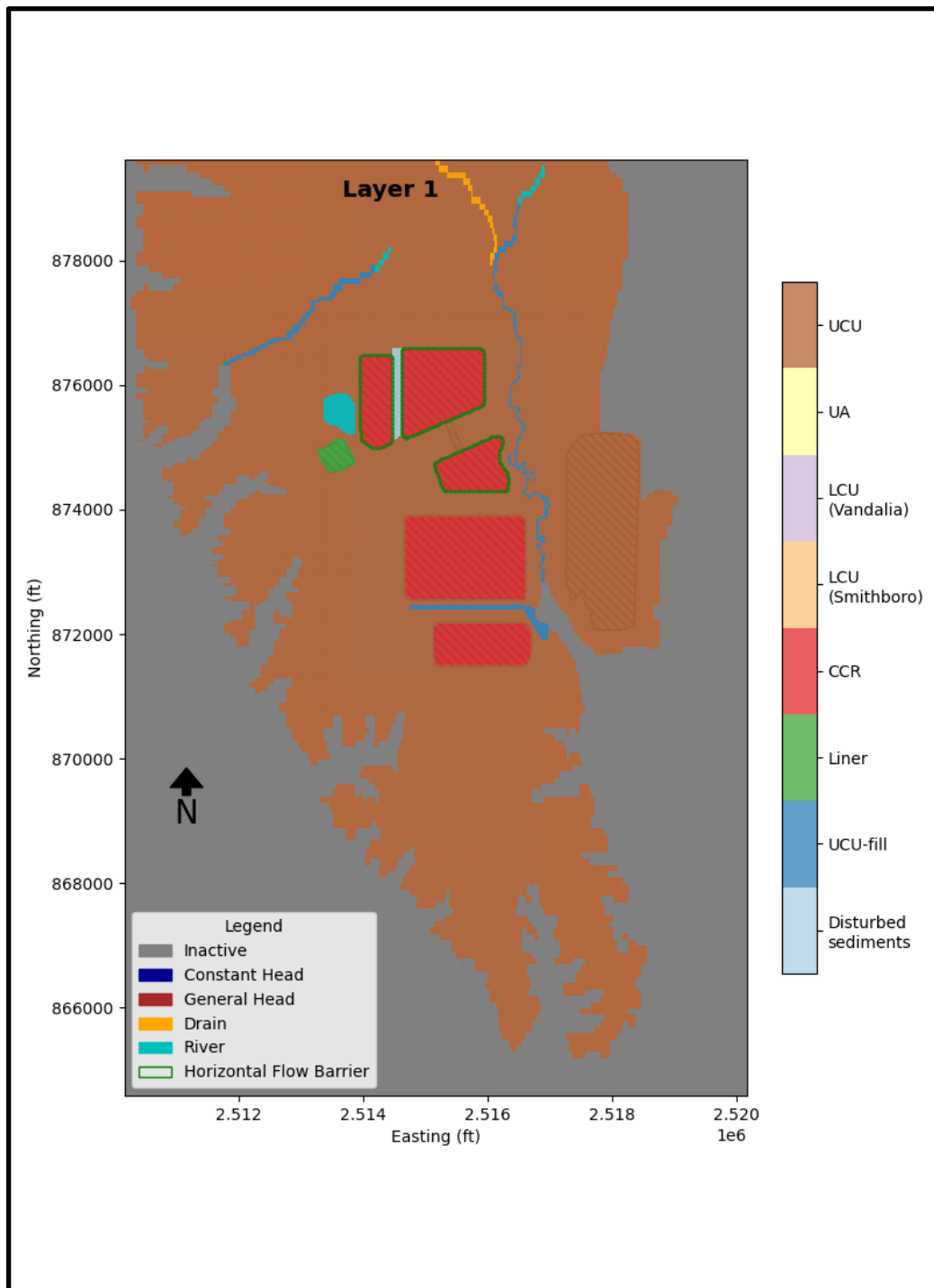
BASE OF MODEL FOR LAYER 4

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



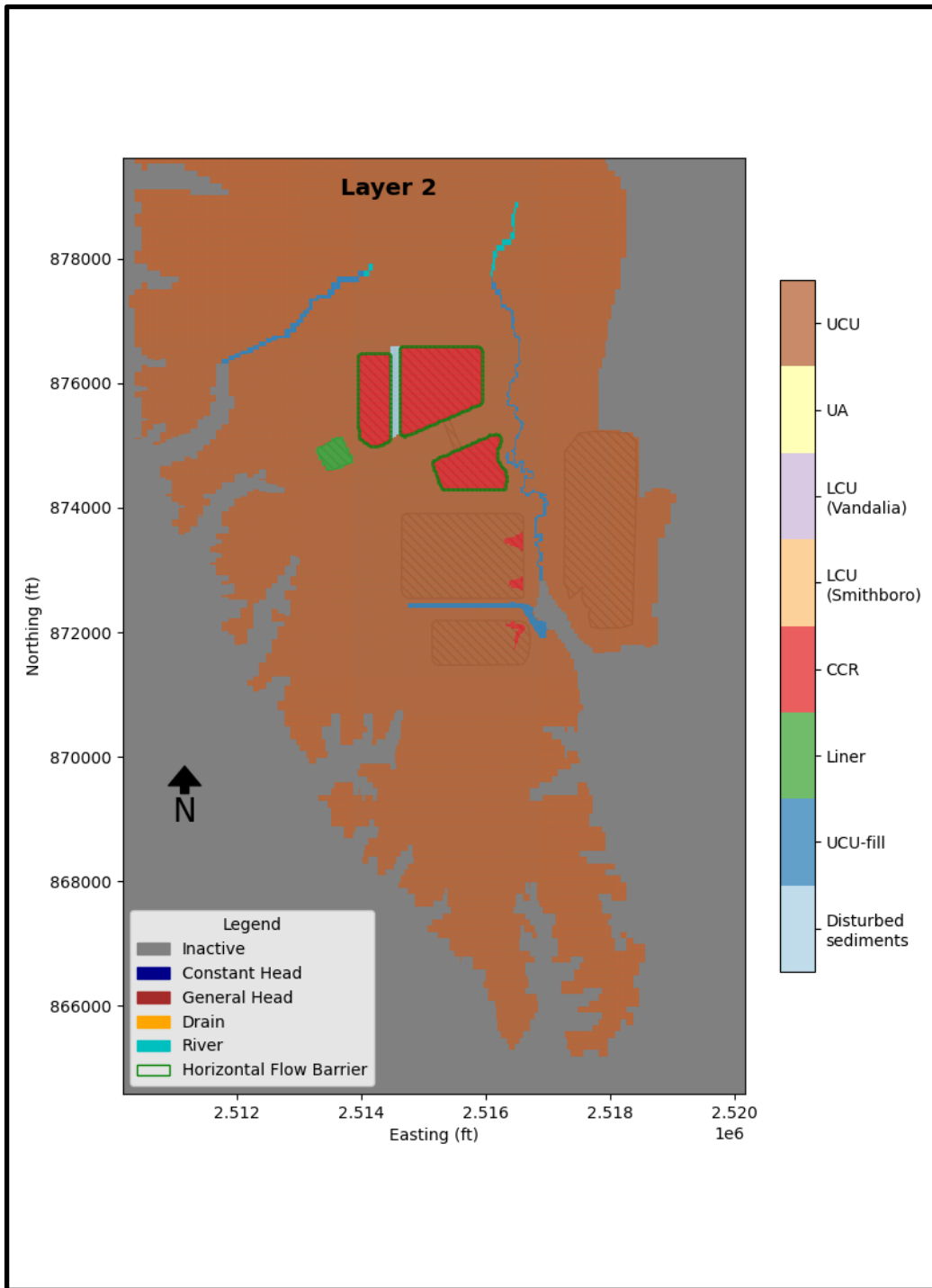
BASE OF MODEL FOR LAYER 5

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



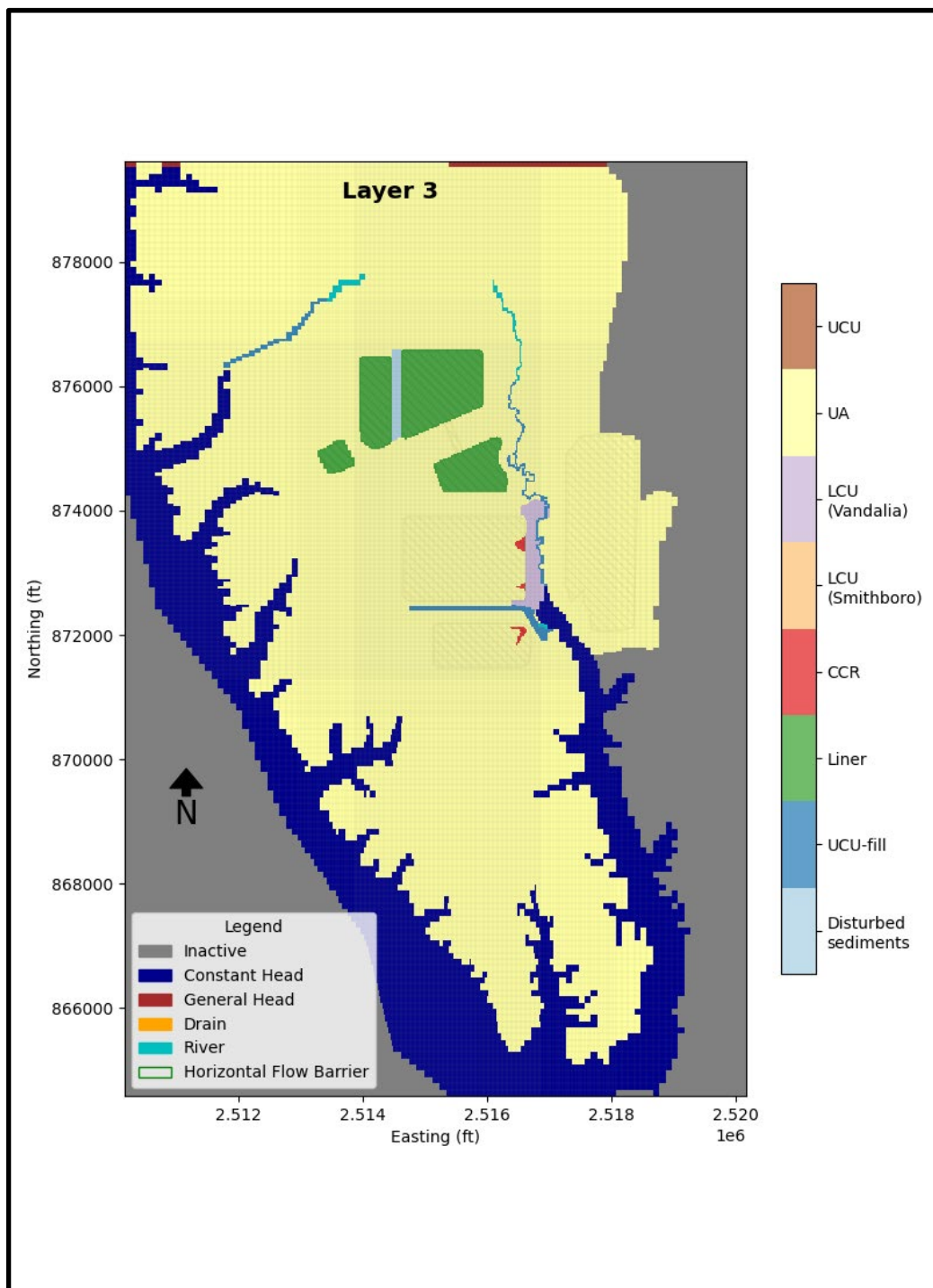
HYDRAULIC CONDUCTIVITY ZONES FOR LAYER 1

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



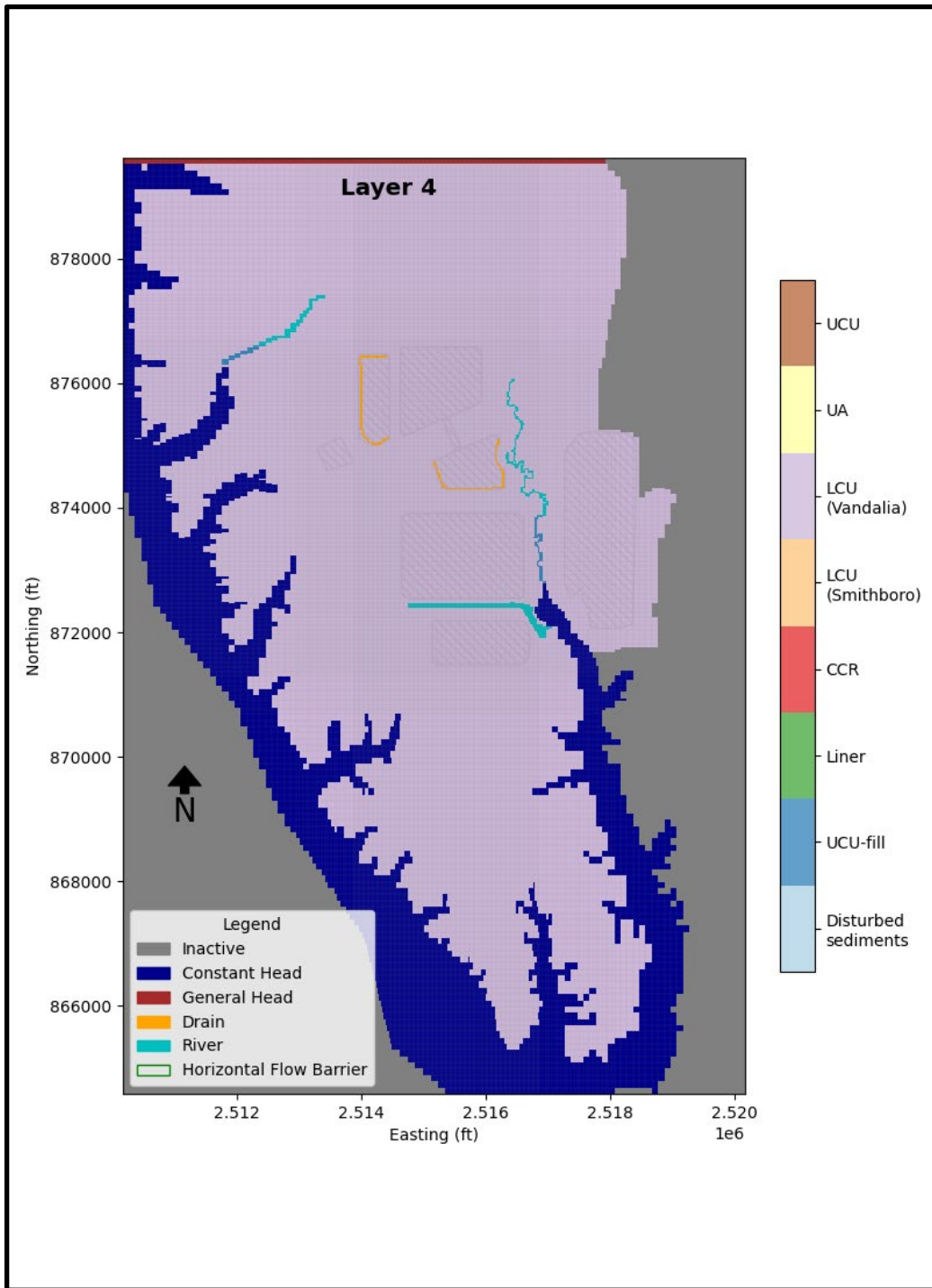
HYDRAULIC CONDUCTIVITY ZONES FOR LAYER 2

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



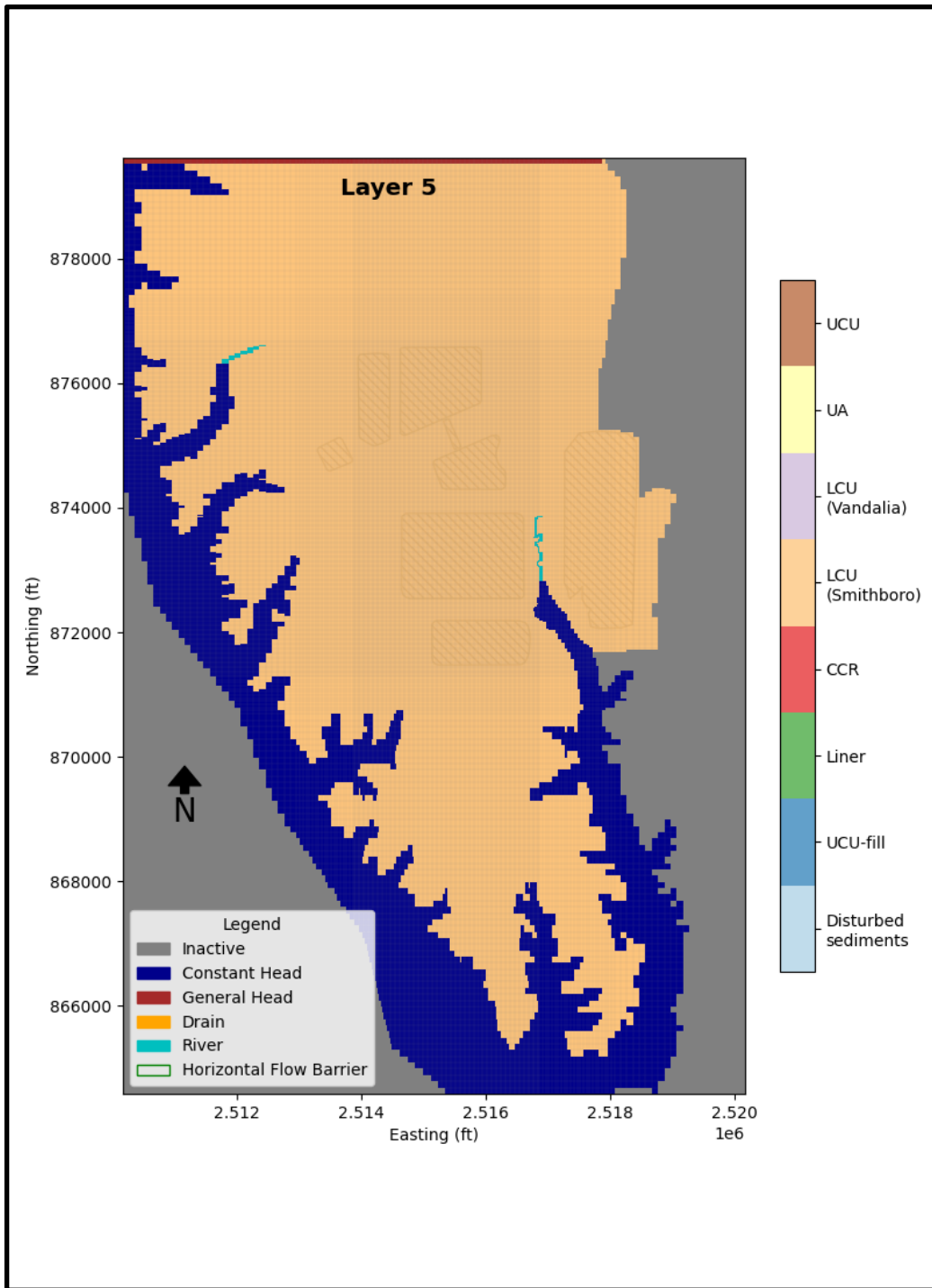
HYDRAULIC CONDUCTIVITY ZONES FOR LAYER 3

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



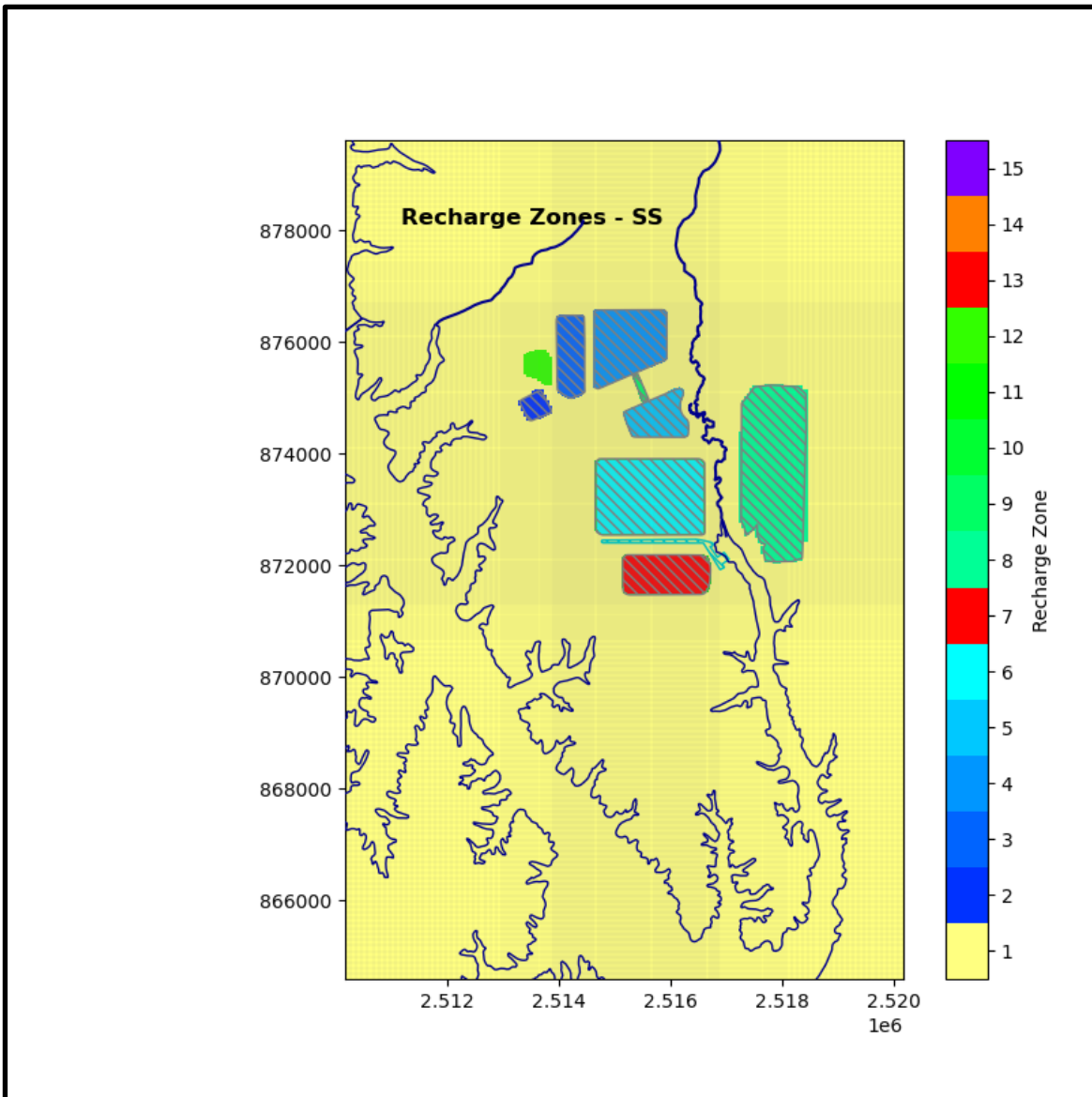
HYDRAULIC CONDUCTIVITY ZONES FOR LAYER 4

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



HYDRAULIC CONDUCTIVITY ZONES FOR LAYER 5

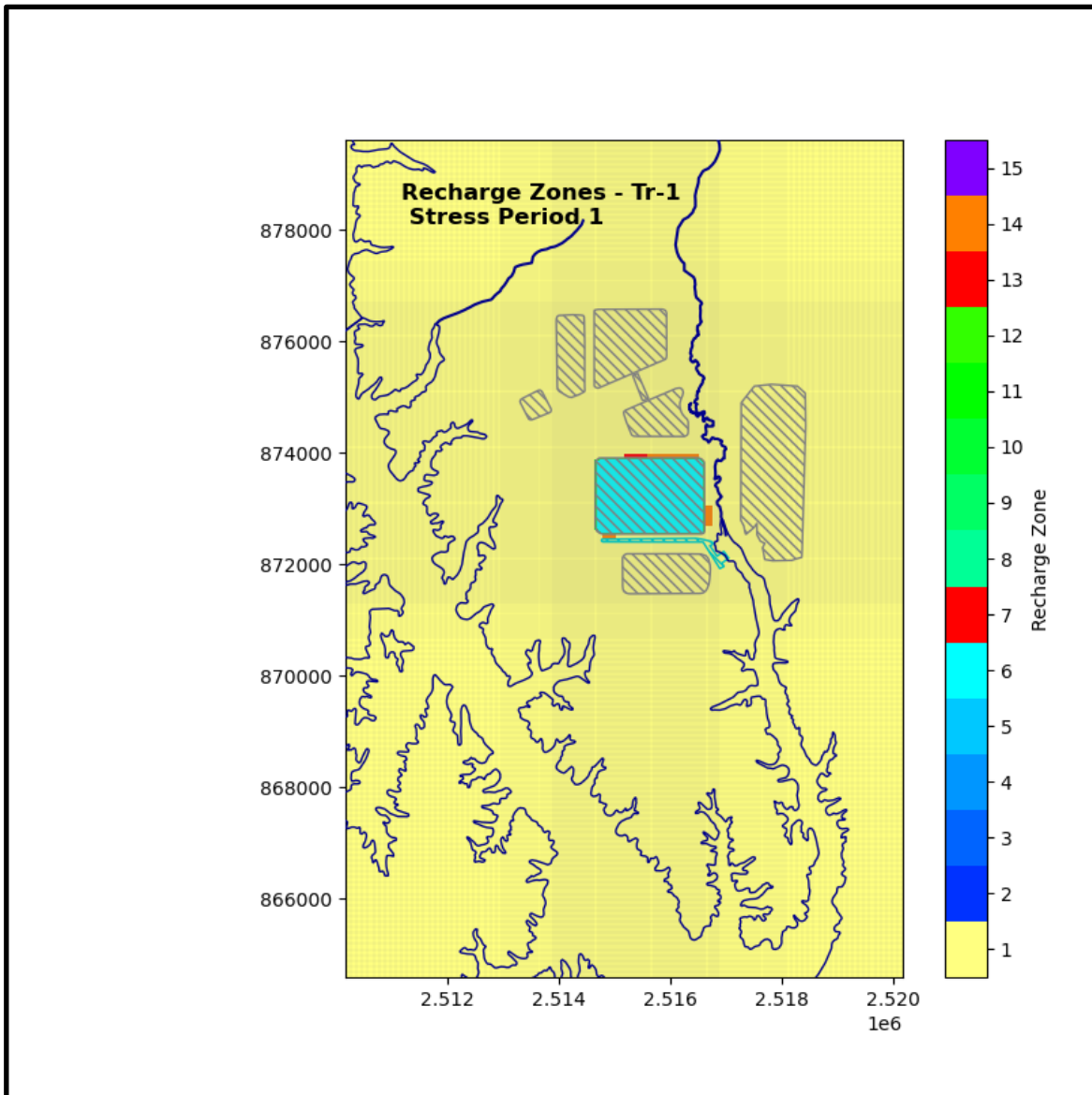
GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



MODEL RECHARGE DISTRIBUTION STEADY STATE (SS) MODEL

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

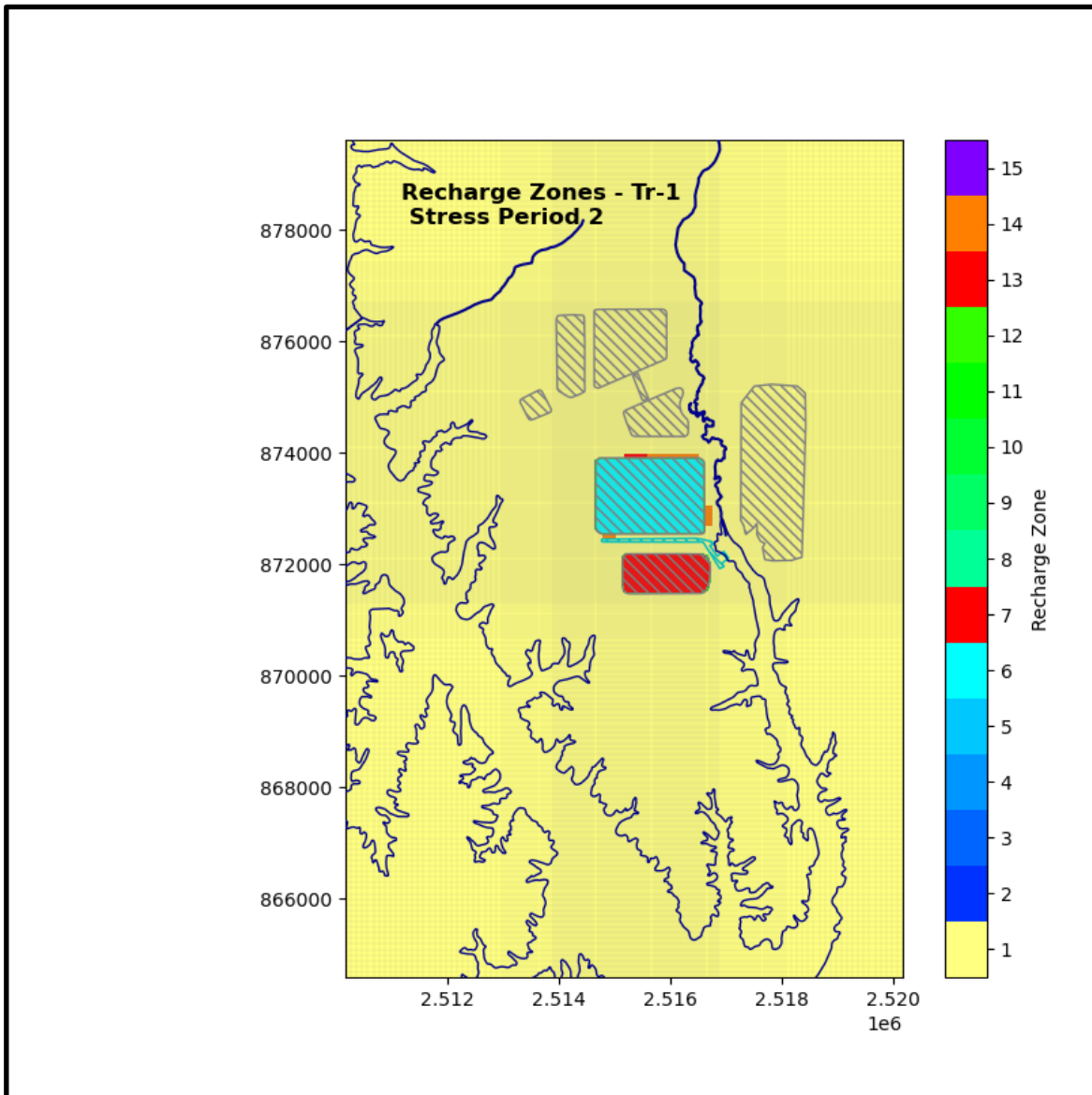




MODEL RECHARGE DISTRIBUTION FOR THE TRANSIENT (TR) MODEL TR-1
STRESS PERIOD 1

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

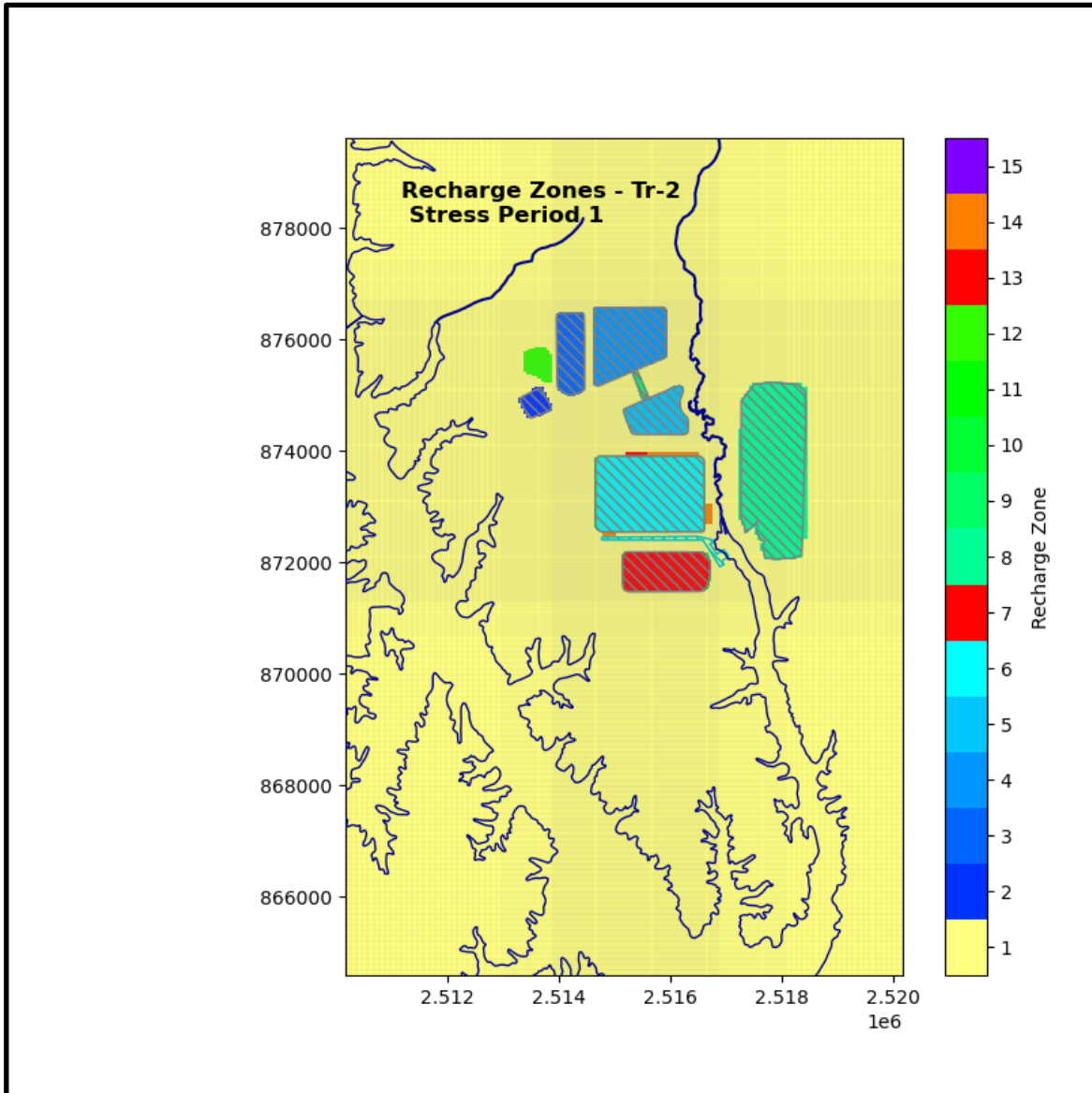




MODEL RECHARGE DISTRIBUTION FOR THE TRANSIENT (TR) MODEL TR-1
STRESS PERIOD 2

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

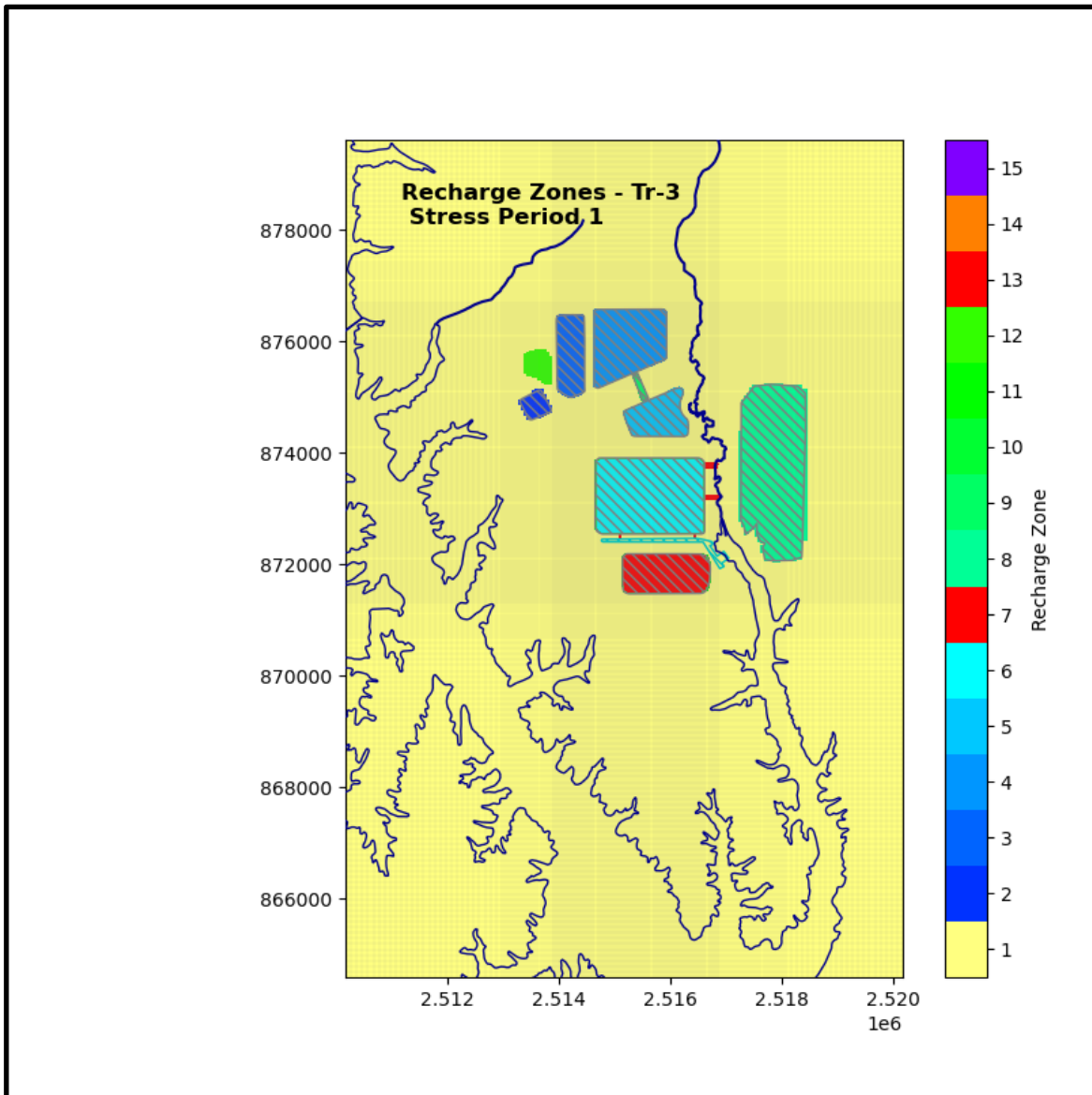




MODEL RECHARGE DISTRIBUTION FOR THE TRANSIENT (TR) MODEL TR-2
STRESS PERIOD 1

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

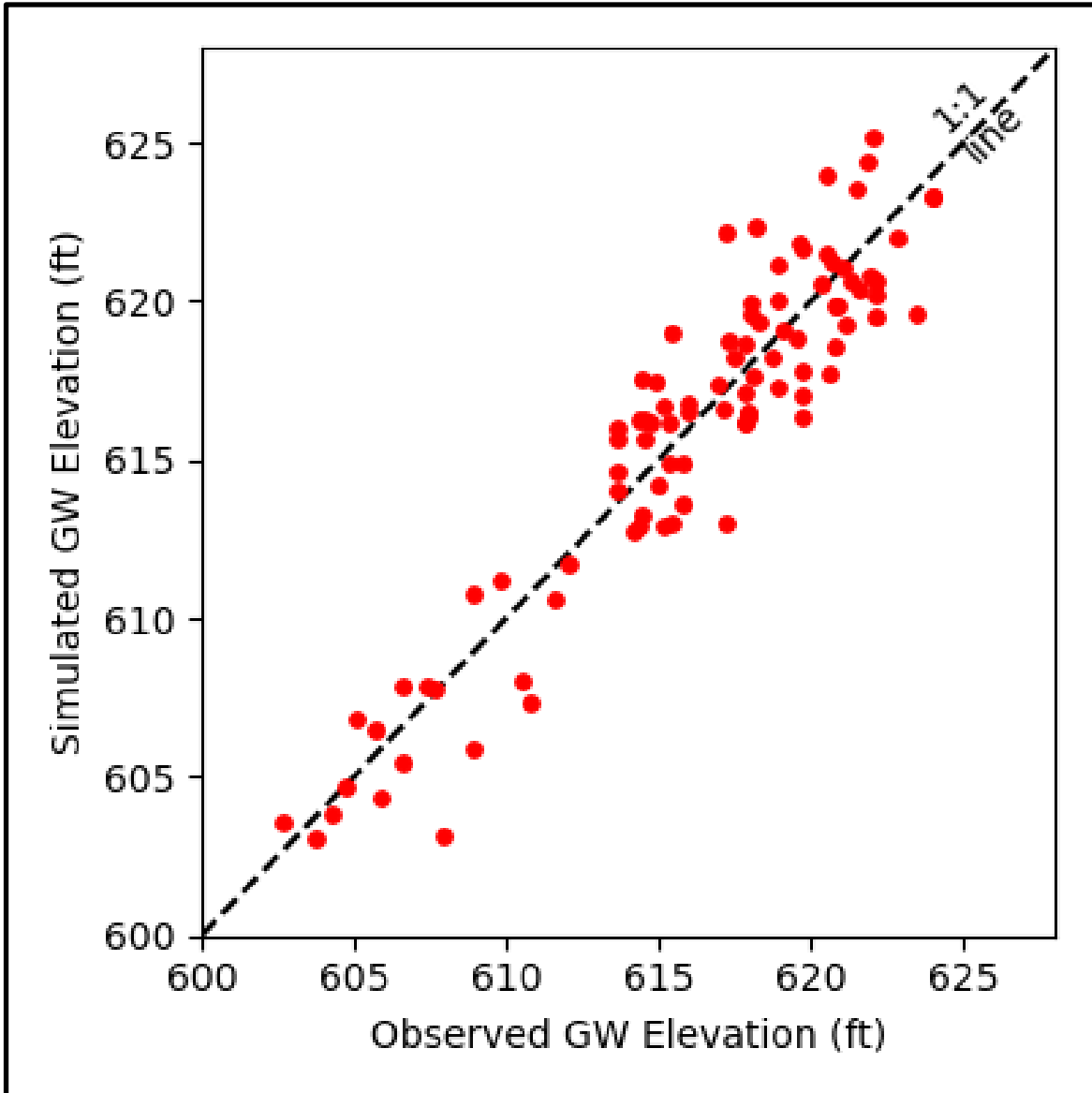




MODEL RECHARGE DISTRIBUTION FOR THE TRANSIENT (TR) MODEL TR-3
STRESS PERIOD 1

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

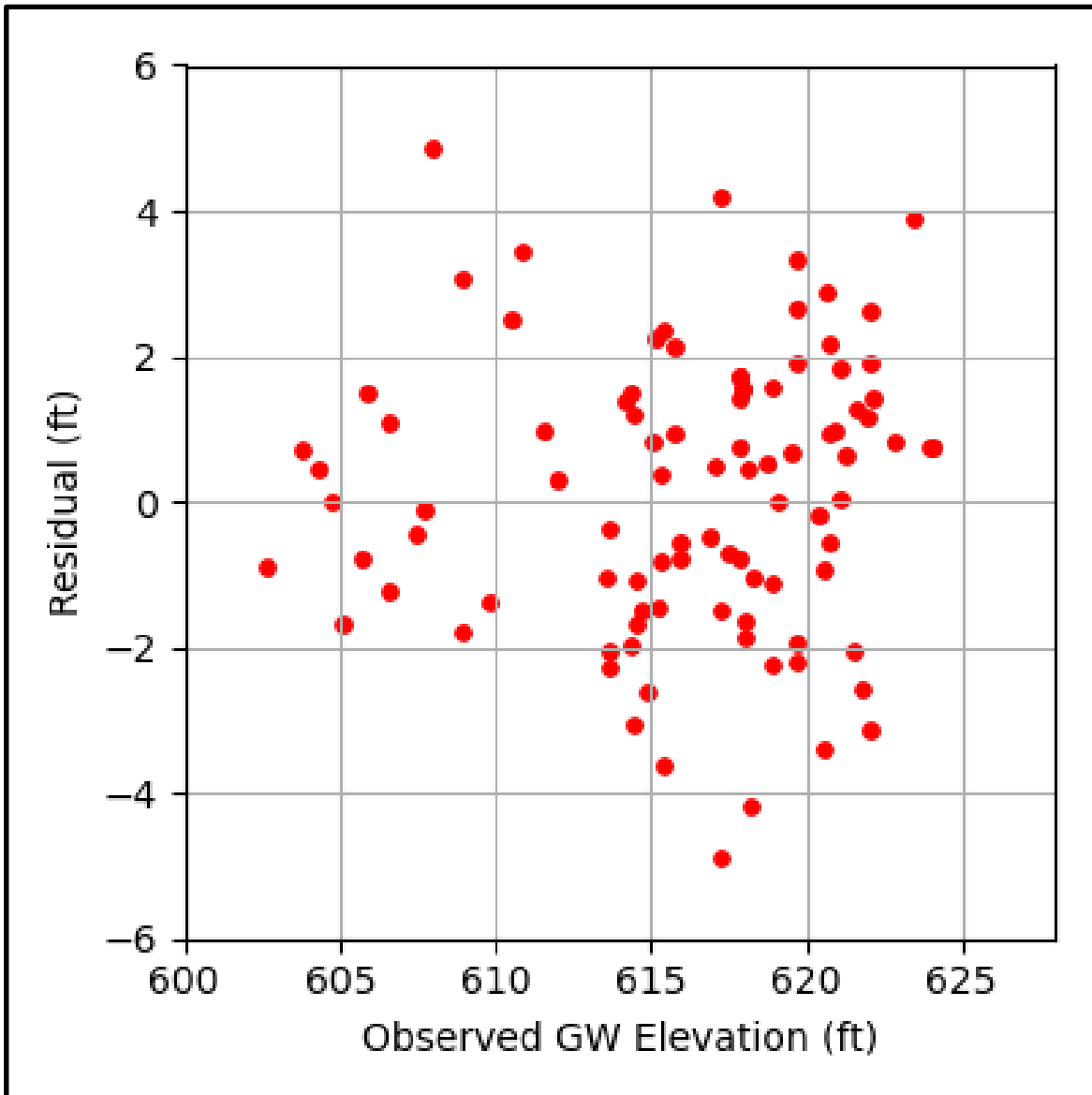




OBSERVED VERSUS SIMULATED STEADY STATE GROUNDWATER LEVELS FROM THE CALIBRATED MODEL

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

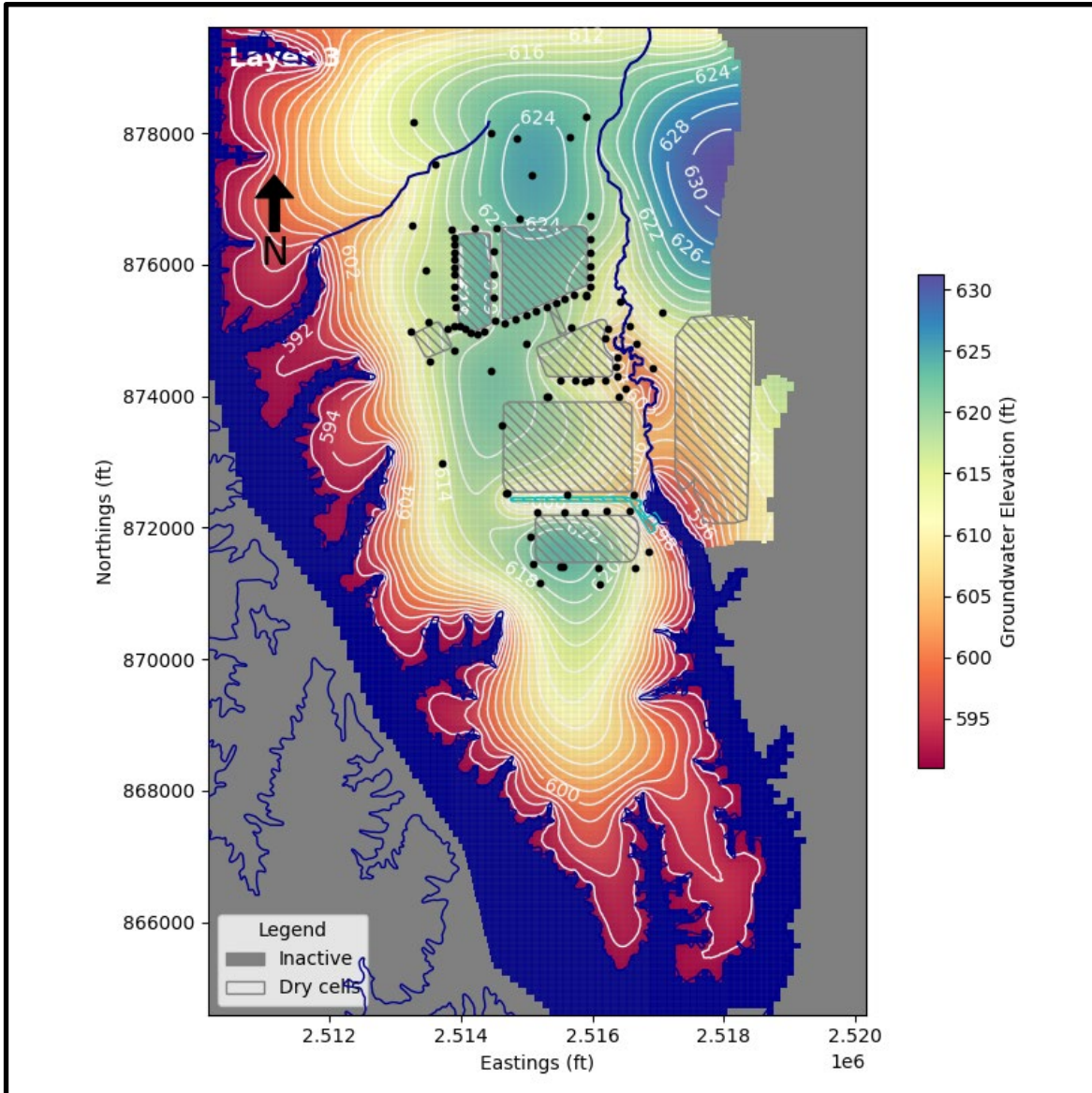




SIMULATED GROUNDWATER LEVEL RESIDUALS FROM THE CALIBRATED MODEL

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

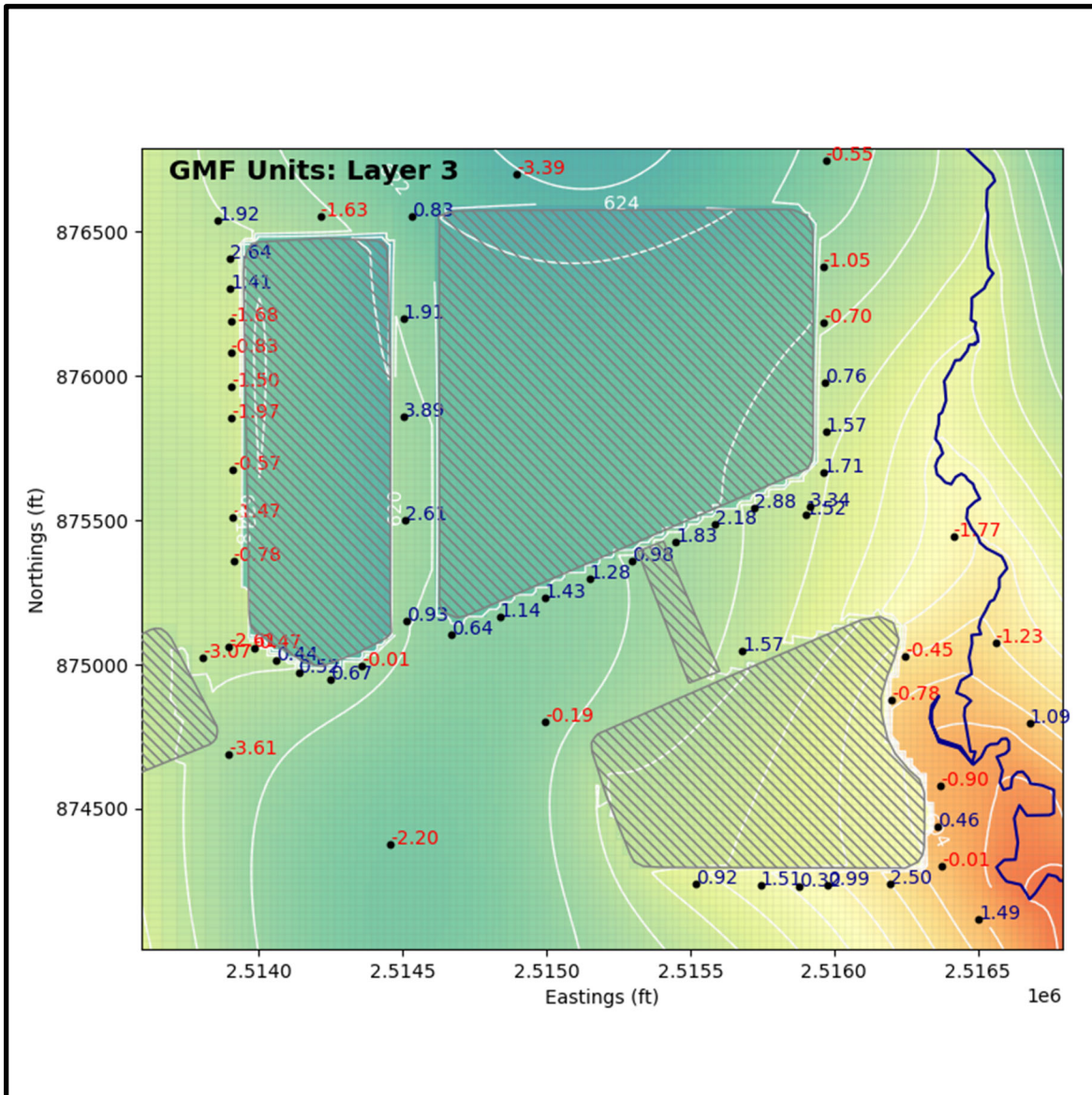




SIMULATED STEADY STATE GROUNDWATER LEVEL CONTOURS FROM UA (LAYER 3) FROM THE CALIBRATED MODEL

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

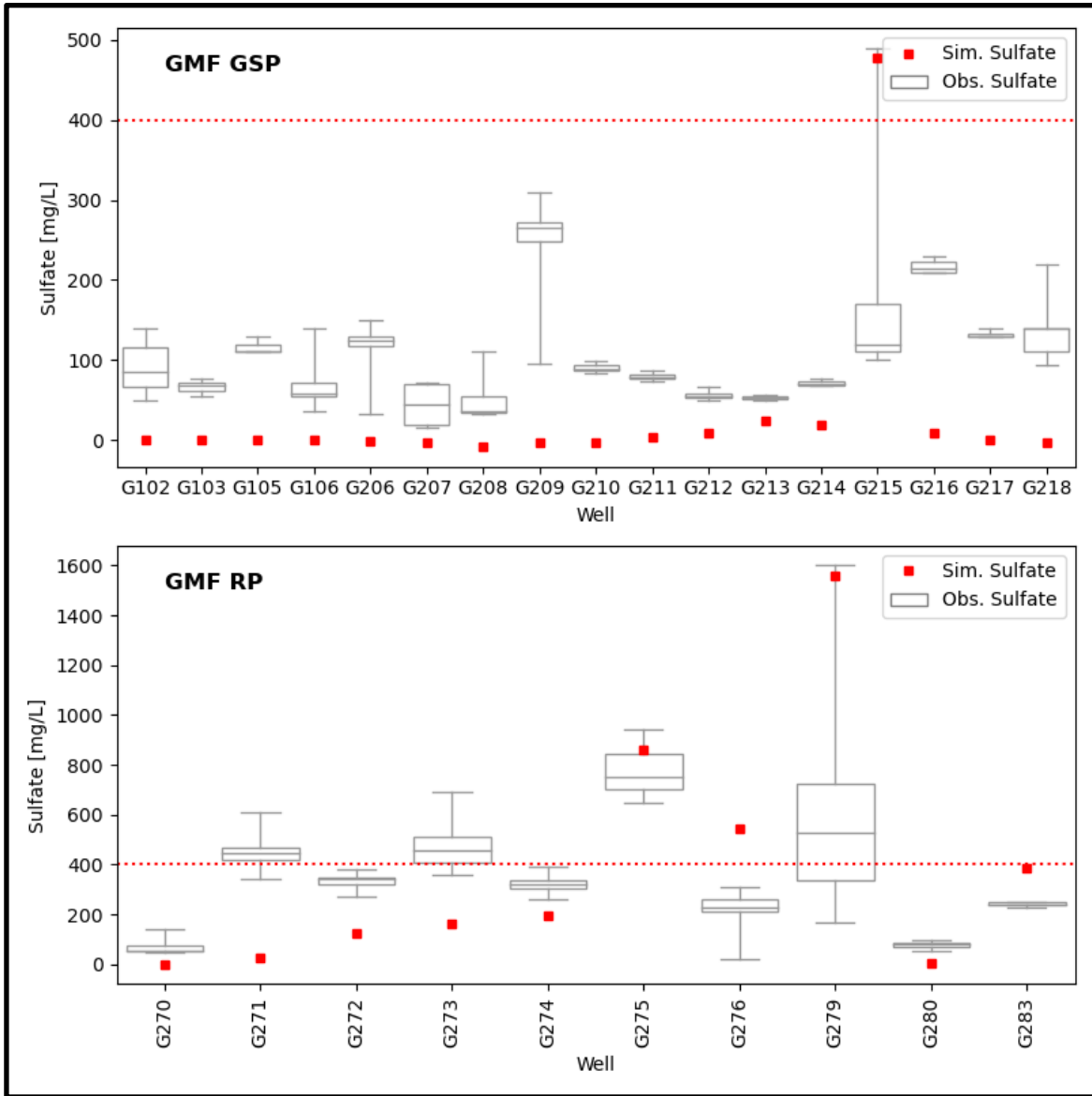




SIMULATED STEADY STATE GROUNDWATER LEVEL CONTOURS IN PROXIMITY TO THE GMF GSP AND GMF RP FROM UA (LAYER 3) FROM THE CALIBRATED MODEL

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

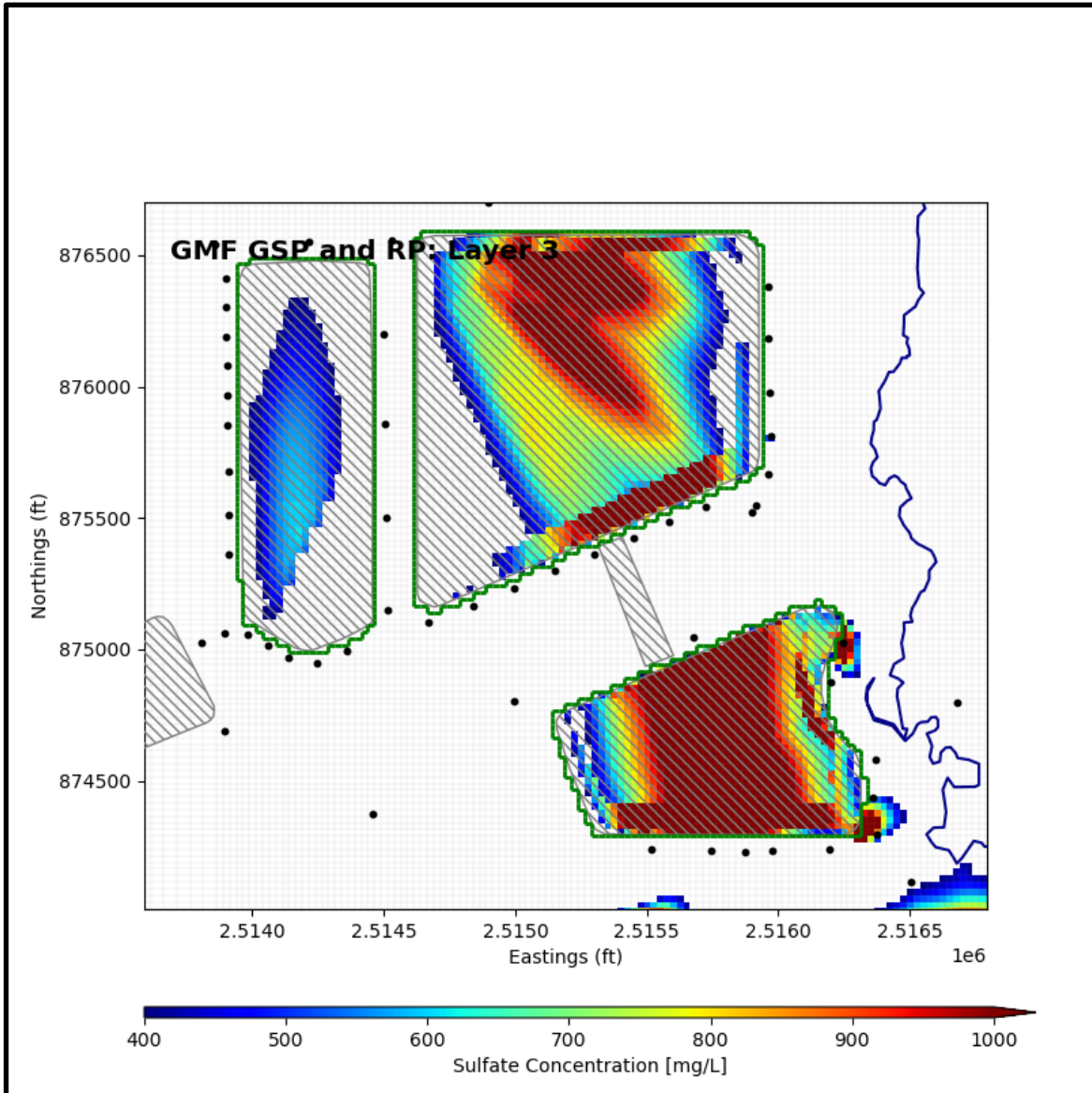




OBSERVED AND SIMULATED SULFATE CONCENTRATIONS FOR THE GMF GSP AND GMF RP [mg/L]

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

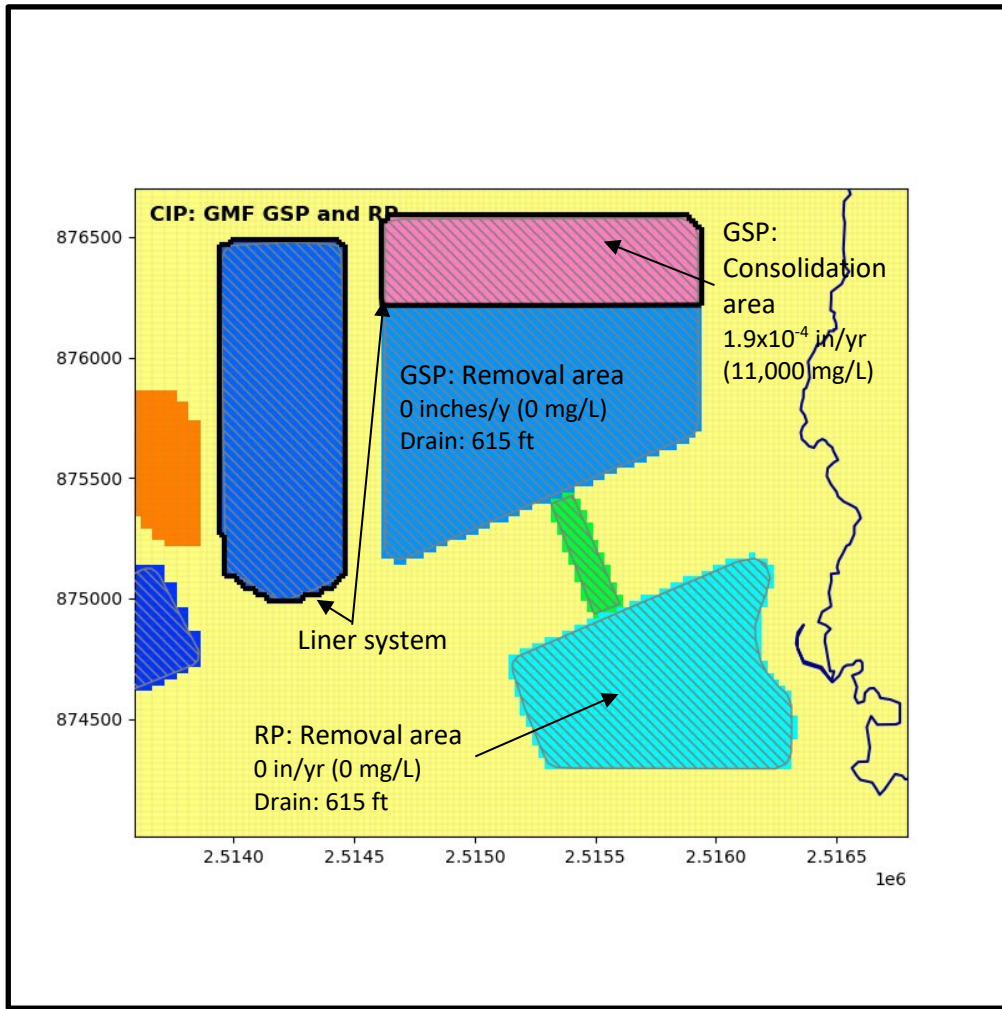




SIMULATED SULFATE PLUME IN THE UA FROM THE TRANSIENT MODEL

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

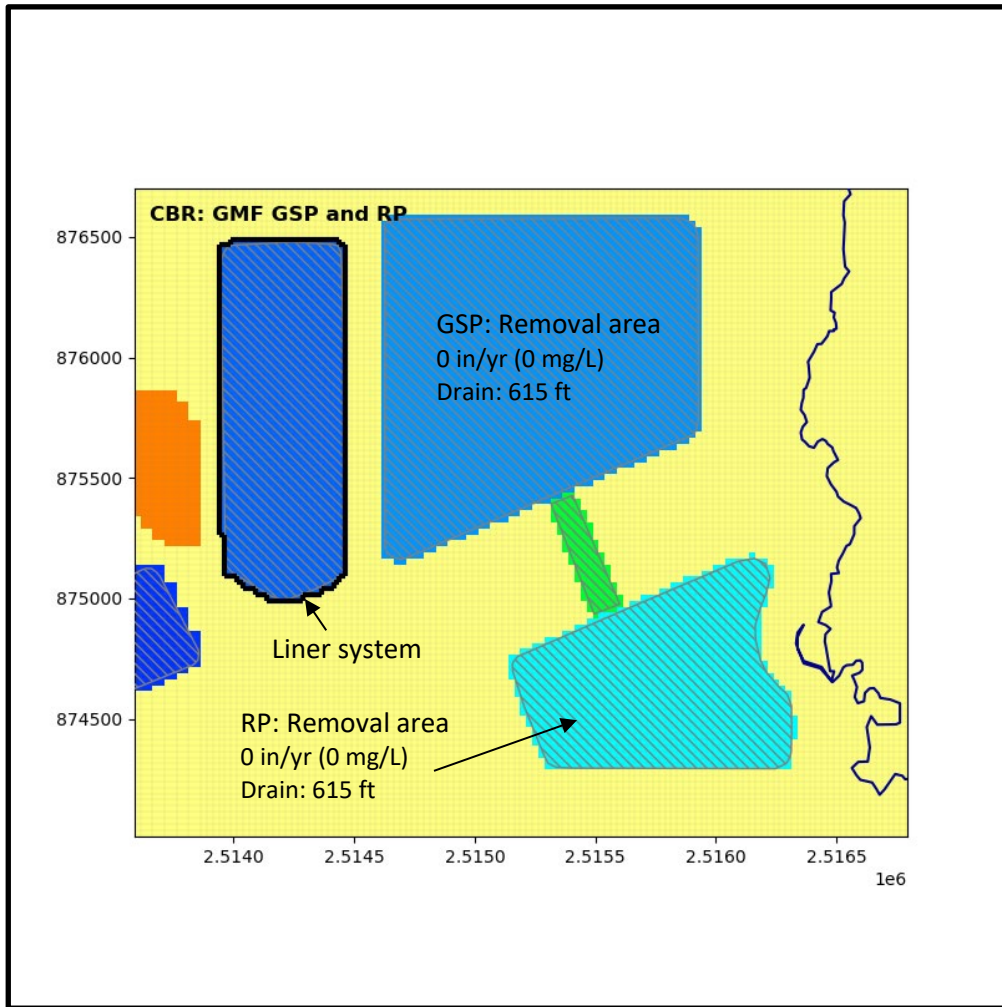




CIP RECHARGE AND STORMWATER POND MODIFICATIONS

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

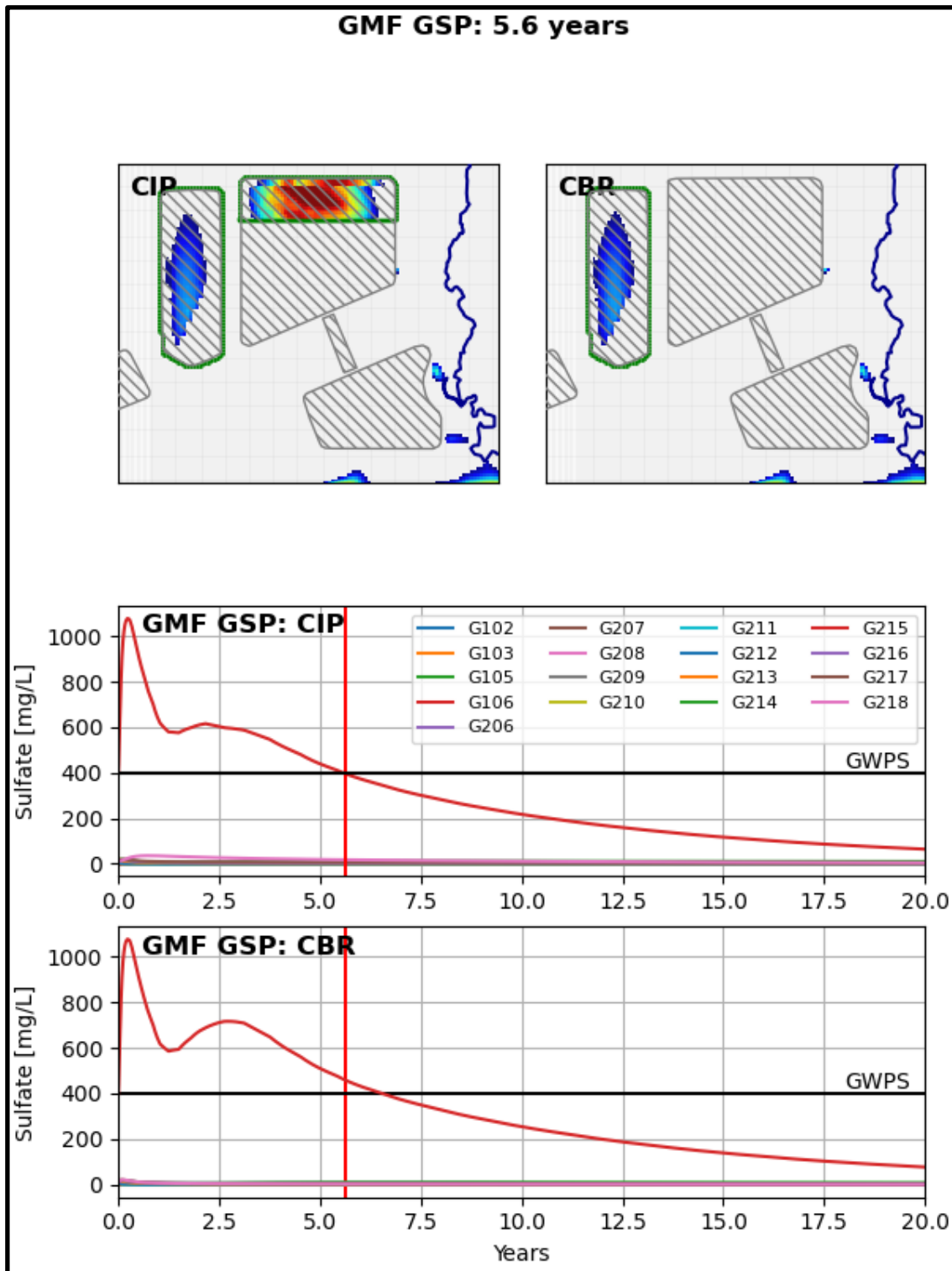




CBR RECHARGE AND STORMWATER POND MODIFICATIONS

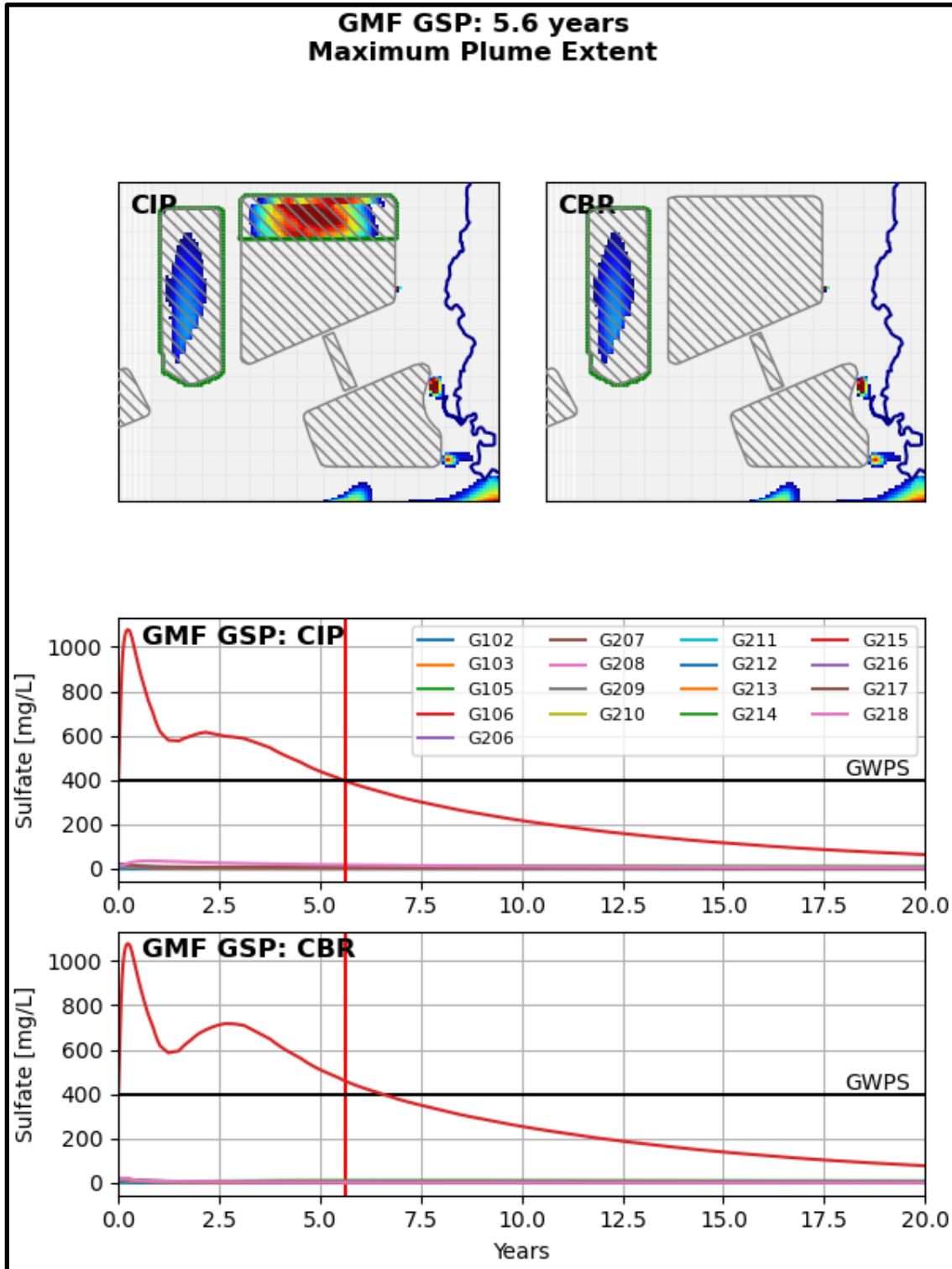
GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS





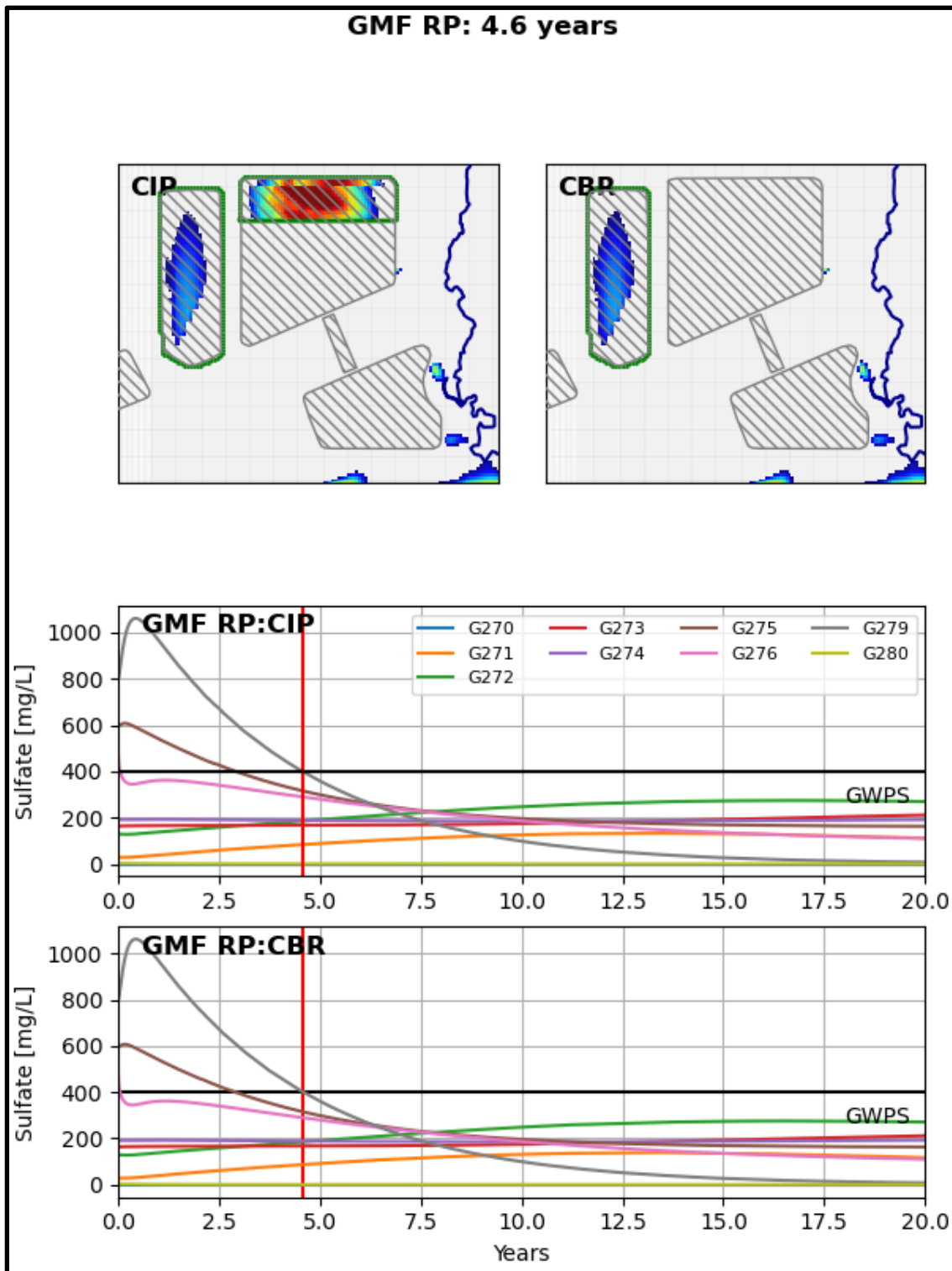
SIMULATED SULFATE PLUME OF THE UA FOR THE CIP AND CBR SCENARIOS AT GMF GSP AFTER 5.6 YEARS

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



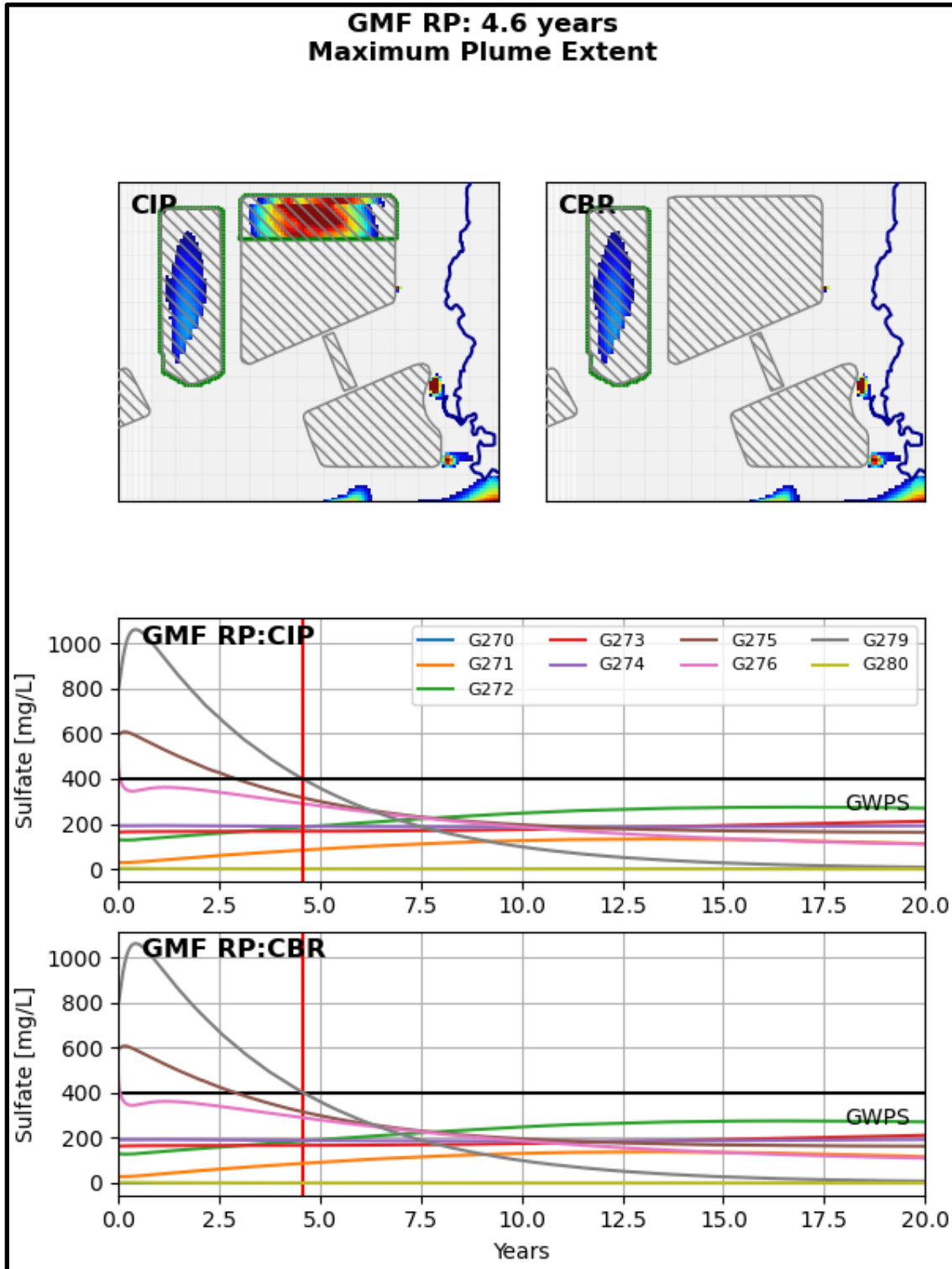
SIMULATED MAXIMUM EXTENT SULFATE PLUME FOR THE CIP AND CBR SCENARIOS AT GMF GSP AFTER 5.6 YEARS

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS



SIMULATED SULFATE PLUME OF THE UA FOR THE CIP AND CBR SCENARIOS AT GMF RP AFTER 4.6 YEARS

GROUNDWATER MODELING REPORT
 GMF GYPSUM STACK POND AND GMF RECYCLE POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS



SIMULATED MAXIMUM EXTENT SULFATE PLUME FOR THE CIP AND CBR SCENARIOS AT GMF RP AFTER 4.6 YEARS

GROUNDWATER MODELING REPORT
GMF GYPSUM STACK POND AND GMF RECYCLE POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

APPENDICES

APPENDIX A
MODFLOW, MT3DMS, and HELP MODEL FILES
(ELECTRONIC ONLY)

**APPENDIX B
EVALUATION OF PARTITION COEFFICIENT RESULTS
(GEOSYNTEC CONSULTANTS, INC., 2022)**

Memorandum

Date: July 5, 2022

To: David Mitchell, Stu Cravens, Vic Modeer
Illinois Power Generating Company

Copies to: Brian Hennings - Ramboll

From: Allison Kreinberg, Ryan Fimmen – Geosyntec Consultants, Inc.

Subject: Evaluation of Partition Coefficient Results – Coffeen GMF Recycle Pond
CCR Unit 104, Coffeen Power Plant, Coffeen, Illinois

INTRODUCTION

The Illinois Power Generation Company (IPGC) currently operates the Coffeen Power Plant (CPP) in Coffeen, Illinois. The coal combustion residuals (CCR) Unit referred to as the Gypsum Management Facility (GMP) Recycle Pond (RP) (Vistra identification [ID] number [No.] 104; Illinois Environmental Protection Agency [IEPA] ID No. W1350150004-04; National Inventory of Dams [NID] No. IL50578) is a 17-acre pond that receives blowdown from the air emission scrubber. The pond was in operation starting in 2010 until April 11, 2021, when IPGC ceased receipt of waste to the GMF RP. Geosyntec Consultants (Geosyntec) is assisting IPGC with Part 845 compliance at the Site.

IPGC is currently preparing a Construction Permit application for the GMF RP as required under Section 845.220. As part of the Construction Permit application, groundwater modeling is being completed for known potential exceedances of groundwater protection standards (GWPS) as outlined in the Operating Permit (Burns & McDonnell, 2021). In the Operating Permit (October 2021), Burns & McDonnell identified potential GWPS exceedances for several compounds potentially associated with the GMF RP, including boron, sulfate, and total dissolved solids (TDS). Batch adsorption testing was conducted for boron and sulfate to generate site-specific partition coefficients. This technical memorandum summarizes the results of the batch adsorption testing and calculation of partition coefficients.

BATCH ATTENUATION TESTING

In 2021 Geosyntec conducted a field investigation at the GMF RP which included completion of two (2) soil/rock borings ranging in depth from 18 to 28 feet below ground surface. As part of that investigation, soil and groundwater samples were submitted to SiREM Laboratories (Guelph, ON) for batch solid/liquid partitioning testing. A summary of the soil samples used for the batch testing is provided in **Table 1**.

One groundwater sample (G215) and one soil sample (SB-215) were used for batch attenuation testing at five (5) soil:solution ratios (**Table 1**), each ran in duplicate. For each treatment, 0.1 L of groundwater was brought into contact with varying amounts of soil (0.004 to 0.2 kg, depending on the ratio) and equilibrated over a seven-day period. One set of microcosms was amended (i.e., spiked) with sodium sulfate (Na_2SO_4) and another with boric acid (H_3BO_3) to achieve target concentrations of sulfate and boron, respectively (**Table 2**).

An initial sample of the stock solution for each experimental design was collected on Day 0, and a control sample (i.e., only amended G215 groundwater with no aquifer solids) was collected on Day 7 after tumbling in polypropylene bottleware to evaluate any loss to interactions with the bottleware or ambient conditions. Duplicates were constructed for each microcosm, including the control samples. After seven days of contact time, an aliquot of the free liquid was collected and filtered through a 0.45 micron (μm) filter prior to analysis for dissolved concentrations of sulfate and/or boron. The oxidation/reduction potential (redox) and pH were measured for each batch test at the beginning and end of the contact period and in the control samples.

Data obtained from the tests (**Tables 3 and 4**) were used to construct isotherms for boron and sulfate; 5-point isotherms were constructed by averaging duplicate results for each soil:solution ratio. Mathematical fitting was used to calculate the partition coefficients (K_d), assuming linear adsorption. The linear adsorption equation was used:

$$q_e = K_d \times C_e \quad \text{Eq. 1}$$

where q_e is the mass of constituent adsorbed to the solid phase at equilibrium, C_e is the remaining aqueous constituent concentration at equilibrium, and K_d is the linear sorption coefficient (reported in liters per kilogram [L/kg]). Some of the data showed a deviation from a linear trend, and so were also fitted using non-linear isotherms. The non-linear Langmuir isotherm was used:

$$q_e = \frac{q_m K_L C_e}{1 + K_L C_e} \quad \text{Eq. 2}$$

where q_m is the inverse of the slope and K_L is the Langmuir partition coefficient. The adsorption data were linearized according to:

$$\frac{C_e}{q_e} = \frac{1}{(K_L \times q_m)} + \frac{C_e}{q_m} \quad \text{Eq. 3}$$

A common non-linear Freundlich equation was also used:

$$q_e = K_F(C_e)^{1/n} \quad \text{Eq. 4}$$

where q_e is the mass of constituent adsorbed to the solid phase at equilibrium, C_e is the remaining aqueous constituent concentration at equilibrium, K_F is the Freundlich partition coefficient, and $1/n$ is a non-linearity constant. The adsorption data were plotted as log-transformed values to perform the non-linear isotherm fitting using the linearized Freundlich equation:

$$\log(q_e) = \log(K_F) + (1/n)\log(C_e) \quad \text{Eq. 5}$$

The calculated linear, Langmuir, and Freundlich partition coefficients (K_d , K_L , and K_F , respectively) and $1/n$ values are shown in **Table 5**.

SUMMARY OF RESULTS

The partition coefficient values for each amendment (denoted as G215-SO₄ when amended with sodium sulfate and G215-B when amended with boric acid) are presented in **Table 5**. Figures which show the linear, Langmuir, and Freundlich isotherms for boron and sulfate are provided in **Appendix A**. Measurements of soil boron concentrations in SB-215 are pending; a surrogate value of 0 mg/kg was used, consistent with soil boron concentrations from other areas at the CPP.

A boron partition coefficient was not determined for any isotherm for the boron amended microcosms. Both the linear and linearized Langmuir isotherms yielded negative partition coefficients, and the linearized Freundlich could not be calculated as the data were not conducive to log transformation. Other studies have reported low partition coefficients for boron ranging from 0.19 to 1.3 L/kg, depending on pH conditions and the amount of sorbent present (EPRI, 2005; Strenge & Peterson, 1989).

A sulfate partition coefficient was not determined for any isotherm for the sulfate amended microcosms. The linear isotherm yielded a partition coefficient of 0.1 L/kg but had a very poor goodness-of-fit, and the Langmuir isotherm yielded a negative coefficient. As in the boron-amended microcosms, the Freundlich isotherm could not be calculated because the data were not conducive to log transformation. These results are consistent with the findings of Strenge and Peterson (1989), who found that partition coefficients for sulfate are 0.0 L/kg, regardless of pH conditions and the amount of sorbent present.

REFERENCES

EPRI, 2005. *Chemical constituents in coal combustion product leachate: boron. Final Report 1005258.*

Burns & McDonnell. 2021. Initial Operating Permit Coffeen GMF Recycle Pond. October

Streng, D. and Peterson, S. 1989. Chemical Data Bases for the Multimedia Environmental Pollutant Assessment System (MEPAS) (No. PNL-7145). Pacific Northwest National Laboratory, Richland, WA (USA).

TABLES

Table 1 - Batch Attenuation Testing Data Summary *Geosyntec Consultants*
 Coffeen GMF RP

Groundwater Sample ID	Soil Sample ID	Soil: Water Ratio
G215	SB-215 (19-24.5 ft bgs) Sodium Sulfate Amendment	2:1.5
		1:1.3
		1:5.8
		1:11.5
		1:27.2
G215	SB-215 (19-24.5 ft bgs) Boric Acid Amendment	2:1.5
		1:1.3
		1:5.8
		1:11.5
		1:28.1

Notes:

ft bgs = feet below ground surface

Table 2 - Microcosm Amendment and Target Concentrations
Coffeen GMF RP

Groundwater Sample ID	Soil Sample ID	Compound	Amendment	Target Concentration (mg/L)
G215	SB-215 (19-24.5 ft bgs)	Boron	31.93 mL of a 2 g/L H ₃ BO ₃	6
		Sulfate	3.41 g of Na ₂ SO ₄	1500

Notes:

ft bgs - feet below ground surface

mg/L - milligrams per liter

Na₂SO₄ - sodium sulfate

H₃BO₃ - boric acid

Table 3 - Batch Attenuation Testing Results
 Coffeen GMF RP - Sodium Sulfate Amendment

Groundwater Sample ID	Geologic Material Sample ID	Treatment	Date	Day	Replicate	Dissolved Sulfate	pH	ORP	
						mg/L	SU	mV	
G215	--	Groundwater Only Control	25-Jan-22	0	G215-1a (SO ₄ ²⁻)	1,589	6.98	83	
					G215-2a (SO ₄ ²⁻)	1,826	6.99	79	
			Average Concentration (mg/L)		1,708	6.99	81		
			7-Feb-22	7	G215-1 (SO ₄ ²⁻)	1,617	6.8	26	
	G215-2 (SO ₄ ²⁻)	1,478			6.81	13			
	Average Concentration (mg/L)		1,548	6.81	20				
	G215 SB-215 Geologic Material	2:1 Soil:Water Ratio	31-Jan-22	0					
			7-Feb-22	7	SB-215-(19-24.5):G215 2:1-1 (SO ₄ ²⁻)	1,321	6.92	57	
					SB-215-(19-24.5):G215 2:1-2 (SO ₄ ²⁻)	1,302	6.94	103	
			Average Concentration (mg/L)		1,311	6.93	80		
			31-Jan-22	0					
		7-Feb-22	7	SB-215-(19-24.5):G215 1:1-1 (SO ₄ ²⁻)	1,727	6.89	85		
				SB-215-(19-24.5):G215 1:1-2 (SO ₄ ²⁻)	860	6.91	91		
		Average Concentration (mg/L)		1,294	6.90	88			
		31-Jan-22	0						
	7-Feb-22	7	SB-215-(19-24.5):G215 1:5-1 (SO ₄ ²⁻)	1,326	6.92	29			
			SB-215-(19-24.5):G215 1:5-2 (SO ₄ ²⁻)	1,516	6.87	15			
Average Concentration (mg/L)		1,421	6.90	22					
31-Jan-22	0								
7-Feb-22	7	SB-215-(19-24.5):G215 1:10-1 (SO ₄ ²⁻)	1,570	6.87	23				
		SB-215-(19-24.5):G215 1:10-2 (SO ₄ ²⁻)	1,551	6.85	30				
Average Concentration (mg/L)		1,560	6.86	27					
31-Jan-22	0								
7-Feb-22	7	SB-215-(19-24.5):G215 1:20-1 (SO ₄ ²⁻)	1,511	6.83	32				
		SB-215-(19-24.5):G215 1:20-2 (SO ₄ ²⁻)	1,588	6.84	79				
Average Concentration (mg/L)		1,550	6.84	56					

Notes:

mg/L - milligrams per liter

mV - millivolts

SU - Standard Units

ORP - oxidation/reduction potential

Table 4 - Batch Attenuation Testing Results
Coffeen GMF RP - Boric Acid Amendment

Groundwater Sample ID	Geologic Material Sample ID	Treatment	Date	Day	Replicate	Dissolved Boron	pH	ORP		
						mg/L	SU	mV		
G215	--	Groundwater Only Control	25-Jan-22	0	G215-1a (B)	4.6	6.88	90		
					G215-2a (B)	4.7	6.85	72		
					Average Concentration (mg/L)	4.7	6.87	81		
			7-Feb-22	7	G215-1 (B)	5.3	6.9	57		
					G215-2 (B)	5.4	7.03	13		
					Average Concentration (mg/L)	5.4	6.97	35		
	G215 SB-215 Geologic Material	2:1 Soil:Water Ratio	31-Jan-22	0						
					7-Feb-22	7	SB-215-(19-24.5) :G215 2:1-1 (B)	3.4	6.91	9
							SB-215-(19-24.5) :G215 2:1-2 (B)	3.4	7.05	11
			Average Concentration (mg/L)	3.4	6.98	10				
			31-Jan-22	0						
					7-Feb-22	7	SB-215-(19-24.5) :G215 1:1-1 (B)	4.3	6.98	15
							SB-215-(19-24.5) :G215 1:1-2 (B)	4.3	7.06	31
			Average Concentration (mg/L)	4.3	7.02	23				
			31-Jan-22	0						
					7-Feb-22	7	SB-215-(19-24.5) :G215 1:5-1 (B)	5.0	6.96	49
							SB-215-(19-24.5) :G215 1:5-2 (B)	5.2	7.00	19
			Average Concentration (mg/L)	5.1	6.98	34				
			31-Jan-22	0						
					7-Feb-22	7	SB-215-(19-24.5) :G215 1:10-1 (B)	5.5	6.95	20
SB-215-(19-24.5) :G215 1:10-2 (B)							5.3	6.95	29	
Average Concentration (mg/L)			5.4	6.95	25					
31-Jan-22			0							
				7-Feb-22	7	SB-215-(19-24.5) :G215 1:20-1 (B)	5.6	6.93	174	
	SB-215-(19-24.5) :G215 1:20-2 (B)	5.5				6.84	102			
Average Concentration (mg/L)	5.5	6.89	138							

Notes:

mg/L - milligrams per liter

mV - millivolts

SU - Standard Units

ORP - oxidation/reduction potential

Table 5 - Partition Coefficient Results
Coffeen GMF RP

Analyte	Amendment	Isotherm	Variable	Value
Boron	Boric Acid	Linear	R^2	0.518
			K_D (L/kg)	-8.45
		Langmuir	R^2	0.47
			q_m (mg/g)	0.000
			K_L (L/kg)	-1.87E+05
		Freundlich	R^2	--
			1/n	--
			K_F (L/kg)	--
		Sulfate	Sodium Sulfate	Linear
K_D (L/kg)	0.10			
Langmuir	R^2			0.66
	q_m (mg/g)			-0.028
	K_L (L/kg)			-8.94E+02
Freundlich	R^2			--
	1/n			--
	K_F (L/kg)			--

Notes:

The Freundlich isotherm was not calculated for boron or sulfate
because the data were not conducive to log transformation

K_D - linear partition coefficient

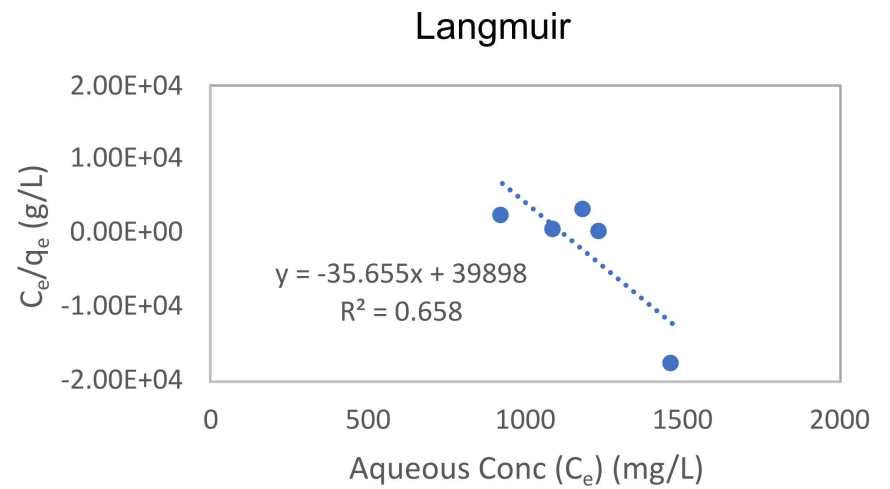
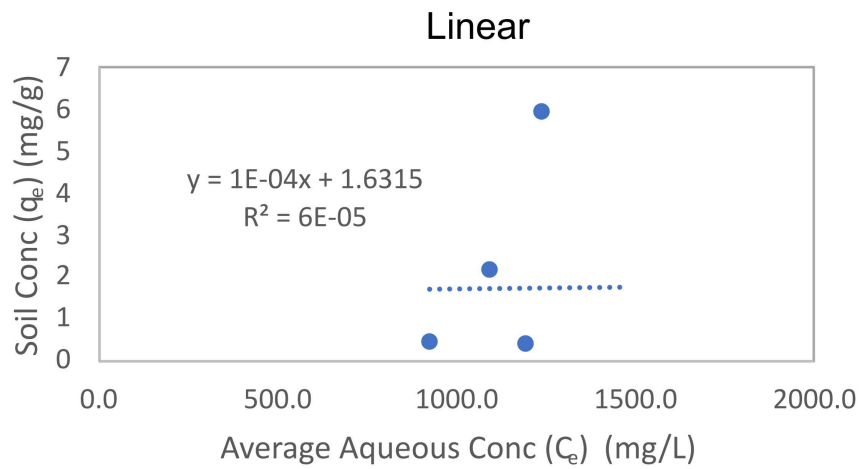
K_L - Langmuir partition coefficient

K_F - Freundlich partition coefficient

q_m - inverse of the slope of the linearized Langmuir isotherm

n - non-linearity constant of the Freundlich isotherm

APPENDIX A
BATCH TESTING ISOTHERM PLOTS



Notes:

The Freundlich isotherm was not calculated because the data were not conducive to log transformation.

q_e - mass of constituent adsorbed to the solid phase
 C_e - remaining aqueous constituent concentration
 mg/L - milligrams per liter
 mg/g - milligrams per gram
 g/L - grams per liter

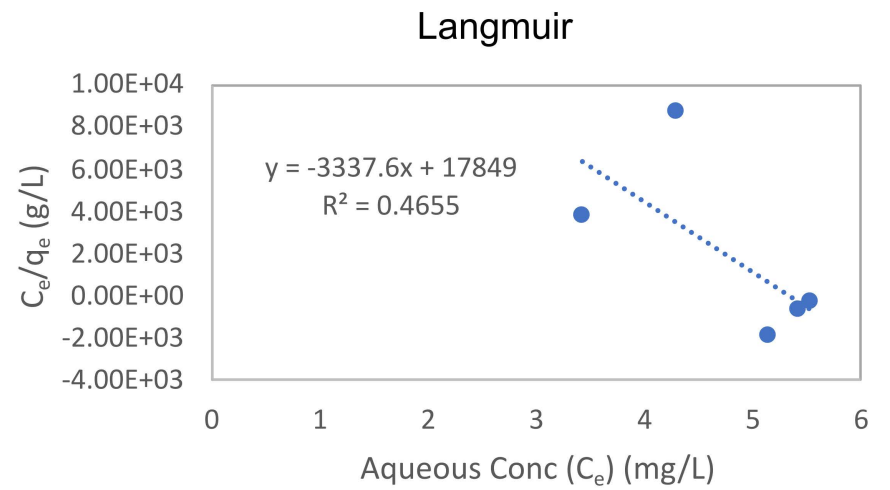
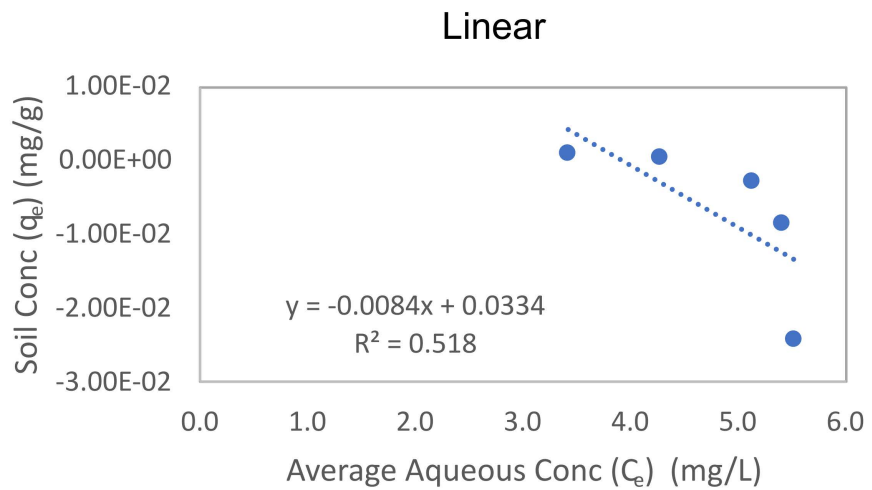
G215-SO₄ Sulfate Partitioning Coefficients
 Coffeen Power Plant GMF RP
 Coffeen, Illinois

Geosyntec
 consultants

Columbus, OH

May 2022

Figure
1



Notes:

The Freundlich isotherm was not calculated because the data were not conducive to log transformation.

- q_e - mass of constituent adsorbed to the solid phase
- C_e - remaining aqueous constituent concentration
- mg/L - milligrams per liter
- mg/g - milligrams per gram
- g/L - grams per liter

G215-B Boron Partitioning Coefficients
 Coffeen Power Plant GMF RP
 Coffeen, Illinois



Columbus, OH

May 2022

Figure
2

APPENDIX C

HELP MODEL OUTPUT FILES

HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
HELP MODEL VERSION 4.0 BETA (2018)
DEVELOPED BY USEPA NATIONAL RISK MANAGEMENT RESEARCH LABORATORY

Title: Cof GMF - CIP Cons **Simulated On:** 7/6/2022 11:42

Layer 1

Type 1 - Vertical Percolation Layer (Cover Soil)

SiCL - Silty Clay Loam

Material Texture Number 12

Thickness	=	6 inches
Porosity	=	0.471 vol/vol
Field Capacity	=	0.342 vol/vol
Wilting Point	=	0.21 vol/vol
Initial Soil Water Content	=	0.2608 vol/vol
Effective Sat. Hyd. Conductivity	=	4.20E-05 cm/sec

Layer 2

Type 1 - Vertical Percolation Layer

SiC - Silty Clay

Material Texture Number 14

Thickness	=	18 inches
Porosity	=	0.479 vol/vol
Field Capacity	=	0.371 vol/vol
Wilting Point	=	0.251 vol/vol
Initial Soil Water Content	=	0.3538 vol/vol
Effective Sat. Hyd. Conductivity	=	2.50E-05 cm/sec

Layer 3

Type 2 - Lateral Drainage Layer

Drainage Net (0.5 cm)

Material Texture Number 20

Thickness	=	0.2 inches
Porosity	=	0.85 vol/vol
Field Capacity	=	0.01 vol/vol
Wilting Point	=	0.005 vol/vol
Initial Soil Water Content	=	0.01 vol/vol
Effective Sat. Hyd. Conductivity	=	1.00E+01 cm/sec
Slope	=	4 %
Drainage Length	=	650 ft

Layer 4

Type 4 - Flexible Membrane Liner
LDPE Membrane

Material Texture Number 36

Thickness	=	0.04 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 5

Type 1 - Vertical Percolation Layer (Waste)

Gypsum waste material (Sandy Loam)

Material Texture Number 83

Thickness	=	144 inches
Porosity	=	0.437 vol/vol
Field Capacity	=	0.105 vol/vol
Wilting Point	=	0.047 vol/vol
Initial Soil Water Content	=	0.105 vol/vol
Effective Sat. Hyd. Conductivity	=	6.70E-04 cm/sec

Layer 6

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.06 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 7

Type 3 - Barrier Soil Liner

C (Moderate)

Material Texture Number 29

Thickness	=	36 inches
Porosity	=	0.451 vol/vol
Field Capacity	=	0.419 vol/vol
Wilting Point	=	0.332 vol/vol
Initial Soil Water Content	=	0.451 vol/vol
Effective Sat. Hyd. Conductivity	=	6.80E-07 cm/sec

Note: Initial moisture content of the layers and snow water were computed as nearly steady-state values by HELP.

General Design and Evaporative Zone Data

SCS Runoff Curve Number	=	87.5
Fraction of Area Allowing Runoff	=	100 %
Area projected on a horizontal plane	=	12.4 acres
Evaporative Zone Depth	=	18 inches
Initial Water in Evaporative Zone	=	5.708 inches
Upper Limit of Evaporative Storage	=	8.574 inches
Lower Limit of Evaporative Storage	=	4.272 inches
Initial Snow Water	=	0.088112 inches
Initial Water in Layer Materials	=	39.293 inches
Total Initial Water	=	39.381 inches
Total Subsurface Inflow	=	0 inches/year

 Note: SCS Runoff Curve Number was calculated by HELP.

Evapotranspiration and Weather Data

Station Latitude	=	39.07 Degrees
Maximum Leaf Area Index	=	4.5
Start of Growing Season (Julian Date)	=	97 days
End of Growing Season (Julian Date)	=	302 days
Average Wind Speed	=	8 mph
Average 1st Quarter Relative Humidity	=	72 %
Average 2nd Quarter Relative Humidity	=	64 %
Average 3rd Quarter Relative Humidity	=	71 %
Average 4th Quarter Relative Humidity	=	72 %

 Note: Evapotranspiration data was obtained for Coffeen, Illinois

Normal Mean Monthly Precipitation (inches)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
2.353618	2.511085	2.81508	3.241374	3.956664	4.312863
4.375035	2.656228	3.284204	3.675466	3.677412	3.106835

 Note: Precipitation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Normal Mean Monthly Temperature (Degrees Fahrenheit)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
37.3	37.1	50	61.9	69.7	80.6
84.2	81	72.2	62.4	48.1	38

Note: Temperature was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4
 Solar radiation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Average Annual Totals Summary

Title: Cof GMF - CIP Cons
Simulated on: 7/6/2022 11:43

	Average Annual Totals for Years 1 - 30*			
	(inches)	[std dev]	(cubic feet)	(percent)
Precipitation	39.97	[4.83]	1,798,943.4	100.00
Runoff	3.963	[2.529]	178,369.0	9.92
Evapotranspiration	29.888	[3.283]	1,345,302.1	74.78
Subprofile1				
Lateral drainage collected from Layer 3	6.1459	[2.4217]	276,641.4	15.38
Percolation/leakage through Layer 4	0.002554	[0.001385]	115.0	0.01
Average Head on Top of Layer 4	0.0155	[0.0128]	---	---
Subprofile2				
Percolation/leakage through Layer 7	0.000172	[0.000108]	7.7305	0.00
Average Head on Top of Layer 6	0.0947	[0.0648]	---	---
Water storage				
Change in water storage	-0.0306	[1.2297]	-1,376.8	-0.08

* Note: Average inches are converted to volume based on the user-specified area.

HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
HELP MODEL VERSION 4.0 BETA (2018)
DEVELOPED BY USEPA NATIONAL RISK MANAGEMENT RESEARCH LABORATORY

Title: Cof LF **Simulated On:** 7/6/2022 12:31

Layer 1

Type 1 - Vertical Percolation Layer (Cover Soil)

SiCL - Silty Clay Loam

Material Texture Number 12

Thickness	=	6 inches
Porosity	=	0.471 vol/vol
Field Capacity	=	0.342 vol/vol
Wilting Point	=	0.21 vol/vol
Initial Soil Water Content	=	0.2594 vol/vol
Effective Sat. Hyd. Conductivity	=	4.20E-05 cm/sec

Layer 2

Type 1 - Vertical Percolation Layer

SiC - Silty Clay

Material Texture Number 14

Thickness	=	18 inches
Porosity	=	0.479 vol/vol
Field Capacity	=	0.371 vol/vol
Wilting Point	=	0.251 vol/vol
Initial Soil Water Content	=	0.3541 vol/vol
Effective Sat. Hyd. Conductivity	=	2.50E-05 cm/sec

Layer 3

Type 2 - Lateral Drainage Layer

Drainage Net (0.5 cm)

Material Texture Number 20

Thickness	=	0.2 inches
Porosity	=	0.85 vol/vol
Field Capacity	=	0.01 vol/vol
Wilting Point	=	0.005 vol/vol
Initial Soil Water Content	=	0.01 vol/vol
Effective Sat. Hyd. Conductivity	=	1.00E+01 cm/sec
Slope	=	25 %
Drainage Length	=	250 ft

Layer 4

Type 4 - Flexible Membrane Liner
LDPE Membrane

Material Texture Number 36

Thickness	=	0.04 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 5

Type 1 - Vertical Percolation Layer (Waste)

Landfill CCR Material

Material Texture Number 83

Thickness	=	720 inches
Porosity	=	0.437 vol/vol
Field Capacity	=	0.105 vol/vol
Wilting Point	=	0.047 vol/vol
Initial Soil Water Content	=	0.105 vol/vol
Effective Sat. Hyd. Conductivity	=	2.69E-04 cm/sec

Layer 6

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.06 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 7

Type 3 - Barrier Soil Liner

C (Moderate)

Material Texture Number 29

Thickness	=	36 inches
Porosity	=	0.451 vol/vol
Field Capacity	=	0.419 vol/vol
Wilting Point	=	0.332 vol/vol
Initial Soil Water Content	=	0.451 vol/vol
Effective Sat. Hyd. Conductivity	=	6.80E-07 cm/sec

Note: Initial moisture content of the layers and snow water were computed as nearly steady-state values by HELP.

General Design and Evaporative Zone Data

SCS Runoff Curve Number	=	88.6
Fraction of Area Allowing Runoff	=	100 %
Area projected on a horizontal plane	=	13.5 acres
Evaporative Zone Depth	=	18 inches
Initial Water in Evaporative Zone	=	5.704 inches
Upper Limit of Evaporative Storage	=	8.574 inches
Lower Limit of Evaporative Storage	=	4.272 inches
Initial Snow Water	=	0.088112 inches
Initial Water in Layer Materials	=	99.768 inches
Total Initial Water	=	99.857 inches
Total Subsurface Inflow	=	0 inches/year

 Note: SCS Runoff Curve Number was calculated by HELP.

Evapotranspiration and Weather Data

Station Latitude	=	39.07 Degrees
Maximum Leaf Area Index	=	4.5
Start of Growing Season (Julian Date)	=	97 days
End of Growing Season (Julian Date)	=	302 days
Average Wind Speed	=	8 mph
Average 1st Quarter Relative Humidity	=	72 %
Average 2nd Quarter Relative Humidity	=	64 %
Average 3rd Quarter Relative Humidity	=	71 %
Average 4th Quarter Relative Humidity	=	72 %

 Note: Evapotranspiration data was obtained for Coffeen, Illinois

Normal Mean Monthly Precipitation (inches)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
2.353618	2.511085	2.81508	3.241374	3.956664	4.312863
4.375035	2.656228	3.284204	3.675466	3.677412	3.106835

 Note: Precipitation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Normal Mean Monthly Temperature (Degrees Fahrenheit)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
37.3	37.1	50	61.9	69.7	80.6
84.2	81	72.2	62.4	48.1	38

Note: Temperature was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4
 Solar radiation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Average Annual Totals Summary

Title: Cof LF
Simulated on: 7/6/2022 12:32

	Average Annual Totals for Years 1 - 30*			
	(inches)	[std dev]	(cubic feet)	(percent)
Precipitation	39.97	[4.83]	1,958,527.1	100.00
Runoff	4.515	[2.615]	221,272.7	11.30
Evapotranspiration	29.778	[3.239]	1,459,247.7	74.51
Subprofile1				
Lateral drainage collected from Layer 3	5.7043	[2.3371]	279,540.0	14.27
Percolation/leakage through Layer 4	0.000094	[0.000034]	4.6022	0.00
Average Head on Top of Layer 4	0.0004	[0.0002]	---	---
Subprofile2				
Percolation/leakage through Layer 7	0.000014	[0.000005]	0.6704	0.00
Average Head on Top of Layer 6	0.0036	[0.0022]	---	---
Water storage				
Change in water storage	-0.0313	[1.2433]	-1,533.8	-0.08

* Note: Average inches are converted to volume based on the user-specified area.

HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
HELP MODEL VERSION 4.0 BETA (2018)
DEVELOPED BY USEPA NATIONAL RISK MANAGEMENT RESEARCH LABORATORY

Title: Cof GMF - Default **Simulated On:** 7/6/2022 13:25

Layer 1

Type 1 - Vertical Percolation Layer (Cover Soil)

SiCL - Silty Clay Loam

Material Texture Number 12

Thickness	=	6 inches
Porosity	=	0.471 vol/vol
Field Capacity	=	0.342 vol/vol
Wilting Point	=	0.21 vol/vol
Initial Soil Water Content	=	0.2608 vol/vol
Effective Sat. Hyd. Conductivity	=	4.20E-05 cm/sec

Layer 2

Type 1 - Vertical Percolation Layer

SiC - Silty Clay

Material Texture Number 14

Thickness	=	30 inches
Porosity	=	0.479 vol/vol
Field Capacity	=	0.371 vol/vol
Wilting Point	=	0.251 vol/vol
Initial Soil Water Content	=	0.3648 vol/vol
Effective Sat. Hyd. Conductivity	=	2.50E-05 cm/sec

Layer 3

Type 2 - Lateral Drainage Layer

Drainage Net (0.5 cm)

Material Texture Number 20

Thickness	=	0.2 inches
Porosity	=	0.85 vol/vol
Field Capacity	=	0.01 vol/vol
Wilting Point	=	0.005 vol/vol
Initial Soil Water Content	=	0.0229 vol/vol
Effective Sat. Hyd. Conductivity	=	1.00E+01 cm/sec
Slope	=	4 %
Drainage Length	=	650 ft

Layer 4

Type 4 - Flexible Membrane Liner
LDPE Membrane

Material Texture Number 36

Thickness	=	0.04 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 5

Type 1 - Vertical Percolation Layer (Waste)

Gypsum waste material (Sandy Loam)

Material Texture Number 83

Thickness	=	144 inches
Porosity	=	0.437 vol/vol
Field Capacity	=	0.105 vol/vol
Wilting Point	=	0.047 vol/vol
Initial Soil Water Content	=	0.105 vol/vol
Effective Sat. Hyd. Conductivity	=	6.70E-04 cm/sec

Layer 6

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.06 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 7

Type 3 - Barrier Soil Liner

C (Moderate)

Material Texture Number 29

Thickness	=	36 inches
Porosity	=	0.451 vol/vol
Field Capacity	=	0.419 vol/vol
Wilting Point	=	0.332 vol/vol
Initial Soil Water Content	=	0.451 vol/vol
Effective Sat. Hyd. Conductivity	=	6.80E-07 cm/sec

Note: Initial moisture content of the layers and snow water were computed as nearly steady-state values by HELP.

General Design and Evaporative Zone Data

SCS Runoff Curve Number	=	87.5
Fraction of Area Allowing Runoff	=	100 %
Area projected on a horizontal plane	=	12.4 acres
Evaporative Zone Depth	=	18 inches
Initial Water in Evaporative Zone	=	5.708 inches
Upper Limit of Evaporative Storage	=	8.574 inches
Lower Limit of Evaporative Storage	=	4.272 inches
Initial Snow Water	=	0.088112 inches
Initial Water in Layer Materials	=	43.871 inches
Total Initial Water	=	43.959 inches
Total Subsurface Inflow	=	0 inches/year

 Note: SCS Runoff Curve Number was calculated by HELP.

Evapotranspiration and Weather Data

Station Latitude	=	39.07 Degrees
Maximum Leaf Area Index	=	4.5
Start of Growing Season (Julian Date)	=	97 days
End of Growing Season (Julian Date)	=	302 days
Average Wind Speed	=	8 mph
Average 1st Quarter Relative Humidity	=	72 %
Average 2nd Quarter Relative Humidity	=	64 %
Average 3rd Quarter Relative Humidity	=	71 %
Average 4th Quarter Relative Humidity	=	72 %

 Note: Evapotranspiration data was obtained for Coffeen, Illinois

Normal Mean Monthly Precipitation (inches)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
2.353618	2.511085	2.81508	3.241374	3.956664	4.312863
4.375035	2.656228	3.284204	3.675466	3.677412	3.106835

 Note: Precipitation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Normal Mean Monthly Temperature (Degrees Fahrenheit)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
37.3	37.1	50	61.9	69.7	80.6
84.2	81	72.2	62.4	48.1	38

Note: Temperature was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4
 Solar radiation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Average Annual Totals Summary

Title: Cof GMF - Default
Simulated on: 7/6/2022 13:27

	Average Annual Totals for Years 1 - 30*			
	(inches)	[std dev]	(cubic feet)	(percent)
Precipitation	39.97	[4.83]	1,798,943.4	100.00
Runoff	3.962	[2.529]	178,352.1	9.91
Evapotranspiration	29.888	[3.285]	1,345,324.5	74.78
Subprofile1				
Lateral drainage collected from Layer 3	6.1507	[2.452]	276,855.0	15.39
Percolation/leakage through Layer 4	0.001913	[0.000901]	86.1	0.00
Average Head on Top of Layer 4	0.0068	[0.0082]	---	---
Subprofile2				
Percolation/leakage through Layer 7	0.000141	[0.000083]	6.3679	0.00
Average Head on Top of Layer 6	0.0760	[0.0487]	---	---
Water storage				
Change in water storage	-0.0354	[1.3493]	-1,594.5	-0.09

* Note: Average inches are converted to volume based on the user-specified area.

Material Texture Number 83

Thickness	=	144 inches
Porosity	=	0.437 vol/vol
Field Capacity	=	0.105 vol/vol
Wilting Point	=	0.047 vol/vol
Initial Soil Water Content	=	0.1159 vol/vol
Effective Sat. Hyd. Conductivity	=	6.70E-04 cm/sec

Layer 5

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.06 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 6

Type 3 - Barrier Soil Liner

C (Moderate)

Material Texture Number 29

Thickness	=	36 inches
Porosity	=	0.451 vol/vol
Field Capacity	=	0.419 vol/vol
Wilting Point	=	0.332 vol/vol
Initial Soil Water Content	=	0.451 vol/vol
Effective Sat. Hyd. Conductivity	=	6.80E-07 cm/sec

Note: Initial moisture content of the layers and snow water were computed as nearly steady-state values by HELP.

General Design and Evaporative Zone Data

SCS Runoff Curve Number	=	87.5
Fraction of Area Allowing Runoff	=	100 %
Area projected on a horizontal plane	=	12.4 acres
Evaporative Zone Depth	=	18 inches
Initial Water in Evaporative Zone	=	5.667 inches
Upper Limit of Evaporative Storage	=	8.574 inches
Lower Limit of Evaporative Storage	=	4.272 inches
Initial Snow Water	=	0.088112 inches
Initial Water in Layer Materials	=	62.588 inches
Total Initial Water	=	62.676 inches

Total Subsurface Inflow = 0 inches/year

Note: SCS Runoff Curve Number was calculated by HELP.

Evapotranspiration and Weather Data

Station Latitude = 39.07 Degrees
Maximum Leaf Area Index = 4.5
Start of Growing Season (Julian Date) = 97 days
End of Growing Season (Julian Date) = 302 days
Average Wind Speed = 8 mph
Average 1st Quarter Relative Humidity = 72 %
Average 2nd Quarter Relative Humidity = 64 %
Average 3rd Quarter Relative Humidity = 71 %
Average 4th Quarter Relative Humidity = 72 %

Note: Evapotranspiration data was obtained for Coffeen, Illinois

Normal Mean Monthly Precipitation (inches)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
2.353618	2.511085	2.81508	3.241374	3.956664	4.312863
4.375035	2.656228	3.284204	3.675466	3.677412	3.106835

Note: Precipitation was simulated based on HELP V4 weather simulation for:
Lat/Long: 39.07/-89.4

Normal Mean Monthly Temperature (Degrees Fahrenheit)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
37.3	37.1	50	61.9	69.7	80.6
84.2	81	72.2	62.4	48.1	38

Note: Temperature was simulated based on HELP V4 weather simulation for:
Lat/Long: 39.07/-89.4
Solar radiation was simulated based on HELP V4 weather simulation for:
Lat/Long: 39.07/-89.4

Average Annual Totals Summary

Title: Cof GMF - Default Earth
Simulated on: 7/6/2022 13:23

	Average Annual Totals for Years 1 - 30*			
	(inches)	[std dev]	(cubic feet)	(percent)
Precipitation	39.97	[4.83]	1,798,943.4	100.00
Runoff	7.080	[3.444]	318,685.9	17.72
Evapotranspiration	30.973	[3.376]	1,394,153.9	77.50
Subprofile1				
Percolation/leakage through Layer 3	1.976199	[0.075661]	88,952.7	4.94
Average Head on Top of Layer 3	21.2644	[2.1745]	---	---
Subprofile2				
Percolation/leakage through Layer 6	0.094011	[0.06158]	4,231.6	0.24
Average Head on Top of Layer 5	81.6880	[52.8561]	---	---
Water storage				
Change in water storage	1.8189	[1.5745]	81,872.0	4.55

* Note: Average inches are converted to volume based on the user-specified area.

APPENDIX G

Final Closure Plan



REPORT

**Final Closure Plan for the Gypsum Management Facility
Gypsum Stack Pond
*Coffeen Power Plant***

Submitted to:

Illinois Power Resources Generating, LLC

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July 28, 2022

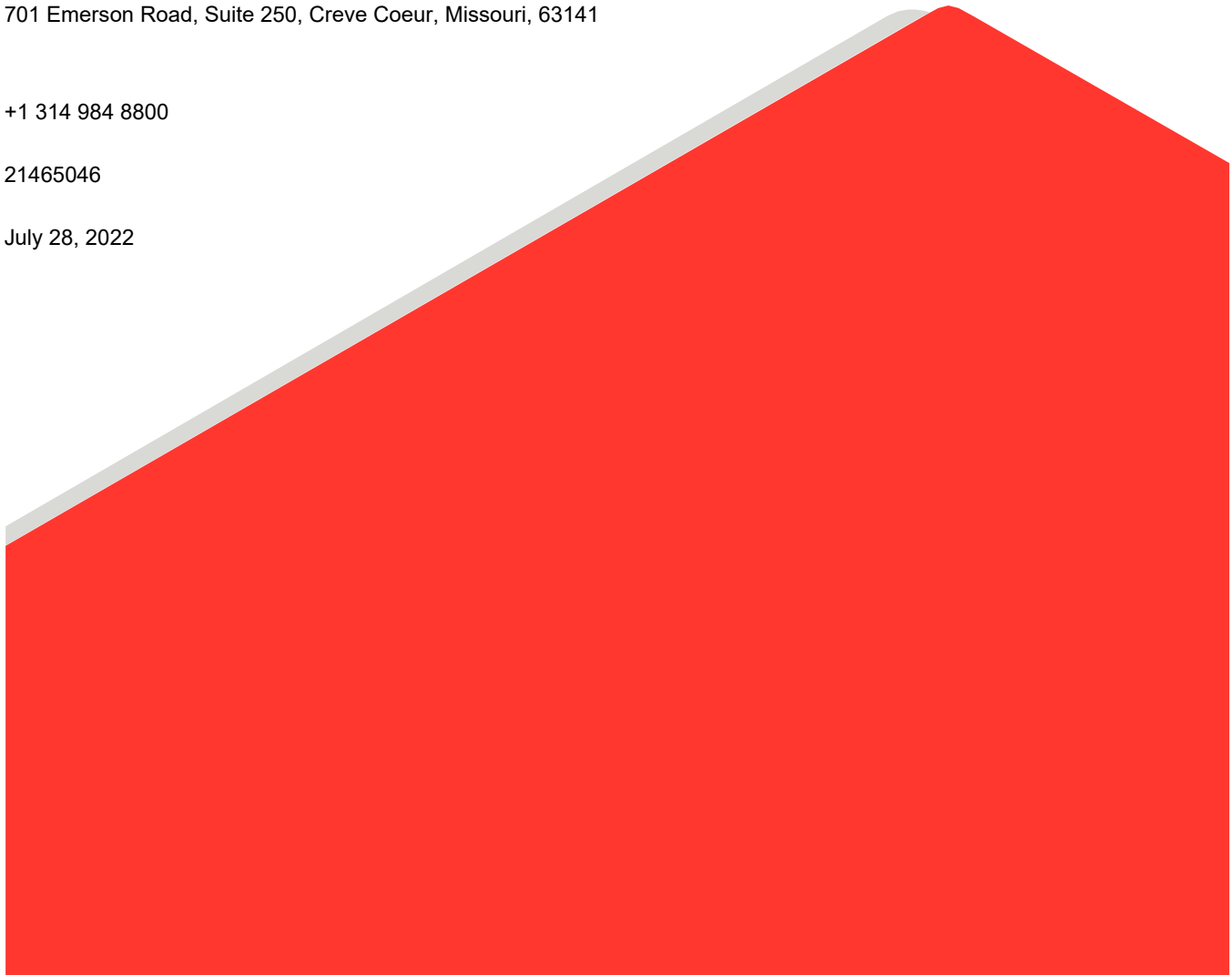


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1.0 INTRODUCTION

This Final Closure Plan has been prepared to address certain requirements of Illinois Administrative Code Title 35, Part 845, Standards for the Disposal of Coal Combustion Residuals (CCR) in Surface Impoundments (Part 845) for Illinois Power Resources Generating, LLC's (IPRG's) Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) at the Coffeen Power Plant near Coffeen, Illinois. Specifically, this document addresses requirements pertaining to the development of a Final Closure Plan for the GMF GSP. The GMF GSP has identification codes as follow:

- IPRG ID Number: CCR Unit ID 103
- IEPA ID Number: W1350150004-03
- IDNR Dam ID Number: IL50579

1.1 Proposed Selected Closure Method

Part 845.720 (b)(3): The final closure plan must identify the proposed selected closure method, and must include the information required in subsection (a)(1) and the closure alternatives analysis specified in Section 845.710.

IPRG evaluated closure with a final cover system (hereafter referred to as closure-in-place or CIP) (Section 845.750) and closure-by-removal of CCR (CBR) (Section 845.740). An analysis of these closure alternatives is summarized in Attachment 1. Based on the Closure Alternatives Analysis, a hybrid closure with a final cover system has been identified as the most appropriate closure method. All gypsum (CCR) in the southern portion of the impoundment will be removed and relocated to the northern portion of the impoundment, which will be closed in accordance with Section 845.750. Under this hybrid approach, approximately 63% of the current CCR footprint within the impoundment will be removed. In combination with the existing liner system beneath the GMF GSP, the final cover system will provide complete encapsulation of CCR in the GMF GSP, physically isolating it from contact with surrounding soils, groundwater, surface water, and the atmosphere, and minimizing the potential for release of CCR or leachate. The final cover system has been designed to minimize the post-closure infiltration of liquids into the waste.

During the closure process, IPRG will continue to assess off-Site CCR beneficial use opportunities. Ash consolidation and closure in place in combination with offsite beneficial use may result in a smaller footprint for purposes of our ultimate cap design along with a reduced construction schedule.

2.0 FINAL CLOSURE PLAN

2.1 Narrative Closure Description

Part 845.720(a)(1)(A): A narrative description of how the CCR surface impoundment will be closed in accordance with this Part.

Closure grades and details are shown in the Drawings included as Attachment 2. The closure-in-place concept for the GMF GSP was developed to reduce the waste footprint at closure. The proposed closure-in-place option would have final cover slopes of 25H:1V (4%) to accommodate moderate settlement and promote drainage. A berm will be constructed at the south end of the consolidated footprint for stability. The location of the berm has been selected to accommodate the estimated 295,500 CY of CCR and 38,000 CY of clayey soil from the GMF GSP to be contained within the consolidated footprint based on the grading plan presented. The general sequencing plan for the closure-in-place option is as follows:

- Pump out ponded water [approximately 106 million gallons (MG)] from the GMF GSP to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- Resume pumping of the perimeter drains surrounding the GSP to lower the groundwater level beneath the GSP and outside the existing liner system to facilitate closure construction. Discharge water to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- A temporary water management system will be constructed within the GMF GSP, including ditches and sumps. The system will maintain the GMF GSP in an unwatered state by collecting contact stormwater during closure construction. Stormwater flow will be conveyed through Outfall 023 to the existing drainage to the east where it will be managed in accordance with the NPDES permit for the site.
- Once the ponded water has been removed from the GMF GSP, the CCR in the consolidated footprint will be dewatered. All of the approximately 131,000 CY of CCR will be dewatered as needed to enable relocation. It is anticipated that approximately 8.8 MG of water removal will be required to dewater the CCR. The CCR will dewater to some degree by gravity, but dewatering by pumping from trenches and sumps is expected to be necessary. Liquid waste and water flowing to sumps will be managed in accordance with the NPDES permit for the site and discharged through Outfall 023.
- CCR will be removed from the berm footprint and relocated into the consolidated footprint. The berm will be constructed in an east-west orientation at the east end of the consolidated footprint. The upstream face of the berm will be lined with a composite liner system consisting of 60-mil HDPE geomembrane overlying a compacted clay layer, which will tie into the existing composite liner system.
- All remaining CCR south of the berm in the GMF GSP will be collected and deposited north of the berm.
- Geosynthetic components of the existing liner system and PWRS south of the berm in the GMF GSP will be removed and disposed in the closure footprint or hauled away for disposal. It is anticipated that up to 1 foot of clay soil beneath the geomembrane may also be removed. The soils will be visually observed for signs of CCR. If soils with signs of CCR are observed, they will be removed and deposited north of the berm (for the purposes of conceptual design, assume 1 foot, or approximately 38,000 CY, will need to be removed).
- Compacted fill, composed of locally available soils, would be placed only as needed to achieve final cover subgrade. The compacted fill is anticipated to be compacted to a minimum of 95% of the standard Proctor maximum dry density to reduce settlement.
- Construction of an alternate final cover system, consisting of (from top to bottom):
 - 24-inch final protective soil layer. The final protective soil layer would include 18 inches of protective soil cover overlain by a 6-inch-thick topsoil layer, and be revegetated with grass species selected to reduce maintenance based on soil type. The 18 inches of protective soil cover will be constructed of locally available soils removed from the embankment containment berm and compacted to between 80% and 95% of the standard Proctor maximum dry density for establishment of vegetation and protection of the underlying geomembrane. Protective soil layer material is likely to be primarily low-plasticity silt or clay based on review of site geotechnical information.
 - Drainage geocomposite

- 40-mil linear low-density polyethylene (LLDPE) geomembrane layer
- All areas of the cover system will be sloped at a minimum of 4% to positively drain to the exterior of the GMF GSP.
- To prevent impoundment of water in the south end of the current GMF GSP footprint after CCR removal, existing earthen embankments not required for the consolidated footprint will be removed on the eastern side of the GMF GSP and a channel will be excavated to allow stormwater to flow through existing NPDES Outfall 023 into the existing drainage to the east.
- The final ground surface of the southern part of the GMF GSP will be sloped to drain a minimum of 0.5% towards the channel excavated in the southeast corner, in order to allow post-closure, non-contact stormwater to gravity flow into the existing drainage. Soil fill, sourced from existing berms no longer required to contain waste in the consolidated footprint or from the on-Site soil borrow area southeast of AP1, will be used to achieve the necessary slopes. Additional fill will come from off-Site borrow sources assumed to be located within 2 miles of the GMF GSP.
- Vegetation will be established on the final surface of the GMF GSP. Stormwater best management practices (BMPs) such as erosion control blankets will be used, as needed, to reduce erosion during vegetation establishment.
- After vegetation is established, BMPs will be removed, and closure construction will be considered complete.

2.2 Decontamination of CCR Surface Impoundment

Part 845.720(a)(1)(B): If closure of the CCR surface impoundment will be accomplished through removal of CCR from the CCR surface impoundment, a description of the procedures to remove the CCR and decontaminate the CCR surface impoundment in accordance with Section 845.740.

After all CCR south of the berm has been relocated to within the closure footprint, the geosynthetic components of the existing liner system and PWRS south of the berm in the GMF GSP will be removed and disposed in the closure footprint or hauled away for disposal. The soils will be visually observed for signs of CCR. If soils with the presence of CCR are observed, they will be removed and deposited north of the berm. It is anticipated that up to 1 foot of subsoils may be removed; however, visual inspection will be conducted to confirm that all CCR in the southern portion is removed. Decontamination of areas outside of the southern portion of the GMF GSP will not be required because there have been no releases of CCR from the southern portion of the GMF GSP.

2.3 Final Cover System Performance Standards

Part 845.720(a)(1)(C): If closure of the CCR surface impoundment will be accomplished by leaving CCR in place, a description of the final cover system, designed in accordance with Section 845.750, and the methods and procedures to be used to install the final cover. The closure plan must also discuss how the final cover system will achieve the performance standards specified in Section 845.750.

The final cover system is described in Section 2.1 and shown in the Drawings (Attachment 2). Documentation in support of the final cover system achieving the performance standards of Section 845.750 is provided in Section 4.7.

2.4 Maximum CCR Inventory Estimate

Part 845.720(a)(1)(D): An estimate of the maximum inventory of CCR ever on-site over the active life of the CCR surface impoundment.

Based on Golder’s comparison (using Autodesk Civil 3D) of the existing conditions (December 2020 survey by IngenAE) and the approximate top-of-liner-system grades developed from the as-built top of liner, the estimated volume of CCR in the GMF GSP is approximately 298,500 CY. No additional CCR will be placed in the GMF GSP before it is closed.

2.5 Largest Surface Area Estimate

Part 845.720(a)(1)(E): An estimate of the largest area of the CCR surface impoundment ever requiring a final cover (see Section 845.750), at any time during the CCR surface impoundment’s active life.

In the Closure Plan developed for compliance with the United States Environmental Protection Agency’s (USEPA’s) CCR Rule (40 CFR 257, Subpart D), the largest area of the GMF GSP ever requiring a final cover system was estimated to be approximately 36 acres. This area represents the entire footprint of the GMF GSP. The area of the closure footprint requiring a final cover system under this Final Closure Plan is approximately 12.4 acres.

2.6 Closure Completion Schedule

Part 845.720(a)(1)(F): A schedule for completing all activities necessary to satisfy the closure criteria in this Section, including an estimate of the year in which all closure activities for the CCR surface impoundment will be completed. The schedule should provide sufficient information to describe the sequential steps that will be taken to close the CCR surface impoundment, including identification of major milestones such as coordinating with and obtaining necessary approvals and permits from other agencies, the dewatering and stabilization phases of CCR surface impoundment closure, or installation of the final cover system, and the estimated timeframes to complete each step or phase of CCR surface impoundment closure. When preparing the preliminary written closure plan, if the owner or operator of a CCR surface impoundment estimates that the time required to complete closure will exceed the timeframes specified in Section 845.760(a), the preliminary written closure plan must include the site-specific information, factors and considerations that would support any time extension sought under Section 845.760(b).

Table 1: Closure Completion Milestone Schedule

Milestone	Timeframe (Preliminary Estimates)
Final Closure Plan Submittal	August 2022
Final Design and Bid Process <ul style="list-style-type: none"> ■ Complete final design of the closure and select a construction contractor Agency Coordination, Approvals, and Permitting <ul style="list-style-type: none"> ■ Obtain state permits, as needed, for dewatering, water discharge, land disturbance, and dam modifications 	8 to 12 months after Final Closure Plan Approval
Dewater and Stabilize CCR	11 to 16 months after issuance of necessary permits, design completion, and bid award

Milestone	Timeframe (Preliminary Estimates)
<ul style="list-style-type: none"> ■ Complete contractor mobilization, installation of stormwater BMPs, and unwatering of GMF GSP ■ Pump water from GMF GSP ■ Dewater and stabilize GMF GSP 	
<p>Consolidate Waste Footprint</p> <ul style="list-style-type: none"> ■ Construct east-west berm ■ Install new liner system on upstream face of berm ■ Relocate CCR south of berm to closure footprint 	4 to 6 months after dewatering and CCR stabilization
<p>Installation of Final Cover System</p> <ul style="list-style-type: none"> ■ Prepare top of CCR for cover system installation ■ Install geomembrane ■ Install geocomposite drainage layer ■ Place protective cover soil 	4 to 6 months after CCR relocation to closure footprint
<p>Site Restoration</p> <ul style="list-style-type: none"> ■ Remove existing liner south of berm ■ Excavate drainage channels ■ Seed and stabilize GMF GSP 	3 to 5 months after the final cover system is complete
<p>Timeframe to Complete Closure</p>	Prior to November 2026

3.0 AMENDMENT OF THE FINAL CLOSURE PLAN

Part 845.720(b)(4): If a final written closure plan revision is necessary after closure activities have started for a CCR surface impoundment, the owner or operator must submit a request to modify the construction permit within 60 days following the triggering event.

IPRG will submit a written request to modify the construction permit within 60 days of a triggering event.

4.0 CLOSURE WITH A FINAL COVER SYSTEM

4.1 Minimization of Post-Closure Infiltration and Releases

Part 845.750(a): The owner or operator of a CCR surface impoundment must ensure that, at a minimum, the CCR surface impoundment is closed in a manner that will:

- 1) *Control, minimize or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere.*

The proposed CIP design will control, minimize, or eliminate as much as feasible “post-closure infiltration of liquids” and releases of CCR, leachate, or contaminated runoff as interpreted by Illinois EPA in the Part 845 rulemaking. The final cover system will consist of (from top to bottom) the following:

- Based on a demonstration to be submitted to IEPA for approval pursuant to Section 845.750(c)(2), a 2-foot-thick final protective layer consisting of locally available soils compacted to between 80% and 95% of the standard Proctor maximum dry density. The uppermost 6 inches of the final protective layer will be tracked in place with a density suitable for establishment of vegetation. Soils are likely to consist primarily of low-plasticity silt or clay based on a review of site geotechnical information.
- Drainage geocomposite. This layer will provide lateral drainage to limit the potential for the final protective layer to become saturated.
- 40-mil LLDPE geomembrane.

The use of HDPE geomembrane was considered, but LLDPE geomembrane was selected because it can be installed more easily in a wider range of cold-temperature conditions. This final cover system is compliant with the Part 845 requirements, as described in Section 4.7, and will minimize the post-closure infiltration of liquids into the waste. After closure, the CCR stored in the facility will be completely encapsulated by the final cover and liner systems, physically isolating it from contact with surrounding soils, groundwater, surface water, and the atmosphere, and minimizing the potential for release of CCR or leachate. This is supported by groundwater modeling, as presented in Appendix G to the Part 845 Construction Permit Application for the GMF GSP.

- The existing liner system physically isolates the CCR stored in the GMF GSP consolidation area from the surrounding soils and groundwater, minimizing water flux through the CCR material. Removal of free liquids, consolidation, and placement of the final cover system will minimize the volume of water within the facility, further reducing the potential for water flux through the CCR material.

4.2 Preclusion of Future Impoundment

Part 845.750(a): The owner or operator of a CCR surface impoundment must ensure that, at a minimum, the CCR surface impoundment is closed in a manner that will:

- 2) *Preclude the probability of future impoundment of water, sediment, or slurry.*

The final cover system will be crowned with 4% slopes to direct surface water away from the facility. Beyond the final cover system, channels will direct surface water away from the GMF GSP to existing site drainages.

4.3 Provisions for Preventing Instability, Sloughing and Movement

Part 845.750(a): The owner or operator of a CCR surface impoundment must ensure that, at a minimum, the CCR surface impoundment is closed in a manner that will:

- 3) *Include measures that provide for major slope stability to prevent the sloughing or movement of the final cover system during the closure and post-closure care period.*

An assessment of the GMF GSP structural stability was completed as part of compliance with USEPA's CCR Rule (AECOM 2016). This assessment concluded that the GMF GSP meets stability factor of safety requirements and does not pose a significant risk of instability.

A new earthen berm is provided in the closure design to enhance stability along the southern end of the closure footprint. Slope stability calculations are included in Attachment 3 to demonstrate that factors of safety for static and seismic stability after closure are acceptable. The slope stability calculations also considered veneer stability to verify that the final cover system will not be susceptible to instability, sloughing, or movement during the closure and post-closure care period.

4.4 Minimize the Need for Future Maintenance

Part 845.750(a): The owner or operator of a CCR surface impoundment must ensure that, at a minimum, the CCR surface impoundment is closed in a manner that will:

4) *Minimize the need for further maintenance of the CCR surface impoundment*

The 4% design closure slopes are sufficient to adequately shed water from the facility but are flat enough to limit erosion of the final protective layer. Minor maintenance of the final cover system (potentially including filling of low areas, reseeding, fertilizing, etcetera) will likely be necessary for several years after completion of final cover system construction, as described in the Post-closure Care Plan (Appendix J to the Part 845 Construction Permit Application for the GMF Pond). The need for long-term future maintenance is expected to be minimal after installation of the final cover system has been completed and vegetation has been established.

The channels designed to convey surface water runoff away from the closed facility have been sized to accommodate the 25-year, 24-hour storm event. The design calculations are provided in Attachment 4.

4.5 Be Completed in the Shortest Amount of Time

Part 845.750(a): The owner or operator of a CCR surface impoundment must ensure that, at a minimum, the CCR surface impoundment is closed in a manner that will:

5) *Be completed in the shortest amount of time consistent with recognized and generally accepted engineering practices.*

The CIP method will require significantly less material handling compared with a CBR approach. Both approaches require the removal of liquid wastes, but the CIP method will require relocation of less than 60% of the CCR present in the GMF GSP. This reduced material handling volume means that the CIP construction can be completed in approximately 30 to 45 months, compared with 41 to 60 months, or possibly more, for CBR.

4.6 Drainage and Stabilization

Part 845.750(b): Drainage and Stabilization of CCR Surface Impoundments. The owner or operator of a CCR surface impoundment or any lateral expansion of a CCR surface impoundment must meet the requirements of this subsection (b) before installing the final cover system required by subsection (c).

- 1) *Free liquids must be eliminated by removing liquid wastes or solidifying the remaining wastes and waste residues.*
- 2) *Remaining wastes must be stabilized sufficiently to support the final cover system.*

Approximately 106 million gallons of water will be pumped from the GMF GSP as the initial step for facility closure. After removal of the ponded water, the CCR will still be unsuitable for supporting heavy construction traffic over much of the footprint. Careful planning will be required to safely work on the wet CCR within the GMF GSP. The planned CCR removal and relocation will rely on a series of trenches or other engineering measures to remove liquid wastes or solidify the remaining wastes. Trenches will shorten drainage routes to facilitate gravity removal of liquid wastes in the CCR in the vicinity of each trench and direct the liquid wastes to sumps. Other engineering measures may be considered to facilitate removal of liquid wastes. Sumps will be used to collect liquid wastes, which will be managed in accordance with the NPDES permit for the site. Using the process described or other engineering measures for removal of liquid wastes or solidification of the remaining wastes, the CCR remaining in place will be stabilized sufficiently to support the final cover system.

4.7 Final Cover System

Part 845.750(c): Final Cover System. If a CCR surface impoundment is closed by leaving CCR in place, the owner or operator must install a final cover system that is designed to minimize infiltration and erosion, and, at a minimum, meets the requirements of this subsection (c). The final cover system must consist of a low permeability layer and a final protective layer. The design of the final cover system must be included in the preliminary and final written closure plans required by Section 845.720 and the construction permit application for closure submitted to the Agency.

4.7.1 Low-Permeability Layer

Part 845.750(c)(1) Standards for the Low Permeability Layer. The low permeability layer must have a permeability less than or equal to the permeability of any bottom liner system or natural subsoils present, or a hydraulic conductivity no greater than 1×10^{-7} cm/sec, whichever is less. The low permeability layer must be constructed in accordance with the standards in either subsection (c)(1)(A) or (c)(1)(B), unless the owner or operator demonstrates that another low permeability layer construction technique or material provides equivalent or superior performance to the requirements of either subsection (c)(1)(A) or (c)(1)(B) and is approved by the Agency.

- A) *A compacted earth layer constructed in accordance with the following standards:*
 - i) *The minimum allowable thickness must be 0.91 meter (three feet); and*
 - ii) *The layer must be compacted to achieve a hydraulic conductivity of 1×10^{-7} cm/sec or less and minimize void spaces.*
- B) *A geomembrane constructed in accordance with the following standards:*
 - i) *The geosynthetic membrane must have a minimum thickness of 40 mil (0.04 inches) and, in terms of hydraulic flux, must be equivalent or superior to a three-foot layer of soil with a hydraulic conductivity of 1×10^{-7} cm/sec;*
 - ii) *The geomembrane must have strength to withstand the normal stresses imposed by the waste stabilization process; and*
 - iii) *The geomembrane must be placed over a prepared base free from sharp objects and other materials that may cause damage.*

The final cover system will include a 40-mil LLDPE geomembrane placed on a prepared subgrade of CCR (see the Drawings in Attachment 2). The prepared subgrade will be free of sharp objects prior to geomembrane

installation. The geomembrane material will conform with the specifications of Geosynthetic Institute GRI-GM17 “Test Methods, Test Properties and Testing Frequency for Linear Low Density Polyethylene (LLDPE) Smooth and Textured Geomembranes” and will be installed per GRI-GM19a “Seam Strength and Related Properties of Thermally Bonded Homogeneous Polyolefin Geomembranes/Barriers” so that the material itself and the seams between panels will withstand the expected normal and tensile stress conditions. Furthermore, a 40-mil LLDPE geomembrane manufactured and installed to these specifications is widely accepted to be equivalent or superior to a 3-foot-thick layer of soil with a hydraulic conductivity of 1×10^{-7} cm/sec.

4.7.2 Final Protective Layer

Part 845.750(c)(2): Standards for the Final Protective Layer. The final protective layer must meet the following requirements, unless the owner or operator demonstrates that another final protective layer construction technique or material provides equivalent or superior performance to the requirements of this subsection (c)(2) and is approved by the Agency.

- A) *Cover the entire low permeability layer;*
- B) *Be at least three feet thick, be sufficient to protect the low permeability layer from freezing, and minimize root penetration of the low permeability layer;*
- C) *Consist of soil material capable of supporting vegetation;*
- D) *Be placed as soon as possible after placement of the low permeability layer; and*
- E) *Be covered with vegetation to minimize wind and water erosion.*

Based on a demonstration to be submitted to IEPA for approval pursuant to Section 845.750(c)(2), a 2-foot-thick final protective layer will be installed for the final cover system, immediately overlaying the drainage geocomposite and covering the entire low-permeability layer (see the Drawings in Attachment 2). The final protective layer will comprise locally available soils compacted to between 80% and 95% of the standard Proctor maximum dry density. The uppermost 6 inches of the final protective layer will be tracked in place to a density suitable for establishment of vegetation. This soil is expected to consist primarily of low-plasticity silt or clay based on a review of site geotechnical information. This soil is capable of supporting vegetation, will be placed as soon as possible after placement of the low-permeability layer, and will be covered with vegetation to limit wind and water erosion.

4.8 Final Cover System Settling

Part 845.750(c)(3): The disruption of the integrity of the final cover system must be minimized through a design that accommodates settling and subsidence.

The closure slopes are designed at 4% to accommodate settlement while still maintaining positive drainage off the facility. Additional discussion on this subject is provided in Section 4.4.

4.9 Use of CCR in Closure

Part 845.750(d): This subsection specifies the allowable uses of CCR in the closure of CCR surface impoundments closing under Section 845.700. Notwithstanding the prohibition on further placement in Section 845.700, CCR may be placed in these surface impoundments, but only for purposes of grading and contouring in the design and construction of the final cover system, if:

- 1) *The CCR placed was generated at the facility and is located at the facility at the time closure was initiated;*

- 2) *CCR is placed entirely above the elevation of CCR in the surface impoundment, following dewatering and stabilization (see subsection (b));*
- 3) *The CCR is placed entirely within the perimeter berms of the CCR surface impoundment; and*
- 4) *The final cover system is constructed with either:*
 - A) *A slope not steeper than 5% grade after allowance for settlement; or*
 - B) *At a steeper grade, if the Agency determines that the steeper slope is necessary, based on conditions at the site, to facilitate run-off and minimize erosion, and that side slopes are evaluated for erosion potential based on a stability analysis to evaluate possible erosion potential. The stability analysis, at a minimum, must evaluate the site geology; characterize soil shear strength; construct a slope stability model; establish groundwater and seepage conditions, if any; select loading conditions; locate critical failure surface; and iterate until minimum factor of safety is achieved.*

The GMF GSP is not closing under Section 845.700. Following dewatering of the GMF GSP, CCR currently located within the GMF GSP (which was generated at Coffeen Power Plant) will be relocated to within the closure footprint. Closure of the GMF GSP will comply with the requirements of Subsection 845.750(d) in the event CCR from a unit other than the GMF GSP is utilized.

5.0 CERTIFICATION

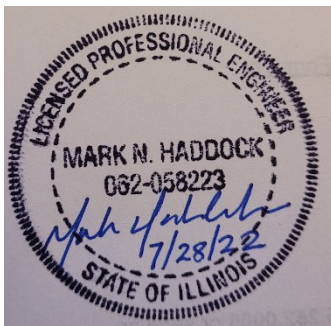
Part 845.750(c)(4): The owner or operator of the CCR surface impoundment must obtain and submit with its construction permit application for closure a written certification from a qualified professional engineer that the design of the final cover system meets the requirements of this Section.

The undersigned qualified professional engineer registered in Illinois certifies that the design of the final cover system meets the requirements of Section 845.750.

Signature Page

Golder Associates USA Inc.

I, Mark Haddock, being a registered professional engineer in good standing in the State of Illinois, certify to the best of my knowledge that this Final Closure Plan meets the requirements of Illinois Administrative Code Title 35, Part 845.



Mark Haddock
Principal

[https://golderassociates.sharepoint.com/sites/145229/project files/6 deliverables/reports/05-closure_plan_gmf_and_recycle/gmf_gsp_20220712-reva/21465046-closure_plan_gmf_gsp_20220712-rev0.docx](https://golderassociates.sharepoint.com/sites/145229/project%20files/6%20deliverables/reports/05-closure_plan_gmf_and_recycle/gmf_gsp_20220712-reva/21465046-closure_plan_gmf_gsp_20220712-rev0.docx)

6.0 REFERENCES

AECom. 2016. CCR Certification Report: GMF Pond, At Coffeen Power Station. October.

ATTACHMENT 1

Closure Alternatives Analysis

Closure Alternatives Analysis for the Gypsum Management Facility Stack Pond and Recycle Pond at the Coffeen Power Plant Coffeen, Illinois

July 28, 2022



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Abbreviations

AACE	Association for the Advancement of Cost Engineering
BMP	Best Management Practice
CAA	Closure Alternatives Analysis
CBR-Offsite	Closure-by-Removal with Off-Site CCR Disposal
CBR-Onsite	Closure-by-Removal with On-Site CCR Disposal
CCR	Coal Combustion Residual
CFR	Code of Federal Regulation
CIP	Closure-in-Place
CO	Carbon Monoxide
CO ₂	Carbon Dioxide
CY	Cubic Yard
DA	Deep Aquifer
DCU	Deep Confining Unit
EJ	Environmental Justice
FEMA	Federal Emergency Management Agency
GHG	Greenhouse Gas
GMF	Gypsum Management Facility
GSP	Gypsum Stack Pond
GWPS	Groundwater Protection Standard
HUC	Hydrologic Unit Code
IAC	Illinois Administrative Code
IDNR	Illinois Department of Natural Resources
IEPA	Illinois Environmental Protection Agency
IPGC	Illinois Power Generating Company
ISGS	Illinois State Geological Survey
LCU	Lower Confining Unit
LLDPE	Linear Low-Density Polyethylene
N ₂ O	Nitrous Oxide
NID	National Inventory of Dams
NO _x	Nitrogen Oxides
NPDES	National Pollutant Discharge Elimination System
PM	Particulate Matter
RP	Recycle Pond
SFWA	State Fish and Wildlife Area
TMDL	Total Maximum Daily Load
TVA	Tennessee Valley Authority
UA	Uppermost Aquifer
UCU	Upper Confining Unit
US DOT	United States Department of Transportation
US FWS	United States Fish & Wildlife Service
VOC	Volatile Organic Compound
WPC Permit	Water Pollution Control Construction and Operating Permit

Summary of Findings

Title 35, Part 845 of the Illinois Administrative Code (IAC; IEPA, 2021a) requires the development of a Closure Alternatives Analysis (CAA) prior to undertaking closure activities at certain surface impoundments containing coal combustion residuals (CCRs) in the state of Illinois. Pursuant to requirements under IAC Section 845.710, this report presents a CAA for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) and the GMF Recycle Pond (RP) located on Illinois Power Generating Company's (IPGC) Coffeen Power Plant property near the City of Coffeen, Illinois. The goal of a CAA is to holistically evaluate potential closure scenarios with respect to a wide range of factors, including the efficiency, reliability, and ease of implementation of the closure scenario; its potential positive and negative short- and long-term impacts on human health and the environment; and its ability to address concerns raised by residents (IAC Part 845; IEPA, 2021a). For the GMF GSP, Gradient evaluated two specific closure scenarios: Closure-in-Place (CIP) with CCR excavation and consolidation and Closure-by-Removal with off-Site disposal (CBR-Offsite). The CIP scenario entails excavating CCR from the southern portion of the GMF GSP and consolidating it into the northern portion of the GMF GSP and capping it with a new cover system consisting of, from bottom to top, a geomembrane layer, a geotextile cushion if needed, and 24 inches of low-permeability soil with a vegetated surface. The CBR-Offsite scenario entails excavating all of the CCR and liner system materials from the GMF GSP and transporting it to an off-Site landfill for disposal. Even though capping the entire GMF GSP (without any excavation or consolidation) would be an acceptable closure approach based on IAC Section 845.710 (IEPA, 2021a), it was not evaluated in this CAA. For the GMF RP, Gradient evaluated one closure scenario: Closure-by-Removal with on-Site disposal (CBR-Onsite). The CBR-Onsite scenario entails excavating the CCR and liner system materials from the GMF RP and transporting these materials to an on-Site landfill for disposal. IPGC will also continue to evaluate potential opportunities for beneficial re-use of CCR excavated from the GMF GSP and the GMF RP as an alternative to disposal.

IAC Section 845.710(c)(2) requires CAAs to "[i]dentify whether the facility has an onsite landfill with remaining capacity that can legally accept CCR, and, if not, whether constructing an onsite landfill is possible" (IEPA, 2021a). There is an existing on-Site landfill at the Coffeen Power Plant Site with enough capacity to accept CCR from the GMF RP, but it does not have enough capacity to contain all of the material that would also be removed from the GMF GSP. Furthermore, due to the planned redevelopment of the Site as a utility-scale solar energy generation and battery energy storage facility, there is not sufficient space available to expand the existing landfill. Thus, for the CBR scenarios evaluated in this CAA, Gradient assumed that CCR from the GMF GSP would be disposed in an off-Site landfill, but CCR from the GMF RP would be disposed in an on-Site landfill.

Table S.1 summarizes the expected impacts of the CIP (GMF GSP), CBR-Offsite (GMF GSP), and CBR-Onsite (GMF RP) closure scenarios with regard to each of the factors specified under IAC Section 845.710 (IEPA, 2021a). Based on this evaluation and the additional details provided in Section 2 of this report, CIP of the GMF GSP and CBR-Onsite of the GMF RP have been identified as the most appropriate closure scenarios for the GMF GSP and the GMF RP. Key benefits of the CIP scenario relative to the CBR-Offsite scenario for the GMF GSP include reduced impacts to workers, community members, and the environment during construction (*e.g.*, fewer construction-related accidents, lower energy demands, less air pollution and greenhouse gas [GHG] emissions, less traffic-related impacts).

Table S.1 Comparison of Proposed Closure Scenarios

Evaluation Factor (Report Section; IAC Part 845 Section)	Closure Scenario		
	CIP (GMF GSP)	CBR-Offsite (GMF GSP)	CBR-Onsite (GMF RP)
Closure Alternative Descriptions (Section 2.1, IAC Section 845.710(c))	CCR would be excavated from the southern portion of the GMF GSP and consolidated into the northern portion and then capped in place with a new cover system consisting of, from bottom to top, a geomembrane layer, a geotextile cushion if needed, and 24 inches of low-permeability soil with a vegetated surface. During the closure process, we will continue to assess off-Site CCR beneficial use opportunities. Ash consolidation and CIP in combination with off-Site beneficial use may result in a smaller footprint for purposes of our ultimate cap design along with a reduced construction schedule.	All CCR and existing liner materials would be excavated from the GMF GSP and transported <i>via</i> truck to an off-Site landfill for disposal. Expansion of the off-Site landfill may be necessary in order to accept all of the CCR and liner materials from the GMF GSP. The on-Site landfill does not have the capacity for all of the CCR from both the GMF GSP and the GMF RP, nor can it be expanded due to future redevelopment plans. This scenario meets the requirements of IAC Section 845.710(c)(2) (IEPA, 2021a), which requires an assessment be included in the CAA of whether the Site has an on-Site landfill with available capacity or whether an on-Site landfill can be constructed.	All CCR and existing liner materials would be excavated from the GMF RP and transported <i>via</i> truck to an on-Site landfill for disposal. This scenario meets the requirements of IAC Section 845.710(c)(2) (IEPA, 2021a), which requires an assessment be included in the CAA of whether the Site has an on-Site landfill with available capacity or whether an on-Site landfill can be constructed.
Type and Degree of Long-Term Management, Including Monitoring, Operation, and Maintenance (Section 2.2.3, IAC Section 845.710(b)(1)(C))	Monitoring would be performed for 30 years post-closure or until GWPSs are achieved, whichever is longer. Additionally, the final cover system for the GMF GSP would undergo 30 years of annual inspections, mowing, and maintenance.	Monitoring would be performed for 3 years post-closure or until GWPSs are achieved, whichever is longer.	Monitoring would be performed for 3 years post-closure or until GWPSs are achieved, whichever is longer. Additionally, the on-Site landfill cover would undergo 30 years of annual inspections, mowing, and maintenance.
Magnitude of Reduction of Existing Risks (Section 2.2.1, IAC Sections 845.710(b)(1)(A) and 845.710(b)(1)(F))	There are no current unacceptable risks to any human or ecological receptors associated with the GMF GSP. Because there are no current risks, and dissolved constituent concentrations would be expected to decline post-closure, no risks to human or ecological receptors would be expected post-closure.	There are no current unacceptable risks to any human or ecological receptors associated with the GMF GSP. Because there are no current risks, and dissolved constituent concentrations would be expected to decline post-closure, no risks to human or ecological receptors would be expected post-closure.	There are no current unacceptable risks to any human or ecological receptors associated with the GMF RP. Because there are no current risks, and dissolved constituent concentrations would be expected to decline post-closure, no risks to human or ecological receptors would be expected post-closure.
Likelihood of Future Releases of CCR (Section 2.2.2, IAC Sections 845.710(b)(1)(B) and 845.710(b)(1)(F))	During closure, there would be minimal risk of dike failure occurring at the GMF GSP (due to, <i>e.g.</i> , flooding or seismic activity) and minimal risk of dike overtopping during flood conditions. Post-closure, the risks of overtopping and dike failure would be even smaller than they are currently, due to the installation of a protective soil cover and new stormwater control structures. Dikes, final cover, and stormwater control features have been designed to withstand earthquakes and storm events.	During closure, there would be minimal risk of dike failure occurring at the GMF GSP (due to, <i>e.g.</i> , flooding or seismic activity) and minimal risk of dike overtopping during flood conditions. Following excavation, there would be no risk of CCR releases due to dike failure. Changing geochemical conditions during an extended excavation can be a mechanism that results in the mobilization and increased transport in groundwater for some constituents.	During closure, there would be minimal risk of dike failure occurring at the GMF RP (due to, <i>e.g.</i> , flooding or seismic activity) and minimal risk of dike overtopping during flood conditions. Following excavation, there would be no risk of CCR releases due to dike failure. Changing geochemical conditions during an extended excavation can be a mechanism that results in the mobilization and increased transport in groundwater for some constituents.
Worker Risks (Section 2.2.4.1, IAC Sections 845.710(b)(1)(D) and 845.710(b)(1)(F))	An estimated 0.0027 worker fatalities and 0.42 worker injuries would be expected to occur due to on-Site activities under this closure scenario. An additional 0.0027 worker fatalities and 0.21 worker injuries would be expected to occur off-Site due to vehicle accidents during hauling, labor and equipment mobilization and demobilization, and material deliveries. In total, 0.0054 worker fatalities and 0.62 worker injuries would be expected under this closure scenario. Overall, risks to workers would likely be higher under the CBR-Offsite scenario and lower under the CIP scenario.	An estimated 0.0017 worker fatalities and 0.26 worker injuries would be expected to occur due to on-Site activities under this closure scenario. An additional 0.0083 worker fatalities and 0.52 worker injuries would be expected to occur off-Site due to vehicle accidents during hauling, labor and equipment mobilization and demobilization, and material deliveries. In total, 0.010 worker fatalities and 0.78 worker injuries would be expected under this closure scenario. Overall, risks to workers would likely be higher under the CBR-Offsite scenario and lower under the CIP scenario.	An estimated 0.00088 worker fatalities and 0.14 worker injuries would be expected to occur due to on-Site activities under this closure scenario. An additional 0.0010 worker fatalities and 0.072 worker injuries would be expected to occur off-Site due to vehicle accidents during hauling, labor and equipment mobilization and demobilization, and material deliveries. In total, 0.0018 worker fatalities and 0.21 worker injuries would be expected under this closure scenario.

Evaluation Factor (Report Section; IAC Part 845 Section)	Closure Scenario		
	CIP (GMF GSP)	CBR-Offsite (GMF GSP)	CBR-Onsite (GMF RP)
Community Risks (Section 2.2.4.2, IAC Sections 845.710(b)(1)(D) and 845.710(b)(1)(F))			
<ul style="list-style-type: none"> <i>Off-Site Impacts on Nearby Residents and EJ Communities</i> 	Off-Site impacts on nearby residents (including accidents, traffic, noise, and air pollution) would be less under this closure scenario than under the CBR-Offsite scenario because it would require less off-Site vehicle and equipment travel miles than the CBR scenarios. In total, an estimated 0.0021 fatalities and 0.11 injuries would be expected to occur among community members due to off-Site activities under this scenario. No off-Site transport of CCR and/or borrow soil is required under this closure scenario. No impacts to nearby EJ communities are anticipated under this closure scenario.	Off-Site impacts on nearby residents would be greater under the CBR-Offsite closure scenario than under the CIP scenario because they would require significantly more off-Site vehicle and equipment travel miles and a longer construction duration. In total, an estimated 0.019 fatalities and 0.63 injuries would be expected to occur among community members due to off-Site activities under this scenario. With regard to traffic impacts, a haul truck would be likely to pass a location near the Site every 9.9 minutes on average during working hours for approximately 31-46 months under this closure scenario. No impacts to nearby EJ communities are anticipated under this closure scenario.	Limited off-Site impacts on nearby residents from the CBR-Onsite (GMF RP) would occur. In total, the CBR-Onsite (GMF RP) would result in an estimated 0.00085 fatalities and 0.041 injuries. No off-Site transport of CCR and/or borrow soil is required under this closure. No impacts to nearby EJ communities are anticipated under this closure scenario.
<ul style="list-style-type: none"> <i>Impacts on Scenic, Historical, and Recreational Value</i> 	<p>Due to (e.g.) noise and visual disturbances, construction activities may have short-term negative impacts on the recreational use of the Coffeen Lake State Fish and Wildlife Area. Because the expected duration of construction activities is shorter under this closure scenario compared to the CBR-Offsite scenario, short-term impacts on the scenic and recreational value of natural areas near the Site would be less under this closure scenario than under the CBR-Offsite scenario.</p> <p>There are no historic sites in the vicinity of the impoundment, the on-Site landfill, or the on-Site borrow soil location. Thus, no impacts on historic sites would be expected under any closure scenario.</p>	<p>Due to (e.g.) noise and visual disturbances, construction activities may have short-term negative impacts on the recreational use of the Coffeen Lake State Fish and Wildlife Area. Because the expected duration of construction activities is longer under this scenario than under the CIP scenario, short-term impacts on the scenic and recreational value of natural areas near the Site would be greater under this closure scenario than under the CIP scenario.</p> <p>There are no historic sites in the vicinity of the impoundment or the on-Site borrow soil location. Thus, no impacts on historic sites would be expected under any closure scenario.</p>	<p>Due to (e.g.) noise and visual disturbances, construction activities may have short-term negative impacts on the recreational use of the Coffeen Lake State Fish and Wildlife Area.</p> <p>There are no historic sites in the vicinity of the impoundment or the on-Site landfill. Thus, no impacts on historic sites would be expected under any closure scenario.</p>
Environmental Risks (Section 2.2.4.3, IAC Sections 845.710(b)(1)(D) and 845.710(b)(1)(F))			
<ul style="list-style-type: none"> <i>Impacts on Greenhouse Gas Emissions and Energy Consumption</i> 	<p>Total energy demands and GHG emissions would be smaller under this closure scenario than under the CBR-Offsite scenarios, because the total equipment and vehicle mileages required under this closure scenario would be smaller than those required under the CBR-Offsite scenario</p> <p>The CIP scenario would have an additional, unquantified carbon footprint due to the need to manufacture geomembranes for use in the final cover system.</p> <p>At the grid scale, construction of a solar facility at the Site would put energy back on the grid and reduce reliance on non-renewable energy sources.</p>	<p>Total energy demands and GHG emissions would be greater under the CBR-Offsite closure scenario than under the CIP scenario, because the total equipment and vehicle mileages required under this closure scenarios would be greater than those required under the CIP scenario.</p> <p>If expansion of the off-Site landfill becomes necessary in order to accept all of the CCR and liner materials from the GMF GSP, then the CBR-Offsite scenario would have an additional, unquantified carbon footprint due to the need to manufacture geomembranes for use in the expanded landfill liner.</p> <p>At the grid scale, construction of a solar facility at the Site would put energy back on the grid and reduce reliance on non-renewable energy sources.</p>	<p>Energy demands and GHG emissions would result from equipment and vehicle mileage required for the CBR-Onsite scenario.</p> <p>The CBR-Onsite scenario would have an additional, unquantified carbon footprint due to the need to manufacture geomembranes for use in the final on-Site landfill cover system.</p> <p>At the grid scale, construction of a solar facility at the Site would put energy back on the grid and reduce reliance on non-renewable energy sources.</p>

Evaluation Factor (Report Section; IAC Part 845 Section)	Closure Scenario		
	CIP (GMF GSP)	CBR-Offsite (GMF GSP)	CBR-Onsite (GMF RP)
<ul style="list-style-type: none"> Impacts on Natural Resources and Habitat 	Construction activities may have short-term negative impacts on species located near the GMF GSP, the on-Site borrow soil location, the off-Site borrow soil location, the on-Site landfill, and the off-Site landfill. Short-term impacts on natural resources and habitat would be smaller under the CIP scenario than under the CBR-Offsite scenario, because the overall duration of construction is shorter under the former scenario.	Construction activities may have short-term negative impacts on species located near the GMF GSP, the on-Site borrow soil location, the off-Site borrow soil location, the on-Site landfill, and the off-Site landfill. Short-term impacts on natural resources and habitat would be greater under the CBR-Offsite scenario than under the CIP scenario, because the overall duration of construction is longer under the former scenarios.	Construction activities may have short-term negative impacts on species located near the GMF GSP, the on-Site borrow soil location, the off-Site borrow soil location, the on-Site landfill, and the off-Site landfill.
Time Until Groundwater Protection Standards Are Achieved (Section 2.2.5, IAC Sections 845.710(b)(1)(E) and 845.710(d)(2 and 3))	Groundwater modeling was performed to evaluate future groundwater quality in the vicinity of the GMF GSP under each of the proposed closure scenarios (Ramboll, 2022). The modeling demonstrated that there are limited differences between CIP and CBR-Offsite in the timeframes to achieve the GWPSs at the GMF GSP. For most constituents at the GMF GSP, the GWPSs will be achieved in the uppermost aquifer (UA) in approximately 7 years after implementation of closure for both scenarios.	Groundwater modeling was performed to evaluate future groundwater quality in the vicinity of the GMF GSP under each of the proposed closure scenarios (Ramboll, 2022). The modeling demonstrated that there are limited differences between CIP and CBR-Offsite in the timeframes to achieve the GWPSs at the GMF GSP. For most constituents at the GMF GSP, the GWPSs will be achieved in approximately 7 years in the UA after implementation of closure for both scenarios. Additionally, changing geochemical conditions during an extended excavation can be a mechanism that results in the mobilization and increased transport in groundwater for some constituents. This may result in GWPS exceedance durations in excess of the model predictions.	Groundwater modeling was performed to evaluate future groundwater quality in the vicinity of the GMF RP under each of the proposed closure scenarios (Ramboll, 2022). The modeling demonstrated that for most constituents at the GMF RP, the GWPSs will be achieved in the UA in approximately 4.6 years after implementation of the CBR-Offsite closure scenario. Additionally, changing geochemical conditions during an extended excavation can be a mechanism that results in the mobilization and increased transport in groundwater for some constituents. This may result in GWPS exceedance durations in excess of the model predictions.
Long-Term Reliability of the Engineering and Institutional Controls (Section 2.2.7; IAC Section 845.710(b)(1)(G))	CIP would be expected to be a reliable closure alternative over the long term.	CBR-Offsite would be expected to be a reliable closure alternative over the long term.	CBR-Onsite would be expected to be a reliable closure alternative over the long term.
Potential Need for Future Corrective Action (Section 2.2.8; IAC Section 845.710(b)(1)(H))	Corrective action is expected at the Site. An evaluation of potential corrective measures and corrective actions has not yet been completed, but will be conducted consistent with the requirements in IAC Section 845.660 and IAC Section 845.670.	Corrective action is expected at the Site. An evaluation of potential corrective measures and corrective actions has not yet been completed, but will be conducted consistent with the requirements in IAC Section 845.660 and IAC Section 845.670.	Corrective action is expected at the Site. An evaluation of potential corrective measures and corrective actions has not yet been completed, but will be conducted consistent with the requirements in IAC Section 845.660 and IAC Section 845.670.
Effectiveness of the Alternative in Controlling Future Releases (Section 2.3; IAC Section 845.710(b)(2)(A and B))	There are no current or future risks to any human or ecological receptors associated with the GMF GSP. During closure, there would be minimal risk of dike failure occurring and minimal risk of dike overtopping during flood conditions. Post-closure, the risks of overtopping and dike failure would be even smaller than they are currently, due to the installation of a protective soil cover and new stormwater control structures. Dikes, final cover, and stormwater control features have been designed to withstand earthquakes and storm events.	There are no current or future risks to any human or ecological receptors associated with the GMF GSP. During closure, there would be minimal risk of dike failure occurring and minimal risk of dike overtopping during flood conditions. Following excavation, there would be no risk of CCR releases due to dike failure.	There are no current or future risks to any human or ecological receptors associated with the GMF RP. During closure, there would be minimal risk of dike failure occurring and minimal risk of dike overtopping during flood conditions. Following excavation, there would be no risk of CCR releases due to dike failure.

Evaluation Factor (Report Section; IAC Part 845 Section)	Closure Scenario		
	CIP (GMF GSP)	CBR-Offsite (GMF GSP)	CBR-Onsite (GMF RP)
Ease or Difficulty of Implementing the Alternative (Section 2.4, IAC Section 845.710(b)(3))			
<ul style="list-style-type: none"> <i>Degree of Difficulty Associated with Construction</i> 	<p>CIP is a reliable and standard method for managing and closing waste impoundments. Dewatering saturated CCR to construct a stabilized final cover system subgrade may present challenges during closure; however, these challenges are common to most CCR surface impoundment closures and are commonly addressed <i>via</i> surface water management and dewatering techniques.</p>	<p>Relative to CIP, CBR-Offsite poses additional implementation difficulties due to higher earthwork volumes, higher dewatering volumes, longer construction schedules, and the need to remove and dispose of the existing bottom liner geomembrane.</p> <p>Hauling would be more difficult to implement under the CBR-Offsite scenario than under the CIP scenario, due to the longer haul distance required and the need to use public roads for hauling. Because the CCR would be hauled on public roads, it would require haul trucks with a smaller capacity (16.5 cubic yards <i>versus</i> 34 cubic yards) and would also need to be dewatered to a greater extent than would be necessary under the CIP scenario. Off-Site landfilling would additionally require the development of a disposal plan and could raise issues related to the co-disposal of CCR and other non-hazardous wastes. The off-Site landfill may also need to be expanded to receive all of the CCR generated during excavation.</p>	<p>CBR-Onsite poses several implementation difficulties due to earthwork volumes, dewatering volumes, construction schedules, and the need to remove and dispose of the existing bottom liner geomembrane.</p>
<ul style="list-style-type: none"> <i>Expected Operational Reliability</i> 	Operational reliability would be expected under all closure scenarios.	Operational reliability would be expected under all closure scenarios.	Operational reliability would be expected under all closure scenarios.
<ul style="list-style-type: none"> <i>Need for Permits and Approvals</i> 	Permits required under all closure scenarios would include a modification to the existing NPDES permit; a construction permit from the IDNR Dam Safety Program to allow the embankment and spillways of the GMF GSP to be modified as part of closure; a construction stormwater permit through IEPA; and a joint water pollution control construction and operating permit (WPC permit).	Permits required under all closure scenarios would include a modification to the existing NPDES permit; a construction permit from the IDNR Dam Safety Program to allow the embankment and spillways of the GMF GSP to be modified as part of closure; a construction stormwater permit through IEPA; and a WPC permit. Additional permits and approvals may be required under this scenario if the off-Site landfill must be expanded to receive all of the CCR from the GMF GSP.	Permits required under all closure scenarios would include a modification to the existing NPDES permit; a construction permit from the IDNR Dam Safety Program to allow the embankment and spillways of the GMF RP to be modified as part of closure; a construction stormwater permit through IEPA; and a WPC permit.
<ul style="list-style-type: none"> <i>Availability of Equipment and Specialists</i> 	CIP and CBR rely on common construction equipment and materials and typically do not require the use of specialists. However, global supply chains have been disrupted due to the COVID-19 pandemic, resulting in shortages in the availability of construction equipment and parts. There may be delays in construction under all scenarios if supply chain resilience does not improve by the time of construction. Due to smaller earthwork volumes and a lesser need for construction equipment under the CIP scenario than under the CBR scenarios, shortages may cause fewer challenges under the CIP scenario than under the CBR scenarios.	CIP and CBR rely on common construction equipment and materials and typically do not require the use of specialists. However, global supply chains have been disrupted due to the COVID-19 pandemic, resulting in shortages in the availability of construction equipment and parts. There may be delays in construction under all scenarios if supply chain resilience does not improve by the time of construction. Due to higher earthwork volumes and a greater need for construction equipment under the CBR scenarios than under the CIP scenario, shortages may cause greater challenges under the CBR scenarios than under the CIP scenario. The current shortage of truck drivers may be particularly impactful under the CBR-Offsite scenario, due to the large volumes of borrow soil required and CCR to be hauled to and from the Site.	CBR relies on common construction equipment and materials and typically do not require the use of specialists. However, global supply chains have been disrupted due to the COVID-19 pandemic, resulting in shortages in the availability of construction equipment and parts. There may be delays in construction under all scenarios if supply chain resilience does not improve by the time of construction.

Evaluation Factor (Report Section; IAC Part 845 Section)	Closure Scenario		
	CIP (GMF GSP)	CBR-Offsite (GMF GSP)	CBR-Onsite (GMF RP)
<ul style="list-style-type: none"> Available Capacity and Location of Treatment, Storage, and Disposal Services 	<p>Under the CIP scenario, all of the CCR currently within the GMF GSP would be stored within the existing footprint of the impoundment. Treatment would consist of unwatering the GMF GSP at the start of construction, performing limited dewatering to stabilize the CCR subgrade, and managing stormwater inflow.</p> <p>Water from unwatering and dewatering of the GMF GSP would be discharged in accordance with the NPDES permit for the facility.</p>	<p>The capacity remaining at the chosen off-Site landfill in Litchfield, Illinois, would be sufficient to receive all of the CCR and liner materials in the GMF GSP. However, due to the relatively short period over which CCR would be received at the landfill, vertical and/or lateral expansions may become necessary. Additionally, the landfill operators may need to develop a disposal plan to account for the increased volume of material that would be received and the unique CCR waste characteristics. If expansion of the chosen off-Site landfill were found to be impractical or infeasible, then an alternative landfill located farther from the Site would need to be identified. A likely alternative to the Litchfield-Hillsboro Landfill is the Five Oaks Landfill in Taylorville, Illinois.</p> <p>Water from unwatering and dewatering of the GMF GSP would be discharged in accordance with the NPDES permit for the facility.</p>	<p>The capacity remaining at the on-Site landfill would be sufficient to receive all of the CCR and liner materials in the GMF RP.</p> <p>Water from unwatering and dewatering of the GMF RP would be discharged in accordance with the NPDES permit for the facility.</p>
Impact of Alternative on Waters of the State (Section 2.5, IAC Section 845.710(d)(4))	No current or future exceedances of any screening benchmarks for surface water would be expected under any closure scenario.	No current or future exceedances of any screening benchmarks for surface water would be expected under any closure scenario.	No current or future exceedances of any screening benchmarks for surface water would be expected under any closure scenario.
Potential Modes of Transportation Associated with CBR (Section 2.1; IAC Section 845.710(c)(1))	This factor is not relevant for CIP.	IAC Section 845.710(c)(1) requires CBR alternatives to consider multiple methods for transporting CCR off-Site, including <i>via</i> rail, barge, and trucks. Golder evaluated the feasibility of transporting CCR to the off-Site landfill <i>via</i> rail or barge and found that neither option is viable at this Site. Truck transport has been identified as the preferred option for transport of CCR to the off-Site landfill. The local availability and use of natural gas-powered trucks, or other low-polluting trucks, will be evaluated prior to the start of construction.	This factor is not relevant for CBR-Onsite.
Concerns of Residents Associated with Alternatives (Section 2.6, IAC Section 845.710(b)(4))	<p>Despite the preference for CBR that has been expressed by nonprofits representing community interests near the Site, CIP would effectively address residents' concerns regarding potential impacts to groundwater and surface water quality at the Site. Relative to CBR-Offsite, CIP also presents less risks to nearby residents and potentially EJ communities in the form of accidents, traffic, noise, and air pollution.</p> <p>A public meeting was held on June 14, 2022, pursuant to requirements under IAC Section 845.710(e). Questions raised by attendees were addressed at the meeting; subsequently, a written summary of the questions and responses was prepared.</p>	<p>Nonprofits representing community interests near the Site have expressed a preference for CBR over CIP. However, the CBR-Offsite scenario has several disadvantages with regard to potential community concerns. Relative to CIP, the CBR-Offsite scenario presents greater risks to nearby residents and potentially EJ communities in the form of accidents, traffic, noise, and air pollution.</p> <p>A public meeting was held on June 14, 2022, pursuant to requirements under IAC Section 845.710(e). Questions raised by attendees were addressed at the meeting; subsequently, a written summary of the questions and responses was prepared.</p>	<p>CBR-Onsite would effectively address residents' concerns regarding potential impacts to groundwater and surface water quality at the Site.</p> <p>A public meeting was held on June 14, 2022, pursuant to requirements under IAC Section 845.710(e). Questions raised by attendees were addressed at the meeting; subsequently, a written summary of the questions and responses was prepared.</p>

Notes:

CBR = Closure-by-Removal; CBR-Offsite = Closure-by-Removal with Off-Site CCR Disposal; CBR-Onsite = Closure-by-Removal with On-Site CCR Disposal; CCR = Coal Combustion Residual; CIP = Closure-in-Place; EJ = Environmental Justice; GHG = Greenhouse Gas; GMF = Gypsum Manufacturing Facility; GSP = Gypsum Stack Pond; GWPS = Groundwater Protection Standard; IAC = Illinois Administrative Code; IDNR = Illinois Department of Natural Resources; IEPA = Illinois Environmental Protection Agency; NPDES = National Pollutant Discharge Elimination System; RP = Recycle Pond.

1 Introduction

1.1 Site Description and History

1.1.1 Site Location and History

Illinois Power Generating Company's (IPGC) Coffeen Power Plant is an electric power generating facility with coal-fired units located approximately two miles south of Coffeen, Illinois, and is situated between two lobes of Coffeen Lake. Historically, three room and pillar coal mines operated within the boundaries of the Site. From north to south, they are the Clover Leaf No. 1 Mine, which operated from 1889 to 1901; the Clover Leaf No. 4 Mine, which operated from 1906 to 1924; and the Hillsboro Mine, which operated from 1964 to 1983 (Ramboll, 2021a; ISGS and University of Illinois at Urbana-Champaign, 2011). The Coffeen Power Plant began operating in 1964 and was retired in November 2019 (Ramboll, 2021a,b).

1.1.2 CCR Impoundments

The Coffeen Power Plant produced and stored coal combustion residuals (CCRs) as a part of its historical operations. There are two gypsum management facility (GMF) units that are subject of this report: (1) the GMF Gypsum Stack Pond (GMF GSP); Vistra ID No. CCR Unit 103, Illinois Environmental Protection Agency [IEPA] ID No. W1350150004-03, and National Inventory of Dams [NID] ID No. IL50579), and (2) the GMF Recycle Pond (GMF RP); Vistra ID No. CCR Unit 104, IEPA ID No. W1350150004-04, and NID ID No. IL50578).

The GMF GSP (Figure 1.1) is a 43.3-acre surface impoundment constructed in 2010 to manage synthetic gypsum generated by the wet scrubber system and to clarify recycled process water from the plant (Ramboll, 2021a,b; Appendix B). It operated from 2010 until 2021 (Ramboll, 2021a,b). The GMF GSP received inflow from the Coffeen Power Plant. Clear water discharge from the GMF GSP flowed south to the GMF RP *via* a 580 foot lined transfer channel and a low-flow high-density polyethylene (HDPE) pipe beneath the transfer channel. The GMF GSP has a liner system consisting of a composite 60-millimeter (mil) HDPE geomembrane and a 36-inch layer of compacted clay with internal piping and drains to collect contact water (Ramboll, 2021a,b, Appendix B).

The GMF RP (Figure 1.1) is an 18.3 acre surface impoundment constructed in 2010 and located immediately south of the GMF GSP. Acting as a polishing pond, the GMF RP received decanted water from the GMF GSP (Ramboll, 2021a,b; Appendix B). It operated from 2010 until 2021 (Ramboll, 2021a,b). Outflow from the GMF RP was pumped back to the Coffeen Power Plant for use in the wet scrubber system (Appendix B). The GMF RP has an emergency spillway that discharges to the Unnamed Tributary *via* a National Pollutant Discharge Elimination System (NPDES)-permitted outfall. The GMF RP has a liner system consisting of a composite 60-mil textured HDPE geomembrane installed over smooth drum-rolled native soil with internal piping and drains to collect contact water (Ramboll, 2021a,b; Appendix B).

There are three former room and pillar coal mines within 1,000 meters of the GMF GSP and the GMF RP. From north to south, they are Clover Leaf No. 1 Mine (ISGS Mine No. 3001), Clover Leaf No. 4 Mine (ISGS Mine No. 442), and Hillsboro Mine (ISGS Mine No. 871). The GMF GSP overlies the

southernmost portion of the Clover Leaf No. 4 Mine. The GMF RP does not directly overlie any of the former mines on the Site (Ramboll, 2021a,b).

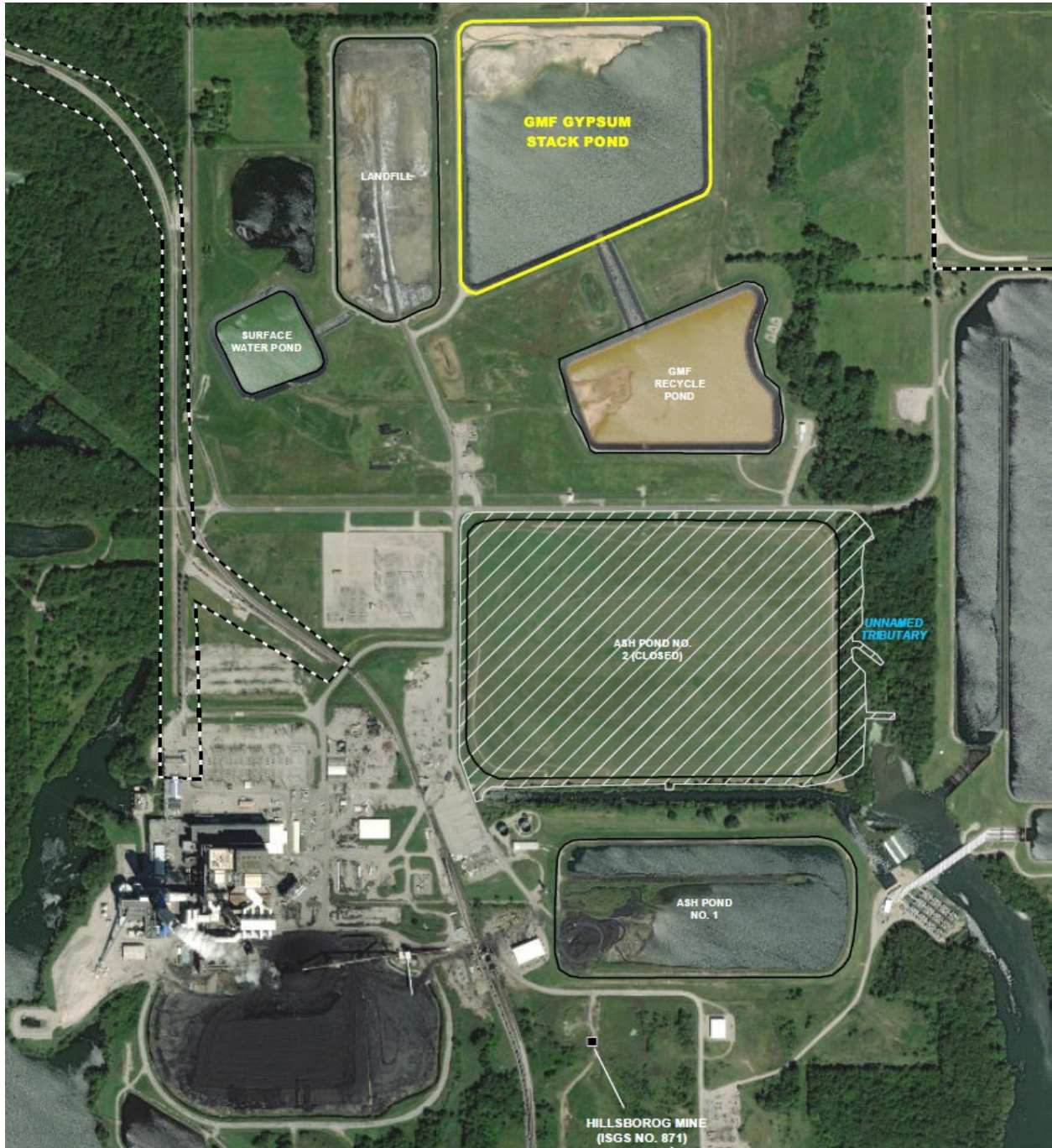


Figure 1.1 Site Location Map. Adapted from Ramboll (2021a).

1.1.3 Surface Water Hydrology

Coffeen Lake has two lobes that border the Coffeen Power Plant on the west, south, and part of the eastern Site boundary. East of the Site, the Unnamed Tributary flows into the eastern lobe of Coffeen

Lake. The facility is permitted to discharge to Coffeen Lake under NPDES Permit No. IL 0000108 and an emergency spillway is located in the northeast corner of the GMF RP (Ramboll, 2021b).

The GMF GSP and the GMF RP are located within the Shoal Creek Watershed (Hydrologic Unit Code [HUC] 07140203) and the outer perimeters of the impoundments are located 400 feet and 150 feet west of the Unnamed Tributary, respectively (Ramboll, 2021a,b). Within 1,000 meters of the GMF GSP and the GMF RP, there are several unnamed freshwater ponds and wetlands (Figure 1.2). The ponds range in size from 0.1 acre to 0.8 acre. There is an approximately 0.6 acre freshwater forested/shrub wetland located northwest of the GMF GSP and a 1.6 acre freshwater emergent wetland located to the southeast of the GMF RP where the Unnamed Tributary enters Coffeen Lake (Ramboll, 2021a,b).

The 1,100-acre Coffeen Lake was built by damming the McDavid Branch of the East Fork of Shoal Creek to aid with cooling for the facility (Ramboll, 2021a,b). The IEPA classifies Coffeen Lake as a General Use Water (IEPA, 2007): it is designated for aquatic life and use in primary contact recreation; however, it is not designated for use in food processing or as a public water supply. Coffeen Lake (Assessment Unit ID IL_ROG) is listed on the 2018 Illinois Section 303(d) List as being impaired for fish consumption due to mercury (IEPA, 2019a; US EPA, 2022). In addition, US EPA approved in 2007 a Total Maximum Daily Load (TMDL) for phosphorus to address aesthetic quality impairments in Coffeen Lake due to excess algae and total suspended solids (IEPA, 2007).

Surface water samples were collected from six locations in Coffeen Lake south of the GMF GSP and the GMF RP in August 2021 (Geosyntec Consultants, 2021). These data are summarized in Gradient's Human Health and Ecological Risk Assessment for the Site, which is provided as Appendix A of this report.

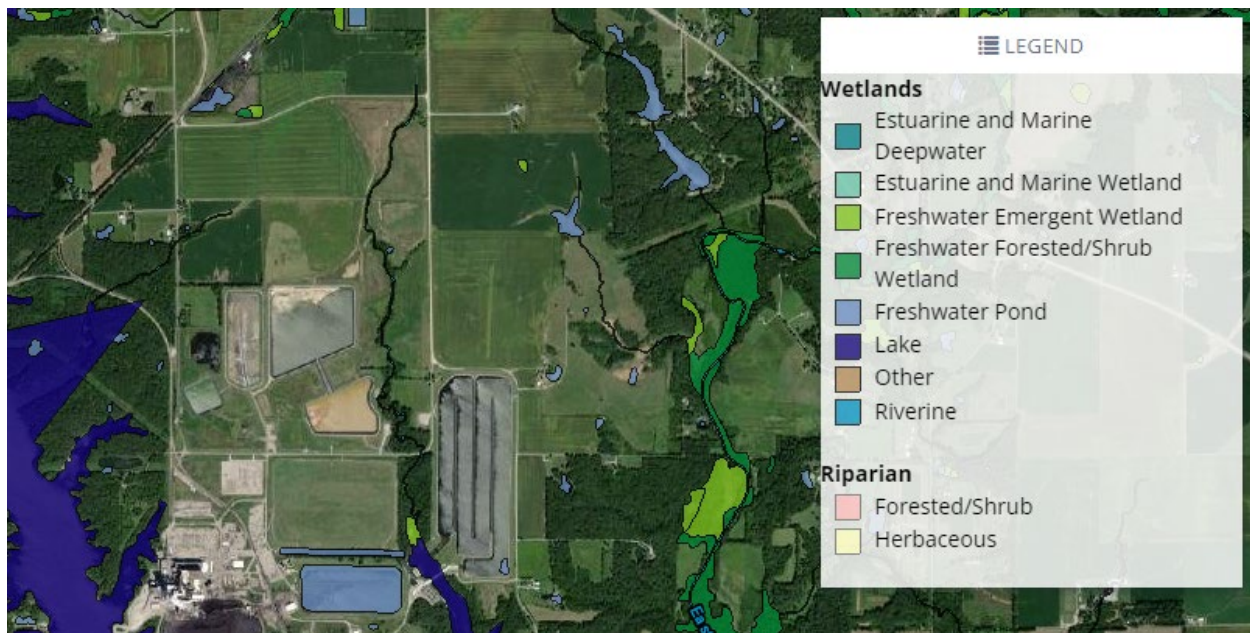


Figure 1.2 Wetlands and Surface Water Bodies in the Vicinity of the Coffeen Power Plant Gypsum Management Facility Gypsum Stack Pond and Recycle Pond. Adapted from US FWS (2021).

1.1.4 Hydrogeology

The geology underlying the Site in the vicinity of the GMF GSP and the GMF RP consists of several distinct layers (Ramboll, 2021a,b).

- Upper Confining Unit (UCU): The UCU underlies the GMF GSP and the GMF RP. It consists of the Loess Unit and the upper portion of the Hagarstown Member which has low permeability clays and silts with generally greater than 60 percent fines. The UCU was encountered across most of the Coffeen Power Plant Site, except near the Unnamed Tributary, where the unit was eroded where it has been excavated for construction.
- Uppermost Aquifer (UA): The UA is comprised of moderately permeable sands, silty sand, and clayey gravel of the Hagarstown Member and, in some portions of the Site, the Vandalia Member. The UA unit is missing in several locations due to both excavation and weathering.
- Lower Confining Unit (LCU): The LCU underlies the UA. It consists of three low hydraulic conductivity soils: the sandy clay till of the Vandalia Member, the silt of the Mulberry Grove Formation, and the compacted clay till of the Smithboro Member.
- Deep Aquifer (DA): The DA is a thin (generally less than 5 feet thick), discontinuous unit composed of sands and silty sands.
- Deep Confining Unit (DCU): The DCU underlies the DA. It consists of the Lierle Clay of the Banner Formation and acts as an aquitard due to its low hydraulic conductivity.

The Hydrogeological Site Characterization Reports prepared by Ramboll (2021a,b) provide more details regarding the hydrostratigraphic units in the vicinity of the GMF GSP and the GMF RP.

There is a groundwater flow divide within the UA in the center of the Coffeen Power Plant property between the two lobes of Coffeen Lake. Groundwater in the UA flows from the center of the Coffeen Power Plant property west toward Coffeen Lake and east toward the Unnamed Tributary. Groundwater predominantly flows east/southeast across the GMF GSP to the Unnamed Tributary; however, the western side of the GMF GSP aligns with the groundwater divide and groundwater in this area flows west toward Coffeen Lake. The GMF RP is located east of the divide, and groundwater flows southeast across the GMF RP. The Unnamed Tributary serves as a regional sink for shallow groundwater discharge and shallow groundwater migration beneath or beyond the tributary is unlikely (Ramboll, 2021a,b). Groundwater flow within the UA is mostly in the horizontal direction because the UA is underlain by the low-permeability LCU (Ramboll, 2021a,b).

During groundwater interaction with surface water, CCR-related constituents may partition between sediments and the surface water column. It should be noted that many CCR-related constituents occur naturally in sediments and surface water (and can also arise from other industrial sources). As a result, their presence in the sediments and/or surface water of the Coffeen Lake and the Unnamed Tributary does not necessarily signify contributions from the GMF GSP and the GMF RP.

The Hydrogeologic Site Characterization Reports prepared by Ramboll as part of the operating permit for the GMF GSP and the GMF RP include an evaluation of groundwater data collected from GMF GSP and GMF RP monitoring wells between 2015 and 2021 (Ramboll, 2021a,b).

1.1.5 Site Vicinity

The Coffeen Power Plant property is bordered by Coffeen Lake to the west and south, by the Unnamed Tributary and Coffeen Lake to the east, and by agricultural land to the north (Ramboll, 2021a, Figure 1.1). Coal mining operations occurred in the vicinity of the GMF GSP and the GMF RP from 1906 until 1983. Three mines were identified within a 1,000 meter radius of the GMF GSP and the GMF RP. From north to south, they are the Clover Leaf No. 1 Mine (Illinois State Geological Survey [ISGS] Mine No. 3001), which operated from 1889 to 1901; the Clover Leaf No. 4 Mine (ISGS Mine No. 442), which operated from 1906 to 1924; and the Hillsboro Mine (ISGS Mine No. 871), which operated from 1964 to 1983. (Ramboll, 2021a,b; ISGS and University of Illinois at Urbana-Champaign, 2011). The GMF GSP partially overlies the southernmost extent of the Clover Leaf No. 4 Mine. The GMF RP does not directly overlie any of the former mines.

Although the area surrounding the Coffeen Power Plant is predominantly agricultural, Coffeen Lake and the surrounding land are used for recreational activities. Since 1986, Coffeen Lake State Fish and Wildlife Area (SFWA) has been open to the public under a lease and management agreement between the Illinois Department of Natural Resources (IDNR) and Ameren Energy Generating Company (IDNR, 1999). To the north of the Coffeen Power Plant, there are walking and hiking trails and bank fishing. Coffeen Lake also entertains fishing and picnicking on the western shore. Based on a review of the IDNR Historic Preservation Division database and the Illinois State Archaeological Survey database, there are no historic sites located within 1,000 meters of the GMF GSP and the GMF RP (Ramboll, 2021a,b).

1.2 IAC Part 845 Regulatory Review and Requirements

Title 35, Part 845 of the Illinois Administrative Code (IAC; IEPA, 2021a) requires the development of a Closure Alternatives Analysis (CAA) prior to undertaking closure activities at certain CCR-containing surface impoundments in the State of Illinois. Section 2 of this report presents a CAA for the GMF GSP and GMF RP pursuant to requirements under IAC Section 845.710. The goal of a CAA is to holistically evaluate each potential closure scenario with respect to a wide range of factors, including the efficiency, reliability, and ease of implementation of the closure scenario; its potential positive and negative short- and long-term impacts on human health and the environment; and its ability to address concerns raised by residents (IEPA, 2021a). A CAA is a decision-making tool that is designed to aid in the selection of an optimal closure alternative for the impoundments at a site.

2 Closure Alternatives Analysis

2.1 Closure Alternative Descriptions (IAC Section 845.710(c))

This section of the report presents a CAA for the GMF GSP and the GMF RP pursuant to requirements under IAC Section 845.710 (IEPA, 2021a). For the GMF GSP, Gradient evaluated two specific closure scenarios: Closure-in-Place (CIP) and Closure-by-Removal with off-Site disposal (CBR-Offsite). The CIP scenario entails consolidating CCR into the northern portion of the GMF GSP and capping it with a new cover system consisting of, from bottom to top, a geomembrane layer, a geotextile cushion if needed, and 24 inches of vegetated soil. The CBR-Offsite scenario entails excavating all of the CCR and liner system materials from the GMF GSP and transporting it to an off-Site landfill for disposal. For the GMF RP, Gradient evaluated one closure scenario: Closure-by-Removal with on-Site disposal (CBR-Onsite). The CBR-Onsite scenario entails excavating the CCR and liner system materials from the GMF RP and transporting these materials to an on-Site landfill for disposal. IPGC will also continue to evaluate potential opportunities for beneficial re-use of CCR excavated from the GMF GSP and GMF RP as an alternative to disposal.

IAC Section 845.710(c)(2) requires CAAs to, "[i]dentify whether the facility has an onsite landfill with remaining capacity that can legally accept CCR, and, if not, whether constructing an onsite landfill is possible" (IEPA, 2021a). There is an existing on-Site landfill at the Coffeen Power Plant Site with some remaining capacity to accept CCR. It has enough capacity to contain all of the material that would be removed from the GMF RP, but it does not have enough capacity to also contain the material that would be removed from the GMF GSP. Due to the planned redevelopment of the Site as a utility-scale solar energy generation and battery storage facility, there is not sufficient space available to expand the existing landfill. Thus, for the CBR scenarios evaluated in this CAA, Gradient assumed that the on-Site landfill would be used for disposal of CCR from the GMF RP, but CCR from the GMF GSP would be disposed in an off-Site landfill.

Sections 2.1.1 and 2.1.2 provide detailed descriptions of the CIP and CBR-Offsite closure scenarios for the GMF GSP, and Section 2.1.3 provides a detailed description of the CBR-Onsite closure scenario for the GMF RP. These scenarios are based on closure documents and analyses provided to Gradient by Golder, which are attached to this report as Appendix B.

2.1.1 GMF GSP Closure-in-Place

Under the CIP scenario, CCR would be excavated from the southern portion of the GMF GSP and consolidated into the northern portion, then capped in place with a final cover system. This scenario includes the following work elements (Golder Associates USA Inc., 2022a):

- Unwatering and dewatering of the impoundment *via* pumping and the construction of drilled sumps, engineered trenches, and/or horizontal wells. In addition to pumping ponded water, perimeter drains would also be pumped to lower the groundwater level beneath the GMF GSP, if needed. The CCR would dewater to some degree by gravity, but pumping from trenches and sumps may also be necessary. Water would be pumped to the existing drainage to the east of the GMF GSP and managed in accordance with the NPDES permit for the facility.

- Excavation of CCR from the southern portion of the GMF GSP and relocation of it into the northern portion of the GMF GSP.
- Construction of a berm with a composite liner system consisting of a 60-mil HDPE geomembrane overlying a compacted clay layer that would be oriented east-west on the south end of the consolidated footprint.
- Removal and disposal of the existing liner system, components of the process water recovery system, and subsoil with CCR staining (up to 1 foot) within the consolidated footprint.
- Placement of compacted fill as needed to achieve final cover subgrade.
- Construction of an alternative cover system consisting of a 40-mil linear low-density polyethylene (LLDPE) geomembrane layer, a nonwoven geotextile cushion, and 24 inches of protective soil cover suitable for supporting vegetative growth. An alternative cover performance demonstration will be submitted to IEPA for approval pursuant to Section 845.750(c)(2) (Geosyntec Consultants, 2022).
- Removal of existing earthen embankments not required for the consolidated footprint and excavation of a channel to allow stormwater to flow off Site in accordance with the NPDES permit for the facility.
- Contouring and grading to manage stormwater.
- Site restoration including placement of vegetative cover on the final surface of the GMF GSP. Implementation of stormwater best management practices (BMPs) to reduce erosion, such as the use of erosion control blankets.
- Long-term (post-closure) monitoring and maintenance, including at least 30 years of groundwater monitoring at the impoundment, or until such time as groundwater protection standards (GWPSs) are achieved. Additionally, 30 years of post-closure care would be undertaken for the final cover system, including annual cap inspections, mowing, and maintenance.

This CIP plan meets all closure requirements of IAC Section 845.750 (IEPA, 2021a). Key closure elements that address the Part 845 closure requirements are summarized below. Further details are provided in the Closure Plan (Golder Associates USA Inc., 2022a).

- An alternative cover system would be installed over the CCR that remains in the GMF GSP. The cover, consisting of a 40-mil LLDPE geomembrane low-permeability layer, a geotextile cushion if needed, and 24 inches of soil, would minimize vertical infiltration of precipitation into the basin (Section 845.750(a)(1)) (Geosyntec Consultants, 2022).
- The final cover system would be gently sloped to direct surface water away from the impoundment. Beyond the final cover system, channels would direct surface water away from the GMF GSP to existing Site drainages (Section 845.750(a)(2)).
- Impounded water would be removed from the GMF GSP and managed in accordance with the NPDES permit for the facility (Sections 845.750(b)(1) and 845.750(b)(2)).
- Free liquids in the CCR would be eliminated by removing liquid wastes. Engineered trenches would facilitate gravity drainage of liquid wastes in the CCR and direct the liquid wastes to sumps. Other engineering measures, such as drilled sumps and/or horizontal wells, may also be considered to facilitate removal of liquid wastes and stabilization of wastes. Liquid wastes will be managed in accordance with the NPDES permit for the Site (Sections 845.750(b)(1) and 845.750(b)(2)).

- After closure, the GMF GSP would provide complete encapsulation of the CCR, physically isolating it from contact with surrounding soils, groundwater, surface water, and the atmosphere (Golder Associates USA Inc., 2022a). Thus, the proposed CIP design will control, minimize, or eliminate as much as feasible "post-closure infiltration of liquids" and releases of CCR, leachate, or contaminated runoff as interpreted by IEPA in the Part 845 rulemaking.

Furthermore, during the closure process, we will continue to assess off-Site CCR beneficial use opportunities. Ash consolidation and CIP in combination with off-Site beneficial use may result in a smaller footprint for purposes of our ultimate cap design along with a reduced construction schedule.

Under this scenario, approximately 177,000 cubic yards (CY) of CCR and subsoil would be relocated to the northern portion of the GMF GSP (an assumed travel distance of 2,000 feet; Appendix B). Additionally, 271,000 CY of material would be required for contouring and grading of the GMF GSP. Of this material, 229,000 CY would be sourced from an on-Site soil borrow area within 2,000 feet of the GMF GSP. Construction of the final cover system would require an additional 42,100 CY of soil to be hauled from a second on-Site borrow area within 2 miles of the GMF GSP (Appendix B). Haul truck capacities are assumed to be 34 CY for the on-Site transport of borrow soil (Appendix B).

Under the CIP (GMF GSP) scenario, the overall expected duration of construction and earthwork activities (including closure of the impoundment and Site restoration) is approximately 22-33 months (1.8-2.8 years; Golder Associates USA Inc., 2022a). The total expected number of on-Site workdays is 758 (Appendix B). The CIP (GMF GSP) scenario will meet the required closure schedule (*i.e.*, closure completed by October 2028) defined in IAC Section 845.700(d)(2)(C)(ii) (IEPA, 2021a). Key parameters for the CIP (GMF GSP) scenario are shown in Table 2.1.

Table 2.1 Key Parameters for the GMF GSP Closure-in-Place Scenario

Parameter	Value
Surface Area of GMF GSP	43.3 acres
Surface Area of Final Cover System	12.4 acres
Average Travel Distance for Relocation of CCR	2,000 feet
Hauled Volume of CCR and Liner to be Relocated	177,000 CY
Average Distance to On-Site Borrow Soil Location	2,000 feet
Hauled Volume of Soil from On-Site Borrow Location	229,000 CY
Average Distance to Second On-Site Borrow Soil Location	2 miles
Hauled Volume of Soil from Second On-Site Borrow Location	42,100 CY
Duration of Construction Activities	758 days
Labor Hours	
Total On-Site Labor	36,200 hours
Total Off-Site Labor	4,660 hours
30% Contingency	12,200 hours
Total Labor Hours:	53,100 hours
Vehicle and Equipment Travel Miles	
Vehicles On-Site	13,400 miles
Equipment On-Site	56,900 miles
On-Site Haul Trucks (Unloaded + Loaded)	14,000 miles
Labor Mobilization	319,000 miles
Equipment Mobilization (Unloaded + Loaded)	65,000 miles
Off-Site Haul Trucks (Unloaded + Loaded)	0 miles
Material Deliveries (Unloaded + Loaded)	13,900 miles
Total On-Site Vehicle and Equipment Travel:	84,200 miles
Total Off-Site Vehicle and Equipment Travel:	397,000 miles
Total Vehicle and Equipment Travel:	482,000 miles

Notes:

CCR = Coal Combustion Residual; CY = Cubic Yards; GMF GSP = Gypsum Management Facility Gypsum Stack Pond.

Hauled volumes of CCR and soil are 5% greater than "in-place" volumes.

Due to rounding, totals may not match the sum of the values.

Source: Appendix B.

2.1.2 GMF GSP Closure-by-Removal with Off-Site CCR Disposal

Under the CBR-Offsite (GMF GSP) scenario, all CCR and liner system materials would be excavated from the GMF GSP and transported to an off-Site landfill for disposal. Evaluation of landfill capacity and permitted use must be taken into consideration for each landfill considered for off-Site disposal. For example, a municipal landfill is often designed and permitted to accept waste from the local community at a specific rate. The landfill owner relies on this information to determine the remaining life of a landfill and determine when it will be necessary to expand or close the landfill. Due to the lengthy permitting and construction process, a landfill would need to continue accepting current waste streams and ash for a significant period of time to be a viable option, assuming the landfill owner and state approve. Furthermore, given the volume of ash that would need to be transported, it is important to evaluate impacts to communities that will be affected by the increase in truck traffic to and from the landfill. The nearest operating landfill to meet these criteria is Republic Services' Litchfield-Hillsboro Landfill in Litchfield, Illinois, which is located approximately 18 miles from the Site (Appendix B). CCR would be hauled to the off-Site landfill using haul trucks with a capacity of 16.5 CY, a smaller capacity than that of the haul trucks that would haul CCR to the consolidated footprint under the CIP (GMF GSP) scenario (34 CY) due to restrictions placed on the size of trucks that can be used on public roadways. As is

described below in Section 2.4.5, it is possible that the Litchfield-Hillsboro Landfill would have to be expanded in order to accept all of the material excavated from the GMF GSP.

IAC Section 845.710(c)(1) requires CBR alternatives to consider multiple methods for transporting CCR off-Site, including *via* rail, barge, and trucks. Golder evaluated the feasibility of transporting CCR to the off-Site landfill *via* rail or barge and found that neither option is viable at this Site (Appendix B). Transporting CCR by rail would require the construction of a new rail loading terminal on-Site and the construction of a new rail unloading terminal near the off-Site landfill. The construction of new rail terminals would require coordination with the railroad and additional permitting, which could negatively impact the project schedule. Trucks would still be needed to haul CCR to and from the terminals, and additional CCR exposures could occur during the loading and unloading of CCR into trucks and rail cars. Moreover, because there is no direct rail route from the Site to the off-Site landfill, the transport of CCR to the off-Site landfill would require 25 miles of rail transport on tracks owned by three separate rail lines.

The Coffeen Power Plant is not located near a navigable waterway; thus, transportation of CCR by barge is not feasible. For these reasons, truck transport has been identified as the preferred option for transport of CCR to the off-Site landfill. Transport *via* truck would not require the construction of additional loading or unloading infrastructure and would not result in project delays due to permitting and coordination with other parties. The existing travel routes from the Site to the off-Site landfill are suitable for CCR transport *via* truck (Appendix B). The local availability and use of natural gas-powered trucks, or other low-polluting trucks, will be evaluated prior to the start of construction.

This scenario includes the following work elements (Appendix B):

- Unwatering and dewatering of the impoundment *via* pumping and passive dewatering methods. In addition to pumping ponded water, perimeter drains would also be pumped to lower the groundwater level beneath the GMF GSP, if needed. The CCR would dewater to some degree by gravity, but pumping from trenches and sumps may also be necessary. Water would be pumped to the existing drainage to the east of the GMF GSP and managed in accordance with the NPDES permit for the facility.
- Excavation of CCR from the impoundment and transport of these materials to the off-Site landfill.
- Removal and disposal of the existing liner system, components of the process water recovery system, and subsoil with CCR staining (up to 1 foot) in the off-Site landfill.
- Removal of existing earthen embankments and excavation of a channel to allow stormwater to flow off Site in accordance with the NPDES permit for the facility.
- Contouring and grading to manage stormwater.
- Site restoration, including the placement of protective cover soil along the side slopes and bottom of the GMF GSP and revegetation with native grasses.
- Monitoring for 3 years post-closure or until such time as GWPSs are achieved, whichever is longer.

Under this scenario, approximately 556,000 CY of CCR and subsoil would be relocated to the off-Site landfill (an assumed travel distance of 18 miles; Appendix B). Additionally, 377,500 CY of material would be required for contouring and grading of the GMF GSP and Site restoration. Of this material, 256,000 CY would be sourced from an on-Site soil borrow area within 2,000 feet of the GMF GSP, 95,600 CY of soil would be sourced from an on-Site borrow area within 2 miles of the GMF GSP, and

25,900 CY of soil would be sourced from an off-Site borrow area within 2 miles of the Site. It is expected that a suitable off-Site borrow location can be identified within 2 miles of the Site (Appendix B). Haul truck capacities are assumed to be 34 CY for the on-Site transport of borrow soil and 16.5 CY for the off-Site transport of borrow soil (Appendix B).

Under the CBR-Offsite (GMF GSP) scenario, the overall expected duration of construction and earthwork activities (including closure of the impoundment and Site restoration) is approximately 31-46 months (2.6-3.8 years). The total expected number of on-Site workdays is 1,160 (Appendix B). The CBR-Offsite (GMF GSP) scenario will meet the required closure schedule (*i.e.*, closure completed by October 2028) defined in IAC Section 845.700(d)(2)(C)(ii) (IEPA, 2021a). Key parameters for the CBR-Offsite (GMF GSP) scenario are shown in Table 2.2.

Table 2.2 Key Parameters for the GMF GSP Closure-by-Removal with Off-Site CCR Disposal Scenario

Parameter	Value
Surface Area of GMF GSP	43.3 acres
Distance to the Off-Site Landfill	18 miles
Hauled Volume of CCR and Liner to Off-Site Landfill	556,000 CY
Distance from the GMF GSP to the On-Site Borrow Location	2,000 feet
Hauled Volume of Borrow Soil from On-Site Borrow Location	256,000 CY
Distance from the GMF GSP to the Second On-Site Borrow Location	2 miles
Hauled Volume of Borrow Soil from the Second On-Site Borrow Location	95,600 CY
Distance from the GMF GSP to the Off-Site Borrow Location	2 miles
Hauled Volume of Borrow Soil from Off-Site Borrow Location	25,900 CY
Duration of Construction Activities	1,160 days
Labor Hours	
Total On-Site Labor	22,700 hours
Total Off-Site Labor	53,600 hours
30% Contingency	22,900 hours
Total Labor Hours:	99,300 hours
Vehicle and Equipment Travel Miles	
Vehicles On-Site	21,700 miles
Equipment On-Site	104,000 miles
On-Site Haul Trucks (Unloaded + Loaded)	17,000 miles
Labor Mobilization	569,000 miles
Equipment Mobilization (Unloaded + Loaded)	99,500 miles
Off-Site Haul Trucks (Unloaded + Loaded)	1,210,000 miles
Material Deliveries (Unloaded + Loaded)	9,000 miles
Total On-Site Vehicle and Equipment Travel:	143,000 miles
Total Off-Site Vehicle and Equipment Travel:	1,890,000 miles
Total Vehicle and Equipment Travel:	2,030,000 miles

Notes:

CCR = Coal Combustion Residual; CY = Cubic Yard; GMF GSP = Gypsum Management Facility Gypsum Stack Pond.

Hauled volumes of CCR and soil are 5% greater than "in-place" volumes.

Due to rounding, totals may not match the sum of the values.

Source: Appendix B.

2.1.3 GMF RP Closure-by-Removal with On-Site CCR Disposal

Under the CBR-Onsite (GMF RP) scenario, all CCR and existing liner system materials would be excavated from the GMF RP and transported to the on-Site landfill for disposal (Appendix B).

This scenario includes the following work elements (Golder Associates USA Inc., 2022b):

- Unwatering and dewatering of the impoundment *via* pumping and passive dewatering methods. The CCR would dewater to some degree by gravity, but pumping from trenches and sumps may also be necessary. Water would be pumped to the existing drainage to the east of the GMF RP and managed in accordance with the NPDES permit for the facility.
- Excavation of CCR, the existing liner system, and subsoil with CCR staining (up to 1 foot) from the impoundment and disposal in the on-Site landfill.
- Removal of existing earthen embankments and excavation of a channel to allow stormwater to flow off Site in accordance with the NPDES permit for the facility.
- Contouring and grading to manage stormwater.
- Site restoration including placement of vegetative cover on the final surface of the GMF RP. Implementation of stormwater BMPs to reduce erosion, such as the use of erosion control blankets.
- Monitoring for 3 years post-closure or until such time as GWPSs are achieved, whichever is longer.

For the CBR-Onsite (GMF RP) closure scenario, approximately 83,000 CY of CCR and subsoil would be relocated to the on-Site landfill (an assumed travel distance of 2,000 feet; Appendix B). Additionally, 153,000 CY of material would be required for contouring and grading of the GMF RP and Site restoration. This material would be sourced from an on-Site soil borrow area within 2,000 feet of the GMF RP. Haul truck capacities are assumed to be 16.5 CY for the on-Site transport of borrow soil (Appendix B).

The on-Site landfill currently has approximately 375,500 CY of available capacity. Thus, the on-Site landfill has sufficient capacity to receive all of the CCR and liner materials from the GMF RP that are slated for disposal. This scenario meets the requirements of IAC Section 845.710(c)(2) (IEPA, 2021a), which requires an assessment be included in the CAA of whether the Site has an on-Site landfill with available capacity or whether an on-Site landfill can be constructed.

For the CBR-Onsite (GMF RP) closure scenario, the overall expected duration of construction and earthwork activities (including closure of the impoundment and Site restoration) is approximately 12-17 months (1.0-1.4 years; Golder Associates USA Inc., 2022b). The total expected number of on-Site workdays is 389 (Appendix B). Key parameters for the CBR-Onsite (GMF RP) scenario are shown in Table 2.3.

Table 2.3 Key Parameters for the GMF RP Closure-by-Removal with On-Site CCR Disposal Scenario

Parameter	Value
Surface Area of GMF RP	18.3 acres
Distance to the On-Site Landfill	2,000 feet
Hauled Volume of CCR and Liner	83,000 CY
Distance to the Borrow Site	2,000 feet
Hauled Volume of Borrow Soil	153,000 CY
Duration of Construction Activities	389 days
Labor Hours	
Total On-Site Labor	11,800 hours
Total Off-Site Labor	2,320 hours
30% Contingency	4,240 hours
Total Labor Hours:	18,400 hours
Vehicle and Equipment Travel Miles	
Vehicles On-Site	5,990 miles
Equipment On-Site	17,500 miles
On-Site Haul Trucks (Unloaded + Loaded)	5,250 miles
Labor Mobilization	109,000 miles
Equipment Mobilization (Unloaded + Loaded)	33,400 miles
Off-Site Haul Trucks (Unloaded + Loaded)	0 miles
Material Deliveries (Unloaded + Loaded)	4,400 miles
Total On-Site Vehicle and Equipment Travel:	28,700 miles
Total Off-Site Vehicle and Equipment Travel:	147,000 miles
Total Vehicle and Equipment Travel:	175,000 miles

Notes:

CCR = Coal Combustion Residual; CY = Cubic Yard; GMF RP = Gypsum Management Facility Recycle Pond.

Hauled volumes of CCR and soil are 5% greater than "in-place" volumes.

Due to rounding, totals may not match the sum of the values.

Source: Appendix B.

2.2 Long- and Short-Term Effectiveness of the Closure Alternative (IAC Section 845.710(b)(1))

2.2.1 Magnitude of Reduction of Existing Risks (IAC Section 845.710(b)(1)(A))

This section of the report addresses the potential risks to human and ecological receptors due to exposure to CCR-associated constituents in groundwater or surface water. Gradient has performed a Human Health and Ecological Risk Assessment for the Site (Appendix A of this report), which provides a detailed evaluation of the magnitude of existing risks to human and ecological receptors associated with the GMF GSP and the GMF RP. This report concluded that there are no current unacceptable risks to any human or ecological receptors associated with the GMF GSP and the GMF RP. Because there are no current risks to any human or ecological receptors, and dissolved constituent concentrations would be expected to decline post-closure, no post-closure risks would be expected under any closure scenario. Thus, there would be no current risk or future risk under any closure scenario, and the magnitude of reduction of existing risks would be the same under each closure scenario (CIP [GMF GSP], CBR-Offsite [GMF GSP], and CBR-Onsite [GMF RP]).

2.2.2 Likelihood of Future Releases of CCR (IAC Section 845.710(b)(1)(B))

This section of the report quantifies the risk of future releases of CCR that may occur during dike failure and storm-related events.

Storm-Related Releases and Dike Failure During Flood Conditions

Based on the effective Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map for the Site, the GMF GSP and the GMF RP are not located within the 100-year flood zone for Coffeen Lake and the Unnamed Tributary (FEMA, 1981; Ramboll, 2021a,b). Engineering analyses show that the risk of overtopping occurring during flood conditions is also minimal under current conditions. Specifically, AECOM and Hanson Professional Services (Hanson) evaluated the risk of flood overtopping occurring at the GMF GSP and the GMF RP, respectively, and found that the impoundments can adequately manage flow during peak discharge periods from the probable maximum flood (PMF) storm event, thus preventing overtopping (AECOM, 2016a; Hanson Professional Services Inc., 2016a). Additionally, engineering analyses show that the GMF GSP and the GMF RP dikes are expected to remain stable under static, seismic, and flood conditions (AECOM, 2016b; Hanson Professional Services Inc., 2016b). Prior to closure (*i.e.*, under current conditions), the risk of dike failure occurring during floods or other storm-related events is therefore minimal. Post-closure, the risks of overtopping and dike failure occurring due to floods or other storm-related events would be even smaller than they are currently. Under the CIP (GMF GSP) scenario, a new cover system would be installed, which would include 24 inches of soil and a geomembrane liner, as well as new stormwater control structures. Relative to current conditions, this cover system would provide increased protection against berm and surface erosion, groundwater infiltration, and other adverse effects that could potentially trigger a dike slope failure event. Under the CBR scenarios, all of the CCR in the GMF GSP and the GMF RP would be excavated and relocated, eliminating the risk of a CCR release occurring post-closure. In summary, there is minimal current or future risk of sudden CCR releases occurring under any closure scenario either during or following closure.

Dike Failure Due to Seismicity

Sites in Illinois may be subject to seismic risks arising from the Wabash Valley Seismic Zone and the New Madrid Seismic Zone (IEMA, 2020). The Coffeen Power Plant property lies within a seismic impact zone (Ramboll, 2021a,b). However, all structural components of the GMF GSP and the GMF RP have been designed to resist the maximum horizontal acceleration in lithified earth material for the Site. The GMF GSP and the GMF RP therefore meet the seismic safety requirements of 40 Code of Federal Regulations (CFR) Section 257.63(a) and IAC Section 845.330, and the overall risk of dike failure due to seismicity is expected to be low (Haley & Aldrich, 2018a,b; Ramboll, 2021a,b). Additionally, the GMF GSP and the GMF RP do not lie within 200 feet of an active fault or fault damage zone at which displacement has occurred within the current geological epoch (*i.e.*, within the last ~11,650 years; Haley & Aldrich, Inc., 2018c,d). The nearest known mapped faults are the Crown Fault, which is located about 31 miles northwest of the Coffeen Power Plant, and the Centralia Fault zone, which is located about 35 miles southeast of the Coffeen Power Plant. These faults do not have known recent activity (Haley & Aldrich, 2018a,b); however, a magnitude 3.8 earthquake occurred approximately 15 miles south of the Site in 1981 and a magnitude 3.6 earthquake occurred approximately 20 miles southeast of the Site in 1990 (Ramboll, 2021a,b). Having met the seismic safety requirements, the risk of dike failure occurring during or following closure activities due to seismic activity is low at the GMF GSP and the GMF RP.

2.2.3 Type and Degree of Long-Term Management, Including Monitoring, Operation, and Maintenance (IAC Section 845.710(b)(1)(C))

The long-term operation and management plans for the GMF GSP and the GMF RP are described in Section 2.1 (Closure Alternatives Descriptions). In summary, under the CIP scenario, the GMF GSP would undergo monitoring for 30 years post-closure, or until such time as GWPSs are achieved. Under the CBR-Offsite scenario for the GMF GSP and the CBR-Onsite scenario for the GMF RP, the GMF GSP and the GMF RP would undergo monitoring for 3 years post-closure, or until such time as GWPSs are achieved. The post-closure care plan for the CIP scenario for the GMF GSP would include annual inspections, mowing, and maintenance of the final cover system.

2.2.4 Short-Term Risks to the Community or the Environment During Implementation of Closure (IAC Section 845.710(b)(1)(D))

This section of the report evaluates the risks to workers, the community, and the environment during closure implementation. Relative comparisons are made between risks associated with the CIP (GMF GSP) scenario and the CBR-Offsite (GMF GSP) scenario. Risks for the CBR-Onsite (GMF RP) scenario are also presented.

2.2.4.1 Worker Risks

Best practices would be employed during construction in order to ensure worker safety and comply with all relevant regulations, permit requirements, and safety plans. However, it is impossible to completely eliminate the risk of accidents occurring during construction activities, both on- and off-Site. On-Site accidents include injuries and deaths arising from the use of heavy equipment and/or earthmoving operations during construction activities. Off-Site accidents include injuries and deaths due to vehicle accidents during labor and equipment mobilization/demobilization, material deliveries, and the hauling of borrow soil and CCR.

As shown in Tables 2.1 through 2.3, Golder estimates that the CIP (GMF GSP) scenario would require 36,200 on-Site labor hours (Appendix B). The CBR-Offsite (GMF GSP) scenario would require approximately 22,700 on-Site labor hours, and the CBR-Onsite (GMF RP) scenario would require approximately 11,800 on-Site labor hours. The US Bureau of Labor Statistics (US DOL, 2020a,b) provides an estimate of the hourly fatality and injury rates for construction workers. Based on the accident rates reported by US Bureau of Labor Statistics and the on-Site labor hours reported in Appendix B, we estimate that approximately 0.42 worker injuries and 0.0027 worker fatalities would occur on-Site under the CIP (GMF GSP) scenario; approximately 0.26 worker injuries and 0.0017 worker fatalities would occur on-Site under the CBR-Offsite (GMF GSP) scenario; and approximately 0.14 worker injuries and 0.00088 worker fatalities would occur on-Site under the CBR-Onsite (GMF RP) scenario (Table 2.4). The rate of on-Site worker accidents is therefore expected to be higher under the CIP (GMF GSP) scenario and lower under the CBR-Offsite (GMF GSP) scenario.

Table 2.4 Expected Number of On-Site Worker Accidents Under Each Closure Scenario

Closure Scenario	Injuries	Fatalities
CIP (GMF GSP)	0.42	0.0027
CBR-Offsite (GMF GSP)	0.26	0.0017
CBR-Onsite (GMF RP)	0.14	0.00088

Notes:

CBR-Offsite = Closure-by-Removal with Off-Site CCR Disposal; CBR-Onsite = Closure-by-Removal with On-Site CCR Disposal; CCR = Coal Combustion Residual; CIP = Closure-in-Place; GMF = Gypsum Manufacturing Facility; GSP = Gypsum Stack Pond; RP = Recycle Pond.

Off-Site, a greater number of haul truck miles, labor and equipment mobilization/demobilization miles, and material delivery miles would be required under the CBR-Offsite (GMF GSP) scenario than would be required under the CIP (GMF GSP) scenario (Tables 2.1 through 2.3). For example, under the CBR-Offsite (GMF GSP) scenario, 1,210,000 haul truck miles would be required to haul CCR and borrow soil to and from the Site, and for the CIP (GMF GSP) scenario and the CBR-Onsite (GMF RP) scenario, off-Site hauling is not required (Appendix B). The United States Department of Transportation (US DOT, 2020) provides estimates of the expected number of fatalities and injuries "per vehicle mile driven" for drivers and passengers of large trucks and passenger vehicles. Table 2.5 shows the expected number of off-Site accidents under each closure scenario due to all categories of off-Site vehicle usage. For these calculations, it was assumed that labor mobilization/demobilization would rely on passenger vehicles (cars or light trucks, including pickups, vans, and sport utility vehicles) and that hauling, equipment mobilization/demobilization, and material deliveries would rely on large trucks. Based on US DOT's accident statistics and the mileage estimates in Appendix B, an estimated 0.21 worker injuries and 0.0027 worker fatalities would be expected to occur due to off-Site activities under the CIP (GMF GSP) scenario; an estimated 0.52 worker injuries and 0.0083 worker fatalities would be expected to occur due to off-Site activities under the CBR-Offsite (GMF GSP) scenario; and an estimated 0.072 worker injuries and 0.0010 worker fatalities would be expected to occur due to off-Site activities under the CBR-Onsite (GMF RP) scenario (Table 2.5).

Table 2.5 Expected Number of Off-Site Worker Accidents Under Each Closure Scenario

Off-Site Vehicle Use Category	CIP (GMF GSP)		CBR-Offsite (GMF GSP)		CBR-Onsite (GMF RP)	
	Injuries	Fatalities	Injuries	Fatalities	Injuries	Fatalities
Hauling	0	0	0.16	0.0035	0	0
Labor Mobilization/Demobilization	0.20	0.0025	0.35	0.0045	0.067	0.00085
Equipment Mobilization/Demobilization	0.0083	0.00019	0.013	0.00029	0.0043	0.00010
Material Deliveries	0.0018	0.000040	0.0012	0.000026	0.00056	0.000013
Total:	0.21	0.0027	0.52	0.0083	0.072	0.0010

Notes:

CBR-Offsite = Closure-by-Removal with Off-Site CCR Disposal; CBR-Onsite = Closure-by-Removal with On-Site CCR Disposal; CCR = Coal Combustion Residual; CIP = Closure-in-Place; GMF = Gypsum Manufacturing Facility; GSP = Gypsum Stack Pond; RP = GMF Recycle Pond.

Overall, taking into account accidents occurring both on- and off-Site, 0.62 worker injuries and 0.0054 worker fatalities would be expected under the CIP (GMF GSP) scenario; 0.78 worker injuries and 0.010 worker fatalities would be expected under the CBR-Offsite (GMF GSP) scenario. Thus, overall risks to workers would be higher under the CBR-Offsite (GMF GSP) scenario and lower under the CIP (GMF GSP) scenario. Additionally, 0.21 worker injuries and 0.0018 worker fatalities would be expected under the CBR-Onsite (GMF RP) scenario.

In summary, risks to workers due to accidents would be expected to be greater under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario. Differences in worker risks between the two scenarios would largely be driven by off-Site activities.

2.2.4.2 Community Risks

Accidents

Vehicle accidents that occur off-Site can result in injuries or fatalities among community members, as well as workers. Based on the accident statistics reported by US DOT (2020) and the off-Site travel mileages reported in Appendix B, off-Site vehicle accidents could result in an estimated 0.11 injuries and 0.0021 fatalities among community members (*i.e.*, people involved in haul truck accidents that are neither haul truck drivers nor passengers, including pedestrians, drivers of other vehicles, *etc.*) under the CIP (GMF GSP) scenario (Table 2.6). Under the CBR-Offsite (GMF GSP) scenario, off-Site vehicle accidents could result in an estimated 0.63 community injuries and 0.019 community fatalities. Under the CBR-Onsite (GMF RP) scenario, off-Site vehicle accidents could result in an estimated 0.041 community injuries and 0.00085 community fatalities. Risks to community members arising from vehicle accidents are therefore higher under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario.

Table 2.6 Expected Number of Community Accidents Under Each Closure Scenario

Off-Site Vehicle Use Category	CIP (GMF GSP)		CBR-Offsite (GMF GSP)		CBR-Onsite (GMF RP)	
	Injuries	Fatalities	Injuries	Fatalities	Injuries	Fatalities
Hauling	0	0	0.45	0.016	0	0
Labor Mobilization/Demobilization	0.079	0.0010	0.14	0.0018	0.027	0.00034
Equipment Mobilization/ Demobilization	0.024	0.00087	0.037	0.0013	0.012	0.00044
Material Deliveries	0.0051	0.00018	0.0033	0.00012	0.0016	0.000059
Total:	0.11	0.0021	0.63	0.019	0.041	0.00085

Notes:

CBR-Offsite = Closure-by-Removal with Off-Site CCR Disposal; CBR-Onsite = Closure-by-Removal with On-Site CCR Disposal; CCR = Coal Combustion Residual; CIP = Closure-in-Place; GMF = Gypsum Manufacturing Facility; GSP = Gypsum Stack Pond; RP = Recycle Pond.

Traffic

Haul routes would be expected to use major arterial roads and highways wherever possible, which would reduce the incidence of traffic. However, the heavy use of local roads for construction operations may result in traffic near the Site, the off-Site landfill, and the off-Site borrow site. Traffic could potentially cause travel delays on local roads and damage to local roadways.

Traffic may increase temporarily around the Site under all closure scenarios due to the daily arrival and departure of the workforce, equipment mobilization/demobilization, and material deliveries. However, these impacts would be expected to largely occur at the beginning or end of each workday (during the arrival/departure of the work force), at the beginning or end of the construction period (during equipment mobilization/demobilization), and at specific times throughout the construction period (during material deliveries). These impacts would therefore likely be less disruptive to community members than the constant and steady movement of haul trucks to and from the Site due to CCR hauling and borrow soil hauling.

Off-Site CCR hauling would only be required under the CBR-Offsite (GMF GSP) scenario. Under this scenario, hauling-related construction activities would be expected to span approximately 1,160 workdays and require approximately 35,300 truckloads (33,700 truckloads of CCR and 1,600 truckloads of borrow soil; Appendix B). Assuming 10-hour workdays, a haul truck would need to pass a given location near the Site once every 9.9 minutes on average for the duration of hauling-related activities under this closure scenario.

Noise

Construction generates a great deal of noise, both in the vicinity of the Site and along haul routes. In a closure impact analysis performed by the Tennessee Valley Authority (TVA, 2015), the authors found that "[T]ypical noise levels from construction equipment used for closure are expected to be 85 dBA or less when measured at 50 ft. These types of noise levels would diminish with distance... at a rate of approximately 6 dBA per each doubling of distance and therefore would be expected to attenuate to the recommended EPA noise guideline of 55 dBA at 1,500 ft." There is one residence northwest of the on-Site landfill, and several residences east of the second on-Site borrow area that are within 1,500 feet of construction areas that could be temporarily impacted by construction noise under the CBR-Onsite (GMF RP) and CBR-Offsite (GMF GSP) scenarios. Likewise, recreators and wildlife in the Coffeen Lake SFWA, which lies within 1,500 feet of the GMF GSP and the GMF RP, could be temporarily impacted by construction noise under all scenarios. The duration of noise impacts in the vicinity of the GMF GSP and the GMF RP would be greater under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario, because the expected duration of construction is longer (31-46 months under the CBR-Offsite (GMF GSP) scenario *versus* 22-33 months under the CIP (GMF GSP) scenario). The duration of noise impacts for the CBR-Onsite (GMF RP) scenario is 12-17 months.

In addition to impacts in the immediate vicinity of planned construction areas at the Site, local roads near the Site, the off-Site landfill, and the off-Site borrow site may also experience noise pollution under the CBR-Offsite (GMF GSP) scenario due to high volumes of haul truck traffic. As described above (Traffic), the construction schedule for the CBR-Offsite (GMF GSP) scenario requires haul trucks to pass by a given location every 9.9 minutes on average for 10 hours each day for approximately 31-46 months. Dump trucks generate significant noise pollution, with noise levels of approximately 88 decibels or higher expected within a 50-foot radius of the truck (Exponent, 2018). This noise level is similar to the noise level of a gas-powered lawnmower or leaf blower (CDC, 2019). Decibel levels above 80 can damage hearing after 2 hours of exposure (CDC, 2019).

In addition to haul truck impacts, noise pollution may also arise from the daily arrival and departure of the workforce, equipment mobilization/demobilization, and material deliveries. These impacts would be expected to largely occur at the beginning or end of each workday (during the arrival/departure of the work force), at the beginning or end of the construction period (during equipment mobilization/demobilization), and at specific times throughout the construction period (during material deliveries). These impacts would therefore likely be less disruptive to community members than the constant and steady movement of haul trucks to and from the Site. In summary, noise impacts are likely to be greater under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario.

Air Quality

Construction can adversely impact air quality. Air pollution can occur both on-Site and off-Site (*e.g.*, along haul routes), potentially impacting workers as well as community members. With regard to construction activities, two categories of air pollution are of particular concern: equipment emissions and fugitive dust. The equipment emissions of greatest concern are those found in diesel exhaust. Most

construction equipment is diesel-powered, including the dump trucks that would be used to haul material to and from the Site. Diesel exhaust contains numerous air pollutants, including nitrogen oxides (NO_x), particulate matter (PM), carbon monoxide (CO), and volatile organic compounds (VOCs; Hesterberg *et al.*, 2009; Mauderly and Garshick, 2009). Fugitive dust, another major air pollutant at construction sites, is generated by earthmoving operations and other soil- and CCR-handling activities. Along haul routes, an additional source of fugitive dust is road dust along unpaved dirt roads. Careful planning and the use of BMPs such as wet suppression are used to minimize and control fugitive dust during construction activities; however, it is not possible to prevent dust generation entirely.

On-Site, emissions would be higher under the CBR-Offsite (GMF GSP) scenarios than under the CIP (GMF GSP) scenario, due to the greater amount of on-Site vehicle and equipment travel miles required under these scenarios (84,200 total on-Site travel miles under the CIP (GMF GSP) scenario *versus* 143,000 total on-Site travel miles under the CBR-Offsite (GMF GSP) scenario with an additional 28,700 total on-Site travel miles under the CBR-Onsite (GMF RP) scenario; Tables 2.1 through 2.3). Off-Site, emissions would similarly be higher under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario due to the greater amount of off-Site vehicle and equipment travel miles required under these scenarios (397,000 total off-Site travel miles under the CIP [GMF GSP] scenario *versus* 1,890,000 total off-Site travel miles under the CBR-Offsite [GMF GSP] scenario with an additional 147,000 total off-Site travel miles under the CBR-Onsite [GMF RP] scenario).

Environmental Justice

The State of Illinois defines environmental justice (EJ) communities to be those communities with a minority population above twice the state average and/or a total population below twice the state poverty rate (IEPA, 2019b).

As shown in a map of EJ communities throughout the state (IEPA, 2019b), the outer perimeter of the 1-mile buffer zone for the nearest EJ community lies approximately 10 miles south of the Site near Greenville (Figure 2.1). As described above (Noise), significant noise impacts due to construction are expected to be limited to potential receptors located within 1,500 feet (0.28 miles) of the Site. Similarly, the air quality impacts of construction are expected to be limited to potential receptors located within 1,000 feet (0.19 miles) of the Site (CARB, 2005; BAAQMD, 2017). Along heavily trafficked roadways, air quality impacts are expected to be limited to potential receptors located within 600 feet of the roadway (0.11 miles; US EPA, 2014). The EJ community near Greenville is therefore unlikely to be directly impacted by on-Site air emissions, noise pollution, or other negative impacts arising at the Site. However, they may be impacted by labor and equipment mobilization/demobilization, and material deliveries. Off-Site impacts due to labor and equipment mobilization/demobilization and material deliveries would be expected to be diffuse (*i.e.*, to span a wide range of transport routes originating over a wide area). Additionally, these impacts would be expected to largely occur at the beginning or end of each workday (during the arrival/departure of the work force), at the beginning or end of the construction period (during equipment mobilization/demobilization), and at specific times throughout the construction period (during material deliveries).

Two types of off-Site hauling are evaluated in this report under the CBR-Offsite (GMF GSP) scenario: CCR hauling and borrow soil hauling. Overall, haul truck impacts on EJ communities due to borrow soil hauling are expected to be small, because borrow soil would be sourced from within two miles of the Site, and there are no EJ communities within two miles of the Site. EJ communities located along the haul route to the off-Site landfill or near the off-Site landfill itself may be negatively impacted throughout the excavation period by the air pollution, noise, traffic, and accidents generated by CCR-hauling activities. A review of the Illinois map of EJ communities reveals that the off-Site landfill is not located within the 1-mile buffer zone of an EJ community. Additionally, based on the two major haul routes suggested by

Google Maps (Google, LLC, 2022), transport of CCR to the landfill will not require hauling CCR through the buffer zone of the EJ community (Figure 2.1).

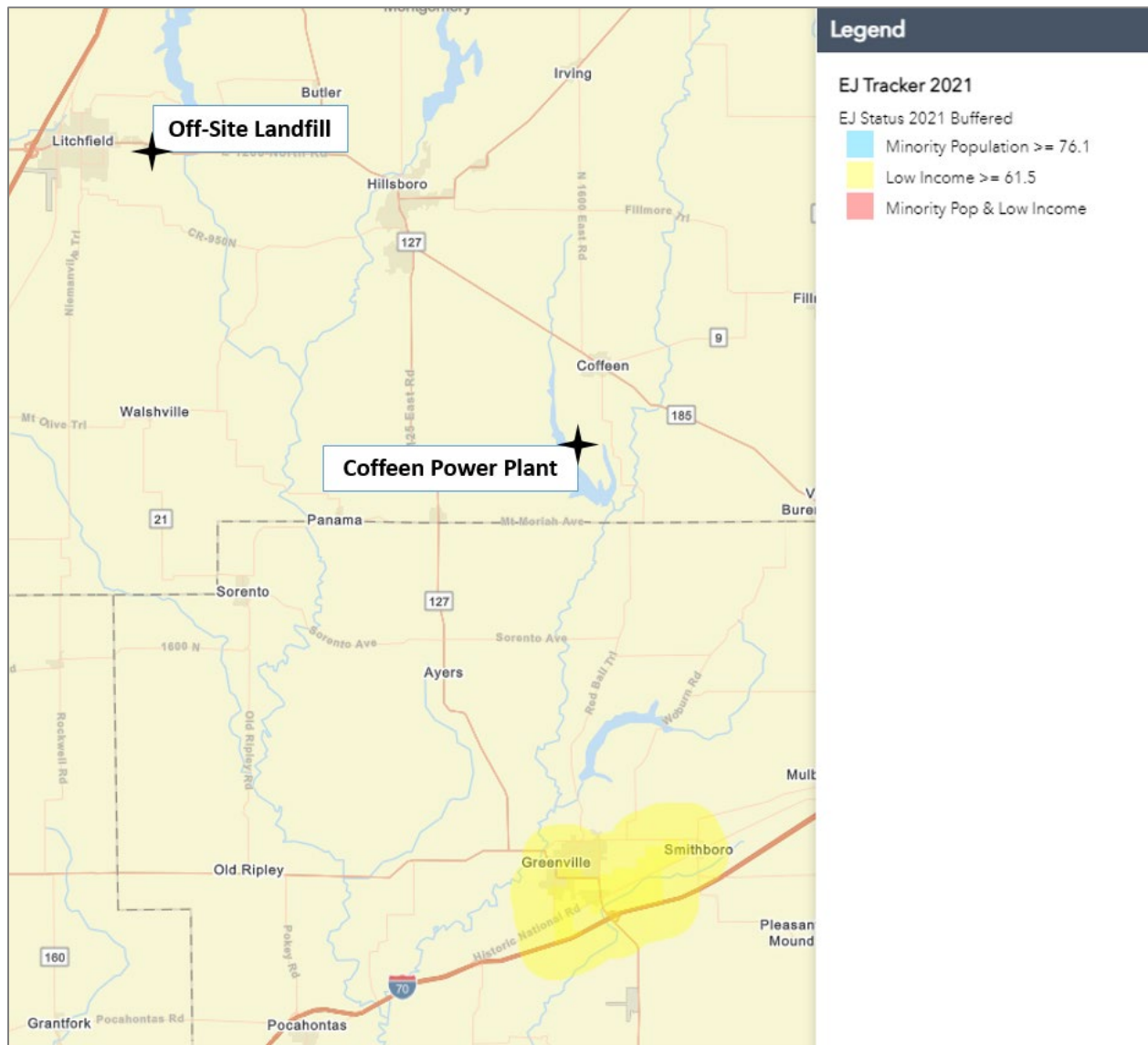


Figure 2.1 Environmental Justice Communities in the Vicinity of the Site and the Off-Site Landfill. EJ = Environmental Justice. Adapted from IEPA (2019b).

Scenic, Historical, and Recreational Value

During construction activities, negative impacts on scenic and recreational value may occur within the Coffeen Lake SFWA. Noise impacts were described above. In addition, construction activities at the GMF GSP and the GMF RP may be visible to recreators using these scenic and recreational areas, potentially interfering with enjoyment of the view. Negative impacts would not be expected to occur within any scenic or recreational areas located further away from the Site. The expected duration of construction activities is longer under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario (22-33 months under the CIP [GMF GSP] scenario *versus* 31-46 months under the CBR-Offsite [GMF GSP] scenario with an additional 12-17 months under the CBR-Onsite [GMF RP] scenario). It is therefore anticipated that short-term impacts on the scenic and recreational value of

natural areas near the Site would be greater under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario.

Based on a review of the IDNR Historic Preservation Division database and the Illinois State Archaeological Survey database, there are no historic sites located within 1,000 meters of the GMF GSP and the GMF RP, the on-Site landfill, or the on-Site borrow soil location (Ramboll, 2021a,b).

2.2.4.3 Environmental Risks

Greenhouse Gas Emissions

In addition to the air pollutants listed above in Section 2.2.4.2, construction equipment emits greenhouse gases (GHGs), including carbon dioxide (CO₂) and possibly nitrous oxide (N₂O). The potential impact of each closure scenario on GHG emissions is proportional to the potential impact of each closure scenario on other emissions from construction vehicles and equipment, as described above in Section 2.2.4.2. In summary, GHG emissions from construction equipment and vehicles would be greater under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario, because the total on-Site and off-Site vehicle and equipment travel miles required under the CBR-Offsite (GMF GSP) scenario (2,030,000 total vehicle and equipment travel miles) are greater than the total required under the CIP (GMF GSP) scenario (482,000 total vehicle and equipment travel miles; Tables 2.1 through 2.3).

We did not quantify the carbon footprint of the approximately 12.4 acres of a 40-mil LLDPE geomembrane liner required for the final GMF GSP cover system under the CIP (GMF GSP) scenario. The carbon footprint of this geomembrane (*i.e.*, the fossil fuel emissions required to manufacture it) is an additional source of GHG emissions at the Site under the CIP (GMF GSP) scenario. The potential expansion of the off-Site landfill under the CBR-Offsite (GMF GSP) scenario would have an additional, unquantified carbon footprint due to the manufacture of geomembranes used in the expanded landfill liners.

Energy Consumption

Energy consumption at a construction site is synonymous with fossil fuel consumption, because the energy to power construction vehicles and equipment comes from the burning of fossil fuels. Fossil fuel demands considered in this analysis include the burning of diesel fuel during construction activities and the carbon footprint of manufacturing geomembrane textiles. Because GHG emission impacts and energy consumption impacts both arise from the same sources at construction sites, the trends discussed above with respect to GHG emissions also apply to the evaluation of energy demands. Specifically, the energy demands of construction equipment and vehicles would be greater under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario. We did not quantify the energy demands of the geomembranes required for the construction of the final cover system under the CIP (GMF GSP) scenario, the geomembranes potentially required for the expansion of the off-Site landfill under the CBR-Offsite scenario.

The Coffeen Power Plant Site is slated for redevelopment as a utility-scale solar power generating facility and battery energy storage facility. At the grid scale, solar generation would add energy back onto the grid and reduce reliance on non-renewable energy sources.

Natural Resources and Habitat

During closure, major construction activities such as the excavation of the impoundment, the excavation of the borrow area, and, potentially, the expansion of the off-Site landfill may require the destruction of

some existing habitat atop portions of these construction areas, resulting in negative impacts to natural resources and habitat within the footprint of these areas. Construction may also have indirect negative impacts on the natural resources and habitat in the immediate vicinity of these locations by causing alarm-and-escape behavior in nearby wildlife (e.g., due to noise disturbances). Finally, although erosion prevention and sediment control measures will be undertaken under all closure scenarios, it is possible that limited negative short-term impacts could occur to sensitive aquatic and wetland species in Coffeen Lake and other wetlands or surface water bodies located near the GMF GSP and the GMF RP (see Section 1.1.3) due to sediment runoff during construction. The duration of time over which various short-term negative habitat impacts might occur due to construction would be longer under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario, due to the longer expected duration of construction activities (22-33 months under the CIP [GMF GSP] scenario *versus* 31-46 months under the CBR-Offsite [GMF GSP] scenario with an additional 12-17 months under the CBR-Onsite [GMF RP] scenario). Thus, negative short-term impacts to natural resources and habitat due to closure activities would likely be greater under the CBR-Offsite (GMF GSP) scenarios than under the CIP (GMF GSP) scenario.

In addition to the short-term negative habitat impacts caused by construction activities, closure may also result in long-term shifts in the habitat types overlying the major construction locations associated with closure. This assessment does not make any value judgments regarding the relative value of the habitat types currently overlying these locations and the habitat types that could potentially overlie these locations post-closure under the various closure scenarios. For example, we did not attempt to determine whether the conversion of open water to grassland within the footprint of the GMF GSP or the GMF RP would constitute a positive or negative long-term change with regard to factors such as biodiversity, ecosystem services, or the preferences of recreators/sightseers.

According to the IDNR Natural Heritage Database and the United States Fish & Wildlife Service (US FWS) Environmental Conservation Online System, there are four state threatened species, five state endangered species, one federally threatened species, and one federally endangered species within Montgomery County (Ramboll, 2021a,b). To our knowledge, however, no threatened or endangered species have been identified at the Site (Ramboll, 2021a,b). Based on the information that is currently available, we do not expect construction activities to have negative impacts on any threatened or endangered species.

2.2.5 Time Until Groundwater Protection Standards Are Achieved (IAC Sections 845.710(b)(1)(E) and 845.710(d)(2 and 3))

The time horizon over which GWPSs would be exceeded at the Site is immaterial from a risk perspective, because there is no unacceptable risk associated with exceedances of a GWPS at the Site (see Section 2.2.1). Nonetheless, pursuant to requirements under IAC Section 845.710, this section of the text describes the time required to achieve GWPSs at the Site.

As described in Section 1.1.4 (Hydrogeology), water and CCR-related constituents from the GMF GSP and the GMF RP may migrate vertically downward until they reach the UA. There is a groundwater flow divide within the UA in the center of the Coffeen Power Plant property between the two lobes of Coffeen Lake. Groundwater in the UA flows from the center of the Coffeen Power Plant property west toward Coffeen Lake and east toward the Unnamed Tributary. Groundwater predominantly flows east/southeast across the GMF GSP to the Unnamed Tributary; however, the western side of the of the GMF GSP aligns with the groundwater divide and groundwater in this area flows west toward Coffeen Lake. The GMF RP is located east of the divide, and groundwater flows southeast across the GMF RP. The Unnamed Tributary serves as a regional sink for shallow groundwater discharge and shallow groundwater migration

beneath or beyond the tributary is unlikely (Ramboll, 2021a,b). Groundwater flow within the UA is mostly in the horizontal direction because the UA is underlain by the low-permeability LCU (Ramboll, 2021a,b).

At the Coffeen Site, no seasonal variation in groundwater levels has been observed. Surface water elevations in Coffeen Lake similarly do not fluctuate significantly over time, since the lake elevation is controlled by a dam. As a result, groundwater flow directions at the Site are not generally affected by seasonal variabilities (Ramboll, 2021a,b).

Groundwater modeling was performed to evaluate future groundwater quality in the vicinity of the GMF GSP and the GMF RP under each of the proposed closure alternatives (Ramboll, 2022). The modeling demonstrated that there are limited differences between CIP and CBR-Offsite in the timeframes to achieve the GWPSs. In general, with the exception of sulfate, the simulated groundwater concentrations in the monitoring wells within the UA will achieve the GWPSs within 7 years after closure implementation for both the CIP and the CBR-Offsite scenarios at the GMF GSP. For the CBR-Onsite scenario at the GMF RP, the simulated groundwater concentrations in the monitoring wells within the UA will achieve the GWPSs in 4.6 years. The model indicates that sulfate will decline over time under both the CIP, CBR-Offsite, and CBR-Onsite scenarios at the GMF GSP and the GMF RP, while remaining in close proximity to the impoundments. For both the CIP and CBR-Offsite scenarios at the GMF GSP, the model predicts that sulfate concentrations will decline below the sulfate GWPS (400 mg/L) in all hydrostratigraphic layers approximately 13 years after closure for CIP and 14 years after closure for CBR-Offsite. For the CBR-Onsite scenario at the GMF RP, the model predicts that sulfate concentrations will decline below the sulfate GWPS in all hydrostratigraphic layers approximately 14 years after the implementation of closure.

Additionally, changing geochemical conditions during an extended excavation associated with the CBR-Offsite and CBR-Onsite scenarios can be a mechanism that results in the mobilization and increased transport in groundwater for some constituents. This may result in GWPS exceedance durations in excess of the model predictions for the CBR-Offsite and CBR-Onsite scenarios.

2.2.6 Potential for Exposure of Humans and Environmental Receptors to Remaining Wastes, Considering the Potential Threat to Human Health and the Environment Associated with Excavation, Transportation, Re-disposal, Containment, or Changes in Groundwater Flow (IAC Section 845.710(b)(1)(F))

Section 2.2.1 evaluates potential risks to human and ecological receptors arising from the leaching of CCR-associated constituents into groundwater during closure activities and following closure of the GMF GSP and the GMF RP. Section 2.2.2 evaluates the potential for CCR releases to occur due to dike failure or overtopping during floods or other storm-related events. In summary, there is no current or future risk to any human or ecological receptors associated with the GMF GSP or the GMF RP. Additionally, there is minimal current or future risk of overtopping occurring at the embankments due to flood conditions at the Site. Dike failure due to, *e.g.*, seismic activity and storm-related events is also exceedingly unlikely.

Section 2.2.4 evaluates several potential risks to human health and the environment during closure activities, including risks of accidents occurring among workers; risks to nearby residents and EJ communities related to accidents, traffic, noise, and air pollution; and risks to natural resources and wildlife. The findings from this section of the text are summarized in Table S.1 (Summary of Findings).

2.2.7 Long-Term Reliability of the Engineering and Institutional Controls (IAC Section 845.710(b)(1)(G))

Post-closure, there is minimal risk of engineering or institutional failures leading to sudden releases of CCR from the impoundment under the CIP (GMF GSP) scenario. There is no post-closure risk of engineering or institutional failures under the CBR-Offsite (GMF GSP) or the CBR-Onsite (GMF RP) scenarios (see Section 2.2.2 above). Additionally, there are no current or future unacceptable risks to any human or ecological receptors under any closure scenario (see Section 2.2.1 above). Moreover, reliable engineering and institutional controls (e.g., a bottom liner, a leachate management system, and groundwater monitoring) would be implemented at the on-Site and off-Site landfills under the CBR-Onsite (GMF RP) and CBR-Offsite (GMF GSP) scenarios. All of the evaluated closure scenarios are therefore reliable with respect to long-term engineering and institutional controls.

2.2.8 Potential Need for Future Corrective Action Associated with the Closure (IAC Section 845.710(b)(1)(H))

Corrective action is expected at the Site. An evaluation of potential corrective measures and corrective actions has not yet been completed, but will be conducted consistent with the requirements in IAC Sections 845.660 and 845.670.

2.3 Effectiveness of the Closure Alternative in Controlling Future Releases (IAC Section 845.710(b)(2))

2.3.1 Extent to Which Containment Practices Will Reduce Further Releases (IAC Section 845.710(b)(2)(A))

The CCR in the GMF GSP and the GMF RP currently poses no unacceptable risks to human health or the environment (Section 2.2.1). Because current conditions do not present a risk to human health or the environment, and dissolved constituent concentrations would be expected to decline post-closure, there would also be no unacceptable risks to human health or the environment following closure, regardless of the closure scenario.

Section 2.2.2 discussed the potential for dike failure or overtopping to occur during or following closure activities, resulting in a sudden release of CCR. That analysis showed that there is minimal risk of sudden CCR releases occurring during or following closure under any closure scenario.

2.3.2 Extent to Which Treatment Technologies May Be Used (IAC Section 845.710(b)(2)(B))

Under all three closure scenarios, water generated during the dewatering and unwatering of the impoundment would be treated if necessary prior to disposal. Following treatment, water from unwatering and dewatering would be discharged to the Unnamed Tributary in accordance with the NPDES permit for the facility.

2.4 Ease or Difficulty of Implementing Closure Alternative (IAC Section 845.710(b)(3))

2.4.1 Degree of Difficulty Associated with Constructing the Closure Alternative

CIP using a final cover system is a reliable and standard method for managing and closing impoundments that relies on common construction activities. Dewatering saturated CCR to construct a stabilized final cover system subgrade can present challenges during closure; however, these challenges are common to most CCR surface impoundment closures and are commonly addressed *via* surface water management and dewatering techniques.

Excavation and landfilling of CCR is also a reliable and standard method for closing impoundments. However, relative to CIP, CBR-Onsite and CBR-Offsite pose additional implementation difficulties due to higher earthwork volumes, higher dewatering volumes, and longer construction schedules. Removal and disposal of the existing bottom liner geomembranes may also prove challenging during CBR activities. Specifically, it may be difficult to remove and handle the geomembranes. Additionally, the geomembranes may need to be decontaminated prior to disposal. Finally, the geomembranes may not be accepted for disposal at the off-Site landfill.

Off-Site hauling would be more difficult to implement under the CBR-Offsite (GMF GSP) scenario than then on-Site hauling under the CIP (GMF GSP) scenario, due to the longer haul distance required for off-Site disposal than for CIP and the need to haul the CCR over public roads. Hauling over public roads would require the use of lower-volume haul trucks, which would increase the number of trucks and trips required for CCR excavation and transport. Additionally, because the CBR-Offsite scenario would involve hauling CCR off-Site (*i.e.*, intrastate travel), a higher level of dewatering would be required under this scenario compared to the CIP scenario. As described in Section 2.2.4.2 ("Community Risks"), off-Site hauling may also have detrimental community impacts due to an increased incidence of vehicle accidents, traffic-related impacts, noise, and air pollution.

In addition to off-Site hauling, off-Site landfilling under the CBR-Offsite scenario may pose particular challenges. A disposal plan would need to be developed between IPGC and the owner/operator of the third-party landfill in order to outline acceptable waste conditions upon delivery, daily waste production rates, and the expected duration of the project. Off-Site landfilling may additionally raise issues related to the co-disposal of CCR and other non-hazardous wastes. Finally, the construction schedule for excavation may be negatively impacted if, during the course of closure, it is determined that the off-Site landfill must be expanded in order to receive all of the materials excavated from the GMF GSP.

2.4.2 Expected Operational Reliability of the Closure Alternative

The operational reliability of the two closure scenarios is expected to be similar. CIP would utilize a final cover system that includes a geomembrane, and the GMF GSP currently includes a bottom liner system. Therefore, under the CIP scenario, the CCR would be surrounded by an engineered containment system on the top, sides, and bottom. The CBR-Offsite (GMF GSP) and CBR-Onsite (GMF RP) scenarios similarly involve placing the CCR in an engineered landfill system that has a bottom liner and a leachate collection system and would eventually be capped with a final cover system, resulting in the CCR being surrounded by an engineered containment system on the top, sides, and bottom. The operational reliability of all closure scenarios is therefore expected to be similar. Moreover, high reliability would be expected under all scenarios due to the full containment of the CCR.

2.4.3 Need to Coordinate with and Obtain Necessary Approvals and Permits from Other Agencies

Permits and approvals would be needed under all closure scenarios. Components of the three closure scenarios that would be expected to require a permit include:

- A modification to the existing NPDES permit through IEPA to allow the disposal of water generated from unwatering and dewatering operations to the Unnamed Tributary *via* the existing NPDES-permitted outfall for the Site;
- A construction permit from the IDNR, Office of Water Resources, Dam Safety Program to allow the embankment and spillways of the GMF GSP and the GMF RP to be modified as part of closure;
- A construction stormwater permit through IEPA, including construction stormwater controls and other BMPs such as silt fences and other measures; and
- A joint water pollution control construction and operating permit (WPC permit).

As discussed below in Section 2.4.5, under the CBR-Offsite (GMF GSP) scenario, it may be necessary to expand the off-Site landfill to accommodate all of the material excavated from the GMF GSP. Additional permitting may be required under this scenario for transport of the CCR and to expand the off-Site landfill. It may also be necessary to modify the operating plan for the off-Site landfill in order to accommodate the increased rate of filling of the landfill and the likely need for additional equipment and personnel to manage the receipt and disposal of the CCR and liner system materials.

2.4.4 Availability of Necessary Equipment and Specialists

CIP, CBR-Onsite, and CBR-Offsite are reliable and standard methods for managing waste that rely on common construction equipment and materials and typically do not require the use of specialists, outside of typical construction labor and equipment operators. However, global supply chains have been disrupted due to the COVID-19 pandemic, resulting in shortages in the availability of construction equipment and parts. There may be some shortages in construction equipment under all scenarios, if supply chain resilience does not improve by the time of construction. Alternatively, extended downtime may be required for equipment repairs and maintenance. A national shortage of truck drivers has also developed during the COVID-19 pandemic. Due to higher earthwork volumes and a longer construction schedule, the CBR-Offsite (GMF GSP) scenario may cause greater challenges than under the CIP (GMF GSP) scenario. The current shortage of truck drivers may be particularly impactful under the CBR-Offsite (GMF GSP) scenario, due to the large volume of CCR and liner materials to be hauled from the Site. If sufficient trucks and truck drivers are not available, the construction schedule at the impoundment may lengthen based on hauling-related delays.

The availability of critical materials such as metal, wood, and electronic chips has also been impacted by the COVID-19 pandemic. However, soil materials and geomembrane liner materials have generally been available during 2021 and early 2022 for landfill development and closure projects.

2.4.5 Available Capacity and Location of Needed Treatment, Storage, and Disposal Services

Under the CIP scenario, all of the CCR currently within the GMF GSP would be stored within the existing footprint of the GMF GSP. Treatment would consist of unwatering the GMF GSP at the start of construction, performing limited dewatering to stabilize the CCR subgrade, and managing stormwater inflow. Water from unwatering and dewatering of the GMF GSP would be discharged in accordance with the NPDES permit for the facility. Under the two CBR scenarios, water treatment would similarly consist of unwatering and dewatering the GMF GSP and the GMF RP at the start of construction and discharging water from unwatering/dewatering in accordance with the NPDES permit for the facility. Due to the need for dewatering prior to CCR hauling, a higher volume of water would be expected to be generated during dewatering under the CBR-Offsite (GMF GSP) scenario than under the CIP (GMF GSP) scenario.

For the CBR-Offsite (GMF GSP) scenario, 556,000 CY of CCR and liner materials would be excavated from the GMF GSP and require disposal. According to the IEPA "Landfill Disposal Capacity Report" for 2020 (IEPA, 2021b), the closest nearby third-party landfill with the ability to receive and dispose of CCR from the Site is the Hillsboro-Litchfield Landfill in Litchfield, Illinois. This facility has 1,540,000 CY of remaining capacity in its current permitted footprint. It receives 83,000 CY of waste annually, and is located 18 miles from the Site by road. The Litchfield-Hillsboro Landfill therefore has sufficient capacity to receive CCR from the GMF GSP. However, closure of the GMF GSP would increase the annual waste receipt rate at the off-Site landfill. Due to the short time frame over which CCR would be received at the landfill, vertical and/or lateral expansions may become necessary. Additionally, the landfill operators may need to develop a disposal plan to account for the increased volume of material that would be received and the unique CCR waste characteristics. Elements of this disposal plan might include increasing daily operational capacity and procedures, expediting planned airspace construction, and potentially expediting landfill expansion.

If expansion of the Litchfield-Hillsboro Landfill is impractical or infeasible, then an alternative landfill located farther from the Site would need to be identified. A likely alternative to the Litchfield-Hillsboro Landfill is the Five Oaks Landfill in Taylorville, Illinois. It has 7,050,000 CY of remaining capacity in its current permitted footprint, receives 250,000 CY of waste annually, and is located 44 miles from the Site (IEPA, 2021b).

2.5 Impact of Closure Alternative on Waters of the State (IAC Section 845.710(d)(4))

As demonstrated in Gradient's Human Health and Ecological Risk Assessment (Appendix A), both modeled and measured surface water concentrations in Coffeen Lake are all below relevant human health and ecological screening benchmarks. Surface water concentrations of CCR-associated constituents would be expected to decline over time under all closure scenarios. Thus, no current or future exceedances of any human health or ecological screening benchmarks would be anticipated under any closure scenario.

The lined landfills that would receive the CCR excavated from the impoundment under the CBR-Onsite (GMF RP) and CBR-Offsite (GMF GSP) scenarios would be managed to ensure that no surface water impacts would occur in the vicinity of the landfill. In summary, no impacts on any waters of the state would be expected under any closure scenario.

2.6 Concerns of Residents Associated with Closure Alternatives (IAC Section 845.710(b)(4))

Several nonprofits representing community interests near the Site have raised concerns regarding the potential impacts of the coal ash impoundments at this Site on groundwater and surface water quality, including Earthjustice, the Prairie Rivers Network, and the Sierra Club (Earthjustice *et al.*, 2018; Sierra Club, 2014; Sierra Club and CIHCA, 2014). These parties generally prefer CBR to CIP, citing fears that allowing CCR to remain in place "allows the widespread groundwater contamination to continue indefinitely" (Earthjustice *et al.*, 2018, p. 24). However, it is not the case that closing the GMF GSP *via* CIP rather than CBR would result in undue risks to groundwater and surface water post-closure. As described in Sections 2.2.1 and 2.2.2, no current or future unacceptable risks to human or ecological receptors are associated with the GMF GSP under any scenario. There is also minimal risk of future CCR releases occurring under any scenario. Furthermore, groundwater modeling conducted at the Site demonstrated that there would be limited differences between CIP and CBR-Offsite in the timeframes to achieve the GWPSs at the GMF GSP. For most constituents at the GMF GSP, the GWPSs will be achieved in approximately 7 years after implementation of closure under both scenarios. For most constituents at the GMF RP, the GWPSs will be achieved in approximately 2.5 years after implementation of the CBR-Onsite closure scenario (Ramboll, 2022). All three closure scenarios are therefore responsive to residents' concerns regarding impacts to groundwater and surface water quality.

The CIP (GMF GSP) scenario has several advantages over the CBR-Offsite (GMF GSP) scenario with regard to likely community concerns. Notably, the CIP scenario presents fewer risks to workers and nearby residents during construction in the form of accidents, traffic-related impacts, noise, and air pollution (Section 2.2.4 above). Closure would also be achieved more rapidly under the CIP scenario than under the CBR-Offsite scenario, due to the shorter duration of construction activities.

A public meeting was held on June 14, 2022, pursuant to requirements under IAC Section 845.710(e). Questions raised by attendees were addressed at the meeting; subsequently, a written summary of the questions and responses was prepared.

2.7 Class 4 Estimate (IAC Section 845.710(d)(1))

Analyses in the Final Closure Plan were prepared consistent with Class 4 estimates based on the Association for the Advancement of Cost Engineering (AACE) Classification Standard (or a comparable classification practice as provided in the AACE Classification Standard), as required by IAC Section 845.710 (IEPA, 2021a).

2.8 Summary

Table S.1 (Summary of Findings) summarizes the expected impacts of the CIP (GMF GSP), CBR-Onsite (GMF RP), and CBR-Offsite (GMF GSP) closure scenarios with regard to each of the factors specified under IAC Section 845.710 (IEPA, 2021a). Based on this evaluation and the details provided in Section 2 above, CIP has been identified as the most appropriate closure scenario for the GMF GSP paired with CBR-Onsite for the GMF RP. Key benefits of the CIP (GMF GSP) scenario relative to the CBR-Offsite (GMF GSP) scenario include greatly reduced impacts to workers, community members, and the environment due to construction activities (*e.g.*, fewer construction-related accidents, lower energy demands, less air pollution and GHG emissions, less traffic-related impacts, and potentially lower impacts to EJ communities).

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Appendix A

Human Health and Ecological Risk Assessment

**Human Health and Ecological Risk Assessment
Gypsum Management Facility Gypsum Stack Pond
and Gypsum Management Facility Recycle Pond
Coffeen Power Plant
Coffeen, Illinois**

July 28, 2022



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Abbreviations

ADI	Acceptable Daily Intake
BCF	Bioconcentration Factor
BCG	Biota Concentration Guide
CAA	Closure Alternatives Assessment
CCR	Coal Combustion Residuals
CEM	Conceptual Exposure Model
COI	Constituent of Interest
COPC	Constituent of Potential Concern
CPP	Coffeen Power Plant
CSF	Cancer Slope Factor
CSM	Conceptual Site Model
CWS	Community Water Supply Well
DA	Deep Aquifer
DCU	Deep Confining Unit
ESV	Ecological Screening Value
GMF	Gypsum Management Facility
GSP	Gypsum Stack Pond
GWPS	Groundwater Protection Standards
GWQS	Groundwater Quality Standards
HTC	Human Threshold Criteria
IAC	Illinois Administrative Code
ID	Identification
IDNR	Illinois Department of Natural Resources
IEPA	Illinois Environmental Protection Agency
ILWATER	Illinois Water and Related Wells
IPGC	Illinois Power Generating Company
ISGS	Illinois State Geological Survey
LCU	Lower Confining Unit
MCL	Maximum Contaminant Level
NID	National Inventory of Dams
NRWQC	National Recommended Water Quality Criteria
ORNL RAIS	Oak Ridge National Laboratory's Risk Assessment Information System
RfD	Reference Dose
RME	Reasonable Maximum Exposure
RP	Recycle Pond
RSL	Regional Screening Level
SI	Surface Impoundment
SWQC	Surface Water Quality Criteria
SWQS	Surface Water Quality Standards
TEC	Threshold Effect Concentration
UA	Uppermost Aquifer
UCU	Upper Confining Unit
US DOE	United States Department of Energy

US EPA
USGS

United States Environmental Protection Agency
US Geological Survey

1 Introduction

Illinois Power Generating Company's (IPGC) Coffeen Power Plant (CPP, or "the Site") is an electric power generating facility with coal-fired units located approximately two miles south of Coffeen, Illinois. The CPP operated as a coal-fired power plant from 1964 until November 2019 and has five coal combustion residuals (CCR) management units (Ramboll, 2021a). The CCR units that are the subjects of this report are two gypsum management facility (GMF) ponds: the GMF gypsum stack pond (GMF GSP, Vistra Identification [ID] Number [No.] 103, Illinois Environmental Protection Agency [IEPA] ID No. W1350150004-03, and National Inventory of Dams [NID] No. IL50579) and the GMF recycle pond (GMF RP, Vistra ID No. 104, IEPA ID No. W1350150004-04, and NID No. IL50578) (Ramboll, 2021a,b). The GMF GSP is a 77-acre lined surface impoundment (SI) and the GMF RP is a 17-acre lined SI; they were used to manage CCR and non-CCR waste streams at the CPP (Ramboll, 2021a,b).

This report presents the results of an evaluation that characterizes potential risk to human and ecological receptors that may be exposed to CCR constituents in environmental media originating from the GMF GSP and RP. This risk evaluation was performed to support the Closure Alternatives Assessment (CAA) for the GMF GSP and the GMF RP in accordance with requirements in Title 35 Part 845 of the Illinois Administrative Code (IAC) (IEPA, 2021). Human and ecological risks were evaluated for Site-specific constituents of interest (COIs). The conceptual site model (CSM) assumed that Site-related COIs in groundwater may migrate to the adjacent Coffeen Lake and affect surface water and sediment in the vicinity of the Site.

Consistent with United States Environmental Protection Agency (US EPA) guidance (US EPA, 1989), this report used a tiered approach to evaluate potential risks, which included the following steps:

1. Identify complete exposure pathways and develop a conceptual exposure model (CEM).
2. Identify Site-related COIs: Constituents detected in groundwater were considered COIs if their maximum detected concentration over the period from 2015 to 2021 exceeded a groundwater protection standard (GWPS) identified in Part 845.600 (IEPA, 2021), or a relevant surface water quality standard (SWQS) (IEPA, 2019; US EPA Region IV, 2018).
3. Perform screening-level risk analysis: Compare maximum measured or modeled COI concentrations in surface water and sediment to conservative, health-protective benchmarks to determine constituents of potential concern (COPCs).
4. Perform refined risk analysis: If COPCs are identified, perform a refined analysis to evaluate potential risks associated with the COPCs.
5. Formulate risk conclusions and discuss any associated uncertainties.

This assessment relies on a conservative (*i.e.*, health-protective) approach and is consistent with the risk approaches outlined in US EPA guidance. Specifically, we considered evaluation criteria detailed in IEPA guidance documents (*e.g.*, IEPA, 2013, 2019), incorporating principles and assumptions consistent with the Federal CCR Rule (US EPA, 2015a) and US EPA's "Human and Ecological Risk Assessment of Coal Combustion Residuals" (US EPA, 2014a).

US EPA has established acceptable risk metrics. Risks above these US EPA-defined metrics are termed potentially "unacceptable risks." Based on the evaluation presented in this report, no unacceptable risks to human or ecological receptors resulting from CCR exposures associated with the GMF GSP or GMF RP were identified. This means that the risks from the site are likely indistinguishable from normal background risks. Specific risk assessment results include the following:

- No completed exposure pathways were identified for any groundwater receptors; consequently, no risks were identified relating to the use of groundwater.
- No unacceptable risks were identified for recreators boating in Coffeen Lake adjacent to the Site.
- No unacceptable risks were identified for recreators exposed to sediment in Coffeen Lake adjacent to the Site.
- No unacceptable risks were identified for anglers consuming locally caught fish.
- No unacceptable risks were identified for ecological receptors exposed to surface water or sediment.
- No bioaccumulative ecological risks were identified.

It should be noted that this evaluation incorporates a number of conservative assumptions that tend to overestimate exposure and risk. Moreover, it should be noted that because current conditions do not present a risk to human health or the environment, there will also be no unacceptable risk to human health or the environment for future conditions when the GMF GSP and the GMF RP are closed. For all future closure scenarios, potential releases of CCR-related constituents will decline over time and consequently potential exposures to CCR-related constituents in the environment will also decline.

2 Site Overview

2.1 Site Description

The CPP is located in Montgomery County, Illinois, approximately two miles south of the city of Coffeen and about eight miles southeast of the city of Hillsboro, Illinois. The CPP operated as a coal-fired power plant from 1964 until November 2019 (Ramboll, 2021a). Five CCR units are present on the CPP property: Ash Pond 1, Ash Pond 2, GMF RP, GMF GSP, and Landfill (Ramboll, 2021a). The GMF GSP and the GMF RP are the subjects of this report. The GMF GSP is a 77-acre lined SI, identified by Vistra ID No. 103, IEPA ID No. W1350150004-03, and NID No. IL50579. The GMF RP is a 17-acre lined SI identified by Vistra ID No. 104, IEPA ID No. W1350150004-04, and NID No. IL50578 (Ramboll, 2021a,b). Both units were put into operation in 2010, and stopped receiving waste prior to April 11, 2021 (Ramboll, 2021a).

The CPP is bordered by Coffeen Lake to the west, east, and south, and is bordered by agricultural land to the north. An unnamed tributary, located east of the GMF GSP, flows south into Coffeen Lake (Figure 2.1) (Ramboll, 2021a). Coffeen Lake (approximately 1,100-acres) was formed in 1963 for use as an artificial cooling lake for the CPP, by damming the McDavid Branch of the East Fork of Shoal Creek (Ramboll, 2021a).



Figure 2.1 Site Location Map. Source: Ramboll (2021a).

2.2 Geology/Hydrogeology

The geology underlying the CPP Site in the vicinity of the GMF GSP and the GMF RP primarily consists of unlithified deposits (Ramboll, 2021a,b). The unlithified deposits were categorized into the following hydrostratigraphic units (from the surface downward): the Upper Confining Unit (UCU), composed of Roxana and Peoria Silts (Loess Unit); the Uppermost Aquifer (UA), primarily composed of sandy to gravelly silts and clays of the Hagarstown Member; the Lower Confining Unit (LCU), comprised of the Vandalia Member, the Mulberry Grove Member, and the Smithboro Member; the Deep Aquifer (DA), comprised of sand and sandy silts/clays of the Yarmouth Soil; and the Deep Confining Unit (DCU), comprised of clays, silts, and sands of the Banner Formation (Ramboll, 2021a).

The Hagarstown Member is separated into two units: a gravelly clay till unit on top of a sandy unit (Ramboll, 2021a,b). The sandy unit at the base of the Hagarstown Member was identified as the UA. However, in some locations, the uppermost weathered sandy clay portion of the Vandalia Member was also identified as the UA (Ramboll, 2021a). The UA (*i.e.*, sandy portion of the Hagarstown Member) is generally less than 3 feet (ft) thick but is absent at several locations due to weathering or construction-related excavation (Ramboll, 2021a,b). The UA is not present beneath the entire footprint of the GMF GSP or the GMF RP (Ramboll, 2021a,b). The top of the UA is separated from the overlying CCR materials in the GMF GSP and the GMF RP by the low permeability Loess (UCU) and the gravelly clay till portions of the Hagarstown Member. The bottom of the UA is separated from the DA by low-permeability tills of the LCU (Ramboll, 2021a,b). The UA has moderate permeability with a geometric mean horizontal hydraulic conductivity of 1.4×10^{-3} cm/s near the GMF GSP (Ramboll, 2021a) and a geometric mean horizontal hydraulic conductivity of 1.2×10^{-3} cm/s near the GMF RP (Ramboll, 2021b).

In the vicinity of the GMF RP, groundwater within the UA generally flows southeast towards an unnamed tributary (Ramboll, 2021b). In the vicinity of the GMF GSP, groundwater within the UA flows southeast/east towards the unnamed tributary and southwest/west towards the western branch of Coffeen Lake (Ramboll, 2021a). For the southeastern/eastern flow component, the horizontal hydraulic gradients within the UA range from 0.003 to 0.01 ft/ft (Ramboll, 2021a,b). For the southwestern/western flow component, the average horizontal hydraulic gradient in the UA is about 0.018 ft/ft (Ramboll, 2021a).

2.3 Conceptual Site Model

A CSM describes sources of contamination, the hydrogeological units, and the physical processes that control the transport of water and solutes. In this case, the CSM describes how groundwater underlying the GMF GSP and the GMF RP migrates and interacts with surface water and sediment in the adjacent unnamed tributary and Coffeen Lake. The CSM was developed using available hydrogeologic data specific to the GMF GSP and the GMF RP, including information on groundwater flow and surface water characteristics (Ramboll, 2021a).

Due to the presence of a groundwater divide on the Site, groundwater in the UA flows both toward the eastern and western branches of Coffeen Lake (Figure 2.2). On the east side of the groundwater divide, groundwater flows east and southeast into an unnamed tributary that flows south into the eastern branch of Coffeen Lake. On the west side of the divide, groundwater flows west and southwest into the western branch of Coffeen Lake.

All groundwater originating from the GMF RP ultimately flows into the unnamed tributary, whereas a component of groundwater originating from the GMF GSP flows into the unnamed tributary, and the rest flows into the western branch of Coffeen Lake (Figure 2.2). The GMF RP and much of the GMF GSP are

located to the east side of the groundwater divide, thus groundwater (and any CCR-related constituents) originating from these SIs may migrate vertically downward through the UCU into the UA and eventually flow into the unnamed tributary (Ramboll, 2021a,b). The western edge of the GMF GSP is on the west side of the groundwater divide; therefore, groundwater (and any CCR-related constituents) originating from this portion of the SI may migrate vertically downward through the UCU into the UA and eventually flow into the western branch of Coffeen Lake. Groundwater flow within the UA is mostly in the horizontal direction because the UA is underlain by the LCU, which is a low-permeability till unit inhibiting vertical flow of groundwater. Groundwater near the GMF ponds may mix with surface water in the unnamed tributary to the east and with surface water in the western branch of Coffeen Lake to the west. The dissolved constituents in groundwater may partition between sediments and surface water.

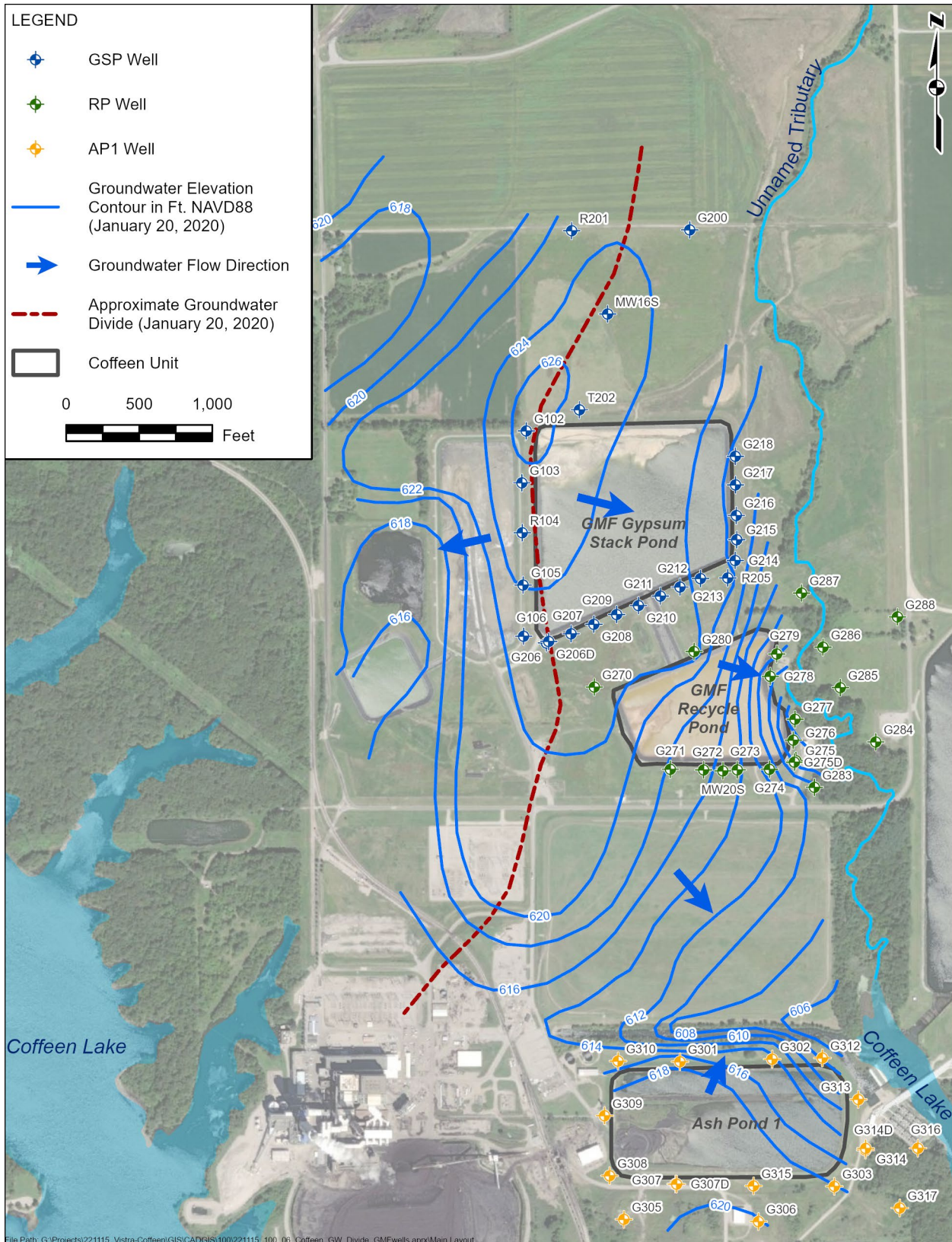


Figure 2.2 Groundwater Flow Direction. Source: Ramboll (2021a).

2.4 Groundwater Monitoring

A total of 43 wells have been used to monitor the groundwater quality near and downgradient of the GMF GSP and the GMF RP; 24 wells are associated with the GMF GSP and 19 wells are associated with the GMF RP. Of the 24 wells associated with the GMF GSP, 23 wells are screened in the UA and 1 well is screened in the DA (Table 2.1). Of the 19 wells associated with the GMF RP, 16 wells are screened in the UA, 2 wells are screened in the LCU, and 1 well is screened in the DA (Table 2.1). The analyses presented in this report conservatively relied on all available data from the 43 wells collected between 2015 and 2021, which is the period subsequent to the promulgation of the Federal CCR Rule. Groundwater samples were analyzed for a suite of total metals, specified in Illinois CCR Rule Part 845.600 (IEPA, 2021).¹ A summary of the groundwater data for the 43 wells used in this risk evaluation is presented in Table 2.2. The GMF GSP and the GMF RP well locations used in this risk evaluation are shown in Figure 2.3. Note that the groundwater data were split into two groups to model surface water concentrations for the unnamed tributary (19 GMF RP and 16 GMF RP wells on the east side of the groundwater divide) and the western branch of Coffeen Lake (8 GMF GSP wells on the west side of the groundwater divide). The use of groundwater data in this risk evaluation does not imply that detected constituents are associated with the GMF GSP or the GMF RP or that they have been identified as potential groundwater exceedances.

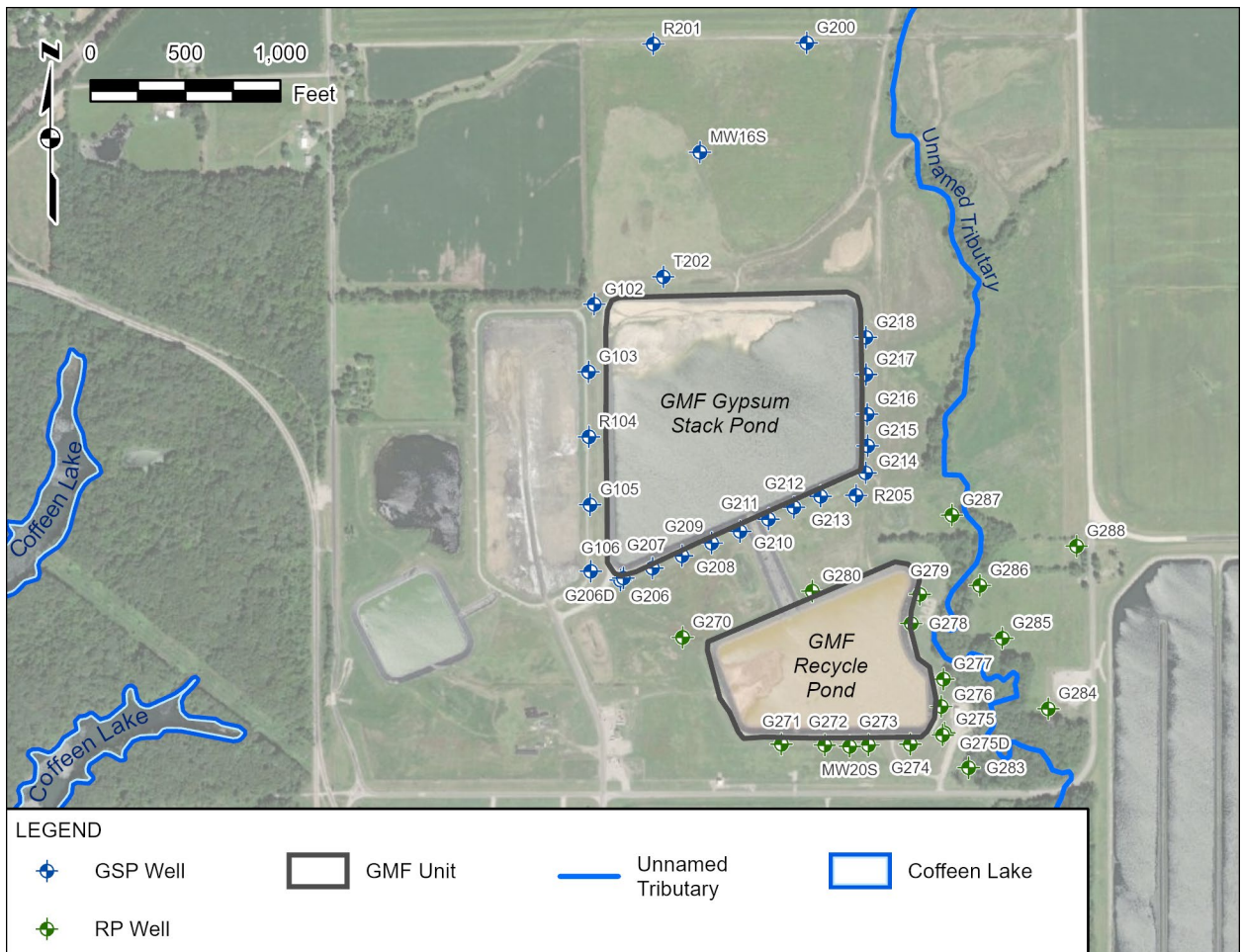


Figure 2.3 Monitoring Well Locations. Source: Ramboll (2021a).

¹ Samples were analyzed for a longer list of inorganic constituents and general water quality parameters (chloride, fluoride, sulfate, and total dissolved solids), but these constituents were not evaluated in the risk evaluation.

Table 2.1 Groundwater Monitoring Wells Related to the GMF GSP and the GMF RP

Associated GMF Pond	Side of Groundwater Divide	Well	Hydrogeologic Unit	Date Constructed	Screen Top Depth (ft bgs)	Screen Bottom Depth (ft bgs)	Well Depth (ft bgs)
GMF GSP	East	G200	UA	2/25/2008	12.19	16.98	17.36
GMF GSP	East	G207	UA	10/8/2010	18.24	22.77	23.30
GMF GSP	East	G208	UA	10/7/2010	17.53	22.06	22.60
GMF GSP	East	G209	UA	10/7/2010	17.74	22.28	22.81
GMF GSP	East	G210	UA	10/6/2010	19.39	23.93	24.46
GMF GSP	East	G211	UA	10/11/2010	17.34	21.88	22.41
GMF GSP	East	G212	UA	10/11/2010	16.74	21.29	21.81
GMF GSP	East	G213	UA	10/12/2010	16.75	21.29	21.82
GMF GSP	East	G214	UA	10/14/2010	17.75	22.14	22.65
GMF GSP	East	G215	UA	10/13/2010	19.41	23.80	24.31
GMF GSP	East	G216	UA	10/13/2010	20.04	24.42	24.93
GMF GSP	East	G217	UA	10/12/2010	20.49	24.88	25.38
GMF GSP	East	G218	UA	10/12/2010	20.33	24.77	25.27
GMF GSP	East	MW16S	UA	4/25/2006	14.59	19.41	19.76
GMF GSP	East	R205	UA	3/20/2017	11.32	16.01	16.42
GMF GSP	East	T202	UA	10/15/2010	12.27	16.65	17.21
GMF GSP	West	G206D	DA	1/25/2021	49.20	59.00	59.39
GMF GSP	West	G102	UA	4/28/2006	12.02	16.78	17.15
GMF GSP	West	G103	UA	2/15/2010	15.88	20.67	21.09
GMF GSP	West	G105	UA	2/16/2010	16.11	20.90	21.37
GMF GSP	West	G106	UA	2/16/2010	14.37	18.96	19.44
GMF GSP	West	G206	UA	10/14/2010	17.51	21.92	22.42
GMF GSP	West	R104	UA	10/8/2010	14.59	19.32	19.85
GMF GSP	West	R201	UA	10/8/2010	14.59	19.32	19.85
GMF RP	East	G275D	DA	1/14/2021	49.76	59.55	59.89
GMF RP	East	G283	LCU	1/14/2021	8.39	18.17	18.36
GMF RP	East	G285	LCU	1/25/2021	13.68	23.45	23.83
GMF RP	East	G270	UA	2/26/2008	13.13	17.92	18.27
GMF RP	East	G271	UA	9/10/2009	9.96	14.31	14.79
GMF RP	East	G272	UA	9/10/2009	9.11	13.98	14.32
GMF RP	East	G273	UA	9/10/2009	9.08	14.56	15.10

Associated GMF Pond	Side of Groundwater Divide	Well	Hydrogeologic Unit	Date Constructed	Screen Top Depth (ft bgs)	Screen Bottom Depth (ft bgs)	Well Depth (ft bgs)
GMF RP	East	G274	UA	9/16/2009	12.90	17.67	18.06
GMF RP	East	G275	UA	9/16/2009	8.22	12.62	13.19
GMF RP	East	G276	UA	9/16/2009	22.41	27.22	27.65
GMF RP	East	G277	UA	9/14/2009	14.29	18.77	19.24
GMF RP	East	G278	UA	9/11/2009	18.93	23.70	24.06
GMF RP	East	G279	UA	9/10/2009	22.40	26.79	27.30
GMF RP	East	G280	UA	2/26/2008	12.79	17.63	17.98
GMF RP	East	G284	UA	2/3/2021	8.08	12.85	13.23
GMF RP	East	G286	UA	1/18/2021	3.37	8.16	8.50
GMF RP	East	G287	UA	1/20/2021	5.43	10.25	10.59
GMF RP	East	G288	UA	1/19/2021	7.59	12.26	12.75
GMF RP	East	MW20S	UA	5/1/2007	8.41	13.22	13.67

Notes:

Source: Ramboll (2021a).

bgs = Below Ground Surface; DA = Deep Aquifer; ft = Feet; LCU = Lower Confining Unit; UA = Uppermost Aquifer.

Table 2.2 Groundwater Data Summary

Constituent	Samples with Constituent Detected	Samples Analyzed	Minimum Detected Value	Maximum Detected Value	Maximum Laboratory Detection Limit
Total Metals (mg/L)					
Antimony	2	434	0.0040	0.0045	0.0030
Arsenic	158	477	0.0010	0.11	0.0010
Barium	452	452	0.018	0.78	0.0010
Beryllium	9	436	0.0013	0.0067	0.0010
Boron	336	549	0.010	4.6	0.015
Cadmium	8	462	0.0012	0.0041	0.0010
Chromium	65	447	0.0040	0.086	0.0040
Cobalt	46	447	0.0021	0.053	0.0020
Lead	85	477	0.0010	0.082	0.0010
Lithium	25	265	0.011	0.030	0.020
Mercury	8	434	0.00024	0.0014	0.00020
Molybdenum	184	422	0.0010	0.044	0.0010
Selenium	189	451	0.0010	0.020	0.0010
Thallium	9	440	0.0010	0.0035	0.0010
Radionuclides (pCi/L)					
Radium-226 + 228	268	268	0	4.2	5.0
Other (mg/L, unless otherwise noted)					
Chloride	552	552	1.7	440	250
Fluoride	480	530	0.25	0.99	0.25
Sulfate	549	549	9.8	1,800	500
Total Dissolved Solids	555	555	230	3,400	26

Note:

pCi/L = PicoCuries Per Liter.

2.5 Surface Water Monitoring

Geosyntec Consultants collected a total of six surface water samples from Coffeen Lake (south of the GMF GSP and the GMF RP) in August 2021 (Geosyntec Consultants, 2021). The sample locations are shown in Figure 2.4, and the sampling results are summarized in Table 2.3.



Figure 2.4 Surface Water Sampling Locations. Source: Geosyntec Consultants (2021).

Table 2.3 Surface Water Data Summary

Constituent	Samples with Constituent Detected	Samples Analyzed	Minimum Detected Value	Maximum Detected Value	Maximum Laboratory Detection Limit
Total Metals (mg/L)					
Boron	5	5	0.086	0.33	0.05
Calcium	5	5	21	53	0.2
Cobalt	0	5	ND	ND	0.005
Iron	5	5	0.23	0.38	0.2
Lithium	0	5	ND	ND	0.01
Magnesium	5	5	10	16	0.1
Manganese	5	5	0.03	0.2	0.01
Potassium	5	5	2.5	4.9	0.5
Sodium	5	5	11	19	1
Other (mg/L)					
Chloride	5	5	7.2	11	0.4
Phosphorus	5	5	0.095	0.24	0.15
Sulfate	5	5	31	110	2
Total Dissolved Solids	5	5	120	240	10

Notes:

ND = Not Detected.

Source: Geosyntec Consultants (2021).

The background sample (BKG-1 on Figure 2.4) was not included in the summary statistics.

Surface water was analyzed for both total and dissolved metals; only the total metals are reported here, because they are generally higher concentrations than dissolved metals. The only exception was iron, which had a maximum dissolved concentration 1.8 times higher than the maximum total concentration. However, iron was not measured in groundwater, therefore, was not identified as a COI.

3 Risk Evaluation

3.1 Risk Evaluation Process

A risk evaluation was conducted to determine whether constituents present in groundwater underlying and downgradient of the GMF GSP and the GMF RP have the potential to pose adverse health effects to human and ecological receptors. The risk evaluation is consistent with the principles of risk assessment established by US EPA and has considered evaluation criteria detailed in Illinois guidance documents (e.g., IEPA, 2013, 2019).

The general risk evaluation approach is summarized in Figure 3.1 and discussed below.

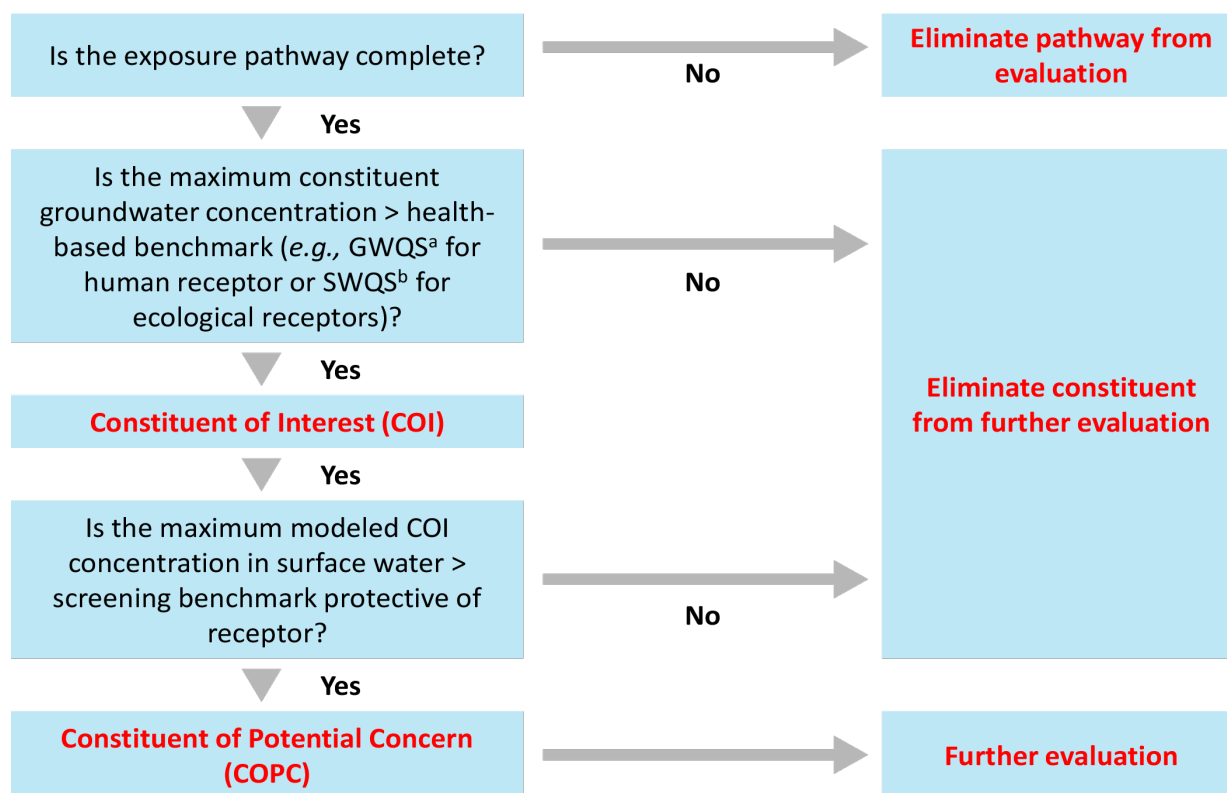


Figure 3.1 Overview of Risk Evaluation Methodology. GWQS = Groundwater Quality Standards; IEPA = Illinois Environmental Protection Agency; SWQS = Surface Water Quality Standards. (a) The IEPA Part 845 groundwater protection standards were used to identify COIs. (b) IEPA SWQS protective of chronic exposures to aquatic organisms were used to identify ecological COIs. In the absence of an SWQS, US EPA Region IV ecological screening values were used.

The first step in the risk evaluation was to develop the CEMs and identify complete exposure pathways. All potential receptors and exposure pathways based on groundwater use and surface water use in the vicinity of the Site were considered. Exposure pathways that were incomplete were excluded from the evaluation.

Groundwater data were used to identify COIs. COIs were identified as constituents with maximum concentrations in groundwater in excess of groundwater quality standards (GWQS)² for human receptors and SWQS for ecological receptors. Based on the CSM (Section 2.2), some groundwater underlying the GMF GSP and the GMF RP has the potential to interact with surface water in the unnamed tributary and Coffeen Lake. Therefore, constituents in groundwater potentially related to the GMF GSP and the GMF RP may potentially flow into the unnamed tributary and subsequently into surface water in Coffeen Lake.

Surface water samples have been collected from Coffeen Lake adjacent to the Site; however, sediment samples have not been collected from the lake. Gradient modeled the potential migration of COIs from groundwater to surface water and sediment to evaluate potential risks to receptors (see Section 3.3.3).

Gradient modeled the COI concentrations in surface water and sediment based on the groundwater data from the wells related to the GMF GSP and the GMF RP. The measured and modeled COI concentrations in surface water and sediment were compared to conservative, generic risk-based screening benchmarks for human health and ecological receptors. These generic screening benchmarks rely on default assumptions with limited consideration of site-specific characteristics. Human health benchmarks are receptor-specific values calculated for each pathway and environmental medium that are designed to be protective of human health. Ecological benchmarks are medium-specific values designed to be protective of all potential ecological receptors exposed to surface water. Ecological and human health screening benchmarks are inherently conservative because they are intended to screen out chemicals that are of no concern with a high level of confidence. Therefore, a measured or modeled COI concentration exceeding a screening benchmark does not indicate an unacceptable risk, but only that further risk evaluation is warranted. COIs with maximum concentrations exceeding a conservative screening benchmark are identified as COPCs requiring further evaluation.

As described in more detail below, this evaluation relied on the screening assessment to demonstrate that constituents present in groundwater underlying the GMF GSP and the GMF RP do not pose an unacceptable human health or ecological risk. That is, after the screening step, no COPCs were identified and further assessment was not warranted.

3.2 Human and Ecological Conceptual Exposure Models

A CEM provides an overview of the receptors and exposure pathways requiring risk evaluation. The CEM describes the source of the contamination, the mechanism that may lead to a release of contamination, the environmental media to which a receptor may be exposed, the route of exposure (exposure pathway), and the types of receptors that may be exposed to these environmental media.

3.2.1 Human Conceptual Exposure Model

The human CEM for the Site depicts the relationships between the off-Site environmental media potentially impacted by constituents in groundwater and human receptors that could be exposed to these media. Figure 3.2 presents a human CEM for the Site. It considers a human receptor who could be exposed to COIs hypothetically released from the GMF GSP and the GMF RP into groundwater, surface water, sediment, and fish. The following human receptors and exposure pathways were evaluated for inclusion in the Site-specific CEM.

² As discussed further in Section 3.3.2, GWQS are protective of human health and not necessarily of ecological receptors. While ecological receptors are not exposed to groundwater, groundwater can potentially enter into the adjacent surface water and impact ecological receptors. Therefore, two sets of COIs were identified: one for humans and another for ecological receptors.

- Residents – exposure to groundwater/surface water as drinking water;
- Residents – exposure to groundwater/surface water used for irrigation;
- Recreators in the lake adjacent to the Site:
 - Boaters – exposure to surface water and sediment while boating;
 - Swimmers – exposure to surface water and sediment while swimming;
 - Anglers – exposure to surface water and sediment and consumption of locally caught fish.

All of these exposure pathways were considered to be complete, except for residential exposure to groundwater or surface water used for drinking water or irrigation, and swimming. Section 3.2.1.1 explains why the residential drinking water and irrigation pathways are incomplete, and Section 3.2.1.2 provides additional description of the recreational exposures. While a recreator's potential exposure to surface water in Coffeen Lake was evaluated, swimming is prohibited in Coffeen Lake and thus was not evaluated (IDNR, 2014). Although swimming and boating are unlikely to occur in the unnamed tributary due to its shallow depth (flow depth of 2.1 feet) (Golder Associates Inc., 2020), the unnamed tributary was evaluated for recreator exposure due to its potential use by recreational anglers.

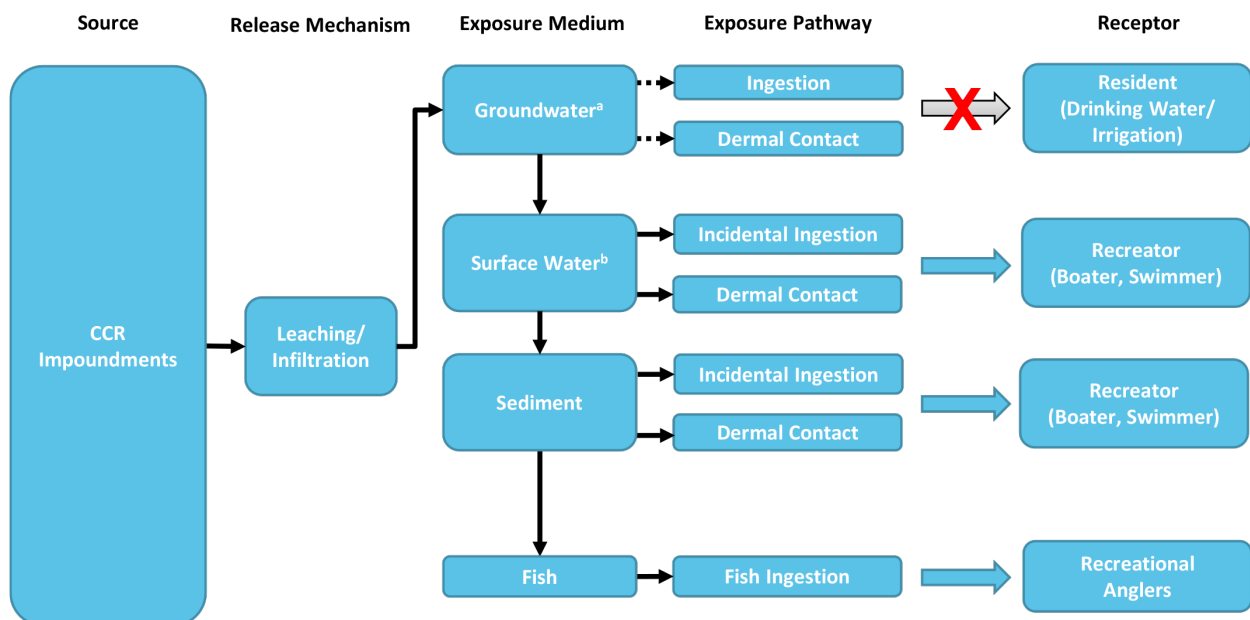


Figure 3.2 Human Conceptual Exposure Model. CCR = Coal Combustion Residuals. Dashed line/Red X = Incomplete or insignificant exposure pathway. (a) Groundwater in the vicinity of the Site is not used as a drinking water or irrigation source. (b) Surface water is not used as a drinking water source.

3.2.1.1 Groundwater or Surface Water as a Drinking Water/Irrigation Source

Using groundwater as a source of drinking water and/or irrigation water is not a complete exposure pathway for CCR-related constituents originating from the GMF GSP or the GMF RP. Specifically, there are no users of shallow groundwater from the UA in the vicinity of the GMF GSP or the GMF RP; thus, no receptors can be exposed to any CCR-related constituents in groundwater originating from either of the GMF ponds.

Relying on State databases, Ramboll completed a water well survey in 2021 (Ramboll, 2021a). A total of 18 water wells were identified within a 1,000-meter radius of the GMF ponds during a comprehensive search of the Illinois State Geological Survey's (ISGS) Illinois Water and Related Wells (ILWATER) Map (Ramboll, 2021a,b). These included 12 monitoring wells, 5 farm/domestic wells,³ and 1 industrial use well (Ramboll, 2021a,b) (Figure 3.3). There is no information available about the current use of these wells. However, site-specific groundwater flow conditions support the conclusion that none of the farm/domestic wells are or can ever be affected by potential CCR-related constituents originating from the GMF GSP/GMF RP.

- **There is no off-Site migration of CCR-related constituents in groundwater.** All groundwater originating from the GMF RP ultimately flows into the unnamed tributary, whereas a component of groundwater originating from the GMF GSP flows into the unnamed tributary, and the rest flows into the western branch of Coffeen Lake. Groundwater from the UA flows southeast/east before flowing into the unnamed tributary, and flows southwest/west before flowing into the western branch of Coffeen Lake (Ramboll, 2021a,b). Three (3) of the 5 farm/domestic wells (*i.e.*, Well IDs 73, 25, and 28) and one industrial well (*i.e.*, 08) located within the 1,000 m buffer area are upgradient of both the GMF GSP and the GMF RP (Figure 3.3). Therefore, there is no plausible mechanism by which those 4 wells (*i.e.*, 73, 08, 25, and 28) could be impacted by any potential constituents in groundwater associated with the GMF GSP and the GMF RP.

Two (2) water wells (73 and 02) are located on the northeast and southeast side of the unnamed tributary, *i.e.*, the opposite side of the tributary from GMF GSP and the GMF RP (Figure 3.3). The surface water bodies in the vicinity of the GMF ponds are hydraulic boundaries that prevent shallow groundwater from flowing past or underneath them. Furthermore, the surface waters are regional "sinks," which means that groundwater flows into the surface water bodies both from the east and the west, but cannot flow past. Thus, because the eastern branch of Coffeen Lake and the unnamed tributary separate those two farm/domestic wells (*i.e.*, 73 and 02) from the GMF GSP and the GMF RP (Figure 3.3), there is no plausible mechanism by which the wells could be impacted by any potential constituents in groundwater associated with the GMF GSP and the GMF RP.

There is one domestic/farm well located southeast of the GMF GSP and the GMF RP (Well ID 32 on Figure 3.3), on the west side of the unnamed tributary. It is likely that this well is not in use and not in existence. The well, which was installed in 1981, is located near the former location of several prior residents (Figure 3.4). However, the property in this area has been purchased by IPGC.

- **Coffeen Lake is not used as a public water supply.** Coffeen Lake is a cooling water pond owned and maintained by IPGC, and IPGC restricts the use of the lake as a source of drinking water. Therefore, the human exposure pathway of surface water ingestion (as potable water) adjacent to the GMF GSP is not a complete pathway and was not evaluated further.

³ The Ramboll 2021 GMF GSP Hydrogeologic Characterization Report states there are four farm/domestic wells, but Figure 3.3 in that report shows that there are five wells (Ramboll, 2021a).

- **The GMF GSP and the GMF RP have a limited hydraulic connection to underlying groundwater.** The LCU underlying the UA forms a hydraulic barrier between the GMF ponds and deeper groundwater resources. Due to the very low hydraulic conductivity of the LCU, downward migration of shallow groundwater is expected to be limited. Therefore, the likelihood of GMF pond-related impacts to deep groundwater is minimal.

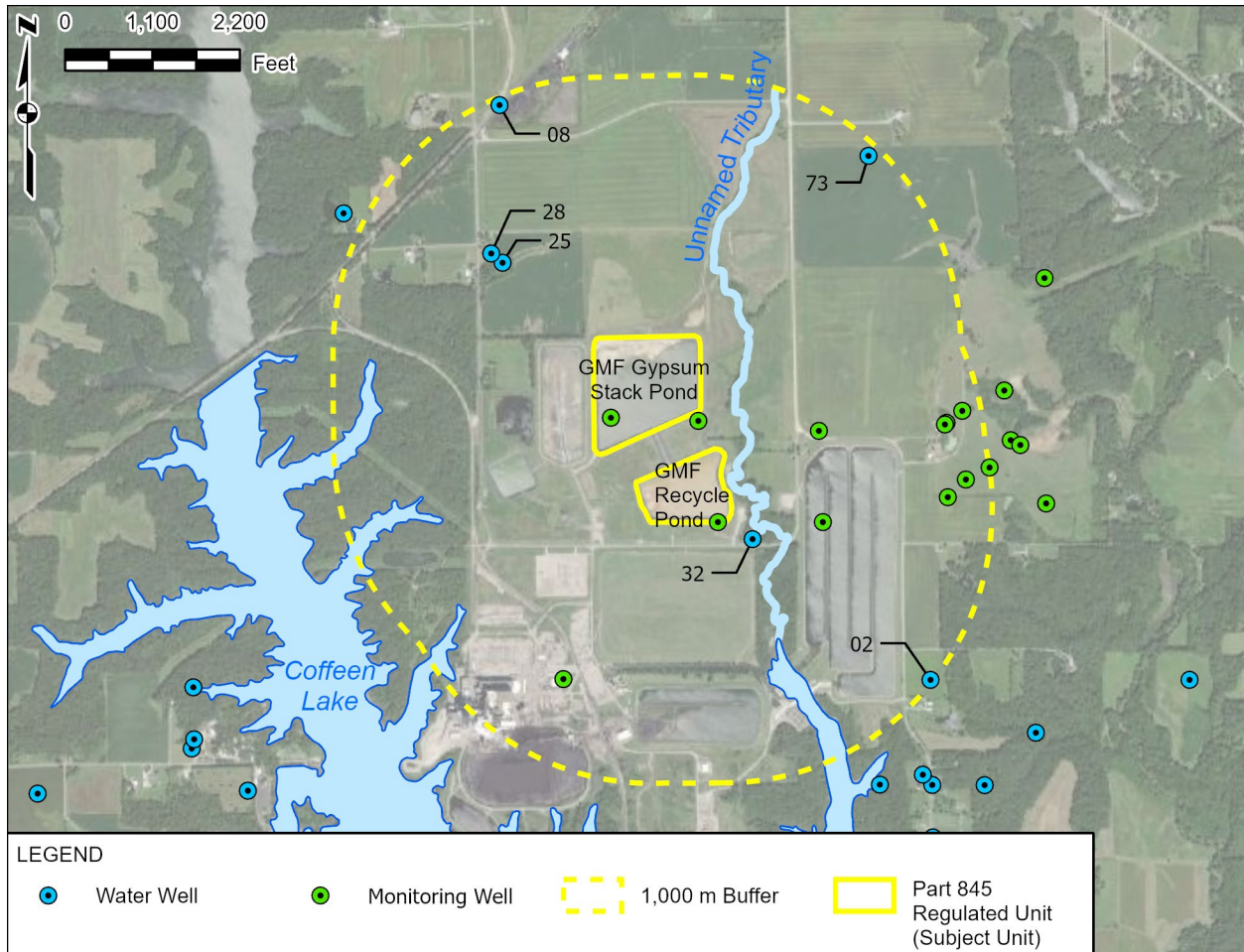


Figure 3.3 Water Wells Within 1,000 Meters of the GMF GSP and the GMF RP. GMF = Gypsum Manufacturing Facility; GSP = Gypsum Stack Pond; RP = Recycle Pond. The industrial well is shown as "08" in the northwest corner of the buffer zone. Sources: Ramboll (2021a,b).



Figure 3.4 Historic Property Use In the Vicinity of Well 32. (a) 1998; (b) 2005; (c) 2009. Sources: USGS (1998a,b, 2005a,b); USDA (2009a,b).

3.2.1.2 Recreational Exposures

Coffeen Lake is located adjacent to the Site, and is owned by IPGC. Property along the lake has been leased to Illinois Department of Natural Resources (IDNR) for use as a State Fish and Wildlife Area (Ramboll, 2021a), and the lake is used for recreational fishing (IDNR, 2022). Recreational exposure to surface water and sediment may occur during activities such as boating or fishing in the lake. Recreational anglers may also consume locally caught fish from Coffeen Lake. While a recreator's potential exposure to surface water in Coffeen Lake was evaluated, swimming is prohibited in Coffeen Lake and thus was not evaluated (IDNR, 2014). Although swimming and boating are unlikely to occur in the unnamed tributary due to its shallow depth (flow depth of 2.1 feet) (Golder Associates Inc., 2020), the unnamed tributary was evaluated for recreator exposure due to its potential use by recreational anglers.

3.2.2 Ecological Conceptual Exposure Model

The ecological CEM for the Site depicts the relationships between off-Site environmental media (surface water and sediment) potentially impacted by COIs in groundwater and ecological receptors that may be exposed to these media. The ecological risk evaluation considered both direct toxicity as well as secondary toxicity *via* bioaccumulation. Figure 3.4 presents the ecological CEM for the Site. The following ecological receptor groups and exposure pathways were considered:

- **Ecological Receptors Exposed to Surface Water:**
 - Aquatic plants, amphibians, reptiles, and fish.
- **Ecological Receptors Exposed to Sediment:**
 - Benthic invertebrates (*e.g.*, insects, crayfish, mussels).
- **Ecological Receptors Exposed to Bioaccumulative COIs:**
 - Higher trophic-level wildlife (avian and mammalian) *via* direct exposures (surface water and sediment exposure) and secondary exposures through the consumption of prey (*e.g.*, plants, invertebrates, small mammals, fish).

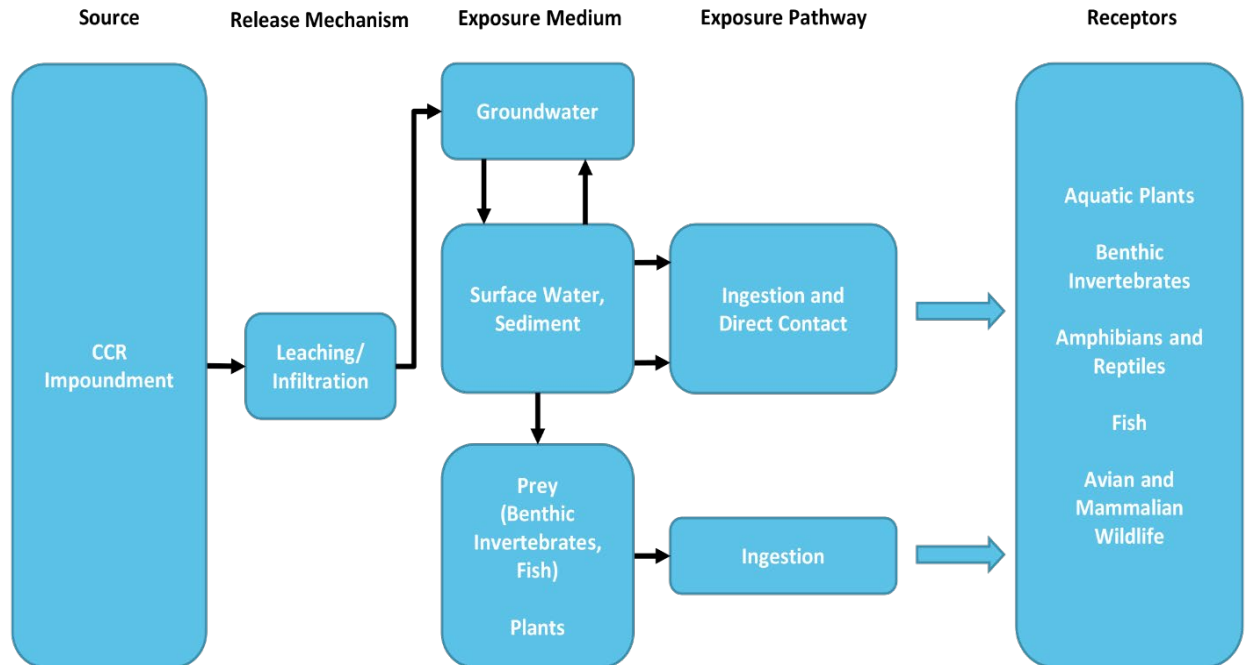


Figure 3.5 Ecological Conceptual Exposure Model. CCR = Coal Combustion Residuals.

3.3 Identification of Constituents of Interest

Risks were evaluated for COIs. A constituent was considered a COI if the maximum detected constituent concentration in groundwater exceeded a health-based benchmark. According to US EPA risk assessment guidance (US EPA, 1989), this screening step is designed to reduce the number of constituents carried through the risk evaluation that are anticipated to have a minimal contribution to the overall risk. Identified COIs are the constituents that are most likely to pose a risk concern in the surface water adjacent to the Site.

3.3.1 Human Health Constituents of Interest

For the human health risk evaluation, COIs were conservatively identified as constituents with maximum concentrations in groundwater above the GWPS listed in the Illinois CCR Rule Part 845.600 (IEPA, 2021). Gradient used the maximum detected concentrations from groundwater samples collected from all of the wells associated with the GMF GSP and the GMF RP, regardless of hydrostratigraphic unit. The use of groundwater data in this risk evaluation does not imply that detected constituents are associated with the GMF GSP or the GMF RP or that they have been identified as potential groundwater exceedances. Using this approach, six COIs (arsenic, beryllium, boron, cobalt, lead, and thallium) were identified for the human health risk evaluation *via* the surface water pathway for the unnamed tributary (Table 3.1) and two COIs (beryllium and lead) were identified for the western branch of Coffeen Lake (Table 3.2).

The water quality parameters that exceeded the GWPS included chloride, sulfate, and total dissolved solids; however, these constituents were not included in the risk evaluation because the GWPS are based on aesthetic quality. The US EPA secondary maximum contaminant levels (MCLs) for chloride, sulfate, and total dissolved solids are based on aesthetic quality. The secondary MCLs for chloride and sulfate (250 mg/L) are based on salty taste (US EPA, 2021a). The secondary MCL for total dissolved solids (500 mg/L) is based on hardness, deposits, colored water, staining, and salty taste (US EPA, 2021a). Given that these

parameters are not likely to pose a human health risk concern in the event of exposure, they were not considered to be human health COIs.

Table 3.1 Human Health Constituents of Interest for the Unnamed Tributary

Constituent ^a	Maximum Concentration	GWPS ^b	Human Health COI ^c
Total Metals (mg/L)			
Antimony	0.0045	0.0060	No
Arsenic	0.11	0.010	Yes
Barium	0.78	2.0	No
Beryllium	0.0042	0.0040	Yes
Boron	4.6	2.0	Yes
Cadmium	0.0041	0.0050	No
Chromium	0.086	0.10	No
Cobalt	0.053	0.0060	Yes
Lead	0.082	0.0075	Yes
Lithium	0.030	0.040	No
Mercury	0.0014	0.0020	No
Molybdenum	0.043	0.10	No
Selenium	0.020	0.050	No
Thallium	0.0035	0.0020	Yes
Radionuclides (pCi/L)			
Radium-226 + 228	4.2	5.0	No
Other (mg/L, unless otherwise noted)			
Chloride	440	200	No ^d
Fluoride	0.73	4.0	No
Sulfate	1,800	400	No ^d
Total Dissolved Solids	3,400	1,200	No ^d

Notes:

COI = Constituent of Interest; GMF = Gypsum Manufacturing Facility; GSP = Gypsum Stack Pond; GWPS = Groundwater Protection Standards; IEPA = Illinois Environmental Protection Agency; MCL = Maximum Contaminant Level; pCi/L = PicoCuries Per Liter; RP = Recycle Pond.

Shaded = Compound identified as a COI.

(a) The constituents are those listed in the Illinois Part 845.600 GWPS (IEPA, 2021). This table presents the maximum concentration from all wells from the GMF GSP and the GMF RP combined.

(b) The Illinois Part 845.600 GWPS (IEPA, 2021) were used to identify COIs.

(c) COIs are constituents for which the maximum concentration exceeds the groundwater standard.

(d) This constituent is not likely to pose a human health risk concern due to the absence of studies regarding toxicity to human health. Therefore, this constituent is not considered a COI.

Table 3.2 Human Health Constituents of Interest for the Western Branch of Coffeen Lake

Constituent ^a	Maximum Concentration	GWPS ^b	Human Health COI ^c
Total Metals (mg/L)			
Antimony	ND	0.0060	No
Arsenic	0.010	0.010	No
Barium	0.17	2.0	No
Beryllium	0.0067	0.0040	Yes
Boron	0.13	2.0	No
Cadmium	0.0012	0.0050	No
Chromium	0.032	0.10	No
Cobalt	0.0058	0.0060	No
Lead	0.0097	0.0075	Yes
Lithium	0.016	0.040	No
Mercury	0.0011	0.0020	No
Molybdenum	0.044	0.10	No
Selenium	0.0091	0.050	No
Thallium	ND	0.0020	No
Radionuclides (pCi/L)			
Radium-226 + 228	2.8	5.0	No
Other (mg/L, unless otherwise noted)			
Chloride	88	200	No ^d
Fluoride	0.99	4.0	No
Sulfate	600	400	No ^d
Total Dissolved Solids	1,300	1,200	No ^e

Notes:

COI = Constituent of Interest; GMF = Gypsum Manufacturing Facility; GSP = Gypsum Stack Pond; GWPS = Groundwater Protection Standards; IEPA = Illinois Environmental Protection Agency; MCL = Maximum Contaminant Level; pCi/L = PicoCuries Per Liter; RP = Recycle Pond.

Shaded = Compound identified as a COI.

(a) The constituents are those listed in the Illinois Part 845.600 GWPS (IEPA, 2021). This table presents the maximum concentration from all wells from the GMF GSP and the GMF RP combined.

(b) The Illinois Part 845.600 GWPS (IEPA, 2021) were used to identify COIs.

(c) COIs are constituents for which the maximum concentration exceeds the groundwater standard.

(d) This constituent is not likely to pose a human health risk concern due to the absence of studies regarding toxicity to human health. Therefore, this constituent is not considered a COI.

(e) Total dissolved solids are not considered a COI because the MCL is based on aesthetic quality.

3.3.2 Ecological Constituents of Interest

The Illinois GWPS, as defined in IEPA's guidance, were developed to protect human health but not necessarily ecological receptors. While ecological receptors are not exposed to groundwater, groundwater can potentially migrate into the adjacent surface water and impact ecological receptors. Therefore, to identify ecological COIs, the maximum concentrations of constituents detected in groundwater were compared to ecological surface water benchmarks protective of aquatic life.

The surface water screening benchmarks for freshwater organisms were obtained from the following hierarchy of sources:

- IEPA (2019) SWQS. IEPA SWQS are health-protective benchmarks for aquatic life exposed to surface water on a long-term basis (*i.e.*, chronic exposure). The SWQS for several metals are hardness-dependent (cadmium, chromium, copper, lead, manganese, nickel, and zinc). Screening benchmarks for these constituents were calculated assuming US EPA's default hardness of 100 mg/L (US EPA, 2022).⁴
- US EPA Region IV (2018) surface water Ecological Screening Values (ESVs) for hazardous waste sites.

Benchmarks from the United States Department of Energy's (US DOE) guidance document ("A Graded Approach for Evaluating Radiation Doses to Aquatic and Terrestrial Biota") were used for radium (US DOE, 2019). US DOE presents benchmarks for radium-226 and radium-228 (4 and 3 pCi/L, respectively). Given that radium concentrations are expressed as total radium (radium-226+228, *i.e.*, the sum of radium-226 and radium-228), Gradient used the lower of the two benchmarks (3 pCi/L for radium-228) to evaluate total radium concentrations.

Consistent with the human health risk evaluation, Gradient used the maximum detected concentrations from groundwater samples collected from all of the wells associated with the GMF GSP and the GMF RP, (regardless of hydrostratigraphic unit) without considering spatial or temporal representativeness for ecological receptor exposures. The use of the maximum constituent concentrations in this evaluation is designed to conservatively identify COIs that warrant further investigation. Cadmium, cobalt, lead, mercury, and radium-226+228 were identified as COIs for ecological receptors at the unnamed tributary (Table 3.3), and cadmium was identified as a COI for the western branch of Coffeen Lake (Table 3.4).

⁴ Hardness data are not available for Coffeen Lake or the unnamed tributary adjacent to the Site; therefore, the US EPA (2022) default hardness of 100 mg/L was used. Use of a higher hardness value would result in less stringent screening values; thus, use of the US EPA default hardness is conservative.

Table 3.3 Ecological Constituents of Interest for the Unnamed Tributary

Constituents ^a	Maximum Groundwater Concentration	Ecological Benchmark ^b	Basis	Ecological COI ^c
Total Metals (mg/L)				
Antimony	0.0045	0.19	US EPA R4 ESV	No
Arsenic	0.11	0.19	IEPA SWQC	No
Barium	0.78	5.0	IEPA SWQC	No
Beryllium	0.0042	0.064	US EPA R4 ESV	No
Boron	4.6	7.6	IEPA SWQC	No
Cadmium	0.0041	0.0011	IEPA SWQC	Yes
Chromium	0.086	0.21	IEPA SWQC	No
Cobalt	0.053	0.019	US EPA R4 ESV	Yes
Lead	0.082	0.020	IEPA SWQC	Yes
Lithium	0.030	0.44	US EPA R4 ESV	No
Mercury	0.0014	0.0011	IEPA SWQC	Yes
Molybdenum	0.043	7.2	US EPA R4 ESV	No
Selenium	0.020	1.0	IEPA SWQC	No
Thallium	0.0035	0.0060	US EPA R4 ESV	No
Radionuclides (pCi/L)				
Radium-226 + 228	4.2	3.0	US DOE	Yes
Other (mg/L, unless otherwise noted)				
Chloride	440	500	IEPA SWQC	No
Fluoride	0.73	4.0	IEPA SWQC	No
Sulfate	1,800	NA	NA	No
Total Dissolved Solids	3,400	NA	NA	No

Notes:

COI = Constituent of Interest; ESV = Ecological Screening Value; GWPS = Groundwater Protection Standards; IEPA = Illinois Environmental Protection Agency; SWQC = Surface Water Quality Criteria; NA = Not Available; pCi/L = PicoCuries Per Liter; US DOE = United States Department of Energy; US EPA R4 = United States Environmental Protection Agency Region IV.

Shaded = Compound identified as a COI.

(a) The constituents are those listed in the Illinois Part 845.600 GWPS (IEPA, 2021). This table presents the maximum concentration from all wells from the GMF GSP and the GMF RP combined.

(b) Ecological benchmarks are from the hierarchy of sources discussed in Section 3.3.2: IEPA SWQS (IEPA, 2019); US EPA R4 "Ecological Risk Assessment Supplemental Guidance" (US EPA Region IV, 2018); and US DOE's guidance document "A Graded Approach for Evaluating Radiation Doses to Aquatic and Terrestrial Biota" (US DOE, 2019).

(c) Constituents with maximum detected concentrations exceeding a benchmark protective of surface water exposure are considered ecological COIs.

Table 3.4 Ecological Constituents of Interest for the Western Branch of Coffeen Lake

Constituents ^a	Maximum Groundwater Concentration	Ecological Benchmark ^b	Basis	Ecological COI ^c
Total Metals (mg/L)				
Antimony	ND	0.19	US EPA R4 ESV	No
Arsenic	0.010	0.19	IEPA SWQC	No
Barium	0.17	5.0	IEPA SWQC	No
Beryllium	0.0067	0.064	US EPA R4 ESV	No
Boron	0.13	7.6	IEPA SWQC	No
Cadmium	0.0012	0.0011	IEPA SWQC	Yes
Chromium	0.032	0.21	IEPA SWQC	No
Cobalt	0.0058	0.019	US EPA R4 ESV	No
Lead	0.0097	0.020	IEPA SWQC	No
Lithium	0.016	0.44	US EPA R4 ESV	No
Mercury	0.0011	0.0011	IEPA SWQC	No
Molybdenum	0.044	7.2	US EPA R4 ESV	No
Selenium	0.0091	1.0	IEPA SWQC	No
Thallium	ND	0.0060	US EPA R4 ESV	No
Radionuclides (pCi/L)				
Radium-226 + 228	2.8	3.0	US DOE	No
Other (mg/L, unless otherwise noted)				
Chloride	88	500	IEPA SWQC	No
Fluoride	0.99	4.0	IEPA SWQC	No
Sulfate	600	0	NA	No
Total Dissolved Solids	1,300	0	NA	No

Notes:

COI = Constituent of Interest; ESV = Ecological Screening Value; GWPS = Groundwater Protection Standards; IEPA = Illinois Environmental Protection Agency; NA = Not Available; pCi/L = PicoCuries Per Liter; SWQC = Surface Water Quality Criteria; US DOE = United States Department of Energy; US EPA R4 = United States Environmental Protection Agency Region IV.

Shaded = Compound identified as a COI.

(a) The constituents are those listed in the Illinois Part 845.600 GWPS (IEPA, 2021). This table presents the maximum concentration from all wells from the GMF GSP and the GMF RP combined.

(b) Ecological benchmarks are from the hierarchy of sources discussed in Section 3.3.2: IEPA SWQS (IEPA, 2019); US EPA R4 "Ecological Risk Assessment Supplemental Guidance" (US EPA Region IV, 2018); and US DOE's guidance document "A Graded Approach for Evaluating Radiation Doses to Aquatic and Terrestrial Biota" (US DOE, 2019).

(c) Constituents with maximum detected concentrations exceeding a benchmark protective of surface water exposure are considered ecological COIs.

3.4 Surface Water and Sediment Modeling

Surface water sampling has been conducted in Coffeen Lake adjacent to the Site. To estimate the potential contribution to surface water (and sediment) from groundwater specifically associated with the GMF GSP and the GMF RP, Gradient modeled concentrations in surface water and sediment from two water bodies (the unnamed tributary and western branch of Coffeen Lake) for the human and ecological COIs in groundwater. This is because the constituents detected in groundwater above an ecological or health-based benchmark are most likely to pose a risk concern in the adjacent surface waters. Gradient modeled human health and ecological COI concentrations in the surface water and sediment using a mass balance calculation based on the surface water and groundwater mixing. The model assumes a well-mixed groundwater-surface water location. The maximum detected concentrations in groundwater (regardless of well location) from 2015 to 2021 were conservatively used to model COI concentrations in surface water and sediment.

Groundwater data from the east side of the groundwater divide were used for modeling surface water concentrations in the unnamed tributary. Groundwater data from the west side of the groundwater divide were used for modeling surface water concentrations in the western branch of Coffeen Lake. All of the GMF RP wells (n = 19) and a subset of the GMF GSP wells (n = 16), located on the east side of the divide, were used to model surface water concentrations in the unnamed tributary. Eight (8) of the GMF GSP wells, located on the west side of the divide, were used to model surface water concentrations in the western branch of Coffeen Lake (Figure 2.1). Due to the fact that different sets of wells were used to model surface water concentrations to the east and west, the list of COIs differs on the east and west sides of the groundwater divide. The surface water modeling for the unnamed tributary and the western branch of Coffeen Lake included only the COIs relevant for the east and west sides of the groundwater divide, respectively. The human health and/or ecological COIs on the east side of the divide included arsenic, beryllium, boron, cadmium, cobalt, lead, mercury, thallium, and radium-226+228. The human health and/or ecological COIs on the west side of the divide included beryllium, cadmium, and lead.

The groundwater data were measured as total metals. Use of the total metals concentration for these COIs may overestimate surface water concentrations because dissolved concentrations, which are lower than total concentrations, represent the mobile fractions of constituents that could likely flow into and mix with surface water.

The modeling approach does not account for geochemical transformations that may occur during groundwater mixing with surface water. Gradient assumed that predicted surface water concentrations were influenced only by the physical mixing of groundwater as it enters the surface water and were not further influenced by the geochemical reactions in the water and sediment, such as precipitation. In addition, the model only predicts surface water and sediment concentrations as a result of the potential migration of COI concentrations in groundwater related to the GMF GSP and the GMF RP and does not account for background concentrations in surface water or sediment.

For this evaluation, Gradient adapted a simplified and conservative form of US EPA's indirect exposure assessment methodology (US EPA, 1998) that was used in US EPA's coal combustion waste risk assessment (US EPA, 2014a). The model is a mass balance calculation based on surface water and groundwater mixing and the concept that the dissolved and sorbed concentrations can be related through an equilibrium partitioning coefficient (K_d). The model assumes a well-mixed groundwater-surface water location, with partitioning among total suspended solids, dissolved water column, sediment pore water, and solid sediments.

Sorption to soil and sediment is highly dependent on the surrounding geochemical conditions. To be conservative, we ignored the natural attenuation capacity of soil and sediment and estimated the surface water concentration based only on the physical mixing of groundwater and surface water (*i.e.*, dilution) at the point of entry of groundwater to the surface water.

The aquifer and surface water properties used to estimate the volume of groundwater flowing into the unnamed tributary and surface water concentrations are presented in Table 3.5. The COI concentrations in sediment in the unnamed tributary were modeled using the COI-specific sediment-to-water partitioning coefficients and the sediment properties presented in Table 3.6. In the absence of Site-specific information, Gradient used default assumptions (*e.g.*, depth of the upper benthic layer and bed sediment porosity) to model sediment concentrations. The modeled surface water and sediment concentrations for the unnamed tributary are presented in Table 3.7. These modeled concentrations reflect conservative contributions from groundwater flow.

The groundwater and surface water properties used in modeling for the western branch of Coffeen Lake are presented in Table 3.8. The sediment properties used in modeling for the western branch of Coffeen Lake are presented in Table 3.9. The modeled surface water and sediment concentrations for the western branch of Coffeen Lake are presented in Table 3.10. A description of the modeling and the detailed results are presented in Appendix A.

Table 3.5 Groundwater and Surface Water Properties Used in Modeling for the Unnamed Tributary

Parameter	Unit	Values	Notes/Source
Groundwater			
COI Concentration	mg/L	Constituent-specific	Maximum detected concentration in groundwater
Cross Section Area for the Uppermost Aquifer ^a	m ²	664	The average thickness of the UA near the GMF ponds (<i>i.e.</i> , approximately 3 ft) multiplied by the total length of the GMF ponds near the unnamed tributary (<i>i.e.</i> , ~726 m) (Ramboll, 2021b).
Hydraulic Gradient	m/m	0.0075	The average hydraulic gradient determined for the UA towards the unnamed tributary (Ramboll, 2021b).
Hydraulic Conductivity of the Uppermost Aquifer	cm/s	0.0013	The average of the geometric mean horizontal hydraulic conductivities measured for the UA (Ramboll, 2021b).
Surface Water			
Surface Water Flow Rate in the Unnamed Tributary	L/yr	8.04×10^{10}	There are no flow records available for the unnamed tributary that flows from north to south into the eastern branch of Coffeen Lake. According to Golder Associates Inc. (2020), the flow rate was assumed to be 90 cfs.
Total Suspended Solids	mg/L	3.2	Average Coffeen Lake concentration (Hanson, 2020).
Depth of the Water Column	m	0.64	Flow depth of the unnamed tributary (Golder Associates Inc., 2020).
Suspended Sediment to Water Partition Coefficient	mg/L	Constituent-specific	Values based on US EPA (2014a).

Notes:

cfs = Cubic Feet per Second; cm/s = Centimeter Per Second; COI = Constituent of Interest; ft = feet; GMF = Gypsum Management Facility; UA = Uppermost Aquifer; US EPA = United States Environmental Protection Agency.

(a) Cross-sectional area represents the area through which groundwater flows from the UA into the unnamed tributary.

Table 3.6 Sediment Properties Used in Modeling for the Unnamed Tributary

Parameter	Unit	Value	Notes/Source
Sediment			
Depth of Upper Benthic Layer	m	0.03	Default (US EPA, 2014a)
Depth of Water Body	m	0.67	Flow depth of the unnamed tributary (Golder Associates Inc., 2020) plus the depth of the upper benthic layer
Bed Sediment Particle Concentration	g/cm ³	1	Default (US EPA, 2014a)
Bed Sediment Porosity	-	0.6	Default (US EPA, 2014a)
TSS Mass Per Unit Area	kg/m ²	0.0038	Depth of water column × TSS × conversion factors (10 ⁻⁶ kg/mg and 1,000 L/m ³)
Sediment Mass Per Unit Area	kg/m ²	30	Depth of upper benthic layer × bed sediment particulate concentration × conversion factors (0.001 kg/g, 10 ⁶ cm ³ /m ³)
Sediment to Water Partition Coefficients	mg/L	Constituent specific	Values based on US EPA (2014a)

Notes:

TSS = Total Suspended Solids; US EPA = United States Environmental Protection Agency.

Table 3.7 Surface Water and Sediment Modeling Results for the Unnamed Tributary

COI	Groundwater Concentration (mg/L or pCi/L)	Mass Discharge Rate (mg/year or pCi/year)	Total Water Column Concentration (mg/L or pCi/L)	Concentration Sorbed to Bottom Sediments (mg/kg or pCi/kg)
Total Metals				
Arsenic	0.11	2.2E+05	2.9E-06	7.0E-04
Beryllium	0.0042	8.6E+03	1.1E-07	6.4E-05
Boron	4.6	9.4E+06	1.2E-04	7.4E-04
Cadmium	0.0041	8.4E+03	1.1E-07	1.5E-04
Cobalt	0.053	1.1E+05	1.4E-06	1.3E-03
Lead	0.082	1.7E+05	2.2E-06	2.2E-02
Mercury	0.0014	2.9E+03	3.7E-08	1.3E-03
Thallium	0.0035	7.1E+03	9.3E-08	1.7E-06
Radionuclides				
Radium-226 + 228	4.2	8.6E+06	1.1E-04	7.9E-01

Notes:

COI = Constituent of Concern; K_d = Equilibrium Partition Coefficient; pCi/L = PicoCuries Per Liter; pCi/kg = PicoCuries Per Kilogram.

Table 3.8 Groundwater and Surface Water Properties Used in Modeling for the Western Branch of Coffeen Lake

Parameter	Unit	Values	Notes/Source
Groundwater			
COI Concentration	mg/L	Constituent-specific	Maximum detected concentration in groundwater
Cross Section Area for the Uppermost Aquifer ^a	m ²	427	The average thickness of the UA near the GMF GSP (<i>i.e.</i> , approximately 3 ft) multiplied by the length of the GMF GSP on the west side of the divide (<i>i.e.</i> , about 467 m) (Ramboll, 2021a).
Hydraulic Gradient	m/m	0.018	The average hydraulic gradient for the UA from the GMF GSP towards the western branch of Coffeen Lake (Ramboll, 2021a).
Hydraulic Conductivity of the Uppermost Aquifer	cm/s	0.0014	The geometric mean horizontal hydraulic conductivity measured for the UA near the GMF GSP (Ramboll, 2021a).
Surface Water			
Surface Water Flow Rate in the western branch of Coffeen Lake	L/yr	9.02×10^{11}	There are no flow records available for the western branch of Coffeen Lake. According to the USGS (2022) Streamstats program, the western branch of Coffeen Lake has a two-year flow peak flow prediction of 1,010 cfs.
Total Suspended Solids	mg/L	3.2	Average Coffeen Lake concentration (Hanson, 2020).
Depth of the Water Column	m	5.7	Mean depth of Coffeen Lake (Austen <i>et al.</i> , 1993).
Suspended Sediment to Water Partition Coefficient	mg/L	Constituent-specific	Values based on US EPA (2014a)

Notes:

cfs = Cubic Feet Per Second; cm/s = Centimeter Per Second; COI = Constituent of Interest; ft = feet; GMF = Gypsum Management Facility; GSP = Gypsum Stack Pond; UA = Uppermost Aquifer; US EPA = United States Environmental Protection Agency; USGS = United States Geological Survey; yr = Year.

(a) Cross-sectional area represents the area through which groundwater flows from the UA into Coffeen Lake (*i.e.*, the groundwater flow area that intersects with Coffeen Lake).

Table 3.9 Sediment Properties Used in Modeling for the Western Branch of Coffeen Lake

Parameter	Unit	Value	Notes/Source
Sediment			
Depth of Upper Benthic Layer	m	0.03	Default (US EPA, 2014a)
Depth of Water Body	m	5.73	Depth of water column (5.7 m, depth of Coffeen Lake) (Austen <i>et al.</i> , 1993) plus depth of upper benthic layer (0.03 m) (US EPA, 2014a)
Bed Sediment Particle Concentration	g/cm ³	1	Default (US EPA, 2014a)
Bed Sediment Porosity	-	0.6	Default (US EPA, 2014a)
TSS Mass per Unit Area	kg/m ²	0.0342	Depth of water column × TSS × conversion factors (10 ⁻⁶ kg/mg and 1,000 L/m ³)
Sediment Mass per Unit Area	kg/m ²	30	Depth of upper benthic layer × bed sediment particulate concentration × conversion factors (0.001 kg/g, 10 ⁶ cm ³ /m ³)
Sediment to Water Partition Coefficients	mg/L	Constituent-specific	Values based on US EPA (2014a)

Notes:

TSS = Total Suspended Solids; US EPA = United States Environmental Protection Agency.

Table 3.10 Surface Water and Sediment Modeling Results for the Western Branch of Coffeen Lake

COI	Groundwater Concentration (mg/L)	Mass Discharge Rate (mg/year)	Total Water Column Concentration (mg/L)	Concentration Sorbed to Bottom Sediments (mg/kg)
Total Metals				
Beryllium	0.01	2.3E+04	2.6E-08	1.5E-05
Cadmium	0.0012	4.1E+03	4.6E-09	6.2E-06
Lead	0.010	3.3E+04	3.7E-08	3.7E-04

Note:

COI = Constituent of Concern.

3.5 Human Health Risk Evaluation

The section below presents the results of the human health risk evaluation for recreators in Coffeen Lake and the unnamed tributary adjacent to the Site. Risks were assessed using the maximum measured or modeled COIs in surface water.

3.5.1 Recreators Exposed to Surface Water

Screening Exposures: Recreators could be exposed to surface water *via* incidental ingestion and dermal contact while boating. In addition, anglers could consume fish caught in Coffeen Lake or the unnamed tributary. The maximum measured or modeled COI concentrations in surface water were used as conservative upper-end estimates of the COI concentrations to which a recreator might be exposed directly (incidental ingestion of COIs in surface water while boating) and indirectly (consumption of locally caught fish exposed to COIs in surface water).

Screening Benchmarks: Illinois surface water criteria (IEPA, 2019), known as human threshold criteria (HTC), are based on incidental exposure through contact or ingestion of small volumes of water while swimming or during other recreational activities, as well as the consumption of fish. The HTC values were calculated from the following equation (IEPA, 2019):

$$HTC = \frac{ADI}{W + (F \times BCF)}$$

where:

HTC = Human health protection criterion in milligrams per liter (mg/L)
ADI = Acceptable daily intake (mg/day)
W = Water consumption rate (L/day)
F = Fish consumption rate (kg/day)
BCF = Bioconcentration factor (L/kg-tissue)

Illinois defines the acceptable daily intake (ADI) as the "maximum amount of a substance which, if ingested daily for a lifetime, results in no adverse effects to humans" (IEPA, 2019). US EPA defines its chronic reference dose (RfD) as an "estimate (with uncertainty spanning perhaps an order of magnitude) of a daily oral exposure for a chronic duration (up to a lifetime) to the human population (including sensitive subgroups) that is likely to be without an appreciable risk of deleterious effects during a lifetime" (US EPA, 2011a). Illinois lists methods to derive an ADI from the primary literature (IEPA, 2019). In accordance with Illinois guidance, Gradient derived an ADI by multiplying the MCL by the default water ingestion rate of 2 L/day (IEPA, 2019). In the absence of an MCL, Gradient applied the RfD used by US EPA to derive its Regional Screening Levels (RSLs) (US EPA, 2021b) as a conservative estimate of the ADI. The RfDs are given in mg/kg-day, while the ADIs are given in mg/day; thus, Gradient multiplied the RfD by a standard body weight of 70 kg to obtain the ADI in mg/day. The calculation of the HTC values is shown in Appendix B, Table B.1.

Gradient used bioconcentration factors (BCFs) from a hierarchy of sources. The primary BCFs were those that US EPA used to calculate the National Recommended Water Quality Criteria (NRWQC) for human health (US EPA, 2002). Other sources included BCFs used in the US EPA coal combustion ash risk assessment (US EPA, 2014a) and BCFs reported by Oak Ridge National Laboratory's Risk Assessment Information System (ORNL RAIS) (ORNL, 2020).⁵ Lithium did not have a BCF value available from any authoritative source; therefore, the water quality criterion for lithium was calculated assuming a BCF of 1. This is a conservative assumption, as lithium does not readily bioaccumulate in the aquatic environment (ECHA, 2020).

Illinois recommends a fish consumption rate of 0.020 kg/day (20 g/day) for an adult weighing 70 kg (IEPA, 2019). Illinois recommends a water consumption rate of 0.01 L/day for "incidental exposure through contact or ingestion of small volumes of water while swimming or during other recreational activities" (IEPA, 2019). Appendix B, Table B.1 presents the calculated HTC for fish and water and for fish consumption only.

Screening Risk Evaluation: The maximum modeled and measured COI concentrations in surface water were compared to the calculated Illinois HTC values for the unnamed tributary and the western branch of Coffeen Lake in Tables 3.11 and 3.12, respectively. Surface water samples were collected from Coffeen Lake, but not the unnamed tributary. All surface water concentrations were below their respective

⁵ Although recommended by US EPA (2015b), US EPA EpiSuite 4.1 (US EPA, 2019) was not used as a source of BCFs because inorganic compounds are outside the estimation domain of the program.

benchmarks. The HTC values are protective of recreational exposure *via* water and/or fish ingestion and do not account for dermal exposures to COIs in surface water while boating. However, given that the measured and modeled COI surface water concentrations are orders of magnitude below HTC protective of water and/or fish ingestion, dermal exposures to COIs are not expected to be a risk concern. Moreover, the dermal uptake of metals is considered to be minimal and only a small proportion of ingestion exposures. Thus, none of the COIs evaluated would be expected to pose an unacceptable risk to recreators exposed to surface water while boating in Coffeen Lake and anglers consuming fish caught in Coffeen Lake or the unnamed tributary.

Table 3.11 Risk Evaluation for Recreators Exposed to Surface Water in the Unnamed Tributary

COI	Modeled Surface Water Concentration ^a	HTC for Water and Fish	HTC for Water Only	HTC for Fish Only	COPC
Total Metals (mg/L)					
Arsenic	2.9E-06	0.022	2.0	0.023	No
Beryllium	1.1E-07	0.021	0.80	0.021	No
Boron	1.2E-04	467	1,400	700	No
Cobalt	1.4E-06	0.0035	2.1	0.0035	No
Lead	2.2E-06	0.015	0.015	0.015	No
Thallium	9.3E-08	0.0017	0.40	0.0017	No

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern; HTC = Human Threshold Criteria.

(a) Surface water samples were not collected from the unnamed tributary.

Table 3.12 Risk Evaluation for Recreators Exposed to Surface Water in the Western Branch of Coffeen Lake

COI	Maximum Surface Water Concentration		HTC for Water and Fish	HTC for Water Only	HTC for Fish Only	COPC	
	Modeled	Measured				Based on Modeled Concentration	Based on Measured Concentration
Total Metals (mg/L)							
Beryllium	2.6E-08	NA	0.021	0.80	0.021	No	NA
Lead	3.7E-08	NA	0.015	0.015	0.015	No	NA

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern; HTC = Human Threshold Criteria.

NA = Not Applicable; COI was not measured in surface water collected from Coffeen Lake.

3.5.2 Recreators Exposed to Sediment

Recreational exposure to sediment may occur during boating activity in Coffeen Lake or while angling in the unnamed tributary; exposure to sediment may occur through incidental ingestion and dermal contact.

Screening Exposures: COIs in impacted groundwater flowing into the river can sorb to sediments. In the absence of sediment data, sediment concentrations were modeled using maximum detected groundwater concentrations.

Screening Benchmarks: There are no established recreator RSLs that are protective of recreational exposures to sediment (US EPA, 2021c). Therefore, benchmarks that are protective of recreational exposures to sediment *via* incidental ingestion and dermal contact were calculated using US EPA's RSL

guidance (US EPA, 2021c). These benchmarks were calculated using the recommended assumptions (*i.e.*, oral bioavailability, body weights, averaging time) and toxicity reference values (*i.e.*, RfD and cancer slope factor [CSF]), with the following changes: Recreators were assumed to be exposed to sediment while recreating 60 days a year (or two weekend days per week for 30 weeks a year, from April to October). The exposure duration was assumed for a child 6 years of age and an adult 20 years of age, per US EPA guidance (US EPA, 2014b). The daily recommended residential soil ingestion rates of 200 mg/day for a child and 100 mg/day for an adult are based on an all-day exposure to residential soils (US EPA, 2011b, 2014b). Since recreational exposures to sediment are assumed to occur for less than four hours per day, one-third of the daily residential soil ingestion (67 mg/day for a child and 33 mg/day for an adult) was used as a conservative assumption. For dermal exposures, recreators were assumed to be exposed to sediment on their lower legs and feet (1,026 cm² for the child and 3,026 cm² for the adult, based on the age-weighted surface areas reported in US EPA (2011b). While other body parts may be exposed to sediment, the contact time will likely be very short, as the sediment would wash off in the surface water. Gradient used US EPA's recommended adherence factor of 0.2 mg/cm² based on child exposure to wet soil (US EPA, 2004, 2014b), which was used in the US EPA RSL User's Guide for a child recreator exposed to soil or sediment (US EPA, 2021c). The sediment screening benchmarks were calculated based on a target hazard quotient of 1, or a target cancer risk of 1×10⁻⁵. Appendix B, Table B.2 presents the calculation of screening benchmarks protective of recreational exposures to sediment.

Screening Risk Evaluation: The modeled sediment concentrations at the unnamed tributary and the western branch of Coffeen Lake were well below the recreational sediment screening benchmarks (Tables 3.13 and 3.14, respectively). Therefore, exposure to sediment is not expected to pose an unacceptable risk to recreators while angling in the unnamed tributary or boating in Coffeen Lake.

Table 3.13 Risk Evaluation for Recreators Exposed to Sediment in the Unnamed Tributary

COI	Modeled Sediment Concentration (mg/kg)	Recreator Sediment Screening Benchmark (mg/kg)	COPC
Total Metals (mg/kg)			
Arsenic	7.0E-04	6.8E+01	No
Beryllium	6.4E-05	2.7E+03	No
Boron	7.4E-04	2.7E+05	No
Cobalt	1.3E-03	4.1E+02	No
Lead	2.2E-02	4.0E+02	No
Thallium	1.7E-06	1.4E+01	No

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern.

Table 3.14 Risk Evaluation for Recreators Exposed to Sediment in the Western Branch of Coffeen Lake

COI	Modeled Sediment Concentration (mg/kg)	Recreator Sediment Screening Benchmark (mg/kg)	COPC
Total Metals (mg/kg)			
Beryllium	1.5E-05	2.7E+03	No
Lead	3.7E-04	4.0E+02	No

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern.

3.6 Ecological Risk Evaluation

Based on the ecological CEM (Figure 3.4), ecological receptors could be exposed to surface water and dietary items (*i.e.*, prey and plants) potentially impacted by identified COIs (cadmium, cobalt, lead, mercury, and radium-226+228).

3.6.1 Ecological Receptors Exposed to Surface Water

Screening Exposures: The ecological evaluation considered aquatic communities in Coffeen Lake and the unnamed tributary potentially impacted by identified ecological COIs. Measured and modeled surface water concentrations were compared to risk-based ecological screening benchmarks.

Screening Benchmarks: Surface water screening benchmarks protective of aquatic life were obtained from the following hierarchy of sources:

- IEPA SWQS (IEPA, 2019), regulatory standards that are intended to protect aquatic life exposed to surface water on a long-term basis (*i.e.*, chronic exposure). For cadmium, the surface water benchmark is hardness dependent and calculated using a default hardness of 100 mg/L (US EPA, 2022)⁶;
- US EPA Region IV (2018) surface water ESVs for hazardous waste sites; and
- US DOE benchmarks from the guidance document, "A Graded Approach for Evaluating Radiation Doses to Aquatic and Terrestrial Biota" (US DOE, 2019).

Risk Evaluation: The maximum modeled COI concentrations in surface water in the unnamed tributary were compared to the benchmarks protective of aquatic life (Table 3.15). The maximum measured COI concentrations in Coffeen Lake, and modeled COI concentrations in the western branch of Coffeen Lake, were compared to the benchmarks protective of aquatic life (Table 3.16). The measured and modeled surface water concentrations for the COIs were below their respective benchmarks. Thus, none of the COIs evaluated are expected to pose an unacceptable risk to aquatic life in the unnamed tributary or Coffeen Lake.

⁶ Conservatism associated with using a default hardness value are discussed in Section 3.6.

Table 3.15 Risk Evaluation for Ecological Receptors Exposed to Surface Water in the Unnamed Tributary

COI	Modeled Surface Water Concentration ^a	Ecological Freshwater Benchmark	Basis	COPC
Total Metals (mg/L)				
Cadmium	1.1E-07	0.0011	IEPA SWQC	No
Cobalt	1.4E-06	0.019	US EPA R4 ESV	No
Lead	2.2E-06	0.020	IEPA SWQC	No
Mercury	3.7E-08	0.0011	IEPA SWQC	No
Radionuclides (pCi/L)				
Radium-226 + 228	1.1E-04	3.0	US DOE	No

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern; ESV = Ecological Screening Value; IEPA = Illinois Environmental Protection Agency; pCi/L = PicoCuries Per Liter; SWQC = Surface Water Quality Criteria; US DOE = United States Department of Energy; US EPA R4 = United States Environmental Protection Agency Region IV.

(a) Surface water samples were not collected from the unnamed tributary.

Table 3.16 Risk Evaluation for Ecological Receptors Exposed to Surface Water in the Western Branch of Coffeen Lake

COI	Maximum Surface Water Concentration		Ecological Freshwater Benchmark	Basis	COPC	
	Modeled	Measured			Based on Modeled Concentration	Based on Measured Concentration
Total Metals (mg/L)						
Cadmium	4.6E-09	NA	0.0011	IEPA SWQC	No	NA

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern; IEPA = Illinois Environmental Protection Agency; SWQC = Surface Water Quality Criteria.

NA = Not Applicable; COI was not measured in surface water collected from Coffeen Lake.

3.6.2 Ecological Receptors Exposed to Sediment

Screening Exposures: COIs in impacted groundwater discharging into Coffeen Lake or the unnamed tributary can sorb to sediments *via* chemical partitioning. In the absence of sediment data, sediment concentrations were modeled using maximum detected groundwater concentrations. Therefore, the modeled COI sediment concentrations reflect the potential maximum Site-related sediment concentration from groundwater discharge.

Screening Benchmarks: Sediment screening benchmarks were obtained from US EPA Region IV (2018). The majority of the sediment ESVs are based on threshold effect concentrations (TECs) from MacDonald *et al.* (2000), which provide consensus values that identify concentrations below which harmful effects on sediment-dwelling organisms are unlikely to be observed. In the absence of an ESV for radium-226+228, a sediment screening value of 90,000 pCi/kg was used, based on the biota concentration guide (BCG) for

radium-228 (US DOE, 2019).⁷ The benchmarks used in this evaluation are listed in Table 3.17 for the unnamed tributary, and Table 3.18 for the western branch of Coffeen Lake.

Screening Risk Results: The maximum modeled COI sediment concentrations for the unnamed tributary and the western branch of Coffeen Lake were below their respective sediment screening benchmarks (Tables 3.17 and 3.18, respectively). The modeled sediment concentrations attributed to potential contributions from Site groundwater for all COIs were less than or equal to 1% of the sediment screening benchmark. Therefore, the modeled sediment concentrations attributed to potential contributions from Site groundwater are not expected to significantly contribute to ecological exposures in the unnamed tributary or Coffeen Lake adjacent to the Site.

Table 3.17 Risk Evaluation for Ecological Receptors Exposed to Sediment in the Unnamed Tributary

COI	Modeled Sediment Concentration	ESV ^a	COPC	Percentage of Benchmark
Total Metals (mg/kg)				
Cadmium	1.5E-04	0.99	No	0.01%
Cobalt	1.3E-03	50	No	0.003%
Lead	2.2E-02	35.8	No	0.06%
Mercury	1.3E-03	0.18	No	0.7%
Radionuclides (pCi/kg)				
Radium-226 + 228	7.9E-01	90,000 ^b	No	0.001%

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern; ESV = Ecological Screening Value; pCi/g = PicoCuries Per Gram; pCi/kg = PicoCuries Per Kilogram; US DOE = United States Department of Energy; US EPA = United States Environmental Protection Agency.

(a) ESV from US EPA Region IV (2018).

(b) ESV from US DOE (2019); value converted from 90 pCi/g to 90,000 pCi/kg.

Table 3.18 Risk Evaluation for Ecological Receptors Exposed to Sediment in the Western Branch of Coffeen Lake

COI	Modeled Sediment Concentration	ESV ^a	COPC	Percentage of Benchmark
Total Metals (mg/kg)				
Cadmium	6.2E-06	0.99	No	0.00063%

Notes:

COI = Constituent of Interest; COPC = Constituent of Potential Concern; ESV = Ecological Screening Value; US EPA = United States Environmental Protection Agency.

(a) ESV from US EPA Region IV (2018).

3.6.3 Ecological Receptors Exposed to Bioaccumulative Constituents of Interest

Screening Exposures: COIs with bioaccumulative properties can impact higher-trophic-level wildlife exposed to these COIs *via* direct exposures (surface water and sediment exposure) and secondary exposures through the consumption of dietary items (*e.g.*, plants, invertebrates, small mammals, and fish).

⁷ The BCG for sediment is 90 pCi/g for Ra-228 and 100 pCi/g for Ra-226; the lower of the two values was used for Ra-226+228, and converted to pCi/kg (US DOE, 2019).

Screening Benchmark: US EPA Region IV (2018) guidance and IEPA SWQS (IEPA, 2019) guidance were used to identify constituents with potential bioaccumulative effects.

Risk Evaluation: With the exception of mercury, the ecological COIs (cadmium, cobalt, lead, and radium-226+228) were not identified as having potential bioaccumulative effects. Therefore, these COIs are not considered to pose an ecological risk *via* bioaccumulation. IEPA (2019) identifies mercury as the only metal with bioaccumulative properties. US EPA Region IV (2018) also identifies mercury (including methyl mercury) as having potential bioaccumulative effects.⁸

The modeled mercury concentration in surface water in the unnamed tributary (3.7×10^{-8} mg/L) was below the mercury surface water ESV for wildlife (1.3×10^{-6} mg/L), and the modeled mercury concentration in sediment at the unnamed tributary (1.3×10^{-3} mg/kg) was below the sediment ESV for wildlife (0.18 mg/kg) (US EPA Region IV, 2018).⁹ Both the modeled surface water and sediment concentrations were below benchmarks protective of receptors accounting for bioaccumulative properties. Therefore, in addition to not posing an ecological risk from direct toxicity, mercury does not pose a risk from bioaccumulation exposures.

3.7 Uncertainties and Conservatism

A number of uncertainties and their potential impacts on the risk evaluation are discussed below. Wherever possible, conservative assumptions were used in an effort to minimize uncertainties and overestimate rather than underestimate risks.

Exposure Estimates:

- The risk evaluation included the Illinois Part 845.600 constituents detected in groundwater samples (above GWPS) collected from wells associated with the GMF GSP and the GMF RP. However, it is possible that not all of the detected constituents are related specifically to the GMF GSP and the GMF RP.
- The human health and ecological risk characterizations were based on the maximum measured or modeled COI concentrations, rather than on averages. Thus, the variability in exposure concentrations was not considered. Assuming continuous exposure to the maximum concentration overestimates human and ecological exposures, given that receptors are mobile and concentrations change over time. For example, US EPA guidance states that risks should be estimated using average exposure concentrations as represented by the 95% upper confidence limit on the mean (US EPA, 1992). Given that exposure estimates based on the maximum concentrations did not exceed risk benchmarks, Gradient has greater confidence that there is no risk concern.
- Only constituents detected in groundwater were used to identify COIs and model COI concentrations in surface water and sediment. For the constituents that were not detected in the GMF GSP and the GMF RP groundwater, the detection limits were below the Illinois Part 845.600 GWPS and thus do not require further evaluation.
- COI concentrations in surface water were modeled using the maximum detected total COI concentrations in groundwater. Modeling surface water concentrations using total metal concentrations may overestimate surface water concentrations because dissolved concentrations,

⁸ US EPA Region IV (2018) identifies selenium as having potential bioaccumulative effects. Although selenium was detected in groundwater, it was not considered an ecological COI.

⁹ Mercury was not an ecological COI on the west side of the groundwater divide; thus, it was not evaluated for the western branch of Coffeen Lake.

which are lower than total concentrations, represent the mobile fractions of constituents that could likely flow into and mix with surface water.

- The COIs identified in this evaluation also occur naturally in the environment. Contributions to exposure from natural sources or other sources unrelated to the GMF GSP and the GMF RP were not considered in the evaluation of modeled concentrations; only exposure contributions potentially attributable to Site groundwater mixing with surface water were evaluated. While not quantified, exposures from potential groundwater contributions related to the GMF GSP and the GMF RP are likely to represent only a small fraction of the overall human and ecological exposure to COIs that also have natural sources or sources unrelated to the GMF GSP and the GMF RP.
- Screening benchmarks for human health were developed using exposure inputs based on US EPA's recommended values for reasonable maximum exposure (RME) assessments (US EPA, 2014b). RME is defined as "the highest exposure that is reasonably expected to occur at a site but that is still within the range of possible exposures" (US EPA, 2004). US EPA states the "intent of the RME is to estimate a conservative exposure case (*i.e.*, well above the average case) that is still within the range of possible exposures" (US EPA, 1989). US EPA also notes that this high-end exposure "is the highest dose estimated to be experienced by some individuals, commonly stated as approximately equal to the 90th percentile exposure category for individuals" (US EPA, 2015c). Thus, most individuals will have lower exposures than those presented in this risk assessment.

Toxicity Benchmarks:

- Screening-level ecological benchmarks were compiled from IEPA and US EPA guidance and designed to be protective of the majority of Site conditions, leaving the option for Site-specific refinement. In some cases, these benchmarks may not be representative of the Site-specific conditions or receptors found at the Site, or may not accurately reflect concentration-response relationships encountered at the Site. For example, the ecological benchmark for cadmium is hardness-dependent. However, hardness data are not available for Coffeen Lake or the unnamed tributary; therefore, Gradient relied on US EPA's default hardness of 100 mg/L. Use of a higher hardness value would increase the cadmium SWQS because benchmarks become less stringent with higher levels of hardness. Regardless of the hardness, the maximum modeled cadmium concentration is orders of magnitude below the SWQS.
- In addition, for the ecological evaluation, Gradient conservatively assumed all constituents to be 100% bioavailable. Modeled COI concentrations in surface water are considered total COI concentrations. In addition, the measured surface water data used in this report represent total concentrations. US EPA recommends using dissolved metals as a measure of exposure to ecological receptors because it represents the bioavailable fraction of metal in water (US EPA, 1993). Therefore, the modeled surface water COI concentrations may be an overestimation of exposure concentrations to ecological receptors.
- In general, it is important to appreciate that the human health toxicity factors used in this risk evaluation are developed to account for uncertainties, such that safe exposure levels used as benchmarks are often many times lower (even orders of magnitude lower) than the levels that cause effects that have been observed in human or animal studies. For example, toxicity factors incorporate a 10-fold safety factor to protect sensitive subpopulations. This means that a risk exceedance does not necessarily equate to actual harm.

4 Summary and Conclusions

A screening-level risk evaluation was performed for potential Site-related constituents in groundwater at the CPP in Coffeen, Illinois. The CSM developed for the Site indicates that groundwater beneath the GMF GSP and the GMF RP flows into Coffeen Lake and the unnamed tributary adjacent to the Site and may potentially impact surface water and sediment.

CEMs were developed for human and ecological receptors. The complete exposure pathways for humans include anglers who consume locally caught fish from Coffeen Lake or the unnamed tributary, and recreators (boaters) in Coffeen Lake, groups which could be exposed to surface water and sediment. Based on the local hydrogeology, residential exposure to groundwater used for drinking water or irrigation is not a complete pathway and was not evaluated. The complete exposure pathways for ecological receptors include aquatic life (including aquatic and marsh plants, amphibians, reptiles, and fish) exposed to surface water; benthic invertebrates exposed to sediment; and avian and mammalian wildlife exposed to bioaccumulative COIs in surface water, sediment, and dietary items.

Groundwater data collected from 2015 to 2021 were used to estimate exposures. Surface water data collected from Coffeen Lake were also evaluated. For groundwater constituents retained as COIs, surface water and sediment concentrations were modeled using the maximum detected groundwater concentration. Surface water and sediment exposure estimates were screened against benchmarks protective of human health and ecological receptors for this risk evaluation.

US EPA has established acceptable risk metrics. Risks above these US EPA-defined metrics are termed potentially "unacceptable risks." Based on the evaluation presented in this report, no unacceptable risks to human or ecological receptors resulting from CCR exposures associated with the GMF GSP or GMF RP were identified. This means that the risks from the site are likely indistinguishable from normal background risks. Specific risk assessment results include the following:

- For recreators exposed to surface water, all COIs were below the conservative risk-based screening benchmarks. Therefore, none of the COIs evaluated in surface water are expected to pose an unacceptable risk to recreators in Coffeen Lake or the unnamed tributary adjacent to the Site.
- For recreators exposed to sediment *via* incidental ingestion and dermal contact, the modeled sediment concentrations were below health-protective sediment benchmarks. Therefore, the modeled sediment concentrations are not expected to pose an unacceptable risk to recreators exposed to sediment in Coffeen Lake or the unnamed tributary adjacent to the Site.
- For anglers consuming locally caught fish, the modeled concentrations of all COIs in surface water were below conservative benchmarks protective of fish consumption. Therefore, none of the COIs evaluated are expected to pose an unacceptable risk to recreators consuming fish caught in Coffeen Lake or the unnamed tributary.
- Ecological receptors exposed to surface water include aquatic and marsh plants, amphibians, reptiles, and fish. The risk evaluation showed that none of the modeled or measured COIs in surface water exceeded protective screening benchmarks. Ecological receptors exposed to sediment include benthic invertebrates. The modeled sediment COIs did not exceed the conservative screening benchmarks; therefore, none of the COIs evaluated in sediment are expected to pose an unacceptable risk to ecological receptors.

- Ecological receptors were also evaluated for exposure to bioaccumulative COIs. This evaluation considered higher-trophic-level wildlife with direct exposure to surface water and sediment and secondary exposure through the consumption of dietary items (e.g., plants, invertebrates, small mammals, fish). Mercury was the only ecological COI identified as having potential bioaccumulative effects. However, the modeled concentrations did not exceed benchmarks protective of bioaccumulative effects. Therefore, mercury is not considered to pose an ecological risk *via* bioaccumulation. Overall, this evaluation demonstrated that none of the COIs evaluated are expected to pose an unacceptable risk to ecological receptors.

It should be noted that this evaluation incorporates a number of conservative assumptions which tend to overestimate exposure and risk. While the risk evaluation was based on the maximum detected COI concentration, US EPA guidance states that risks should be based on a representative average concentration such as the 95% upper confidence limit on the mean; thus, using the maximum concentration tends to overestimate exposure. Although the COIs identified in this evaluation also occur naturally in the environment, the contributions to exposure from natural background sources and nearby industry were not considered; thus, CCR-related exposures were likely overestimated. Exposure estimates assumed 100% metal bioavailability, which likely results in overestimates of exposure and risks. Exposure estimates were based on inputs to evaluate the "reasonable maximum exposure"; thus, most individuals will have lower exposures than those estimated in this risk assessment.

Finally, it should be noted that because current conditions do not present a risk to human health or the environment, there will also be no unacceptable risk to human health or the environment for future conditions when the GMF GSP and the GMF RP are closed. For all future closure scenarios, potential releases of CCR-related constituents will decline over time and, consequently, potential exposures to CCR-related constituents in the environment will also decline.

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Appendix A

Surface Water and Sediment Modeling

Gradient modeled surface water concentrations in the unnamed tributary and the western branch of Coffeen Lake and associated sediments based on available groundwater data. First, Gradient estimated the flow rate of constituents of interest (COIs) that may flow into these waterbodies *via* groundwater. Then, Gradient adapted United States Environmental Protection Agency's (US EPA's) indirect exposure assessment methodology (US EPA, 1998) in order to model surface water and sediment water concentrations in the unnamed tributary and the western branch of Coffeen Lake.

Model Overview

Two separate surface water models were carried out: one for the unnamed tributary, located to the east of a groundwater divide, and one for the western branch of Coffeen Lake, located to the west of a groundwater divide (see Section 2). Groundwater flow into these waterbodies is represented by a one-dimensional steady-state model. In this model, the groundwater plume migrates horizontally in the Uppermost Aquifer (UA) before flowing into surface water. The groundwater flow entering the surface water is the flow going through a cross-sectional area with a length equal to the length of the surface water bodies adjacent to the GMF ponds (*i.e.*, the GMF Gypsum Stack Pond [GSP] and the GMF Recycle Pond [RP]) with potential CCR-related impacts and a height equal to the average saturated thickness of the UA. It was assumed that all groundwater originating from the GMF RP ultimately flows into the unnamed tributary, whereas a component of groundwater originating from the GMF GSP flows into the unnamed tributary, and the rest flows into the western branch of Coffeen Lake.

Groundwater flow into the unnamed tributary mixes with the surface water in the tributary whereas groundwater flow into the western branch of Coffeen Lake mixes with the surface water in the lake. The COIs entering the unnamed tributary as well as the western branch of Coffeen Lake *via* groundwater can dissolve into the water column, sorb to suspended sediments, or sorb to benthic sediments. Using US EPA's indirect exposure assessment methodology (US EPA, 1998), the model evaluates the surface water and sediment concentrations at a location downstream of the groundwater discharge, assuming a well-mixed water column.

Groundwater Flow Rate

The groundwater flow rate was evaluated using conservative assumptions. Gradient conservatively assumed that the groundwater concentrations were uniformly equal to the maximum detected concentration for each individual COI. Gradient ignored adsorption by subsurface soil and assumed that groundwater flowing through the UA was discharged into surface water.

For each groundwater unit, the groundwater flow rate into surface water was derived using Darcy's Law:

$$Q = K \times i \times A$$

where:

- Q = Groundwater flow rate (m³/s)
- K = Hydraulic conductivity (m/s)
- i = Hydraulic gradient (m/m)
- A = Cross-sectional area (m²)

For each COI, the mass discharge rate into surface water was then calculated from the following equation:

$$m_c = C_c \times Q \times CF$$

where:

- m_c = Mass discharge rate of the COI (mg/year)
- C_c = Maximum groundwater concentration of the COI (mg/L)
- Q = Groundwater flow rate (m³/s)
- CF = Conversion factors: 1,000 L/m³; 31,557,600 s/year

The values of the aquifer parameters used for these calculations for the unnamed tributary are provided in Table A.1 and for the western branch of Coffeen Lake are provided in Table A.2. The calculated mass discharge rates were then used as inputs for the surface water and sediment partitioning model.

The cross-sectional area for the UA was approximately 664 m² for the unnamed tributary and approximately 427 m² for the western branch of Coffeen Lake. The length of the water bodies through which groundwater flows was estimated to be approximately 726 m for the unnamed tributary and about 467 m for the western branch of Coffeen Lake. In both cases, the height of the UA was approximately 3 feet (ft) (*i.e.*, 0.91 m) (Ramboll, 2021a,b).

Towards the unnamed tributary, the average hydraulic gradient within the UA was estimated to be 0.0075 m/m (Ramboll, 2021a,b). Towards the western branch of Coffeen Lake, the mean hydraulic gradient determined for the UA was 0.018 m/m (Ramboll, 2021a).

To model surface water concentrations in the unnamed tributary, we used the average of the geometric mean horizontal hydraulic conductivities (*i.e.*, 0.0013 cm/sec) measured for the UA near the GMF GSP and the GMF RP (Ramboll, 2021a,b). To model surface water concentrations in the western branch of Coffeen Lake, we used the geometric mean horizontal hydraulic conductivity (*i.e.*, 0.0014 cm/sec) determined for the UA near the GMF GSP (Ramboll, 2021a).

Surface Water and Sediment Concentration

Groundwater flowing into the lake will be diluted in the surface water flow. Constituents transported by groundwater into the surface water migrate into the water column and the bed sediments. The surface water model Gradient used to estimate the surface water and sediment concentrations is a steady-state model described in US EPA's indirect exposure assessment methodology (US EPA, 1998), and also used in US EPA's "Human and Ecological Risk Assessment of Coal Combustion Residuals" (US EPA, 2014). This model describes the partitioning of constituents between surface water, suspended sediments, and benthic sediments based on equilibrium partition coefficients. It estimates the concentrations of constituents in surface water, suspended sediments, and benthic sediments at steady-state equilibrium at a theoretical location downstream of the discharge point after complete mixing of the water column. In the analysis, Gradient used the partitioning coefficients given in Table J-1 of the US EPA CCR Risk Assessment for all COIs (US EPA, 2014). These coefficients are presented in Table A.3.

To be conservative, Gradient assumed that the constituents were not affected by dissipation or degradation once they entered the water body. The total water body concentration of the COI was calculated as (US EPA, 1998):

$$C_{wtot} = \frac{m_c}{V_f \times f_{water}}$$

where:

- C_{wtot} = Total water body concentration of the constituent (mg/L)
- m_c = Mass discharge rate of the COI (mg/year)
- V_f = Water body annual flow (L/year)
- f_{water} = Fraction of COI in the water column (unitless)

There are no flow records available for the unnamed tributary and the western branch of Coffeen Lake. According to Golder Associates, the flow rate in the unnamed tributary was assumed to be 90 cubic feet per second (cfs) (Golder Associates, Inc., 2020). According to the US Geological Survey (USGS) Streamstats program, the western branch of Coffeen Lake has a two-year flow peak flow prediction of 1,010 cfs (USGS, 2022). The surface water parameters for the unnamed tributary are presented in Table A.4 and the surface water parameters for the western branch of the Coffeen Lake are presented in Table A.5.

The fraction of COI in the water column was calculated for each COI (11 COIs determined for the unnamed tributary and 4 COIs determined for the western branch of Coffeen Lake) using the sediment/water and suspended solids/water partition coefficients (US EPA, 2014, Table J-1). The fraction of COIs in the water column is defined as (US EPA, 2014):

$$f_{water} = \frac{(1 + [K_{dsw} \times TSS \times 0.000001]) \times \frac{d_w}{d_z}}{\left([1 + (K_{dsw} \times TSS \times 0.000001)] \times \frac{d_w}{d_z}\right) + ([bsp + K_{abs} \times bsc] \times \frac{d_b}{d_z})}$$

where:

- K_{dsw} = Suspended sediment-water partition coefficient (mL/g)
- K_{abs} = Sediment-water partition coefficient (mL/g)
- TSS = Total suspended solids in the surface water body (mg/L), set equal to the average Coffeen Lake concentration of 3.2 mg/L (Hanson, 2020)
- 0.000001 = Units conversion factor
- d_w = Depth of the water column (m). The mean depth of the surface water column for the western branch of Coffeen Lake was estimated as 5.7 m (Austen *et al.*, 1993), whereas the flow depth of the unnamed tributary was estimated as 0.6 m (Golder Associates, Inc., 2020).
- d_b = Depth of the upper benthic layer (m), set equal to 0.03 m (US EPA, 2014)
- $d_z = d_w + d_b$ = Depth of the water body (m) = 5.73 m for the western branch of Coffeen Lake and 0.67 m for the unnamed tributary.
- bsp = Bed sediment porosity (unitless), set equal to 0.6 (US EPA, 2014)
- bsc = Bed sediment particle concentration (g/cm³), set equal to 1.0 g/cm³ (US EPA, 2014)

The fraction of COIs dissolved in the water column (f_d) is calculated as (US EPA, 2014):

$$f_d = \frac{1}{1 + K_{dsw} \times TSS \times 0.000001}$$

The values of the fraction of COIs in the water column and other calculated parameters for the unnamed tributary are presented in Table A.6 and for the western branch of Coffeen Lake are presented in Table A.7.

The total water column concentration (C_{wcTot}) of the COIs, comprising both the dissolved and suspended sediment phases, is then calculated as (US EPA, 2014):

$$C_{wcTot} = C_{wtot} \times f_{water} \times \frac{d_z}{d_w}$$

Finally, the dissolved water column concentration (C_{dw}) for the COIs is calculated as (US EPA, 2014):

$$C_{dw} = f_d \times C_{wcTot}$$

The dissolved water column concentration was then used to calculate the concentration of COIs sorbed to suspended solids in the water column (US EPA, 1998):

$$C_{sw} = C_{dw} \times K_{dsw}$$

where:

- C_{sw} = Concentration sorbed to suspended solids (mg/kg)
- C_{dw} = Concentration dissolved in the water column (mg/L)
- K_{dsw} = Suspended solids/water partition coefficient (mL/g)

In the same way, using the total water body concentration and the fraction of COIs in the benthic sediments, the model derives the total concentration in benthic sediments (US EPA, 2014):

$$C_{bstot} = f_{benth} \times C_{wtot} \times \frac{d_z}{d_b}$$

where:

- C_{bstot} = Total concentration in bed sediment (mg/L or g/m³)
- C_{wtot} = Total water body concentration of the constituent (mg/L)
- f_{benth} = Fraction of contaminant in benthic sediments (unitless)
- d_b = Depth of the upper benthic layer (m)
- $d_z = d_w + d_b$ = Depth of the water body (m)

This value can be used to calculate dry weight sediment concentration as follows:

$$C_{sed-dw} = \frac{C_{bstot}}{b_{sc}}$$

where:

- C_{sed-dw} = Dry weight sediment concentration (mg/kg)
- C_{bstot} = Total sediment concentration (mg/L)
- b_{sc} = Bed sediment bulk density (default value of 1 g/cm³ from US EPA, 2014)

The total sediment concentration is composed of the concentration dissolved in the bed sediment pore water (equal to the concentration dissolved in the water column) and the concentration sorbed to benthic sediments (US EPA, 1998).

The concentration sorbed to benthic sediments was calculated from US EPA (1998):

$$C_{sb} = C_{dbs} \times K_{dbs}$$

where:

- C_{sb} = Concentration sorbed to bottom sediments (mg/kg)
- C_{dbs} = Concentration dissolved in the sediment pore water (mg/L)
- K_{dbs} = Sediments/water partition coefficient (mL/kg)

For each COI, the modeled total water column concentration, the modeled dry weight sediment concentration, and the modeled concentration sorbed to sediment for the unnamed tributary are presented in Table A.8 and for the western branch of Coffeen Lake are presented in Table A.9.

Table A.1 Parameters Used to Estimate Groundwater Flow to the Surface Water of the Unnamed Tributary

Groundwater Unit	Parameter	Name	Value	Unit
Uppermost Aquifer	A	Cross-Sectional Area	664	m ²
Uppermost Aquifer	i	Hydraulic Gradient	0.0075	m/m
Uppermost Aquifer	K	Hydraulic Conductivity	0.0013	cm/s

Note:

Sources: Hydraulic gradient and hydraulic conductivity values from Ramboll (2021a,b).

Table A.2 Parameters Used to Estimate Groundwater Flow to the Surface Water of the Western Branch of Coffeen Lake

Groundwater Unit	Parameter	Name	Value	Unit
Uppermost Aquifer	A	Cross-Sectional Area	427	m ²
Uppermost Aquifer	i	Hydraulic Gradient	0.018	m/m
Uppermost Aquifer	K	Hydraulic Conductivity	0.0014	cm/s

Note:

Source: Hydraulic gradient and hydraulic conductivity values from Ramboll (2021a).

Table A.3 Partition Coefficients

Constituent	Sediment-Water, Mean, K_{dbs}		Suspended Sediment-Water, Mean, K_{dsw}	
	Value (\log_{10}) (mL/g)	Value (mL/g)	Value (\log_{10}) (mL/g)	Value (mL/g)
Metals				
Arsenic	2.4	2.51E+02	3.9	7.94E+03
Boron	0.8	6.31E+00	3.9	7.94E+03
Beryllium	2.8	6.31E+02	4.2	1.58E+04
Cadmium	3.3	2.00E+03	4.9	7.94E+04
Cobalt	3.1	1.26E+03	4.8	6.31E+04
Lead	4.6	3.98E+04	5.7	5.01E+05
Mercury	4.9	7.94E+04	5.3	2.00E+05
Thallium	1.3	2.00E+01	4.1	1.26E+04
Radionuclides				
Radium-226+228	-	7.40E+03	-	7.40E+03

Note:

Source: US EPA (2014).

Table A.4 Surface Water Parameters for the Unnamed Tributary

Parameter	Name	Value	Unit
<i>TSS</i>	Total Suspended Solids	6	mg/L
<i>V_{fx}</i>	Surface Water Flow Rate	8.04×10^{10}	L/yr
<i>d_b</i>	Depth of Upper Benthic Layer (default)	0.03	m
<i>d_w</i>	Depth of Water Column	0.64	m
<i>d_z</i>	Depth of Water Body	0.67	m
<i>b_{sc}</i>	Bed Sediment Bulk Density (default)	1	g/cm ³
<i>b_{sp}</i>	Bed Sediment Porosity (default)	0.6	-
<i>M_{TSS}</i>	TSS Mass per Unit Area ^a	0.00384	kg/m ²
<i>M_S</i>	Sediment Mass per Unit Area ^b	30	kg/m ²

Notes:

L/yr = Liter Per Year.

Source of default values: US EPA (2014).

(a) Determined by multiplying total suspended solids, TSS by the depth of water column, *d_w*.

(b) Determined by multiplying depth of the upper benthic layer, *d_b*, by sediment bed particle concentration of 1 g/cc.

Table A.5 Surface Water Parameters for the Western Branch of Coffeen Lake

Parameter	Name	Value	Unit
<i>TSS</i>	Total Suspended Solids	6	mg/L
<i>V_{fx}</i>	Surface Water Flow Rate	9.02×10^{11}	L/yr
<i>d_b</i>	Depth of Upper Benthic Layer (default)	0.03	m
<i>d_w</i>	Depth of Water Column	5.70	m
<i>d_z</i>	Depth of Water Body	5.73	m
<i>b_{sc}</i>	Bed Sediment Bulk Density (default)	1	g/cm ³
<i>b_{sp}</i>	Bed Sediment Porosity (default)	0.6	-
<i>M_{TSS}</i>	TSS Mass per Unit Area ^a	0.0342	kg/m ²
<i>M_S</i>	Sediment Mass per Unit Area ^b	30	kg/m ²

Notes:

L/yr = Liter Per Year.

Source of default values: US EPA (2014).

(a) Determined by multiplying total suspended solids, TSS by the depth of water column, *d_w*.

(b) Determined by multiplying depth of the upper benthic layer, *d_b*, by the default sediment bed particle concentration of 1 g/cc.

Table A.6 Calculated Parameters for the Unnamed Tributary

COI	Fraction of Constituent in the Water Column <i>f_{water}</i>	Fraction of Constituent in the Benthic Sediments <i>f_{benthic}</i>	Fraction of Constituent Dissolved in the Water Column <i>f_{dissolved}</i>
Arsenic	0.082	0.918	0.955
Beryllium	0.0357	0.9643	0.9132
Boron	0.7639	0.2361	0.9545
Cadmium	0.0155	0.9845	0.6772
Cobalt	0.023	0.977	0.725
Lead	0.002	0.998	0.250
Mercury	0.001	0.999	0.455
Thallium	0.528	0.472	0.930
Radium-226+228	0.003	0.997	0.957

Note:

COI = Constituent of Interest.

Table A.7 Calculated Parameters for the Western Branch of Coffeen Lake

COI	Fraction of Constituent in the Water Column <i>f_{water}</i>	Fraction of Constituent in the Benthic Sediments <i>f_{benthic}</i>	Fraction of Constituent Dissolved in the Water Column <i>f_{dissolved}</i>
Beryllium	0.248	0.752	0.913
Cadmium	0.1232	0.8768	0.6772
Lead	0.019	0.981	0.250

Note:

COI = Constituent of Interest.

Table A.8 Surface Water and Sediment Modeling Results for the Unnamed Tributary

COI	Groundwater Concentration (mg/L or pCi/L)	Mass Discharge Rate (mg/year or pCi/year)	Total Water Column Concentration (mg/L or pCi/L)	Concentration Sorbed to Bottom Sediments (mg/kg or pCi/kg)
Arsenic	0.11	2.2E+05	2.9E-06	7.0E-04
Beryllium	0.0042	8.6E+03	1.1E-07	6.4E-05
Boron	4.6	9.4E+06	1.2E-04	7.4E-04
Cadmium	0.0041	8.4E+03	1.1E-07	1.5E-04
Cobalt	0.053	1.1E+05	1.4E-06	1.3E-03
Lead	0.082	1.7E+05	2.2E-06	2.2E-02
Mercury	0.0014	2.9E+03	3.7E-08	1.3E-03
Thallium	0.0035	7.1E+03	9.3E-08	1.7E-06
Radium-226 + 228	4.2	8.6E+06	1.1E-04	7.9E-01

Notes:

COI = Constituent of Concern; K_d = Equilibrium Partition Coefficient; pCi/kg = PicoCuries Per Kilogram; pCi/L = PicoCuries Per Liter; pCi/year = PicoCuries Per Year.

Table A.9 Surface Water and Sediment Modeling Results for the Western Branch of Coffeen Lake

COI	Groundwater Concentration (mg/L)	Mass Discharge Rate (mg/year)	Total Water Column Concentration (mg/L)	Concentration Sorbed to Bottom Sediments (mg/kg)
Beryllium	0.01	2.3E+04	2.6E-08	1.5E-05
Cadmium	0.0012	4.1E+03	4.6E-09	6.2E-06
Lead	0.010	3.3E+04	3.7E-08	3.7E-04

Notes:

COI = Constituent of Concern; K_d = Equilibrium Partition Coefficient.

Appendix A References

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Appendix B

Screening Benchmarks

Table B.1 Calculated Water Quality Standards Protective of Incidental Ingestion and Fish Consumption

Human Health COI	BCF ^a (L/kg-tissue)	Basis	MCL (mg/L)	RfD (mg/kg-day)	ADI ^b (mg/day)	Human Threshold Criteria		
						Water & Fish (mg/L)	Water Only (mg/L)	Fish Only (mg/L)
Arsenic	44	NRWQC (2002)	0.010	0.00030	0.020	0.022	2.0	0.023
Beryllium	19	NRWQC (2002)	0.0040	0.0020	0.0080	0.021	0.80	0.021
Boron	1	(c)	NC	0.20	14	467	1,400	700
Cobalt	300	ORNL (2020)	NC	0.00030	0.021	0.0035	2.1	0.0035
Lead	46	US EPA (2014)	0.015	NC	0.030	0.015	0.015	0.015
Thallium	116	NRWQC (2002)	0.0020	0.000010	0.0040	0.0017	0.40	0.0017

Notes:

ADI = Acceptable Daily Intake; BCF = Bioconcentration Factor; MCL = Maximum Contaminant Level; NC = No Criterion Available; NRWQC = National Recommended Water Quality Criteria; ORNL = Oak Ridge National Laboratory; RAIS = Risk Assessment Information System; RfD = Reference Dose; US EPA = United States Environmental Protection Agency.

(a) BCFs from the following hierarchy of sources:

NRWQC (US EPA, 2002). National Recommended Water Quality Criteria: 2002. Human Health Criteria Calculation Matrix.

US EPA (2014a). Human and Ecological Risk Assessment of Coal Combustion Residuals.

ORNL RAIS (ORNL, 2020). Risk Assessment Information System (RAIS) Toxicity Values and Chemical Parameters.

(b) ADI based on the MCL is calculated as the MCL (mg/L) multiplied by a water ingestion rate of 2 L/day. In the absence of an MCL, the ADI was calculated as the RfD (mg/kg-day) multiplied by the body weight (70 kg).

(c) BCF of 1 was used as a conservative assumption, due to a lack of a published BCF.

Equations from IEPA (2019):

Consumption of Water and Fish

$$HTC = \frac{ADI}{W + (F \times BCF)}$$

Incidental Consumption of Water Only

$$HTC = \frac{ADI}{W}$$

Consumption of Fish Only

$$HTC = \frac{ADI}{F \times BCF}$$

Where:

Human Threshold Criteria (HTC)	Chemical-specific	mg/L
Acceptable Daily Intake (ADI)	Chemical-specific	mg/day
Fish Consumption Rate (F)	0.02	kg/day
Bioconcentration Factor (BCF)	Chemical-specific	L/kg-tissue
Water Consumption Rate (W)	0.01	L/day
Body Weight	70	kg

Table B.2 Recreator Exposure to Sediment

COI	Relative Bioavailability (unitless)	Dermal Absorption Fraction (unitless)	Cancer				Cancer SL (mg/kg)	Non-Cancer								Recreator RSL Sediment (mg/kg)	Basis ^a
			TRV		Child + Adult			TRV		Child		Adult		Child	Adult		
			CSF (mg/kg-day) ⁻¹	Dermal CSF (mg/kg-day) ⁻¹	Incidental Ingestion SL (mg/kg)	Dermal Contact SL (mg/kg)		RfD (mg/kg-day)	Dermal RfD (mg/kg-day)	Incidental Ingestion SL (mg/kg)	Dermal Contact SL (mg/kg)	Incidental Ingestion SL (mg/kg)	Dermal Contact SL (mg/kg)	Non-Cancer SL (mg/kg)			
Total Metals																	
Arsenic	1	3.0E-02	1.5E+00	1.5E+00	8.1E+01	4.1E+02	6.8E+01	3.0E-04	3.0E-04	4.1E+02	4.4E+03	4.4E+03	8.0E+03	3.8E+02	2.8E+03	6.8E+01	c
Beryllium	1	NA	NC	NC	NC	NC	NC	2.0E-03	1.4E-05	2.7E+03	NA	2.9E+04	NA	2.7E+03	2.9E+04	2.7E+03	nc
Boron	1	NA	NC	NC	NC	NC	NC	2.0E-01	2.0E-01	2.7E+05	NA	2.9E+06	NA	2.7E+05	2.9E+06	2.7E+05	nc
Cobalt	1	NA	NC	NC	NC	NC	NC	3.0E-04	3.0E-04	4.1E+02	NA	4.4E+03	NA	4.1E+02	4.4E+03	4.1E+02	nc
Lead	1	NA	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	NC	4.0E+02	L
Thallium	1	NA	NC	NC	NC	NC	NC	1.0E-05	1.0E-05	1.4E+01	NA	1.5E+02	NA	1.4E+01	1.5E+02	1.4E+01	nc

Notes:
 ABS = Dermal Absorption Fraction; COI = Constituent of Interest; CSF = Cancer Slope Factor; NC = No Criterion Available; RfD = Reference Dose; RSL = Regional Screening Level; SL = Screening Level; TRV = Toxicity Reference Value; US EPA = United States Environmental Protection Agency.

(a) Screening benchmark defined as the lower of the Screening Levels for cancer and non-cancer. The basis of the benchmark presented as c = based on cancer endpoint, nc = based on non-cancer endpoint, or L = based on blood lead levels.

Equations for Screening Benchmark and Screening Levels:

$$\text{Screening Benchmark} = \frac{1}{\frac{1}{SL_{ing}} + \frac{1}{SL_{derm}}}$$

$$\text{Non-cancer } SL_{ing} = \frac{\text{THQ} \times \text{RfD}}{\text{Intake}} \quad \text{Cancer } SL_{ing} = \frac{\text{TR}}{\text{Intake} \times \text{CSF}}$$

$$\text{Non-cancer } SL_{derm} = \frac{\text{THQ} \times \text{RfD}}{\text{Intake} \times \text{ABS}} \quad \text{Cancer } SL_{derm} = \frac{\text{TR}}{\text{Intake} \times \text{ABS} \times \text{CSF}}$$

Where:

- Target Risk (TR) 1E-05
- Target Hazard Quotient (THQ) 1
- Reference Dose (RfD) Chemical-specific mg/kg-day
- Dermal Absorption Fraction (ABS) Chemical-specific
- Cancer Slope Factor (CSF) Chemical-specific mg/kg
- Incidental Ingestions Screening Level (SL_{ing}) Chemical-specific mg/kg
- Dermal Contact Screening Level (SL_{derm}) Chemical-specific mg/kg

Sediment – Ingestion (Chemical)

			Non-Cancer		Cancer		
Intake Factor (IF) =	IR x EF x ED x CF	=	7.3E-07 Child	6.8E-08 Adult	6.3E-08 Child	2.0E-08 Adult	Basis
IR	Ingestion Rate (mg/day)		67	33	67	33	One-third of US EPA residential soil ingestion rate (Professional Judgment)
EF	Sediment Exposure Frequency (days/year)		60	60	60	60	
ED	Exposure Duration (years)		6	20	6	20	Default value for Resident (US EPA, 2021b)
CF	Conversion Factor (kg/mg)		0.000001	0.000001	0.000001	0.000001	
BW	Body Weight (kg)		15	80	15	80	Default value for Resident (US EPA, 2021b)
AT	Averaging Time (days)		2,190	7,300	25,550	25,550	Default value for Resident (US EPA, 2021b)

Sediment – Dermal Contact (Chemical)

			Non-Cancer		Cancer		
Intake Factor (IF) =	SA x AF x EF x ED x CF	=	2.2E-06 Child	1.2E-06 Adult	1.9E-07 Child	3.6E-07 Adult	Basis
SA	Surface Area Exposed to Sediment (cm ² /day)		1,026	3,026	1,026	3,026	Age-weighted SA for lower legs and feet (US EPA, 2011b)
AF	Sediment Skin Adherence Factor (mg/cm ²)		0.2	0.2	0.2	0.2	Age-weighted AF for children exposed to sediment (US EPA, 2011b)
EF	Sediment Exposure Frequency (days/year)		60	60	60	60	2 days/week between April and October when air temperature >70°F (Professional Judgment)
ED	Exposure Duration (years)		6	20	6	20	Default value for Resident (US EPA, 2021b)
CF	Conversion Factor (kg/mg)		0.000001	0.000001	0.000001	0.000001	
BW	Body Weight (kg)		15	80	15	80	Default value for Resident (US EPA, 2021b)
AT	Averaging Time (days)		2,190	7,300	25,550	25,550	Default value for Resident (US EPA, 2021b)

Appendix B

Supporting Information for Closure Alternatives Analysis – Gypsum Management Facility Gypsum Stack Pond and Recycle Pond at Coffeen Power Station

TECHNICAL MEMORANDUM

DATE July 28, 2022

Reference No. 21465046

TO Victor Modeer
Illinois Power Generating Company, LLC

CC David Mitchell (Illinois Power Generating Company, LLC)

FROM Michael Dreyer

EMAIL michael_dreyer@golder.com

SUPPORTING INFORMATION FOR CLOSURE ALTERNATIVES ANALYSIS – GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND AND RECYCLE POND AT COFFEEN POWER STATION

Golder Associates USA Inc. (Golder), a Member of WSP, has prepared this technical memorandum for Illinois Power Generating Company, LLC (IPGC) to support the Closure Alternatives Analysis for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) and GMF Recycle Pond (RP) at Coffeen Power Station. The Closure Alternatives Analysis is being completed in accordance with Illinois Administrative Code Title 35, Part 845, Standards for the Disposal of Coal Combustion Residuals (CCR) in Surface Impoundments (Part 845), by Gradient. With this technical memorandum, Golder summarizes the design basis and references used in developing the closure concepts evaluated by the Closure Alternatives Analysis.

Golder reviewed several documents related to the design, construction, and operation of the GMF GSP and GMF RP. Notable documents included the History of Construction (AECOM 2016a), the GMF GSP CCR Certification Report (AECOM 2016b), the GMF RP CCR Documentation Report (Hanson 2016), and the Gypsum Stack Cell G1 Acceptance Report (Hanson 2010).

1.0 INTRODUCTION AND BACKGROUND

1.1 Operational History

The GMF GSP served as the primary wet impoundment basin for gypsum produced by the wet scrubber system at the Coffeen Power Station. The GMF GSP and GMF RP were constructed between July 2008 and October 2010 and operated from 2010 until the Coffeen Power Station was retired in 2019. The GMF GSP has an area of approximately 43.3 acres. Base grade elevations range from approximate El. 605 feet (North American Vertical Datum of 1988) to El. 614 feet. The interior side slopes of the liner system are 3H:1V (horizontal to vertical) and have a height that varies from 15 to 24 feet. An exterior perimeter berm surrounds the entire GMF GSP and has side slopes of 3H:1V. The GMF RP has an area of approximately 18.3 acres and was formed with a continuous embankment ring dike, which has a total length of approximately 3,600 feet and a maximum height of approximately 16 feet above the surrounding grade. Base grade elevations range from approximately El. 604 feet to El. 606 feet. The interior side slopes of the liner system and the exterior slopes of the embankment ring dike are both 3H:1V.

The GMF GSP received inflow from two pairs of high-density polyethylene (HDPE) gypsum slurry pipes, which deposited gypsum from the west side of the GMF GSP and in the northwest corner, which formed a delta or

beach of built-up gypsum in these locations during the operational life. The gypsum would build up to the water level and then expand laterally (rather than vertically) due to the subaqueous deposition method. During the operational life, the beach expanded so that roughly one-third of the GMF footprint had gypsum built up to the typical water level. Clear water discharge from the GMF GSP flowed downstream into the GMF RP via a lined channel (transfer channel) and 14-inch HDPE low-flow pipe buried beneath the transfer channel. The transfer channel effectively acted as a gap in the dike of the GMF GSP, as the bottom elevation of the transfer channel is equal to the adjacent exterior toe elevation of the dike. The transfer channel is approximately 580 feet in length, is trapezoidal in shape, is lined with 60-mil HDPE geomembrane, has 3H:1V side slopes, and has a bottom elevation that decreases from El. 624 feet at the upstream (north) end to El. 622 feet at the downstream (south) end. The 14-inch low flow pipe has an invert of El. 619.0 feet at the upstream end and El. 617.6 feet at the downstream end. The GMF RP acted as a polishing pond, and outflow was pumped to Coffeen Power Station to be recycled for use in the wet scrubber system. The GMF RP has an emergency spillway located at the northeast corner that consists of three precast 6-foot by 6-foot reinforced concrete risers with crest elevations at approximate El. 624 feet. The risers are connected to 48-inch diameter HDPE pipes that convey flow from the risers and discharge through existing NPDES Outfall 023 into the unnamed tributary creek east of the GMF RP that discharges to Coffeen Lake.

1.2 Existing GMF GSP Liner System Information

Based on review of the available documents, a composite liner system was installed for the GMF GSP consisting of (from top to bottom):

- GSE HDT060VW00 - 60-mil textured HDPE geomembrane
- 3-foot compacted clay layer placed in 8-inch lifts, compacted to at least 95% of the standard Proctor maximum dry density at a moisture content between the standard Proctor optimum moisture content (OMC) and 5% wet of the OMC

As documented in the Acceptance Report (Hanson 2010), the liner system was subjected to a rigorous construction quality assurance (CQA) program.

The side slopes and portions of the bottom of the GMF GSP were excavated to foundation grade. The bottom of the GMF GSP was excavated to and into the Vandalia Till. The excavated till was used to raise portions of the GMF GSP bottom to foundation grade. During preparation of the side slope and foundation grades, unsuitable sand materials were removed from several areas and stockpiled separately. These areas were then backfilled with material previously stockpiled or locally available material. Backfilled areas were compacted to at least 95% of the standard Proctor maximum dry density at a moisture content within 2% of the OMC. Four Shelby tube samples collected from the foundation grade berms were used for hydraulic conductivity analysis, with results ranging from 1.5×10^{-9} centimeters per second (cm/s) to 5.4×10^{-9} cm/s.

After certification of the foundation grades, the 3-foot compacted clay layer was constructed in 8-inch lifts and compacted to at least 95% of the standard Proctor maximum dry density at a moisture content between the OMC and 5% wet of the OMC. Twenty-one Shelby tube samples were collected during construction. Hydraulic conductivity results from tests on the Shelby tube samples range from 7.4×10^{-10} cm/s to 2.4×10^{-8} cm/s, significantly less than the construction specification of 1.0×10^{-7} cm/s. The compacted clay layer was smooth drum rolled prior to installation of the overlying GCL.

After placement of the compacted clay layer, a 60-mil textured HDPE geomembrane liner was installed on the floor and side slopes of the GMF GSP.

A 10-oz/yd² cushion geotextile was placed over the 60-mil textured HDPE geomembrane liner in the anchor trench and approximately 3.5 feet down the side slopes before the anchor trench was backfilled with flowable fill. The GMF GSP floor was constructed to slope towards four Process Water Recovery System (PWRS) sumps located at the inside toe of the east and south perimeter berms. An HDPE geomembrane rub sheet was placed on top of the geomembrane liner at the location of the sumps and side slope pipes. A 16-oz/yd² cushion geotextile was placed over the top of the geomembrane rub sheet in each sump. Each sump consists of a 12-inch HDPE SDR 11 perforated pipe placed on top of the 16-oz/yd² cushion geotextile surrounded by coarse aggregate placed over the pipe. A 6-oz/yd² geotextile filter fabric was placed over the top of the coarse aggregate. A sump access riser, 12-inch HDPE solid pipe, was connected to the 12-inch HDPE perforated pipe in each sump and extended upward along the side slopes. Filter sand was then placed over the 6-oz/yd² geotextile fabric at each sump location.

1.3 Existing GMF RP Liner System Information

Based on review of the available documents, a liner system was installed for the GMF RP consisting of (from top to bottom):

- 60-mil textured HDPE geomembrane
- Smooth drum-rolled native soil

Based on the evaluation of design drawings and available construction records, the GMF RP was constructed with a 60-mil textured HDPE geomembrane liner over smooth drum-rolled native soils. No compaction testing was performed on the smooth drum-rolled native soils. Permeability requirements were not specified for the native soils.

1.4 Type and Volume of Materials

Based on Golder's comparison (using Autodesk Civil 3D) of the existing conditions (December 2020 survey by IngenAE) and the approximate top-of-liner-system grades developed from the as-built top of liner, approximately 298,500 cubic yards (CY) of gypsum are present in the GMF GSP and approximately 51,500 CY of gypsum are present in the GMF RP.

The wet scrubber system used for flue gas desulfurization at Coffeen Power Station produced synthetic gypsum (calcium sulfate). The synthetic gypsum is generally of the same chemical structure as natural gypsum. Minimal information on the specific gypsum material produced at Coffeen Power Station is available. Because the material was sluiced, the particle-size distribution of the gypsum in the GMF GSP and GMF RP is expected to be variable, becoming finer with increased distance from the deposition locations. The Federal Highway Administration (FHWA) (2011) indicates that gypsum typically comprises approximately 17% sand-sized particles, 81% silt-sized particles, and 2% clay-sized particles. We Energies (2013) shows a comparable grain-size distribution for gypsum produced at Pleasant Prairie Power Plant, but with a less significant sand-sized fraction. Recent testing of gypsum produced at Duck Creek Power Plant showed the gypsum was comprised of approximately 3.5% sand-sized particles, 91.6% silt-sized particles, and 4.9% clay-sized particles.

We Energies (2013) cites a typical hydraulic conductivity of 1×10^{-5} cm/s for dewatered gypsum but indicates a measured hydraulic conductivity of 3×10^{-3} cm/s for gypsum produced at Pleasant Prairie Power Plant. Recent permeability testing of gypsum produced at Duck Creek Power Plant showed a typical hydraulic conductivity of 6.9×10^{-5} cm/s for gypsum at 10 psi confining pressure.

1.5 Water Levels

At the time of the December 2020 survey by IngenAE, the water level in the GMF GSP was at El. 625.2 feet and the water level in the GMF RP was at El. 617.6 feet. Although the water level would be expected to respond to wet or dry climate conditions, this water level is likely typical. Based on this water level, all the gypsum in the GMF GSP and GMF RP is below the water level and can be considered saturated. The gypsum forms a plateau at the north end of the GMF GSP with the highest point just below the water level at approximately El. 625 feet and the gypsum on the western side of the GMF RP has its highest point at approximately El. 613.5 feet.

Ramboll has provided a surface corresponding to the top of the uppermost aquifer unit. Based on a comparison of this surface and the as-built liner system grades, the top of the liner system appears to be below the top of the uppermost aquifer across the majority of the GMF GSP and GMF RP.

2.0 CLOSURE-BY-REMOVAL INFORMATION

Section 845.710(c)(1) requires the evaluation of complete removal of CCR and Section 845(d)(2) requires Closure Alternatives Analysis to identify if the Power Plant has a landfill that can accept the CCR or if constructing an on-Site landfill is feasible. Additionally, Section 845.710(c)(1) requires the evaluation of multiple modes of transportation of CCR, including rail, barge, and truck. This section includes evaluation of on-Site landfill options, potential off-Site landfills, and potential methods for transporting CCR to off-Site landfills.

2.1 Evaluation of On-Site CCR Landfill Options

There is an existing CCR landfill at the Coffeen Site, which currently has capacity for up to approximately 375,500 CY of additional material. The approximately 79,000 CY of CCR and subsoil resulting from closure-by-removal of the GMF RP are planned for disposal in the on-Site landfill. Under a closure-by-removal scenario, approximately 296,000 CY of material from Ash Pond No. 1 (AP1) would also be disposed of in the on-Site landfill. Therefore, the on-Site landfill does not currently have the capacity to contain all the CCR and subsoil that would be excavated from both the GMF RP and GMF GSP under the closure-by-removal scenario. Under closure-by-removal, material from the GMF GSP will be hauled off-Site for disposal.

Due to planned future land use of the surrounding property dedicated to renewable power generation, the landfill also cannot be expanded to sufficiently increase its capacity. Neither expansion of the existing on-site landfill nor construction of a new on-site landfill is a viable alternative at this site.

2.2 Potential Off-Site CCR Receiving Landfills

Potential off-Site landfills suitable for disposing of the approximately 295,500 CY of CCR and 234,000 CY of clay liner and subsoil within the GMF GSP were evaluated using IEPA's online Illinois Disposal Capacity Report. The closest landfills to the site, by road miles, were determined to be Republic Services' Litchfield-Hillsboro Landfill (a.k.a. Litchfield Landfill) in Litchfield, Illinois and Waste Management's Five Oaks Recycling and Disposal Facility (a.k.a. Five Oaks Landfill) in Taylorville, Illinois.

The Litchfield Landfill is the preferred landfill due to its location being closer to the Coffeen Power Plant (17.9 vs. 43.5 one-way miles, respectively), thereby resulting in reduced hauling mileage. Both landfills have sufficient remaining capacity to receive the approximately 529,500 CY of CCR, clay, and subsoil. Both landfills have been contacted but, as of the date of this memo, only the Litchfield Landfill has confirmed that they would be willing to accept the CCR. Golder has requested a quote for disposal costs (tipping fees) but has not yet received a quote. Information on both landfills is provided in Table 1 below.

Table 1: Off-Site Landfill Information

Landfill Name	Owner	Location	One-Way Distance from Site by Road (Miles)	2020 Five-Year Average Disposal Volume (in-place CY)	2020 Remaining Capacity Reported (in-place CY)
Litchfield Landfill	Republic Services	Litchfield, IL	17.9	82,620	1,535,189
Five Oaks Landfill	Waste Management	Taylorville, IL	43.5	249,664	7,051,864

2.3 Potential Off-Site CCR Transportation Methods

Section 845.710(c)(1) requires Closure-by-Removal to consider multiple methods for transporting removed CCR, including using rail, barge, and trucks. An evaluation of each method is included within this section.

2.3.1 Transportation by Rail

The Coffeen Power Plant currently has a rail spur on-Site that was historically used to receive coal shipments, which were unloaded via an unloading terminal. The terminal is not currently suitable for the loading of CCR into rail cars as it was designed and constructed for unloading, rather than loading. Additionally, the terminal was partially decommissioned by removing associated transformers and disconnecting the electrical supply after the Coffeen Power Plant was closed in 2019. For CCR to be hauled by rail from the Coffeen Power Plant, a new loading terminal would need to be constructed, thereby increasing the project schedule due to the need to complete design, permitting, and construction.

While the Lichfield Landfill is located within approximately 2.3 miles of an existing rail line, an existing terminal suitable for the unloading of CCR is not present. A rail unloading terminal would need to be constructed, which would increase the project schedule due to the need to coordinate with the railroad, complete design and permitting, and construct the terminal. CCR would still need to be hauled by truck from the new off-Site unloading terminal to the landfill, resulting in additional CCR handling and exposure to the surrounding environment. The Five Oaks Landfill has a rail spur on-site.

Furthermore, a direct rail route from the Coffeen Power Plant to either landfill does not exist. Hauling CCR to the Lichfield or Five Oaks Landfills would involve approximately 25 and 63 miles, respectively, of hauling by rail on tracks owned by three separate rail lines (Norfolk Southern Ry. Co., BNSF Ry. Co., and Illinois & Midland R.R. Inc.). The ability of CCR to be hauled over multiple lines and transferred from line to line is currently unknown.

Therefore, transporting CCR by rail is unlikely to be a viable option for the GMF GSP at the Coffeen Power Plant, due to the need to design, permit, and construct additional loading and unloading infrastructure, resulting in corresponding project schedule delays, and the distance and number of rail lines which the CCR would need to be transported over.

2.3.2 Transportation by Barge

The Coffeen Power Plant is not located near a navigable waterway and, therefore, transportation of CCR by barge is not feasible.

2.3.3 Transportation by Truck

The Coffeen Power Plant is located approximately 2.9 miles from Illinois Route 185 (IL-185), which is suitable for receiving truck hauling traffic. Red Ball Trail links the Coffeen Power Plant to IL-185 and routinely receives truck traffic associated with adjacent industrial facilities and the Coffeen Power Plant. Potential travel routes between the Coffeen Power Plant and Litchfield and Five Oaks Landfills have been assumed for cost estimate purposes, although actual travel routes may vary.

Transporting CCR by truck will not require the construction of additional loading or unloading infrastructure at either the receiving landfill or the Coffeen Power Plant. CCR would be loaded into trucks using heavy equipment at the GMF GSP. CCR will then be unloaded at the receiving landfill by the truck directly. Since no construction is required, project delays related to coordination with other entities, design, and permitting are unlikely to occur. Therefore, transporting CCR by truck is a viable option for the GMF GSP at the Coffee Power Plant.

3.0 CLOSURE DESCRIPTION NARRATIVES

Section 845.720(a)(1)(A) requires narrative description of CCR impoundment closures to be prepared. Narrative descriptions have been prepared for both closure-in-place and closure-by-removal and are included in this section.

3.1 GMF GSP Closure-in-Place

The closure-in-place concept for the GMF GSP was developed to reduce the waste footprint at closure. The proposed closure-in-place option would have final cover slopes of 25H:1V (4%) to accommodate moderate settlement and promote drainage. A berm will be constructed at the south end of the consolidated footprint for stability. The location of the berm has been selected to accommodate the estimated 295,500 CY of gypsum and 38,000 CY of clayey soil from the GMF GSP to be contained within the consolidated footprint based on the grading plan presented. The general sequencing plan for the closure-in-place option is as follows:

- Pump out ponded water [approximately 106 million gallons (MG)] from the GMF GSP to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- Resume pumping of the perimeter drains surrounding the GSP to lower the groundwater level beneath the GSP and facilitate closure construction. Discharge water to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- A temporary water management system will be constructed within the GMF GSP, including ditches and sumps. The system will maintain the GMF GSP in an unwatered state by collecting contact stormwater during closure construction. Stormwater flow will be conveyed through Outfall 023 to the existing drainage to the east where it will be managed in accordance with the NPDES permit for the site.

- Once the ponded water has been removed from the GMF GSP, the gypsum in the consolidated footprint will be dewatered. Approximately 131,000 CY of gypsum will be dewatered as needed to enable relocation. It is anticipated that approximately 8.8 MG of water removal will be required to dewater the gypsum. The gypsum will dewater to some degree by gravity, but dewatering by pumping from trenches and sumps is expected to be necessary. Liquid waste and water flowing to sumps will be managed in accordance with the NPDES permit for the site and discharged through Outfall 023.
- Gypsum will be removed from the berm footprint and relocated into the consolidated footprint. The berm will be constructed in an east-west orientation at the east end of the consolidated footprint. The upstream face of the berm will be lined with a composite liner system consisting of 60-mil HDPE geomembrane overlying a compacted clay layer, which will tie into the existing composite liner system.
- The remaining gypsum south of the berm in the GMF GSP will be collected and deposited north of the berm.
- Geosynthetic components of the existing liner system and PWRS south of the berm in the GMF GSP will be removed and disposed in the closure footprint or hauled away for disposal. It is anticipated that up to 1 foot of clay soil beneath the geomembrane may also be removed. The soils will be visually observed for signs of CCR. If soils with signs of CCR are observed, they will be removed and deposited north of the berm (for the purposes of conceptual design, assume 1 foot, or approximately 38,000 CY, will need to be removed).
- Compacted fill, composed of locally available soils, would be placed only as needed to achieve final cover subgrade. The compacted fill is anticipated to be compacted to a minimum of 95% of the standard Proctor maximum dry density to reduce settlement.
- Construction of an alternate final cover system, consisting of (from top to bottom):
 - 24-inch final protective soil layer. The final protective soil layer would include 18 inches of protective soil cover overlain by a 6-inch-thick topsoil layer, and be revegetated with grass species selected to reduce maintenance based on soil type. The 18 inches of protective soil cover will be constructed of locally available soils removed from the embankment containment berm and compacted to between 80% and 95% of the standard Proctor maximum dry density for establishment of vegetation and protection of the underlying geomembrane. Protective soil layer material is likely to be primarily low-plasticity silt or clay based on review of site geotechnical information.
 - Drainage geocomposite
 - 40-mil linear low-density polyethylene (LLDPE) geomembrane layer
- All areas of the cover system will be sloped at a minimum of 4% to positively drain to the exterior of the GMF GSP.
- To prevent impoundment of water in the south end of the current GMF GSP footprint after gypsum removal, existing earthen embankments not required for the consolidated footprint will be removed on the eastern side of the GMF GSP and a channel will be excavated to allow stormwater to flow through existing NPDES Outfall 023 into the existing drainage to the east.
- The final ground surface of the southern part of the GMF GSP will be sloped to drain a minimum of 0.5% towards the channel excavated in the southeast corner, in order to allow post-closure, non-contact stormwater

to gravity flow into the existing drainage. Soil fill, sourced from existing berms no longer required to contain waste in the consolidated footprint or from the on-Site soil borrow area southeast of AP1, will be used to achieve the necessary slopes. Additional fill will come from off-Site borrow sources assumed to be located within 2 miles of the GMF GSP.

- Vegetation will be established on the final surface of the GMF GSP. Stormwater best management practices (BMPs) such as erosion control blankets will be used, as needed to reduce erosion during vegetation establishment.
- After vegetation is established, BMPs will be removed, and closure construction will be considered complete.

3.2 GMF GSP Closure by Removal

A narrative description of closure-by-removal activities associated with the GMF GSP include:

- Pump out ponded water [approximately 106 million gallons (MG)] from the GMF GSP to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- Resume pumping of the perimeter drains surrounding the GSP to lower the groundwater level beneath the GSP and facilitate closure construction. Discharge water to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- A temporary water management system will be constructed within the GMF GSP, including ditches and sumps. The system will maintain the GMF GSP in an unwatered state by collecting contact stormwater during closure construction. Stormwater flow will be conveyed through Outfall 023 to the existing drainage to the east where it will be managed in accordance with the NPDES permit for the site.
- Once the ponded water has been removed from the GMF GSP, the gypsum will be dewatered. Approximately 295,500 CY of gypsum is located below the current water level in the GMF GSP and it is anticipated that approximately 8.8 MG of water removal will be required to dewater the gypsum. The gypsum will dewater to some degree by gravity, but dewatering by pumping from trenches and sumps is expected to be necessary. Liquid waste and water flowing to sumps will be managed in accordance with the NPDES permit for the site and discharged through Outfall 023.
- Gypsum will be removed from the GMF GSP using mass mechanical excavation techniques.
- Approximately 295,500 CY of gypsum will be loaded into over-the-road dump trucks and hauled to the off-Site receiving landfill.
- the PWRS and composite liner system, consisting of the HDPE geomembrane and 3-foot compacted clay layer (176,000 CY), will be removed as required and disposed. It is anticipated that up to 1 foot of subsoil beneath the liner system may also be removed. The subsoils will be visually observed for signs of CCR. If subsoils with signs of CCR are observed, they will be removed and disposed (assume 1 foot of subsoil removal, approximately 58,000 CY, will be required for conceptual designs).
- To prevent impoundment of water in the GMF GSP footprint after gypsum removal, existing earthen embankments will be removed on the eastern side of the GMF GSP and a channel will be excavated to allow stormwater to flow through existing NPDES Outfall 023 into the existing drainage to the east.

- The final ground surface of the GMF GSP will be sloped to drain a minimum of 0.5% towards the channel excavated in the southeast corner, in order to allow post-closure, non-contact stormwater to gravity flow into the existing drainage. Soil fill, sourced from existing berms no longer required to contain waste in the consolidated footprint or from the on-Site soil borrow area southeast of AP1, will be used to achieve the necessary slopes. Additional fill will come from off-Site borrow sources assumed to be located within 2 miles of the GMF GSP.
- Vegetation will be established on the final surface of the GMF GSP. Stormwater best management practices (BMPs) such as erosion control blankets will be used, as needed to reduce erosion during vegetation establishment.
- After vegetation is established, BMPs will be removed, and closure construction will be considered complete.

3.3 GMF RP Closure by Removal

A narrative description of closure-by-removal activities associated with the GMF RP include:

- Pump out ponded water (approximately 45.5 MG) from the GMF RP to the existing drainage to the east through Outfall 023 where it will be managed in accordance with the NPDES permit for the site.
- A temporary water management system will be constructed within the GMF RP, including ditches and sumps. The system will maintain the GMF RP in an unwatered state by collecting contact stormwater during closure construction. Stormwater flow will be conveyed through Outfall 023 to the existing drainage to the east where it will be managed in accordance with the NPDES permit for the site.
- Once the ponded water has been removed from the GMF RP, the gypsum will be dewatered. Approximately 51,000 CY of gypsum is located below the current water level in the GMF RP and it is anticipated that approximately 1.5 MG of water removal will be required to dewater the gypsum. The gypsum will dewater to some degree by gravity, but dewatering by pumping from trenches and sumps is expected to be necessary. Liquid waste and water flowing to sumps will be managed in accordance with the NPDES permit for the site and discharged through Outfall 023.
- Gypsum will be removed from the GMF RP using mass mechanical excavation techniques.
- Approximately 51,000 CY of gypsum will be hauled by truck from the GMF RP to the on-Site CCR Landfill for disposal.
- The geomembrane liner system will be removed as required and disposed. It is anticipated that up to 1 foot of subsoil beneath the geomembrane may also be removed and disposed in the on-Site CCR Landfill. The subsoils will be visually observed for signs of CCR. If subsoils with signs of CCR are observed, they will be removed and disposed (assume 1 foot of subsoil removal, approximately 28,000 CY, will be required for conceptual designs).
- To prevent impoundment of water in the GMF RP footprint after gypsum removal, existing earthen embankments will be removed on the eastern side of the GMF GSP and a channel will be excavated to allow stormwater to flow through existing NPDES Outfall 023 into the existing drainage to the east.
- The final ground surface of the GMF RP will be sloped to drain a minimum of 0.5% towards the channel excavated in the northeast corner, in order to allow post-closure, non-contact stormwater to gravity flow into

the existing drainage. Soil fill, sourced from existing berms no longer required to contain waste will be used to achieve the necessary slopes.

- Vegetation will be established on the final surface of the GMF RP. Stormwater best management practices (BMPs) such as erosion control blankets will be used, as needed to reduce erosion during vegetation establishment.
- After vegetation is established, BMPs will be removed, and closure construction will be considered complete.

4.0 CONSTRUCTION SCHEDULES

Section 845.720(a)(1)(F) requires a schedule including all activities necessary to complete closure to be prepared. Schedules have been prepared for both closure-in-place and closure-by-removal and are included within this section. Schedules were prepared using estimates of task durations based on Golder’s experience, typical weather conditions at the site, and expected construction rates relative to estimated construction quantities.

4.1 GMF GSP Closure-in-Place

The proposed closure completion schedule for GMF GSP closure-in-place is provided in Table 2.

Table 2: Construction Schedule – GMF GSP Closure-in-Place

Milestone	Timeframe (Preliminary Estimates)
Agency Coordination, Approvals, and Permitting <ul style="list-style-type: none"> ■ Obtain state permits, as needed, for dewatering, water discharge, land disturbance, and dam modifications 	8 to 12 months after Final Closure Plan Approval
Final Design and Bid Process <ul style="list-style-type: none"> ■ Complete final design of the closure and select a construction contractor 	8 to 14 months after Agency Coordination, Approvals, and Permitting
Dewater and Stabilize CCR, Install Final Cover System <ul style="list-style-type: none"> ■ Complete contractor mobilization, installation of stormwater BMPs, and unwatering of GMF GSP ■ Stabilize GMF GSP, and complete grading ■ Install the final cover system and stormwater conveyances ■ Winter weather delays are assumed between November and March of each construction year 	18 to 27 months after necessary permits are issued
Site Restoration <ul style="list-style-type: none"> ■ Seed and stabilize GMF GSP 	2 to 3 months after the final cover system is complete

Milestone	Timeframe (Preliminary Estimates)
<ul style="list-style-type: none"> Complete contractor demobilization 	
Timeframe to Complete Closure	36 to 56 months

4.2 GMF GSP Closure-by-Removal

The proposed closure completion schedule for GMF GSP closure-by-removal is provided in Table 3.

Table 3: Construction Schedule – GMF GSP Closure-by-Removal

Milestone	Timeframe (Preliminary Estimates)
Agency Coordination, Approvals, and Permitting <ul style="list-style-type: none"> Obtain state permits, as needed, for dewatering, water discharge, land disturbance, and dam modifications 	8 to 12 months after Final Closure Plan Approval
Final Design and Bid Process <ul style="list-style-type: none"> Complete final design of the closure and select a construction contractor 	8 to 14 months after Agency Coordination, Approvals, and Permitting
Dewater and Excavate CCR, Decontaminate CCR Unit <ul style="list-style-type: none"> Complete contractor mobilization, installation of stormwater BMPs, and unwatering of GMF GSP Complete mass excavation of CCR and decontamination of GMF GSP Winter weather delays are assumed between November and March of each construction year 	23 to 34 months after necessary permits are issued
Backfill with Clean Soil <ul style="list-style-type: none"> Regrade GMF GSP base grade and slope to drain. 	6 to 9 months after decontamination is complete
Site Restoration <ul style="list-style-type: none"> Seed and stabilize GMF GSP Complete contractor demobilization 	2 to 3 months after backfill is complete
Timeframe to Complete Closure	47 to 72 months

4.3 GMF RP Closure-by-Removal

The proposed closure completion schedule for GMF RP closure-by-removal is provided in Table 4.

Table 4: Construction Schedule – GMF RP Closure-by-Removal

Milestone	Timeframe (Preliminary Estimates)
Agency Coordination, Approvals, and Permitting <ul style="list-style-type: none"> ■ Obtain state permits, as needed, for dewatering, water discharge, land disturbance, and dam modifications 	8 to 12 months after Final Closure Plan Approval
Final Design and Bid Process <ul style="list-style-type: none"> ■ Complete final design of the closure and select a construction contractor 	8 to 14 months after Agency Coordination, Approvals, and Permitting
Dewater and Excavate CCR, Decontaminate CCR Unit <ul style="list-style-type: none"> ■ Complete contractor mobilization, installation of stormwater BMPs, and unwatering of GMF RP ■ Complete mass excavation of CCR and decontamination of GMF RP ■ Winter weather delays are assumed between November and March of each construction year 	7 to 11 months after necessary permits are issued
Backfill with Clean Soil <ul style="list-style-type: none"> ■ Regrade GMF RP base grade and slope to drain. 	2 to 3 months after decontamination is complete
Site Restoration <ul style="list-style-type: none"> ■ Seed and stabilize GMR RP ■ Complete contractor demobilization 	1 to 2 months after backfill is complete
Timeframe to Complete Closure	26 to 42 months

5.0 MATERIAL, QUANTITY, LABOR, AND MILEAGE ESTIMATES

Section 845.720(d)(1) requires that an estimate be prepared in accordance with the Class 4 standards of the Association for the Advancement of Cost Engineering (AACE). Estimates for both closure-in-place and closure-by-removal were prepared in accordance with the AACE Class 4 standards.

In addition to construction quantity estimates, Golder has also prepared estimates of construction labor hours, equipment usage, haul truck mileage, daily labor mobilization vehicle mileage, material delivery mileage, and on-Site vehicle mobilization mileage.

Estimates were prepared using the following approach:

- Major construction components and line items were identified, in accordance with the narrative closure description
- Construction quantities were estimated based on volume estimates, area estimates, and proposed construction schedules
- Soil fill beyond what is available on-Site was assumed to come from off-Site borrow sources located within 2 miles of the site, as limited borrow soil is expected to be available at the Coffeen Power Plant, due to planned future land use of the surrounding property dedicated to renewable power generation
- For line items where RS Means data was available, the corresponding RS Means crew size, equipment description, and daily output were used to estimate the total number of man-hours and equipment hours. For line items where RS Means data was unavailable, the crew size, equipment description, and daily output were estimated based on Golder's experience.
- Daily labor mobilization miles were estimated assuming an average one-way commute of 35 miles for each individual working on-Site. The number of working days were estimated from the construction schedules.
- Estimates of haul truck mileage were based on the assumed round-trip haul distance and dump truck size. All dump trucks were assumed to be filled to capacity.
- Estimates of material delivery miles were prepared based on Golder's experience
- A contingency of 30% was applied for the construction hours estimate total, based on the level of design and quantity estimate prepared as part of this Memo

The detailed hours estimate and labor and mileage estimates for GMF GSP closure-in-place are provided in Tables 5 and 6, respectively.

The detailed hours estimate and labor and mileage estimates for GMF GSP closure-by-removal are provided in Tables 7 and 8, respectively.

The detailed hours estimate and labor and mileage estimates for GMF RP closure-by-removal are provided in Tables 9 and 10, respectively.

6.0 REFERENCES

AACE International. 2020. Recommended Practice 18R-97: Cost Estimate Classification System – As Applied in Engineering, Procurement, and Construction for the Process Industries. 2020.

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APPENDIX A

Tables

AACE Class 4 Estimate
Coffeen Power Station
Closure-in-Place of GMF Gypsum Stack Pond

Item No.	Item Description	Quantity	Unit	Crew	Daily Output	Labor Hours	Equipment Hours	Notes/Assumptions/Reference
Pre-Construction								
1	Mobilization and Demobilization (10% of Construction Subtotal)	1	LS					Typical Industry Value
Pre-Construction Subtotal								
Site Preparation								
2	Mow Vegetation in limits of disturbance	250	MSF	B84	22	91	91	RS Means 320190191660: Mowing, mowing brush, light density, tractor with rotary mower
3	Construction Soil Erosion & Sediment Controls (Silt Fence)	7000	LF	B62	650	258	86	RS Means 312514161000: Synthetic erosion control, silt fence, install and remove, 3' high
4	Construction Facilities	25	MO - in use	-	-	-	-	
	Office Trailer	25	MO - in use	-	-	-	-	RS Means 015213200350: Office trailer, furnished, rent per month, 32' x 8', excl. hookups
	Storage Trailers (x2)	25	MO - in use	-	-	-	-	RS Means 015213201350: Storage boxes, rent per month, 40' x 8'
	Portable Toilet (x2)	25	MO - in use	-	-	-	-	RS Means 015433406410: Rent toilet portable chemical, incl. hourly oper. cost
5	Dust Control	217	Day	B59	0.5	3,478	3,478	RS Means 312323202510: Hauling, heavy, dust control, includes loading
6	Haul Road Maintenance	109	Day	B86A	1	869	869	RS Means 312323202600: Hauling, haul road maintenance, includes loading
Site Preparation Subtotal								
4,700 4,520								
Dewatering, Unwatering, and Stormwater Management								
7	Unwatering of GMF GSP ponded water	409	Day	Dewater	4	818	204	RS Means 312319200650: Dewatering, pumping 8 hours, attended 2 hours per day, 4" discharge pump used for 8 hours, includes 20 LF of suction hose and 100 LF of discharge hose
8	Dewatering and Stormwater Management for GMF GSP	350	Day	Dewater	4	699	175	RS Means 312319200650: Dewatering, pumping 8 hours, attended 2 hours per day, 4" discharge pump used for 8 hours, includes 20 LF of suction hose and 100 LF of discharge hose
9	Dewatering Sumps Installation	10	EA	Sump Install	4	40	20	Unit Rate, Crew, and Daily Output based on experience. Materials include 24" corrugated HDPE pipe with geotextile wrapping, and 1 CY of gravel backfill
Dewatering, Unwatering, and Stormwater Management Subtotal								
1,560 400								
GMF Gypsum Stack Pond Closure								
11	Relocation of Gypsum Material and Contaminated Clay	169000	CY - in place	-	-	4,876	3,948	
	Excavation and Loading of Material	177450	CY - as excavated	B14A	3230	659	440	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Hauling of Material	177450	CY - as excavated	B34G	680	2,088	2,088	RS Means 312323206020: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 5 MPH, cycle 4000 feet
	Spreading of Material	177450	CY - as excavated	B10B	1000	2,129	1,420	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction
12	Excavation and Placement of Embankment Fill	111100	CY - in place	-	-	3,718	2,936	
	Excavation and Loading of Material	116655	CY - as excavated	B14A	3230	433	289	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Hauling of Material	116655	CY - as excavated	B34G	680	1,372	1,372	RS Means 312323206020: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 5 MPH, cycle 4000 feet
	Spreading of Material	116655	CY - as excavated	B10B	1000	1,400	933	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction
	Compaction of Material	111100	CY - in place	B10F	2600	513	342	RS Means 312323235100: Compaction; Riding, vibrating roller, 12" lifts, 4 passes (RSMeans Crew is B10Y; altered to B10F based on experience)
15	Geomembrane - Berm Liner	76000	SF - in place	B63B	1600	1,520	380	RS Means 310519531200: Pond and reservoir liners, membrane lining systems HDPE, 100,000 S.F. or more, 60 mil thick, per S.F. (multiplied unit rate by 0.5 based on experience)
14	Excavation and Placement of Soil Fill for Final Grades	106890	CY - in place	-	-	3,577	2,825	
	Excavation and Loading of Material	112235	CY - as excavated	B14A	3230	417	278	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Hauling of Material	112235	CY - as excavated	B34G	680	1,320	1,320	RS Means 312323206020: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 5 MPH, cycle 4000 feet
	Spreading of Material	112235	CY - as excavated	B10B	1000	1,347	898	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction
	Compaction of Material	106890	CY - in place	B10F	2600	493	329	RS Means 312323235100: Compaction; Riding, vibrating roller, 12" lifts, 4 passes (RSMeans Crew is B10Y; altered to B10F based on experience)
15	Final Cover Geomembrane	541400	SF - in place	B63B	1600	10,828	2,707	RS Means 310519531200: Pond and reservoir liners, membrane lining systems HDPE, 100,000 S.F. or more, 60 mil thick, per S.F. (multiplied unit rate by 0.5 based on experience)
16	Final Cover Geocomposite	541400	SF - in place	B63B	4800	3,609	902	Unit Rate, and Output based on experience. Crew based on RS Means 310519531200.
17	Anchor Trench Installation	2900	LF	-	-	87	58	
	Excavation of Material	451	CY - as excavated	B11C	150	48	24	RS Means 312316130050: Excavating, Trench or continuous footing, common earth with no sheeting or dewatering included, 1' to 4' deep, 3/8 C.Y. excavator
	Backfilling Material	451	CY - as excavated	B10R	400	14	9	RS Means 312316133020: Backfill trench, F.E. Loader, wheel mtd., 1 C.Y. bucket, minimal haul
	Compaction of Material	430	CY - in place	A1D	140	25	25	RS Means 312323237040: Compaction, walk behind, vibrating plate 18" wide, 6" lifts, 4 passes
19	Placement of Protective Cover Soil	40100	CY - in place	-	-	1,182	953	
	Excavation and Loading of Material	42105	CY - as excavated	B14A	5000	101	67	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Hauling of Material	42105	CY - as excavated	B34G	680	495	495	RS Means 312323206120: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 10 MPH, cycle 4 miles
	Spreading of Material	42105	CY - as excavated	B10B	1000	505	337	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction
	Finish Grading of Material	59851	SY	B10W	8900	81	54	RS Means 312216103300: Fine grading, Finish grading slopes, gentle. Crew altered to reflect likely equipment to be used based on experience
GMF Gypsum Stack Pond Closure Subtotal								
29,400 14,710								
Site Restoration								
22	Erosion Control Blanket	40000	SF - in place	ECB	22500	43	14	RS Means 312514160100: Rolled erosion control mats and blankets, plastic netting, stapled, 2" x 1" mesh, 20 mil.
23	Straw Wattle Ditch Checks	2500	LF - in place	A2	1000	60	20	RS Means 312514160705: Compost or mulch filter sock, 9" diameter
24	Seed, Mulch, and Maintain Vegetated Surfaces	45	AC	-	-	405	405	
	Lime	1960	MSF	B66	700	22	22	RS Means 329113234250: Soil preparation, structural soil mixing, spread soil conditioners, ground limestone, 1#/S.Y., tractor spreader
	Fertilizer	1960	MSF	B66	700	22	22	RS Means 329113234150: Soil preparation, structural soil mixing, spread soil conditioners, fertilizer, 0.2#/S.Y., tractor spreader
	Seed	1960	MSF	B66	52	302	302	RS Means 329219142300: Seeding athletic fields, seeding fescue, tall, 5.5 lb. per M.S.F., tractor spreader
	Mulch	1960	MSF	B65	530	59	59	RS Means 329113160350: Mulching, Hay, 1" deep, power mulcher, large
Site Restoration Subtotal								
510 440								
Engineering & Construction Support Tasks and Contingency								
25	Final Closure Design and Bid Support (5% of Construction Subtotal)	1	LS	-	-	-	-	Typical Industry Value
26	Engineering Support and CQA During Construction (10% of Construction Subtotal)	466	Day	Eng	1	4,664	1,866	Unit Rate, Crew, and Output based on experience.
Engineering & Construction Support Tasks Subtotal								
4,660 1,870								
Construction Subtotal								
36,200 20,100								
Project Subtotal								
40,900 22,000								
30% Contingency								
12,300 6,600								
ENGINEER'S ESTIMATE OF TOTAL CONSTRUCTION AND ENGINEERING HOURS								
53,200 28,600								

Notes and Assumptions:

1. LS = Lump Sum, AC = Acre, LF = Linear Foot, EA = Each, SY = Square Yard, MO = Month, YR = Year, CY = Cubic Yard, MSF = Thousand Square Feet
2. Where possible, costs were developed using RS Means 2022 Heavy Construction Costs
3. 2022 RS Means unit rates include overhead and profit and refer to standard union labor in Effingham, IL.
4. Subtotal and total costs have been rounded to the nearest \$1,000. Subtotal and total hours have been rounded to the nearest 100.
5. Earthwork quantities assume that the excavation and placement of fill within construction limits will be balanced so that no off-site fill will be required to reach the final contours. The final elevations may need to be adjusted during final design to achieve balanced quantities.

Table 6: Labor, Equipment, and Mileage Estimate - GMF GSP Closure-in-Place

Crew	Labor	Daily Labor Hours	Equipment	Daily Equipment Hours	Project Total	
					Labor Hours	Equipment Hours
B84	Operator x1	8	Rotary Mower/Tractor	8	91	91
B62	Laborer x2 Operator x1	24	Loader, Skid Steer, 30 H.P.	8	258	86
B59	Truck Driver x1	8	Truck Tractor, 220 H.P. Water Tank Trailer, 5000 Gal	8	3478	3478
B86A	Operator x1	8	Grader, 30,000 lbs	8	869	869
B14A	Operator x1 Laborer x0.5	12	Hyd. Excavator, 4.5 CY	8	1610	1074
B34G	Truck Driver x1	8	Dump Truck, Off Hwy, 54 ton	8	5275	5275
B10B	Operator x1 Laborer x0.5	12	Dozer, 200 H.P.	8	5381	3588
B63B	Labor Foreman x1 Laborer x2 Operator (light) x1	32	Loader, Skid Steer, 78 H.P.	8	15957	3989
A2	Laborer x2 Truck Driver x1	24	Flatbed Truck, Gas, 1.5 ton	8	60	20
B66	Operator (light) x1	8	Loader-Backhoe, 40 H.P.	8	346	346
B65	Laborer x1 Truck Driver (light) x1	16	Power Mulcher (large) Flatbed Truck, Gas, 1.5 ton	16	59	59
B11C	Laborer x1 Operator (medium) x1	16	Backhoe Loader, 48 H.P.	8	48	24
B10R	Operator (medium) x1 Laborer x0.5	12	F.E. Loader, W.M., 1 CY	8	14	9
ECB	Laborer x3	24	Tractor	8	43	14
Dewater	Laborer x1	8	8" Diesel Pump	2	1517	379
Sump Install	Laborer x1 Operator x1	16	Hyd. Excavator, 4.5 CY	8	40	20
Eng	Engineering Staff x1.2	10	Side by Side x1	4	4664	1866
A1D	Laborer x1	8	Vibrating Plate, Gas, 18"	8	25	25
B10F	Operator (medium) x1 Laborer x0.5	12	Tandem Roller, 10 ton	8	1006	671
B10W	Operator (medium) x1 Laborer x0.5	12	Dozer, 105 H.P.	8	81	54
PROJECT TOTAL					40822	21937

Notes and Assumptions:

1. Crew names in italics were created by Golder based on experience and are not from RS Means.

Coffeen Power Station
Closure-in-Place of GMF Gypsum Stack Pond

Item	Quantity	Assumptions
Labor Total Hours	40,822	Per projected total in cost estimate
Duration of Onsite Construction - Days	758	Per Construction Schedule
Average Daily Crew Size	6	10 hour days
Labor Mobilization Miles	318,570	Average of 70 miles round trip per day
Vehicle Miles On-Site	13,350	1 mile round trip from gate to parking 5 miles per day for CQA tech and Construction Supervisor 10% Contingency for Site visitors (client and engineering support)
Equipment Mobilization Miles - Unloaded	32,507	Average of 300 miles one way for equipment hauling Average 1 load of equipment per working week
Equipment Mobilization Miles - Loaded	32,507	Average of 300 miles one way for equipment hauling Average 1 load of equipment per working week
Total Equipment Miles On-Site	56,887	Average of 4 of 6 crew members running equipment Assume 15 miles per piece of equipment (based on 15 minute round trip path across GMF GSP 10 miles per day used for water truck 5 miles per day for grader
On-Site Haul Truck Miles - Unloaded	7,004	34 CY Haul Truck 4000 ft and 4 mile cycles
On-Site Haul Truck Miles - Loaded	7,004	34 CY Haul Truck 4000 ft and 4 mile cycles
Off-Site Haul Truck Miles - Unloaded	-	16.5 CY Dump Truck 4 mile cycle
Off-Site Haul Truck Miles - Loaded	-	16.5 CY Dump Truck 4 mile cycle
Material Delivery Miles - Unloaded	6,925	Same geosynthetic material source, trailer quantities, and roll sizes as Coffeen AP2 project assumed 45 extra trips for seed, fertilizer, lime, mulch, ECBS, straw wattles, and concrete
Material Delivery Miles - Loaded	6,925	Same geosynthetic material source, trailer quantities, and roll sizes as Coffeen AP2 project assumed 45 extra trips for seed, fertilizer, lime, mulch, ECBS, straw wattles, and concrete

AACE Class 4 Estimate
Coffeen Power Station
Closure-by-Removal of GMF Gypsum Stack Pond

Item No.	Item Description	Quantity	Unit	Crew	Daily Output	Labor Hours	Equipment Hours	Notes/Assumptions/Reference
Pre-Construction								
1	Mobilization and Demobilization (10% of Construction Subtotal)	1	LS					Typical Industry Value
Pre-Construction Subtotal								
Site Preparation								
2	Mow Vegetation in limits of disturbance	250	MSF	B84	22	91	91	RS Means 320190191660: Mowing, mowing brush, light density, tractor with rotary mower
3	Construction Soil Erosion & Sediment Controls (Silt Fence)	7000	LF	B62	650	258	86	RS Means 312514161000: Synthetic erosion control, silt fence, install and remove, 3' high
4	Construction Facilities	39	MO - in use	-	-	-	-	
	Office Trailer	39	MO - in use	-	-	-	-	RS Means 015213200350: Office trailer, furnished, rent per month, 32' x 8', excl. hookups
	Storage Trailers (x2)	39	MO - in use	-	-	-	-	RS Means 015213201350: Storage boxes, rent per month, 40' x 8'
	Portable Toilet (x2)	39	MO - in use	-	-	-	-	RS Means 015433406410: Rent toilet portable chemical, incl. hourly oper. cost
5	Dust Control	169	Day	B59	0.5	2,708	2,708	RS Means 312323202510: Hauling, heavy, dust control, includes loading
6	Haul Road Maintenance	114	Day	B86A	1	912	912	RS Means 312323202600: Hauling, haul road maintenance, includes loading
Site Preparation Subtotal								
3,970 3,800								
Dewatering, Unwatering, and Stormwater Management								
7	Unwatering, Dewatering, and Stormwater Management for GMF GSP	409	Day	Dewater	4	818	204	RS Means 312319200650: Dewatering, pumping 8 hours, attended 2 hours per day, 4" discharge pump used for 8 hours, includes 20 LF of suction hose and 100 LF of discharge hose
8	Dewatering and Stormwater Management for GMF GSP	751	Day	Dewater	4	1,503	376	RS Means 312319200650: Dewatering, pumping 8 hours, attended 2 hours per day, 4" discharge pump used for 8 hours, includes 20 LF of suction hose and 100 LF of discharge hose
9	Dewatering Sumps Installation	10	EA	Sump Install	4	40	20	Unit Rate, Crew, and Daily Output based on experience. Materials include 24" corrugated HDPE pipe with geotextile wrapping, and 1 CY of gravel backfill
Dewatering, Unwatering, and Stormwater Management Subtotal								
2,360 600								
GMF Gypsum Stack Pond Closure								
12	Disposal of Gypsum/Liner/Subsoil at Off-Site Landfill	529500	CY - in place	-	-	48,679	47,147	
	Excavation and Loading of Material	555975	CY - as excavated	B14A	3230	2,066	1,377	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Hauling of Material	555975	CY - as excavated	B34C	99	44,927	44,927	RS Means 312323203284: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 16.5 C.Y. truck, 20 min wait/ld/uld., 40 MPH, cycle 40 miles
	Finish Grading of Excavation Surface	175645	SY	B32C	5000	1,686	843	RS Means 312216101020: Fine grading, loam or topsoil fine grade for large area, 15,000 S.Y. or more
	Landfill Tipping Fee	714825	Ton	-	-	-	-	Unit Rate based on actual tipping fee from Republic Services Litchfield Landfill (nearest landfill to Site). Unit Rate subject to increase upon Landfill's soil classification.
13	Excavation and Placement of On-Site Soil Fill for Final Grades	244003	CY - in place	-	-	8,166	6,450	
	Excavation and Loading of Material	256203	CY - as excavated	B14A	3230	952	635	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Hauling of Material	256203	CY - as excavated	B34G	680	3,014	3,014	RS Means 312323206020: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 5 MPH, cycle 4000 feet
	Spreading of Material	256203	CY - as excavated	B10B	1000	3,074	2,050	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction
	Compaction of Material	244003	CY - in place	B10F	2600	1,126	751	RS Means 312323235100: Compaction; Riding, vibrating roller, 12" lifts, 4 passes (RSMeans Crew is B10Y; altered to B10F based on experience)
14	Placement of On-Site Borrow Soil Fill for Final Grades	91000	CY - in place	-	-	2,920	2,321	
	Excavation and Loading of Material	95550	CY - as excavated	B14A	5000	229	153	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Hauling of Material	95550	CY - as excavated	B34G	680	1,124	1,124	RS Means 312323206120: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 10 MPH, cycle 4 miles
	Spreading of Material	95550	CY - as excavated	B10B	1000	1,147	764	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction
	Compaction of Material	91000	CY - in place	B10F	2600	420	280	RS Means 312323235100: Compaction; Riding, vibrating roller, 12" lifts, 4 passes (RSMeans Crew is B10Y; altered to B10F based on experience)
15	Placement of Imported Borrow Soil Fill for Final Grades	24697	CY - in place	-	-	1,063	810	
	Excavation and Loading of Material	25932	CY - as excavated	B14A	3230	96	64	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)
	Purchase of Material	25932	CY - as excavated	-	-	-	-	Unit Rate based on experience.
	Hauling of Material	25932	CY - as excavated	B34G	680	305	305	RS Means 312323203032: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 16.5 C.Y. truck, 15 min wait/ld/uld., 20 MPH, cycle 4 miles
	Spreading of Material	25932	CY - as excavated	B10B	1000	311	207	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction
	Finish Grading of Material	175645	SY	B10W	8900	237	158	RS Means 312216103300: Fine grading, Finish grading slopes, gentle. Crew altered to reflect likely equipment to be used based on experience.
	Compaction of Material	24697	CY - in place	B10F	2600	114	76	RS Means 312323235100: Compaction; Riding, vibrating roller, 12" lifts, 4 passes (RSMeans Crew is B10Y; altered to B10F based on experience)
GMF Gypsum Stack Pond Closure Subtotal								
60,830 56,730								
Site Restoration								
16	Erosion Control Blanket	48000	SF - in place	ECB	22500	51	17	RS Means 312514160100: Rolled erosion control mats and blankets, plastic netting, stapled, 2" x 1" mesh, 20 mil.
17	Straw Wattle Ditch Checks	2500	LF - in place	A2	1000	60	20	RS Means 312514160705: Compost or mulch filter sock, 9" diameter
18	Seed, Mulch, and Maintain Vegetated Surfaces	45	AC	-	-	405	405	
	Lime	1960	MSF	B66	700	22	22	RS Means 329113234250: Soil preparation, structural soil mixing, spread soil conditioners, ground limestone, 1#/S.Y., tractor spreader
	Fertilizer	1960	MSF	B66	700	22	22	RS Means 329113234150: Soil preparation, structural soil mixing, spread soil conditioners, fertilizer, 0.2#/S.Y., tractor spreader
	Seed	1960	MSF	B66	52	302	302	RS Means 329219142300: Seeding athletic fields, seeding fescue, tall, 5.5 lb. per M.S.F., tractor spreader
	Mulch	1960	MSF	B65	530	59	59	RS Means 329113160350: Mulching, Hay, 1" deep, power mulcher, large
Site Restoration Subtotal								
520 440								
Engineering & Construction Support Tasks and Contingency								
19	Final Closure Design and Bid Support (1.5% of Construction Subtotal)	1	LS	-	-	-	-	Typical Industry Value
20	Engineering Support and CQA During Construction (2% of Construction Subtotal)	868	Day	Eng	1	8,682	3,473	Unit Rate, Crew, and Output based on experience.
Engineering & Construction Support Tasks Subtotal								
8,682 3,470								
Construction Subtotal								
67,700 61,600								
Project Subtotal								
76,400 65,100								
30% Contingency								
22,900 19,500								
ENGINEER'S ESTIMATE OF TOTAL CONSTRUCTION AND ENGINEERING HOURS								
99,300 84,600								

Notes and Assumptions:

1. LS = Lump Sum, AC = Acre, LF = Linear Foot, EA = Each, SY = Square Yard, MO = Month, YR = Year, CY = Cubic Yard, MSF = Thousand Square Feet
2. Where possible, costs were developed using RS Means 2022 Heavy Construction Costs
3. 2022 RS Means unit rates include overhead and profit and refer to standard union labor in Effingham, IL.
4. Subtotal and total costs have been rounded to the nearest \$1,000. Subtotal and total hours have been rounded to the nearest 100.
5. Earthwork quantities assume that the excavation and placement of fill within construction limits will be balanced so that no off-site fill will be required to reach the final contours. The final elevations may need to be adjusted during final design to achieve balanced quantities.

Table 8: Labor, Equipment, and Mileage Estimate - GMF GSP Closure-by-Removal

Coffeen Power Station
Closure-by-Removal of GMF Gypsum Stack Pond

Crew	Labor	Daily Labor Hours	Equipment	Daily Equipment Hours	Project Total	
					Labor Hours	Equipment Hours
B84	Operator x1	8	Rotary Mower/Tractor	8	91	91
B62	Laborer x2 Operator x1	24	Loader, Skid Steer, 30 H.P.	8	258	86
B59	Truck Driver x1	8	Truck Tractor, 220 H.P. Water Tank Trailer, 5000 Gal	8	2708	2708
B86A	Operator x1	8	Grader, 30,000 lbs	8	912	912
B14A	Operator x1 Laborer x0.5	12	Hyd. Excavator, 4.5 CY	8	3343	2229
B34G	Truck Driver x1	8	Dump Truck, Off Hwy, 54 ton	8	4443	4443
B10B	Operator x1 Laborer x0.5	12	Dozer, 200 H.P.	8	4532	3021
A2	Laborer x2 Truck Driver x1	24	Flatbed Truck, Gas, 1.5 ton	8	60	20
B66	Operator (light) x1	8	Loader-Backhoe, 40 H.P.	8	346	346
B65	Laborer x1 Truck Driver (light) x1	16	Power Mulcher (large) Flatbed Truck, Gas, 1.5 ton	16	59	59
B32C	Labor Foreman x1 Laborer x2 Operator (medium) x3	48	Grader, 30,000 lbs Tandem Roller, 10 ton Dozer, 200 H.P.	24	1686	843
ECB	Laborer x3	24	Tractor	8	51	17
Dewater	Laborer x1	8	8" Diesel Pump	2	2321	580
Sump Install	Laborer x1 Operator x1	16	Hyd. Excavator, 4.5 CY	8	40	20
Eng	Engineering Staff x1.2	10	Side by Side x1	4	8682	3473
B10F	Operator (medium) x1 Laborer x0.5	12	Tandem Roller, 10 ton	8	1660	1107
B34C	Truck Driver (heavy) x1	8	Truck Tractor, 6x4, 380 H.P. Dump Trailer, 16.5 CY	8	44927	44927
B10W	Operator (medium) x1 Laborer x0.5	12	Dozer, 105 H.P.	8	237	158
PROJECT TOTAL					76356	65040

Notes and Assumptions:

1. Crew names in italics were created by Golder based on experience and are not from RS Means.

Item	Quantity	Assumptions
Labor Total Hours	76,356	Per projected total in cost estimate
Duration of Onsite Construction - Days	1,160	Per Construction Schedule
Average Daily Crew Size	7	10 hour days
Labor Mobilization Miles	568,528	Average of 70 miles round trip per day
Vehicle Miles On-Site	21,697	1 mile round trip from gate to parking 5 miles per day for CQA tech and Construction Supervisor 10% Contingency for Site visitors (client and engineering support)
Equipment Mobilization Miles - Unloaded	49,725	Average of 300 miles one way for equipment hauling Average 1 load of equipment per working week
Equipment Mobilization Miles - Loaded	49,725	Average of 300 miles one way for equipment hauling Average 1 load of equipment per working week
Total Equipment Miles On-Site	104,423	Average of 5 of 7 crew members running equipment Assume 15 miles per piece of equipment (based on 15 minute round trip path across AP1 10 miles per day used for water truck 5 miles per day for grader
On-Site Haul Truck Miles - Unloaded	8,475	34 CY Haul Truck 4000 ft cycle and 4 mile cycle on-site
On-Site Haul Truck Miles - Loaded	8,475	34 CY Haul Truck 4000 ft cycle and 4 mile cycle on-site
Off-Site Haul Truck Miles - Unloaded	606,292	16.5 CY Dump Truck 36 mile cycle to off-Site Landfill
Off-Site Haul Truck Miles - Loaded	606,292	16.5 CY Dump Truck 36 mile cycle to off-Site Landfill
Material Delivery Miles - Unloaded	4,500	45 extra trips for seed, fertilizer, lime, mulch, ECBs, straw wattles, and concrete
Material Delivery Miles - Loaded	4,500	45 extra trips for seed, fertilizer, lime, mulch, ECBs, straw wattles, and concrete

AACE Class 4 Estimate
Coffeen Power Station
Closure-by-Removal of GMF Recycle Pond

Item No.	Item Description	Quantity	Unit	Crew	Daily Output	Labor Hours	Equipment Hours	Notes/Assumptions/Reference	
Pre-Construction									
1	Mobilization and Demobilization (10% of Construction Subtotal)	1	LS					Typical Industry Value	
Pre-Construction Subtotal									
Site Preparation									
2	Mow Vegetation in limits of disturbance	250	MSF	B84	22	91	91	RS Means 320190191660: Mowing, mowing brush, light density, tractor with rotary mower	
3	Construction Soil Erosion & Sediment Controls (Silt Fence)	7000	LF	B62	650	258	86	RS Means 312514161000: Synthetic erosion control, silt fence, install and remove, 3' high	
4	Construction Facilities	13	MO - in use	-	-	-	-		
	Office Trailer	13	MO - in use	-	-	-	-	RS Means 015213200350: Office trailer, furnished, rent per month, 32' x 8', excl. hookups	
	Storage Trailers (x2)	13	MO - in use	-	-	-	-	RS Means 015213201350: Storage boxes, rent per month, 40' x 8'	
	Portable Toilet (x2)	13	MO - in use	-	-	-	-	RS Means 015433406410: Rent toilet portable chemical, incl. hourly oper. cost	
5	Dust Control	139	Day	B59	0.5	2,230	2,230	RS Means 312323202510: Hauling, heavy, dust control, includes loading	
6	Haul Road Maintenance	70	Day	B86A	1	560	560	RS Means 312323202600: Hauling, haul road maintenance, includes loading	
Site Preparation Subtotal									
3,140 2,970									
Dewatering, Unwatering, and Stormwater Management									
7	Unwatering, Dewatering, and Stormwater Management for GMF GSP	219	Day	Dewater	4	439	110	RS Means 312319200650: Dewatering, pumping 8 hours, attended 2 hours per day, 4" discharge pump used for 8 hours, includes 20 LF of suction hose and 100 LF of discharge hose	
8	Dewatering and Stormwater Management for GMF GSP	170	Day	Dewater	4	339	85	RS Means 312319200650: Dewatering, pumping 8 hours, attended 2 hours per day, 4" discharge pump used for 8 hours, includes 20 LF of suction hose and 100 LF of discharge hose	
9	Dewatering Sumps Installation	8	EA	Sump Install	4	32	16	Unit Rate, Crew, and Daily Output based on experience. Materials include 24" corrugated HDPE pipe with geotextile wrapping, and 1 CY of gravel backfill	
Dewatering, Unwatering, and Stormwater Management Subtotal									
810 210									
GMF Recycle Pond Closure									
12	Disposal of Gypsum/Subsoil at On-Site Landfill	79000	CY - in place	-	-	2,970	2,024		
	Excavation and Loading of Material	82950	CY - as excavated	B14A	3230	308	205	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)	
	Hauling of Material	82950	CY - as excavated	B34G	680	976	976	RS Means 312323206020: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 5 MPH, cycle 4000 feet	
	Finish Grading of Excavation Surface	175645	SY	B32C	5000	1,686	843	RS Means 312216101020: Fine grading, loam or topsoil fine grade for large area, 15,000 S.Y. or more	
13	Excavation and Placement of On-Site Soil Fill for Final Grades	66670	CY - in place	-	-	2,345	1,838		
	Excavation and Loading of Material	70004	CY - as excavated	B14A	3230	260	173	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)	
	Hauling of Material	70004	CY - as excavated	B34G	680	824	824	RS Means 312323206020: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 5 MPH, cycle 4000 feet	
	Spreading of Material	70004	CY - as excavated	B10B	1000	840	560	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction	
	Finish Grading of Material	84000	SY	B10W	8900	113	76	RS Means 312216103300: Fine grading, Finish grading slopes, gentle. Crew altered to reflect likely equipment to be used based on experience.	
	Compaction of Material	66670	CY - in place	B10F	2600	308	205	RS Means 312323235100: Compaction; Riding, vibrating roller, 12" lifts, 4 passes (RSMeans Crew is B10Y; altered to B10F based on experience)	
14	Excavation and Stockpiling of Excess Cut Material	78573	CY - in place	-	-	2,268	1,835		
	Excavation and Loading of Material	82502	CY - as excavated	B14A	3230	307	204	RS Means 312316435400: Excavating, large volume projects; excavation with truck loading; excavator, 4.5 CY bucket, 95% fill factor (assume 5% shrinkage factor from ground to in-place)	
	Hauling and Dumping of Material	82502	CY - as excavated	B34G	680	971	971	RS Means 312323206020: Hauling; no loading equipment, including hauling, waiting, loading/dumping; 34 C.Y. off road, 15 min wait/ld/uld., 5 MPH, cycle 4000 feet	
	Spreading of Material	82502	CY - as excavated	B10B	1000	990	660	RS Means 312323170020: Fill, dumped material, spread, by dozer, excludes compaction	
GMF Recycle Pond Closure Subtotal									
7,580 5,700									
Site Restoration									
15	Erosion Control Blanket	26000	SF - in place	ECB	22500	28	9	RS Means 312514160100: Rolled erosion control mats and blankets, plastic netting, stapled, 2" x 1" mesh, 20 mil.	
16	Straw Wattle Ditch Checks	1500	LF - in place	A2	1000	36	12	RS Means 312514160705: Compost or mulch filter sock, 9" diameter	
17	Seed, Mulch, and Maintain Vegetated Surfaces	22	AC	-	-	198	198		
	Lime	958	MSF	B66	700	11	11	RS Means 329113234250: Soil preparation, structural soil mixing, spread soil conditioners, ground limestone, 1#/S.Y., tractor spreader	
	Fertilizer	958	MSF	B66	700	11	11	RS Means 329113234150: Soil preparation, structural soil mixing, spread soil conditioners, fertilizer, 0.2#/S.Y., tractor spreader	
	Seed	958	MSF	B66	52	147	147	RS Means 329219142300: Seeding athletic fields, seeding fescue, tall, 5.5 lb. per M.S.F., tractor spreader	
	Mulch	958	MSF	B65	530	29	29	RS Means 329113160350: Mulching, Hay, 1" deep, power mulcher, large	
Site Restoration Subtotal									
260 220									
Engineering & Construction Support Tasks and Contingency									
18	Final Closure Design and Bid Support (1.5% of Construction Subtotal)	1	LS	-	-	-	-	Typical Industry Value	
19	Engineering Support and CQA During Construction (10% of Construction Subtotal)	232	Day	Eng	1	2,324	930	Unit Rate, Crew, and Output based on experience.	
Engineering & Construction Support Tasks Subtotal									
2,320 930									
Construction Subtotal							11,800	9,100	
Project Subtotal							14,100	10,000	
30% Contingency							4,200	3,000	
ENGINEER'S ESTIMATE OF TOTAL CONSTRUCTION AND ENGINEERING HOURS							18,300	13,000	

Notes and Assumptions:

1. LS = Lump Sum, AC = Acre, LF = Linear Foot, EA = Each, SY = Square Yard, MO = Month, YR = Year, CY = Cubic Yard, MSF = Thousand Square Feet
2. Where possible, costs were developed using RS Means 2022 Heavy Construction Costs
3. 2022 RS Means unit rates include overhead and profit and refer to standard union labor in Effingham, IL.
4. Subtotal and total costs have been rounded to the nearest \$1,000. Subtotal and total hours have been rounded to the nearest 100.
5. Earthwork quantities assume that the excavation and placement of fill within construction limits will be balanced so that no off-site fill will be required to reach the final contours. The final elevations may need to be adjusted during final design to achieve balanced quantities.

Table 10: Labor, Equipment, and Mileage Estimate - GMF RP Closure-by-Removal

Crew	Labor	Daily Labor Hours	Equipment	Daily Equipment Hours	Project Total	
					Labor Hours	Equipment Hours
B84	Operator x1	8	Rotary Mower/Tractor	8	91	91
B62	Laborer x2 Operator x1	24	Loader, Skid Steer, 30 H.P.	8	258	86
B59	Truck Driver x1	8	Truck Tractor, 220 H.P. Water Tank Trailer, 5000 Gal	8	2230	2230
B86A	Operator x1	8	Grader, 30,000 lbs	8	560	560
B14A	Operator x1 Laborer x0.5	12	Hyd. Excavator, 4.5 CY	8	875	582
B34G	Truck Driver x1	8	Dump Truck, Off Hwy, 54 ton	8	2771	2771
B10B	Operator x1 Laborer x0.5	12	Dozer, 200 H.P.	8	1830	1220
A2	Laborer x2 Truck Driver x1	24	Flatbed Truck, Gas, 1.5 ton	8	36	12
B66	Operator (light) x1	8	Loader-Backhoe, 40 H.P.	8	169	169
B65	Laborer x1 Truck Driver (light) x1	16	Power Mulcher (large) Flatbed Truck, Gas, 1.5 ton	16	29	29
B32C	Labor Foreman x1 Laborer x2 Operator (medium) x3	48	Grader, 30,000 lbs Tandem Roller, 10 ton Dozer, 200 H.P.	24	1686	843
ECB	Laborer x3	24	Tractor	8	28	9
Dewater	Laborer x1	8	8" Diesel Pump	2	778	195
Sump Install	Laborer x1 Operator x1	16	Hyd. Excavator, 4.5 CY	8	32	16
Eng	Engineering Staff x1.2	10	Side by Side x1	4	2324	930
B10F	Operator (medium) x1 Laborer x0.5	12	Tandem Roller, 10 ton	8	308	205
B34C	Truck Driver (heavy) x1	8	Truck Tractor, 6x4, 380 H.P. Dump Trailer, 16.5 CY	8	0	0
B10W	Operator (medium) x1 Laborer x0.5	12	Dozer, 105 H.P.	8	113	76
PROJECT TOTAL					14118	10024

Notes and Assumptions:

1. Crew names in italics were created by Golder based on experience and are not from RS Means.

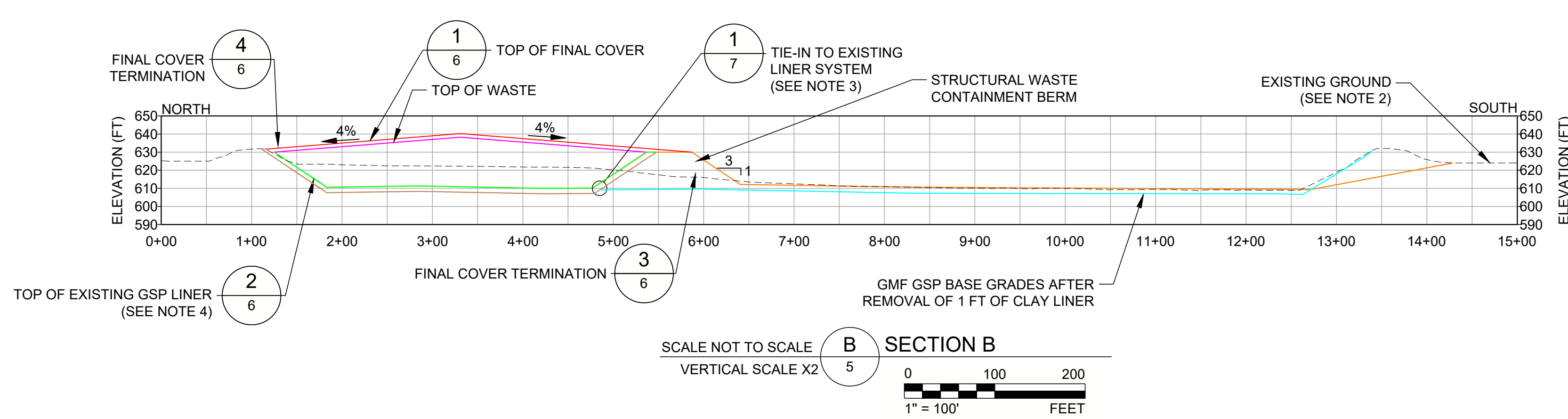
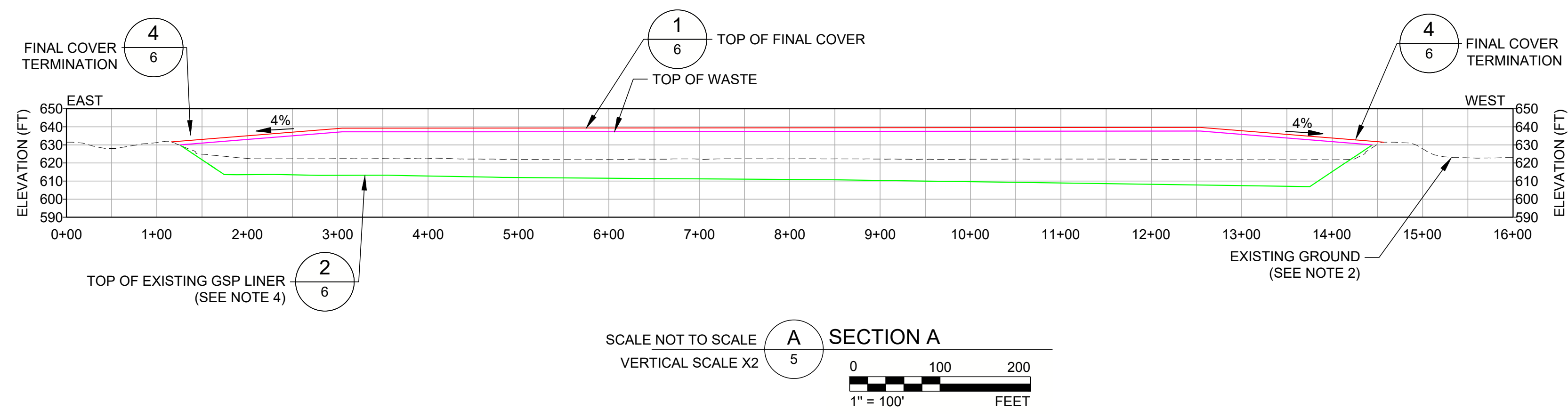
Coffeen Power Station
Closure-by-Removal of GMF Recycle Pond

Item	Quantity	Assumptions
Labor Total Hours	14,118	Per projected total in cost estimate
Duration of Onsite Construction - Days	389	Per Construction Schedule
Average Daily Crew Size	4	10 hour days
Labor Mobilization Miles	108,954	Average of 70 miles round trip per day
Vehicle Miles On-Site	5,992	1 mile round trip from gate to parking 5 miles per day for CQA tech and Construction Supervisor 10% Contingency for Site visitors (client and engineering support)
Equipment Mobilization Miles - Unloaded	16,677	Average of 300 miles one way for equipment hauling Average 1 load of equipment per working week
Equipment Mobilization Miles - Loaded	16,677	Average of 300 miles one way for equipment hauling Average 1 load of equipment per working week
Total Equipment Miles On-Site	17,510	Average of 2 of 4 crew members running equipment Assume 15 miles per piece of equipment (based on 15 minute round trip path across AP1) 10 miles per day used for water truck 5 miles per day for grader
On-Site Haul Truck Miles - Unloaded	2,623	34 CY Haul Truck 4000 ft cycle on-site
On-Site Haul Truck Miles - Loaded	2,623	34 CY Haul Truck 4000 ft cycle on-site
Off-Site Haul Truck Miles - Unloaded	-	16.5 CY Dump Truck 36 mile cycle to off-Site Landfill
Off-Site Haul Truck Miles - Loaded	-	16.5 CY Dump Truck 36 mile cycle to off-Site Landfill
Material Delivery Miles - Unloaded	2,200	45 extra trips for seed, fertilizer, lime, mulch, ECBs, straw wattles, and concrete
Material Delivery Miles - Loaded	2,200	45 extra trips for seed, fertilizer, lime, mulch, ECBs, straw wattles, and concrete

ATTACHMENT 2

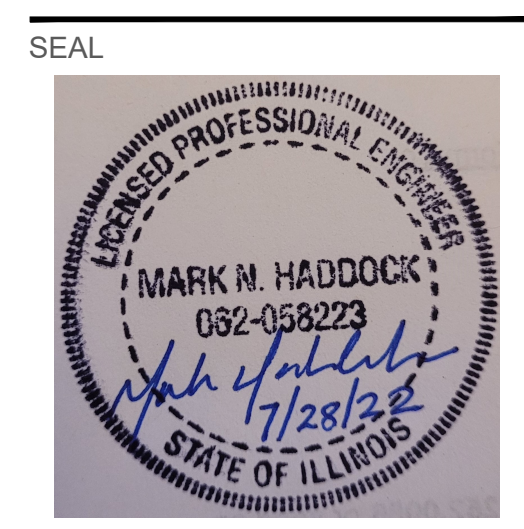
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- NOTE(S)**
1. THE CLOSURE-IN-PLACE CONCEPT FOR THE GYPSUM MANAGEMENT FACILITY (GMF) GYPSUM STACK POND (GSP) INVOLVES REMOVAL OF PONDED WATER, CONSTRUCTION OF A STRUCTURAL BERM (WITH COMPOSITE LINER ON THE UPSTREAM SLOPE), REMOVAL AND RELOCATION OF GYPSUM AND 1 FT (MAX.) OF CLAY LINER SOUTH OF THE BERM TO WITHIN THE CONSOLIDATED FOOTPRINT, PLACEMENT OF SOIL COVER ON GSP FLOOR SOUTH OF THE BERM FOR DRAINAGE, REMOVAL OF PERIMETER EMBANKMENT SOUTH OF RELOCATED WASTE, AND FINAL COVER CONSTRUCTION.
 2. EXISTING CONTOURS ARE A COMPOSITE OF AN AERIAL SURVEY COMPLETED BY DRAGONFLY AEROSOLUTIONS DATED 12/3/2020, TOPOGRAPHIC/BATHYMETRIC SURVEYS COMPLETED BY INGENAE DATED 12/3/2020 & 12/4/2020.
 3. THE GSP EXISTING LINER SYSTEM WILL BE EXTENDED UP THE UPSTREAM SLOPE OF THE BERM.
 4. GMF GSP TOP OF EXISTING LINER GRADES WERE DEVELOPED FROM CONSTRUCTION RECORD DRAWINGS DATED 1/5/2011.

A	2022-07-28	ISSUED FOR PERMIT APPLICATION	DVS	AGD	MWD	MNH
REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED



CLIENT
ILLINOIS POWER RESOURCES GENERATING, LLC
 COFFEEN POWER PLANT

CONSULTANT
wsp GOLDER
 701 EMERSON ROAD, SUITE 250
 CREVE COEUR, MO 63141
 UNITED STATES
 (313) 984 8800

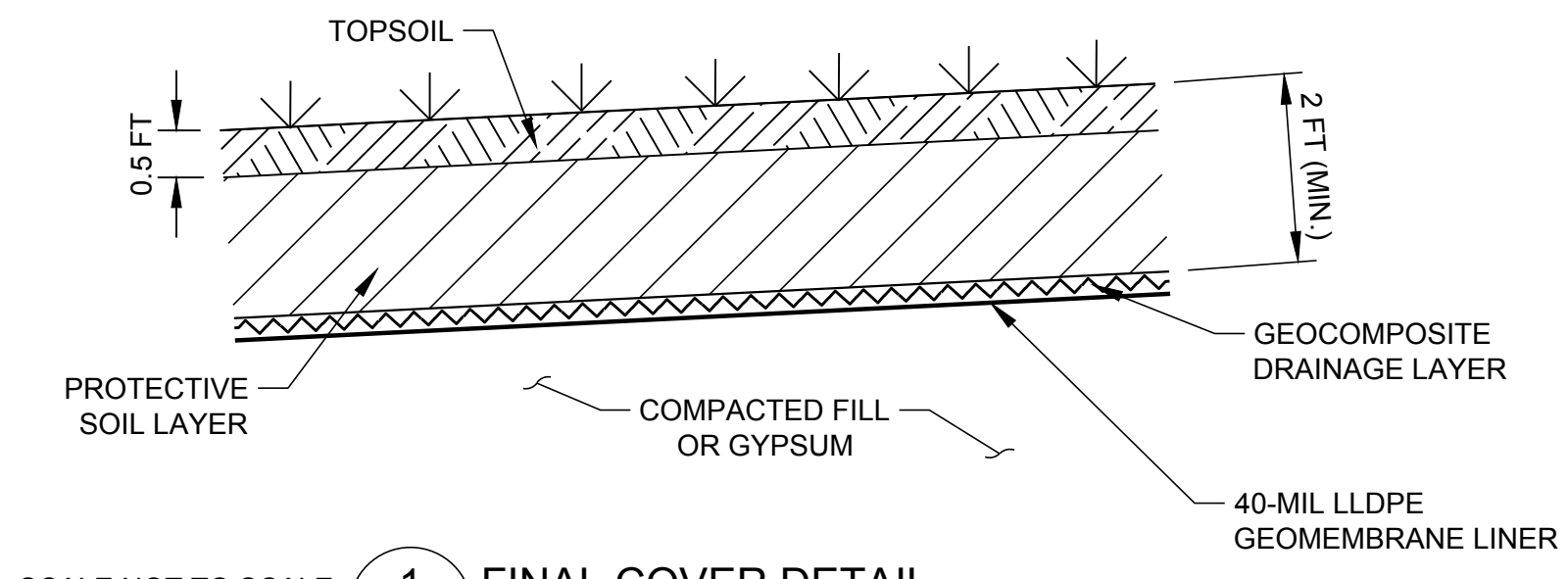
PROJECT
GYPSUM MANAGEMENT FACILITY
 CONSTRUCTION PERMIT APPLICATION

TITLE
CROSS SECTIONS

PROJECT NO. 21465046
 REV. A
 DRAWING 5

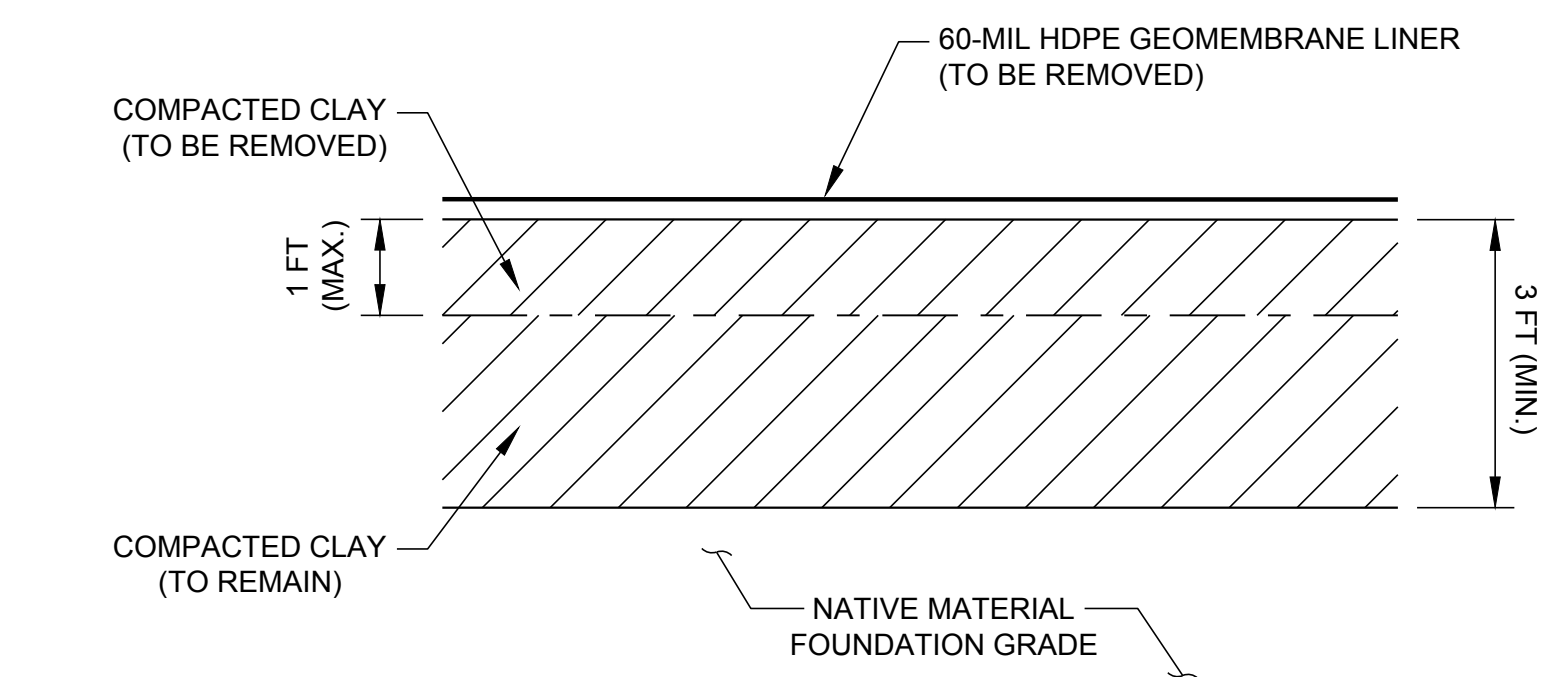
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SCALE NOT TO SCALE **1** FINAL COVER DETAIL
6

NOTE
 THE FINAL PROTECTIVE SOIL LAYER WILL BE COMPOSED OF LOCALLY AVAILABLE SOILS COMPACTED TO BETWEEN 80% AND 95% OF THE STANDARD PROCTOR MAXIMUM DRY DENSITY FOR ESTABLISHMENT OF VEGETATION AND PROTECTION OF THE GEOSYNTHETICS.



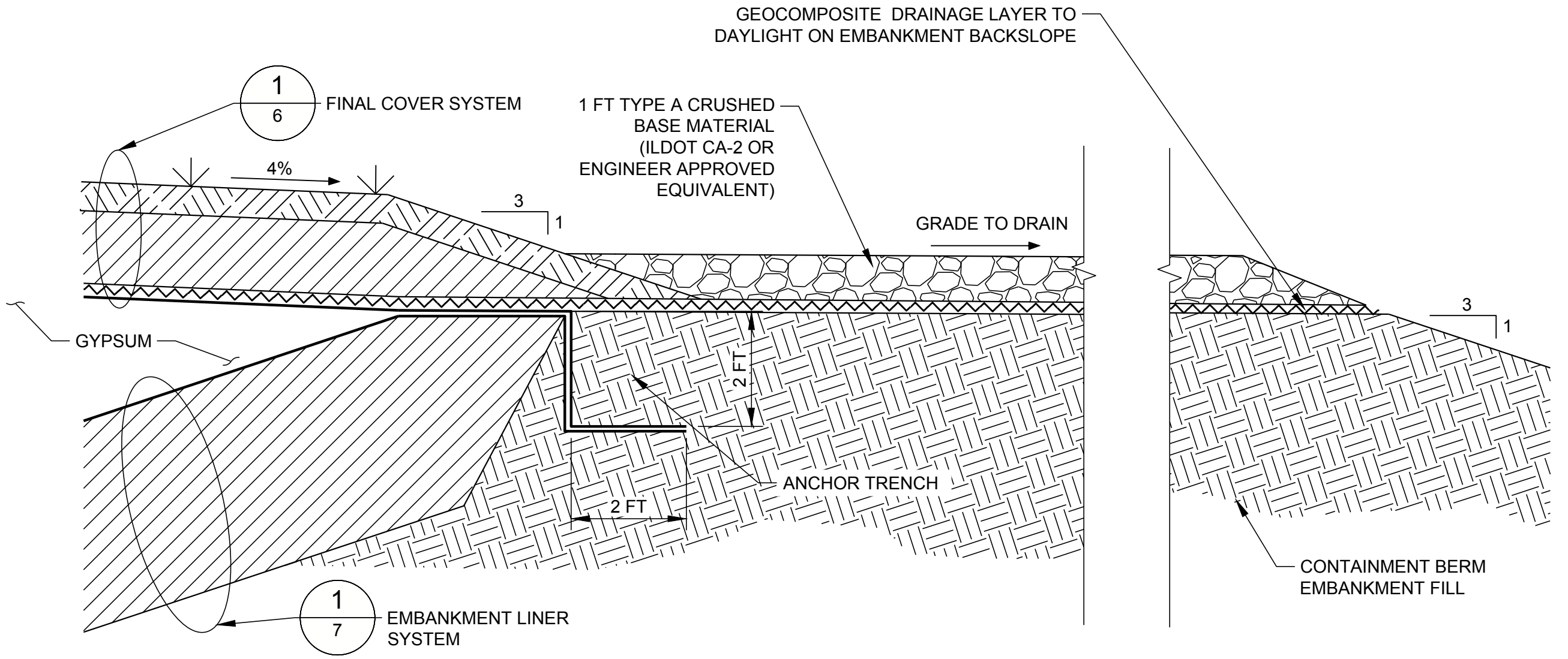
SCALE NOT TO SCALE **2** EXISTING GYPSUM STACK POND LINER SYSTEM DETAIL
6

MATERIAL REMOVAL NOTES (WHERE APPLICABLE)

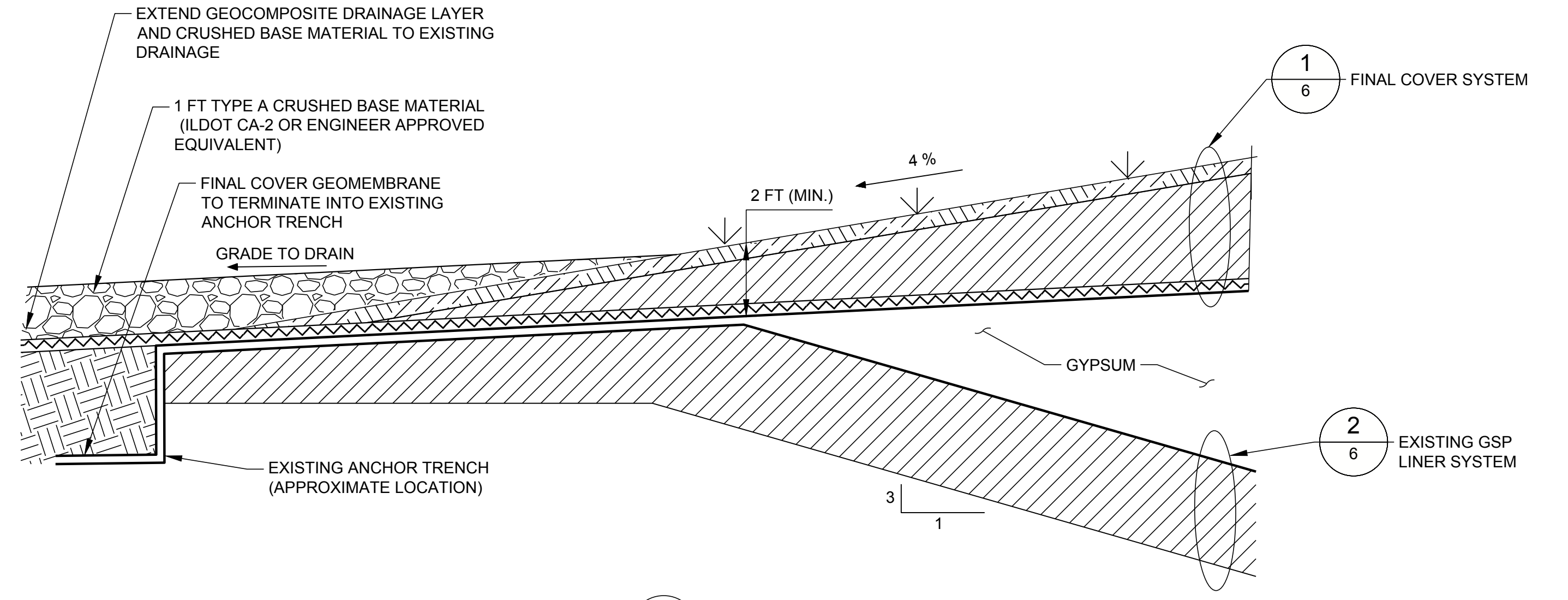
- 60-MIL HDPE GEOMEMBRANE WILL BE REMOVED AND DISPOSED OF IN THE CLOSURE FOOTPRINT OR DISPOSED OFFSITE.
- UP TO 1 FT COMPACTED CLAY LAYER WHERE SIGNS OF CCR ARE OBSERVED WILL BE REMOVED AND RELOCATED TO WITHIN THE CONSOLIDATED FOOTPRINT.

NOTE(S)

- THE CLOSURE-IN-PLACE CONCEPT FOR THE GYPSUM MANAGEMENT FACILITY (GMF) GYPSUM STACK POND (GSP) INVOLVES REMOVAL OF PONDED WATER, CONSTRUCTION OF A STRUCTURAL BERM (WITH COMPOSITE LINER ON THE UPSTREAM SLOPE), REMOVAL AND RELOCATION OF GYPSUM AND 1 FT (MAX.) OF CLAY LINER SOUTH OF THE BERM TO WITHIN THE CONSOLIDATED FOOTPRINT, PLACEMENT OF SOIL COVER ON GSP FLOOR SOUTH OF THE BERM FOR DRAINAGE, REMOVAL OF PERIMETER EMBANKMENT SOUTH OF RELOCATED WASTE, AND FINAL COVER CONSTRUCTION.
- EXISTING CONTOURS ARE A COMPOSITE OF AN AERIAL SURVEY COMPLETED BY DRAGONFLY AEROSOLUTIONS DATED 12/3/2020, TOPOGRAPHIC/BATHYMETRIC SURVEYS COMPLETED BY INGENAE DATED 12/3/2020 & 12/4/2020.
- TOP OF UPPERMOST AQUIFER SURFACE BASED ON GROUNDWATER ELEVATION CONTOURS PROVIDED BY RAMBOLL.
- THE GSP EXISTING LINER SYSTEM WILL BE EXTENDED UP THE UPSTREAM SLOPE OF THE BERM.
- GMF GSP TOP OF EXISTING LINER GRADES WERE DEVELOPED FROM CONSTRUCTION RECORD DRAWINGS DATED 1/5/2011.



SCALE NOT TO SCALE **3** SOUTH FINAL COVER TERMINATION
6



SCALE NOT TO SCALE **4** OUTER PERIMETER FINAL COVER
6

REV.	YYYY-MM-DD	DESCRIPTION	DESIGNED	PREPARED	REVIEWED	APPROVED
A	2022-07-28	ISSUED FOR PERMIT APPLICATION	DVS	AGD	MWD	MNH



CLIENT
ILLINOIS POWER RESOURCES GENERATING, LLC
 COFFEEN POWER PLANT

 CONSULTANT
wsp GOLDER
 PROJECT OFFICE
 701 EMERSON ROAD, SUITE 250
 CREVE COEUR, MO 63141
 UNITED STATES
 (313) 984 8800

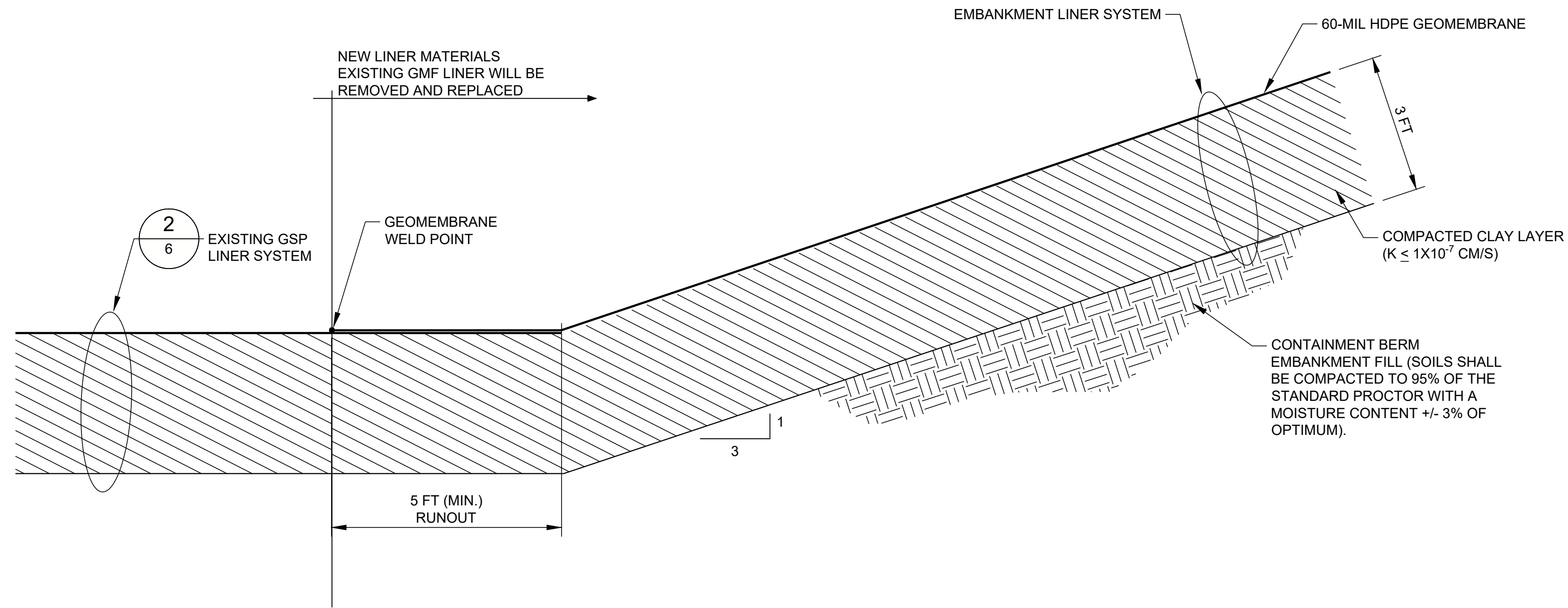
PROJECT
GYPSUM MANAGEMENT FACILITY
CONSTRUCTION PERMIT APPLICATION

 TITLE
DETAILS - 1 OF 2

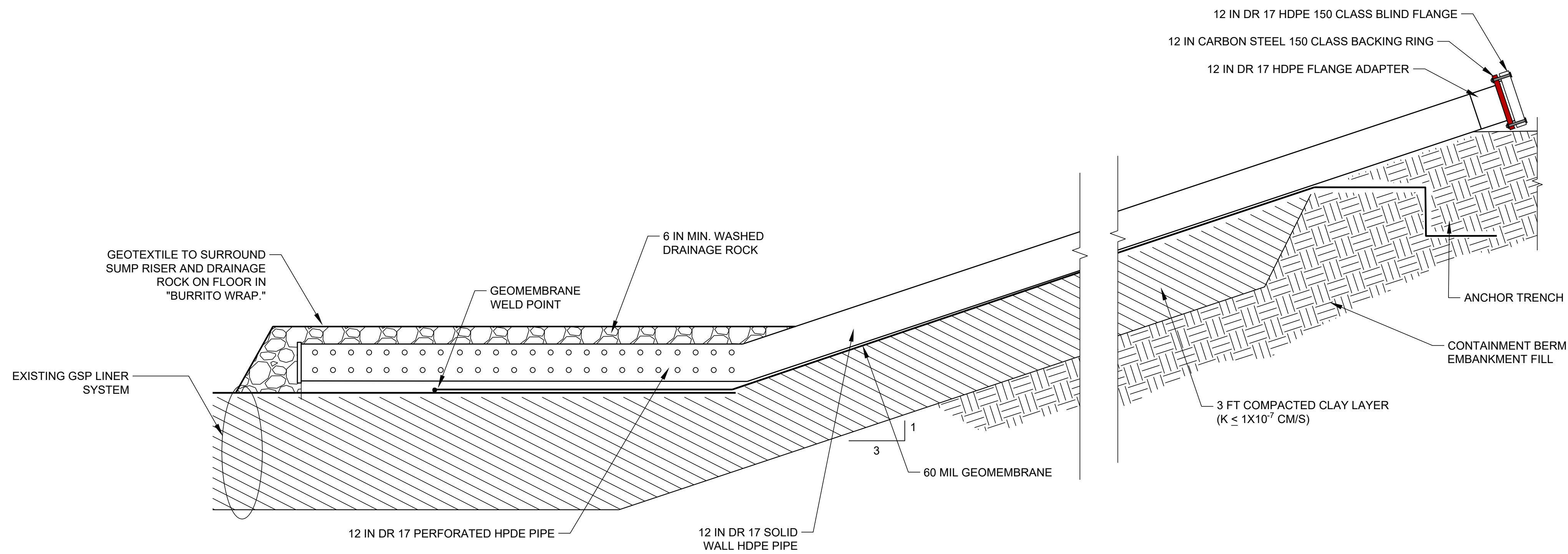
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21465046
 REV. **A**
 DRAWING **6**

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SCALE NOT TO SCALE 1 EMBANKMENT LINER SYSTEM AND TIE-IN TO EXISTING GSP LINER



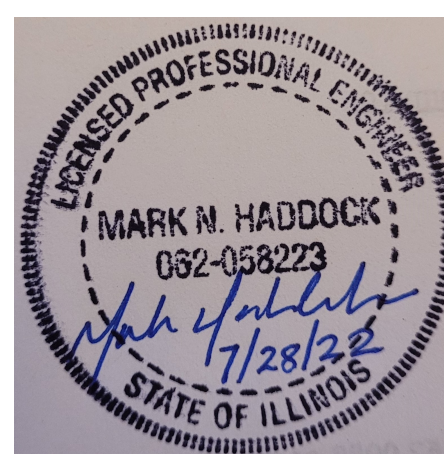
SCALE NOT TO SCALE 2 LEAK DETECTION SYSTEM SUMP AND RISER

A 2022-07-28 ISSUED FOR PERMIT APPLICATION

REV. YYYY-MM-DD DESCRIPTION

DVS AGD MWD MNH
DESIGNED PREPARED REVIEWED APPROVED

SEAL



CLIENT
ILLINOIS POWER RESOURCES GENERATING, LLC
COFFEEN POWER PLANT

CONSULTANT



PROJECT OFFICE
701 EMERSON ROAD, SUITE 250
CREVE COEUR, MO 63141
UNITED STATES
(313) 984 8800

PROJECT
GYPSUM MANAGEMENT FACILITY
CONSTRUCTION PERMIT APPLICATION

TITLE
DETAILS - 2 OF 2

PROJECT NO.
21465046

REV.
A

DRAWING
7

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ATTACHMENT 3

Slope Stability Calculations

CALCULATION

DATE May 12, 2022

Project No. 21465046

PREPARED BY: Elizabeth Hanna

CHECKED BY Michael Dreyer

REVIEWED BY: Jacob Sauer

CLIENT NAME: Illinois Power Resources
Generating, LLC

SLOPE STABILITY ANALYSIS – GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND

1.0 OBJECTIVE

Evaluate slope stability for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) closure design in terms of global stability and veneer stability for the final cover system and containment berm.

2.0 METHODOLOGY

Limit-equilibrium slope stability analyses were performed using Spencer's method of slices (Spencer 1967) in Slide2, a two-dimensional slope stability modeling software platform (Rocscience Inc. 2022). Spencer's method of slices considers both moment and force equilibrium. It is common geotechnical practice to analyze the stability of embankment slopes using limit-equilibrium methods.

2.1 Target Factors of Safety

The following target factors of safety are based on the values presented in Illinois Administrative Code Title 35, Subsection 845.460(a), as pertinent to the GMF GSP following closure:

- Target minimum factor of safety under static long-term conditions = 1.5
- Target minimum factor of safety under seismic loading conditions = 1.0

The locally available soils that will be used for closure construction have relatively high silt and clay contents. Therefore, they are not expected to be susceptible to liquefaction.

3.0 SLOPE STABILITY ANALYSIS

3.1 Geometry

A typical cross-section through the containment berm along the south end of the closure footprint was selected for the slope stability analysis. This is identified as the critical cross-section for slope stability following closure of the GMF GSP, as the GMF GSP is mostly incised around the remainder of the closure footprint perimeter.

The existing liner system will remain in place beneath the closure footprint and consists of (from top to bottom):

- 60-mil textured high-density polyethylene (HDPE) geomembrane

- 3 feet of compacted clay

The containment berm is designed with 3H-to-1V slopes and a crest width of 40 feet. The final cover system will be sloped at 4%. The base of the final cover system is designed to meet the upstream edge of the containment berm crest and terminate with a 3H-to-1V slope to the crest. The final cover system will consist of the following components (from top to bottom):

- 2 feet of protective soil cover, anticipated to consist primarily of locally available low-plasticity silt or clay
- double-sided geocomposite drainage layer
- 40-mil textured LLDPE geomembrane

A liner system will be installed on the upstream containment berm slope and tied into the existing liner system at the toe of the slope. The liner system will be the same as the existing liner system.

Downstream of the containment berm, the closure grades represent soil fill (locally available low-plasticity silt or clay) over the top of the compacted clay layer in the existing liner system, as the geomembrane components of the existing liner system above this layer will be removed during closure of the GMF GSP. Previous stability analyses (AECOM 2016) determined that the GMF GSP is underlain by a native clay layer, a relatively thin layer (approximately 3 feet) of soft native clay, and till.

For simplification of the model geometry, the liner system is represented as a layer having a thickness of 3 feet and the final cover system is represented as a layer having a thickness of 2 feet.

Groundwater levels in the vicinity of the GMF GSP are elevated above the existing liner system. For slope stability analysis, the phreatic surface is modeled along the top of the compacted clay layer in the existing liner system. Within the closure footprint, the liner system will prevent the phreatic surface from rising above this level and the CCR will be dewatered. Downstream of the containment berm, elevated groundwater is expected to present as surface water that will be managed in a stormwater channel, resulting in phreatic levels near the ground surface.

3.2 Approach and Input Parameters

The slope stability analysis uses the following approach and input parameters:

- Circular and non-circular slip surfaces are evaluated. Analysis of non-circular slip surfaces enables evaluation of veneer stability for the final cover system.
- Earthquake (seismic) loading conditions are simulated using a pseudo-static approach. Pseudo-static stability analyses apply a constant horizontal force to the system to represent the forces generated during an earthquake event, with the magnitude of the applied force typically related to the peak ground acceleration (PGA) of a specific earthquake hazard risk. A pseudo-static limit equilibrium analysis was conducted to evaluate the stability of the slope under a seismic load for the earthquake hazard representing a 2% probability of exceedance in 50 years (equaling 0.212g; i.e. a return period of 2475 years) based on the United States Geological Survey (USGS) Hazard Maps. As recommended by Hynes-Griffin and Franklin (1984), a horizontal force of ½ of the maximum PGA (EPA 1995) was used in the analysis (0.106g). In addition, the shear strength properties of the materials were reduced by 20% per the method's requirements.

- Material properties of soils are selected based on previous stability calculations (AECOM 2016). Cohesion is neglected for conservatism.
- For conservatism, undrained strengths are applied for gypsum. A vertical stress ratio (ratio of undrained strength to initial vertical effective stress) of 0.22 is used, consistent with the typical value for normally consolidated fine-grained material (Mesri 1989).
- Strength parameters for the geosynthetic interfaces included in the liner system and final cover system associated with the closed GMF GSP are evaluated from laboratory testing data published by Koerner and Narejo (2005) and summarized in Table 1.

Table 1: Characteristic Geosynthetic Interface Strengths (Koerner and Narejo 2005)

Interface	Peak Friction Angle	Peak Adhesion
Textured geomembrane against cohesive soil	18 degrees	209 psf
Textured geomembrane against granular soil	28 degrees	0 psf
NWNP geotextile against cohesive soil	30 degrees	104 psf
NWNP geotextile against textured geomembrane	25 degrees	167 psf
Geocomposite against cohesive soil	Use NWNP geotextile against cohesive soil	
Geocomposite against textured geomembrane	26 degrees	0 psf

- The lowest geosynthetic interface strength parameters in a given system (liner or final cover) from Table 1 are selected for analysis. Adhesion is conservatively neglected for all geosynthetic interfaces.

A summary of material properties used in the slope stability analysis is presented in Table 2.

Table 2: Material Properties

Material	Unit Weight	Friction Angle	Cohesion or Adhesion	Vertical Stress Ratio
Embankment	135	31	0	N/A
Gypsum	100	N/A	N/A	0.22
Liner System	120	18	0	N/A
Protective Cover	120	26	0	N/A
Native Clay	125	32	0	N/A
Soft Native Clay	125	30	0	N/A
Till	135	40	0	N/A

3.3 Results and Conclusions

The factor of safety for slope stability under static loading conditions is calculated as 1.8, as shown in Figure 1. The critical slip surface is surficial on the downstream face of the containment berm. The factor of safety for global stability under seismic loading conditions is calculated as 1.1, as shown in Figure 2. As with the static analysis, the critical slip surface is surficial on the downstream face of the containment berm.

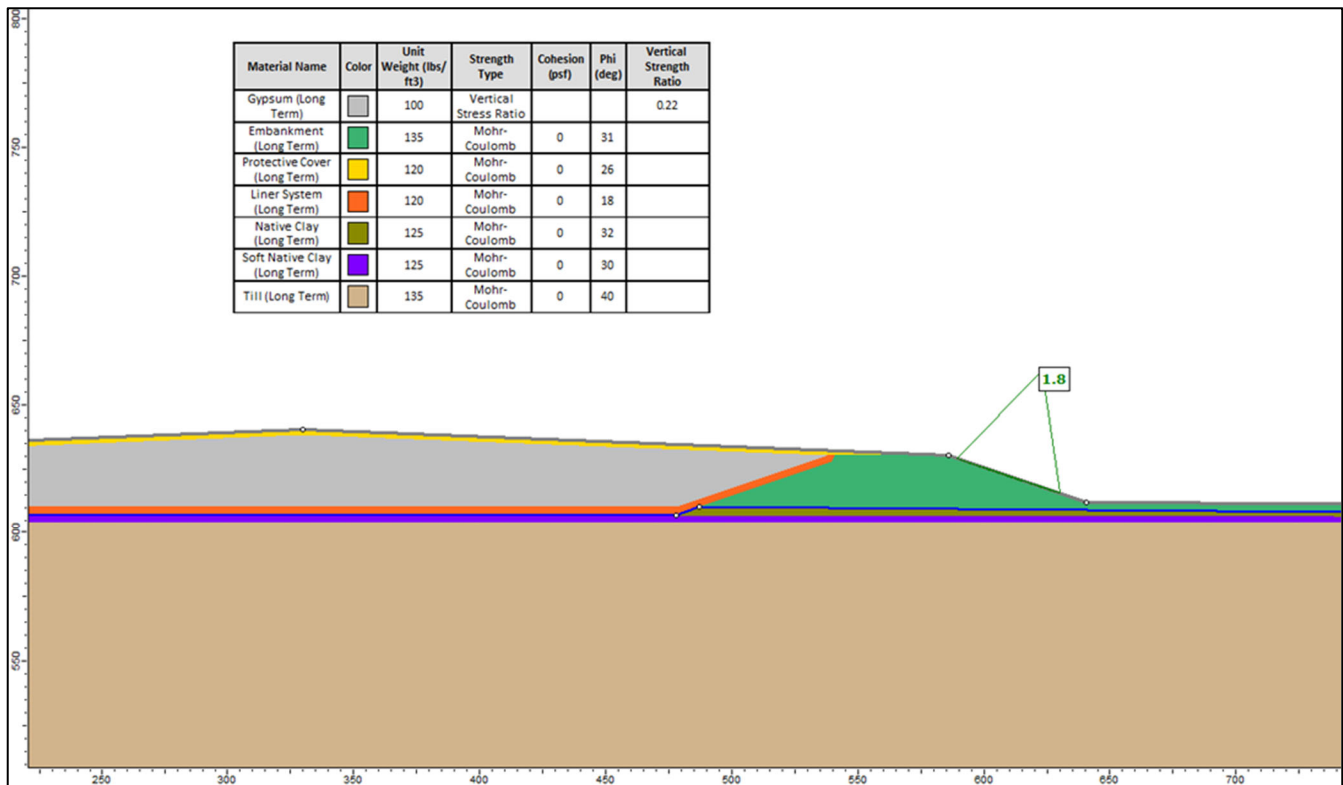


Figure 1: Analysis Result - Static Loading

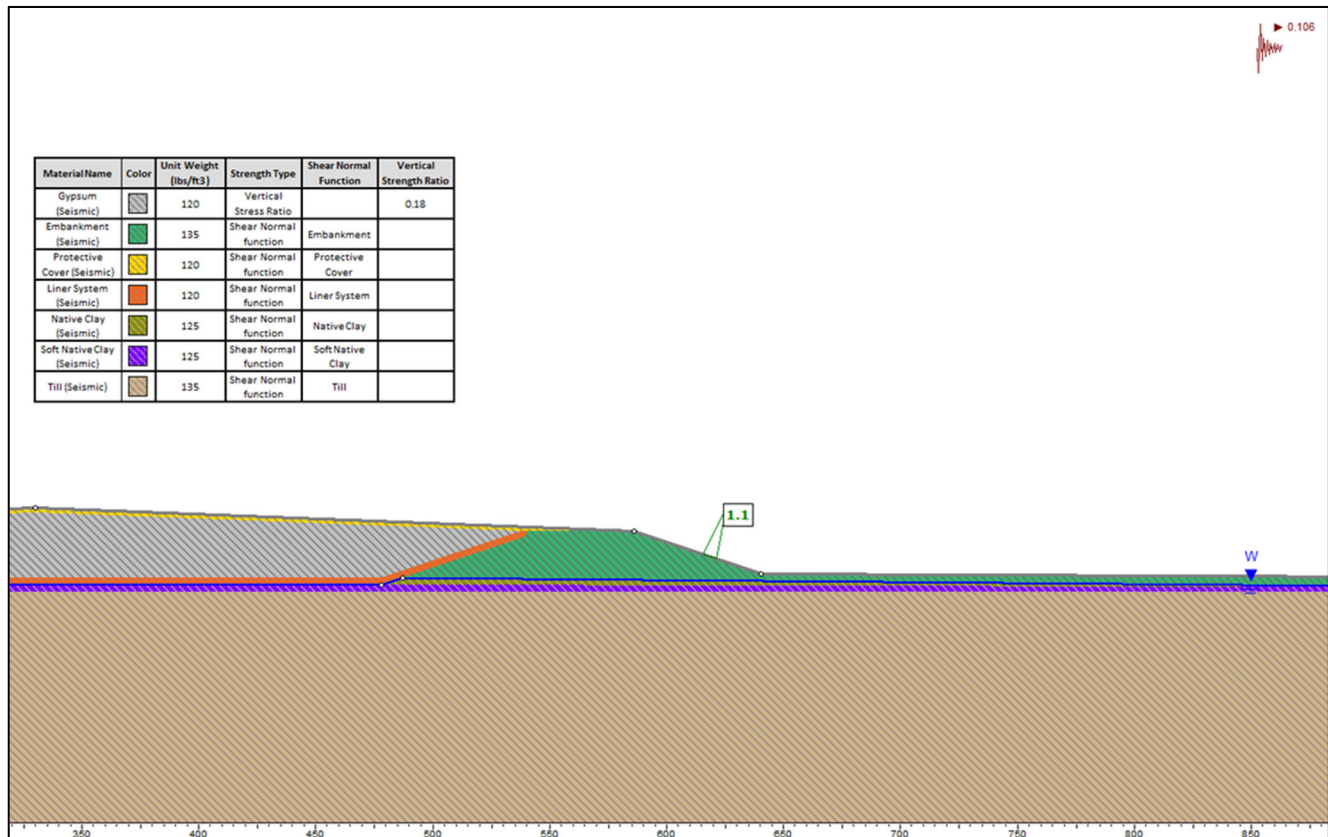


Figure 2: Analysis Result - Seismic Loading

Based on the factors of safety computed using the methods and assumptions described, the closed GMF GSP is expected to remain stable with an acceptable safety margin for global and veneer stability. A factor of safety greater than 1.5 was computed for static loading conditions. A factor of safety greater than 1.0 was computed for seismic loading conditions.

4.0 REFERENCES

AECOM 2016. Geotechnical Report Coffeen Power Station AP1. October 2016.

RocScience Inc. 2022. Slide2 Version 9.022. Build date: April 20, 2022.

Koerner, G.R. and Narejo, D. 2005. Direct Shear Database of Geosynthetic-to-Geosynthetic and Geosynthetic-to-Soil Interfaces. GRI Report #30, 2005.

Mesri, G. 1989. A Reevaluation of $S_{u(mob)} = 0.22\sigma'_p$ Using Laboratory Shear Tests. Canadian Geotechnical Journal, No. 26, pp. 162-164.

Spencer, E. 1967. A Method of Analysis of the Stability of Embankments Assuming Parallel Inter-Slice Forces. Geotechnique, Vol. XVII, No. 1, pp. 11-26.

United States Geological Survey (USGS). 2014. Unified Hazard Tool. Available online: <https://earthquake.usgs.gov/hazards/interactive/index.php> (accessed May 11, 2022).

ATTACHMENT 4

Hydrologic Calculations

CALCULATION

DATE	May 10, 2022	PREPARED BY	Gustavo Guerrero, EIT
PROJECT NO.	21465046	CHECKED BY	DVS
CLIENT NAME	Illinois Power Resource Generating, LLC	REVIEWED BY	MWD

HYDROLOGY CALCULATIONS FOR CLOSURE OF THE GYPSUM MANAGEMENT FACILITY GYPSUM STACK POND AND RECYCLE POND AT THE COFFEEN POWER PLANT

1.0 OBJECTIVE

Evaluate the hydrology (routing of stormwater runoff) after closure of the Coffeen Power Plant Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) and Recycle Pond (RP). These calculations were performed to support the closure plan by determining the minimum channel dimensions.

2.0 METHODOLOGY

The areas contributing to the GMF GSP and GMF RP were delineated in AutoCAD, as shown on Figure 1. The ground conditions were used to estimate a lag time using NRCS methodology (NRCS 1986). The calculations for the hydrologic parameters are included in Tables 1 and 2. The hydrologic parameters were used to model the stormwater runoff reporting to proposed channels to the east of the closed GMF during the 25-year, 24-hour design storm event using HEC-HMS software (USACE 2021). The channels were analyzed using Manning's equation to evaluate the steady-state hydraulics.

3.0 INPUTS AND ASSUMPTIONS

Information and assumptions regarding input parameters used in the analyses include the following:

- A curve number of 58 was used to be consistent with the closed condition of Meadow and hydrologic soil group B (NRCS 1986) based on a review of the Web Soil Survey in the vicinity of the GMF GSP and GMF RP (NRCS 2021).
- The design storm (25-year, 24-hour) depth from NOAA Atlas 14 (NOAA 2006) is 5.33 inches.
- Lag time was estimated using NRCS TR-55 methodology.
- Manning's number used for channel design was 0.030 for capacity and 0.035 for depth assuming a grass-lined channel.

4.0 RESULTS AND CONCLUSIONS

The HEC-HMS model results provide the estimated peak flow from the 25-year, 24-hour design storm to discharge points of interest:

- The peak flow rate at the proposed stormwater channel for the GMF GSP is estimated as 44.0 cubic feet per second (cfs).
- The peak flow rate at the proposed stormwater channel for the GMF RP is estimated as 26.0 cfs.

- The peak flow rate at the proposed stormwater channel north and east of the relocated CCR is estimated as 19.6 cfs.

The output from the HEC-HMS model is shown in Table 3.

The channels were designed with dimensions as indicated in Table 4. Freeboard is shown to be at least 1 foot and at least one-half of the velocity head. The calculations indicate that the channels should function as designed.

5.0 REFERENCES

National Oceanic and Atmospheric Administration (NOAA). 2006. Precipitation-Frequency Atlas of the United States, Volume 2 Version 3.0.

Natural Resources Conservation Service (NRCS). 1986. Urban Hydrology for Small Watersheds. 2nd edition Technical Release 55). June.

Natural Resources Conservation Service (NRCS). 2021. Web Soil Survey. Available online: <http://websoilsurvey.sc.egov.usda.gov/>. Accessed September 22, 2021.

United States Army Corps of Engineers (USACE). 2021. Hydrologic Modeling System (HEC-HMS), Version 4.9.0. Release date: Jan 21, 2022.

TABLES

Table 1: Subbasin Summary Table

**Illinois Power Resource Generating, LLC
Gypsum Management Facility Ponds
Project Number: 21465046**

Date:	5/10/22
By:	GMG
Chkd:	DVS
Apprvd:	MWD

Design Storm 25 -Year Reccurence Interval

Storm Duration (hours)	2-Year Depth (inches)	25 -Year Depth (inches)	Storm Distribution
24	3.14	5.33	II

Subbasin ID	Subbasin Area (ft ²)	Subbasin Area (acres)	Subbasin Area (sq mile)	CN = 58	CN = 99	Composite SCS Curve No.	S = $\frac{1000}{CN} - 10$	Unit Runoff Q (in)	Runoff Volume (ac-ft)	Runoff Volume (ft ³)
				Meadow HSG B (acres)	Open Water or Impervious (acres)					
STK	1,801,416	41.4	0.0646	41.4	0.00	CN = 58	7.24	1.35	4.67	203,356
RCY	1,649,368	37.9	0.0592	37.9	0.00	CN = 58	7.24	1.35	4.27	186,191
CCR	746,014	17.1	0.0268	17.1	0.00	CN = 58	7.24	1.35	1.93	84,215
		0.0	0.0000							
Total:	4,196,798	96.3	0.15						10.88	473,762

**TABLE 2
BASIN TIME OF CONCENTRATION CALCULATIONS**

Table 2: Basin Time of Concentration Calculations
 Illinois Power Resource Generating, LLC
 Gypsum Management Facility Ponds
 Project Number: 21465046

Date:	5/10/22
By:	GMG
Chkd:	DVS
Apprvd:	MWD

Subbasin ID	Subbasin Area (sq mile)	Composite Curve Number	Total Lag (0.67Tc) (min)	Total Travel Time (min)	Flow Segment 1				Flow Segment 2				Flow Segment 3									
					Type of Flow	Length (ft)	Slope (ft/ft)	Roughness Condition ⁽¹⁾	Typical Hydraulic Radius (Channel Only) (ft)	Travel Time (min)	Type of Flow	Length (ft)	Slope (ft/ft)	Roughness Condition ⁽¹⁾	Typical Hydraulic Radius (Channel Only) (ft)	Travel Time (min)	Type of Flow	Length (ft)	Slope (ft/ft)	Roughness Condition ⁽¹⁾	Typical Hydraulic Radius (Channel Only) (ft)	Travel Time (min)
STK	0.0646	58	15.2	25.3	Sheet	100	0.212	G Bermuda Grass	8.6	8.6	Shallow	1230	0.016	U Unpaved	10.2	10.2	Channel	1105	0.005	G Grass-lined	0.89	6.5
RCY	0.0592	58	31.3	52.2	Sheet	100	0.007	G Bermuda Grass	33.8	33.8	Shallow	1765	0.012	U Unpaved	16.7	16.7	Channel	280	0.007	G Grass-lined	0.66	1.7
CCR	0.0268	58	13.4	22.3	Sheet	100	0.205	G Bermuda Grass	0.0	0.0	Channel	2850	0.005	G Grass-lined	0.61	22.3						

Notes:

(1) Refer to Attachment A for Roughness Condition descriptions and Tc Coefficients.

**TABLE 3
FLOW RESULTS FROM HEC-HMS**

Illinois Power Resource Generating, LLC
Gypsum Management Facility Ponds
Project Number: 21465046

Date:	5/10/22
By:	GMG
Chkd:	DVS
Apprvd:	MWD

HEC-HMS Basin Model:	GMF
HEC-HMS Met. Model:	25-yr, 24-hr
HEC-HMS Control Specs:	48-hr, 6-min

Hydrologic Element	Drainage Area (sq mile)	Peak Discharge (cfs)	Time of Peak	Total Volume (ac-ft)
CCR	0.027	19.6	02May2050, 00:06	1.9
CCR-Sink	0.027	19.6	02May2050, 00:06	1.9
Recycle Pond	0.059	26	02May2050, 00:30	4.3
Recycle-Sink	0.059	26	02May2050, 00:30	4.3
Stack	0.065	44	02May2050, 00:12	4.7
Stack-Sink	0.065	44	02May2050, 00:12	4.7

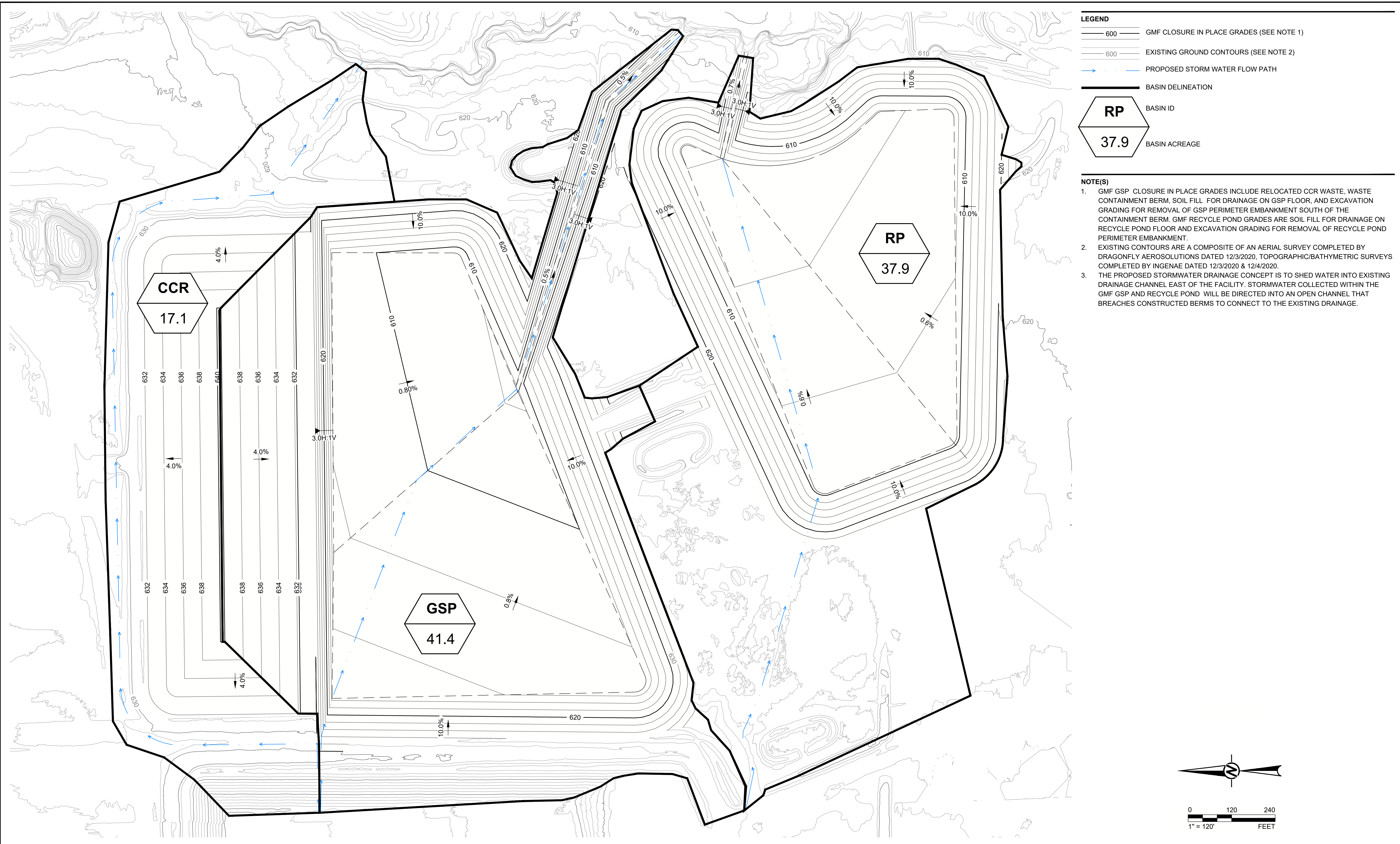
**Table 4
Channel Hydraulic Calculations**

Illinois Power Resource Generating, LLC
Gypsum Management Facility Ponds
PROJECT NO.: 21465046

Date:	5/10/22
By:	GMG
Chkd:	DVS
Apprvd:	MWD

Reach Designation	Q25 from HEC-HMS (cfs)	HEC HMS Element ID for Q	Channel Design Geometry					Channel Roughness Parameters			Hydraulic Calculations						Channel Evaluations			
			Approximate Channel Length (ft)	Bed Slope (ft/ft)	Left Side Slope (H:1V)	Right Side Slope (H:1V)	Bottom Width (ft)	Minimum Channel Depth (ft)	Design Channel Lining	Mannings 'n' for Capacity (Depth Calculation)	Mannings 'n' for Stability (Velocity Calculation)	Maximum Velocity (ft/sec)	Maximum Normal Flow Depth (ft)	Froude Number	Normal Depth Shear Stress (lb/ft ²)	Stream Power (W/m ²)	Top Width of Flow (ft)	Top Width of Channel (ft)	Available Freeboard (ft)	
STK Channel	44.0	Stack-Sink	1105	0.005	3.0	3.0	10	4.0	G Grass-lined	0.035	0.030	3.2	1.15	0.60	0.37	16.92	16.9	34.0	2.8	Suitable
RCY Channel	26.0	Recycle Pond	260	0.007	3.0	3.0	10	5.0	G Grass-lined	0.035	0.030	3.0	0.78	0.67	0.35	15.10	14.7	40.0	4.2	Suitable
CCR Channel	19.6	CCR-Sink	2850	0.005	3.0	3.0	0	3.0	G Grass-lined	0.035	0.030	2.8	1.62	0.56	0.49	19.98	9.7	18.0	1.4	Suitable

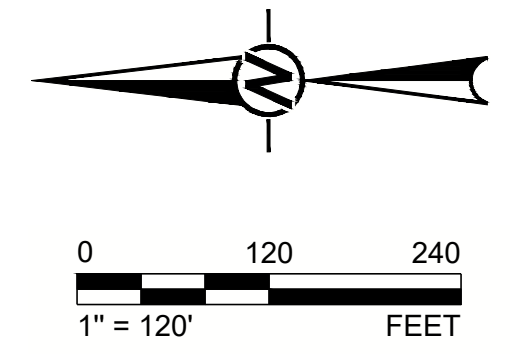
FIGURE



LEGEND

- 600 GMF CLOSURE IN PLACE GRADES (SEE NOTE 1)
- 600 EXISTING GROUND CONTOURS (SEE NOTE 2)
- PROPOSED STORM WATER FLOW PATH
- BASIN DELINEATION
- BASIN ID
37.9 BASIN ACREAGE

- NOTE(S)**
1. GMF GSP CLOSURE IN PLACE GRADES INCLUDE RELOCATED CCR WASTE, WASTE CONTAINMENT BERM, SOIL FILL FOR DRAINAGE ON GSP FLOOR, AND EXCAVATION GRADING FOR REMOVAL OF GSP PERIMETER EMBANKMENT SOUTH OF THE CONTAINMENT BERM. GMF RECYCLE POND GRADES ARE SOIL FILL FOR DRAINAGE ON RECYCLE POND FLOOR AND EXCAVATION GRADING FOR REMOVAL OF RECYCLE POND PERIMETER EMBANKMENT.
 2. EXISTING CONTOURS ARE A COMPOSITE OF AN AERIAL SURVEY COMPLETED BY DRAGONFLY AEROSOLUTIONS DATED 12/3/2020, TOPOGRAPHIC/BATHYMETRIC SURVEYS COMPLETED BY INGENAE DATED 12/3/2020 & 12/4/2020.
 3. THE PROPOSED STORMWATER DRAINAGE CONCEPT IS TO SHED WATER INTO EXISTING DRAINAGE CHANNEL EAST OF THE FACILITY. STORMWATER COLLECTED WITHIN THE GMF GSP AND RECYCLE POND WILL BE DIRECTED INTO AN OPEN CHANNEL THAT BREACHES CONSTRUCTED BERMS TO CONNECT TO THE EXISTING DRAINAGE.



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ATTACHMENT 5



**Final Protective Layer
Demonstration**

Technical Memorandum

Date: July 25, 2022

To: Victor Modeer, P.E., DGE, Vistra on behalf of Illinois Power Resources
Generating, LLC

Copies to: Phil Morris, Rhys Fuller, Vistra on behalf of Illinois Power Resources
Generating, LLC

From: John Seymour, P.E., Geosyntec Consultants (Geosyntec) 
Lucas Carr, P.E., Geosyntec 

Subject: Proposed Alternative Final Protective Layer Equivalency Demonstration
Gypsum Management Facility, Gypsum Stack Pond, Coffeen Power Plant
Coffeen, Illinois
Geosyntec Project: GLP8025

PROPOSAL

An alternative final protective layer is proposed by Illinois Power Resources Generating, LLC (IPRG) for the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP) surface impoundment that will be closed-in-place at the Coffeen Power Plant (CPP). The closure will be in accordance with Illinois Administrative Code (IAC) Part 845 Rule [1] (Part 845). Overall, the proposal will meet the requirements of Section 845.750 c) 2).

This Technical Memorandum presents a demonstration that a 2-foot-thick alternative final protective layer consisting of an 18-inch-thick soil layer and a 6-inch layer of topsoil provide equivalent or superior performance to the default protective layer set forth in Section 845.750 c) 2). The alternative final protective layer works in combination with an underlying low permeability (geomembrane) layer in place of the default three-foot thick, low permeability compacted earth layer required by Section 845.750 c) 1) A). In addition, a geocomposite drainage layer consisting of a geogrid web with geotextiles modeled on top and bottom of the geogrid ("drainage layer") will be placed on top of the geomembrane prior to installation of the final protective layer. The combination of the above materials comprises the final "alternative final cover system".

A discussion of how the closure, including the proposed alternative final cover system discussed herein, meets the performance standards is contained in the Closure Plan [2], which includes the Closure Alternatives Assessment required by Section 845.710.

REQUIREMENTS OF SECTION 845

Section 845.750 provides requirements for both the final protective layer and underlying low permeability layer. They work in tandem to provide protection of groundwater and surface exposure conditions. A principal intention of the low permeability layer is to reduce the infiltration of liquid through the final cover system and into the CCR waste mass during post-closure conditions, in accordance with Section 845.720 (a), which states in part:

The owner or operator of a CCR surface impoundment must ensure that, at a minimum, the CCR surface impoundment is closed in a manner that will:

- 1) Control, minimize or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate or contaminated run-off to the ground or surface waters or to the atmosphere;*

Specific default requirements for the final cover system are included in Section 845.750(c), which requires the final cover system to have either: 1) a three-foot thick soil low permeability compacted earth layer overlain by a three-foot-thick final protective layer (final protective layer), or 2) a geomembrane low permeability layer with a three-foot-thick final protective layer.

The specific Section 845.750 (c) (2) design requirements for the final protective layer are as follows (emphasis added):

*Standards for the Final Protective Layer: The final protective layer must meet the following requirements, **unless the owner or operator demonstrates that another final protective layer construction technique or material provides equivalent or superior performance to the requirements of this subsection (c)(2) and is approved by the Agency.***

Therefore, Section 845.750 (c) (2) specifically allows the use of an alternate final protective layer as long as it provides an equivalent or superior performance to the default standards set forth in Section 845.750(c)(2), which are as follows:

- A) Cover the entire low permeability layer;*
- B) Be at least three feet thick, be sufficient to protect the low permeability layer from freezing, and minimize root penetration of the low permeability layer;*

- C) Consist of soil material capable of supporting vegetation;*
- D) Be placed as soon as possible after placement of the low permeability layer; and*
- E) Be covered with vegetation to minimize wind and water erosion.*

The alternate design is only requesting an alternate to Section 845.740(c)(2)(B) related to the thickness of the of the final protective layer.

PROPOSED FINAL COVER SYSTEM SUMMARY

The proposed final cover systems will include:

- A low permeability layer consisting of a linear low-density polyethylene (LLDPE) geomembrane that is at least 40-mil in thickness, placed on a smooth CCR subgrade;
- A geocomposite drainage layer; and
- A final protective layer consisting of 18 inches of protective cover soil with a 6-inch layer of topsoil capable of supporting vegetation.

The final protective layer will meet all Section 845.750(c)(2) criteria, will not need any supplemental engineering measures, and will be designed by a qualified professional engineer licensed in Illinois.

The concepts of the alternative cover system are illustrated on **Figure 1**.



Figure 1: Proposed Alternative Final Cover System

DEMONSTRATION

The proposed alternate final protective layer will address the five requirements of Section 845.750 (c)(2)(A) to (E), as described in this section.

Section 845.750(c)(2)(A) Cover the entire low permeability layer

The final protective layer will horizontally cover the entire low-permeability layer, as indicated in the drawings in Attachment 2 of the Closure Plan [2].

Therefore, the use of the two-foot-thick final protective layer will meet the minimum requirements of Section 845 750(c)(2)(A) because it will completely cover the low-permeability layer.

Section 845.750(c)(2)(B) Be sufficient to protect the low permeability layer from freezing, and minimize root penetration of the low permeability layer

The existing Part 845, which has the same requirements as Part 814 (closure rule for landfills), requires a three-foot-thick final protective layer to protect the underlying low permeability layer from freeze-thaw effects and root penetration. However, when a geomembrane is used as the low permeability layer it does not need these protections since it is not subject to the same impacts (i.e., causing an increase in hydraulic conductivity) as a compacted earth layer as discussed in more detail below.

A geomembrane low permeability layer will be used for the CPP GSP. Geomembranes have the following characteristics:

- Geomembranes do not have pores that can contain water and are therefore not susceptible to freeze-thaw damage that may reduce their performance as a low permeability layer and/or lead to degradation of the geomembrane.
 - In fact, geomembrane panel strength and stiffness both increase with decreasing temperatures ([3], [4]). In 1996, the United States Bureau of Reclamation [5] (USBR) performed testing of both geomembrane panels and seams subjected to up to 500 freeze-thaw cycles, in both constrained and unconstrained conditions, with temperature cycles as severe as +30° C to -20° C.
 - The testing showed no changes in the strength of the geomembrane panels or seams. The USBR concluded that “...there is simply “no change” in tensile behavior of geomembrane sheets or their seams after freeze-thaw cycling”.
 - In 2013, the Geosynthetic Institute, upon reviewing the results of the USBR and

other studies, concluded that *“the essential question often raised in this regard, i.e., “will freeze-thaw conditions affect geomembrane sheets or their seam behavior,” is answered with a resounding “NO”*” [6].

- Geomembranes are not susceptible to grass plant root penetration because the geomembranes do not provide organic nutrients to plant roots and do not have pores or other areas where roots can enter the geomembrane.
 - Consequently, geomembranes are not a hospitable material that would either encourage root penetration or allow root penetration. Additionally, the geomembrane will be covered with a geocomposite drainage layer with a geotextile filter on top, which will provide an additional barrier to root penetration.

U.S. EPA research [7] states that *“...a typical minimum thickness of the cover soil is 0.45 to 0.6 m...” (18 to 24 inches) thick “... for cover systems with hydraulic barriers”* (low permeability layer). This is particularly appropriate when using a geomembrane low permeability which is not susceptible to any impact from freezing. U.S. EPA research also states that cover thickness design for root penetration into the low permeability layer is only a concern for compacted clay layers or geosynthetic clay barriers. This is when using an appropriate design of cover vegetation.

Therefore, the use of the two-foot-thick final protective layer will provide equivalent or superior performance to the requirements of Section 845.750 (c) (2) (B) when coupled with a geotextile cushion and a geomembrane low permeability layer, as geomembranes are not susceptible to freeze-thaw damage or root penetration as compared to a low permeability compacted earth layer.

Section 845.750(c)(2)(C) Consist of soil material capable of supporting vegetation.

The uppermost six inches of the final protective layer will consist of topsoil that is capable of supporting vegetation, which is the same requirement as the default (three-foot thick) final protective layer. This is also consistent with the Federal CCR Rule, which requires a six-inch-thick “erosion” (topsoil) layer. Research [7] and Geosyntec’s experience indicate topsoil layers are designed to have shallow-rooted grasses and most shallow-rooted grasses do not typically penetrate more than six inches into the subsurface. Shallow-rooted grasses will be specified based on recommendations from specialists at nurseries in the location of CPP and Illinois Department of Transportation guidelines. The topsoil layer will be fertilized and/or amended, as necessary, on a site-specific basis based on agronomical soil testing, to provide a growing medium for the vegetation that provides the required levels of nutrients and water storage during drought conditions.

Grass species will also be selected on a site-specific basis to minimize long-term vegetation maintenance, based on the climatic conditions at each site and the soil types. Vegetation will be established by applying seed and mulch and watering to establish the vegetation. Temporary erosion control measures will also be used during vegetation establishment to protect the topsoil layer from erosion. These measures may include erosion control blankets (ECBs), silt fences, hydroseeding, and/or other methods. The Post-Closure Care Plan includes the commitment to maintain the vegetation of the surface for the closed CPP GSP within the Construction Permit Application [8].

The 18-inches of the protective layer below the topsoil will consist of a soil type suitable for retaining moisture to provide additional support for vegetation during times of drought, and to support any grass species with roots that exceed six inches. Such soil types may include sandy clay loam, silty loam, silts, silty clays, lean clays, sandy clays, and/or sandy silts.

Therefore, the use of the two-foot-thick protective layer will meet the requirements of Section 845.750(c)(2)(C), as the final protective layer will utilize soil capable of supporting vegetation.

Section 845.750(c)(2)(D) Be placed as soon as possible after placement of the low permeability layer

The CPP GSP Closure Plan (Section 4.7.2 [2]) states that the geotextile and cover soil "...will be placed as soon as possible after placement of the low-permeability layer."

The use of a two-foot-thick protective layer will allow the final protective layer to be placed on top of the low permeability layer and vegetation to be established on top of the final protective layer sooner than if a three-foot thick final protective layer is used. This is due to the 33% reduction in earthwork volumes associated with the thinner 2-ft-thick final protective layer.

Therefore, the use of the two-foot-thick final protective layer will exceed the minimum requirements of Section 845.750(c)(2)(D), by allowing the protective layer to be installed sooner than when using a three-foot-thick protective layer.

Section 845.750(c)(2)(E) Be covered with vegetation to minimize wind and water erosion.

The final protective layer will be covered with vegetation to limit wind and water erosion, as noted in the discussion regarding Section 4.7.2 within the Closure Plan [2]. Additionally, the following design and engineering features, construction techniques, and maintenance procedures will be used to reduce the potential for wind and water erosion under both long-term conditions and during vegetation establishment.

- Design and Engineering Features
 - Final cover system slopes will be installed at relatively gentle grades (e.g., typically 4%). The use of gentle grades will reduce water runoff velocities and therefore reduce the potential for water erosion of the final cover soils.
 - Stormwater will be managed by routing it off of the final cover system over relatively short distances (e.g., approximately 220 ft or less), thereby minimizing overland flow distances and limiting stormwater velocities and erosion. Information on the stormwater management system is provided in the drawings within the Closure Plan [2].
- Construction Techniques
 - The final protective layer is typically the most susceptible to wind and water erosion in the period between the placement of the protective layer and the establishment of vegetation. To reduce the potential for both wind and water erosion during this time, the following approaches will be utilized:
 - Temporary erosion and sediment controls (ESCs) will be installed to reduce the potential for erosion, such as erosion control blankets (ECBs), silt socks (e.g., straw wattles), silt fences, and other methods. These ESCs will be regularly inspected and maintained until vegetation is established.
 - The entire surface of the final protective layer will be stabilized during seeding and until vegetation is established. Coverings may consist of straw mulch, hydroseeding binder, ECBs, or engineering growing media.
 - The final protective layer will be regularly inspected and maintained during vegetation establishment. Any areas that become eroded by wind and water will be repaired until vegetation is established to a suitable level over the surface of the final cover.
- Maintenance Procedures
 - During the post-closure care period, vegetation established on the final protective cover layer will be regularly maintained using a written and IEPA-approved maintenance program. The program will consist of regular mowing and inspections. Any bare areas or areas of erosion will be repaired by seeding and stabilizing the area, and observing the area until vegetation becomes re-established.
 - The final cover slopes will be relatively gentle at 4%; these slopes experience less

erosion in general, especially less than typical landfill covers sloped at predominately 25 to 33%. Typically, after three to five years, it is Geosyntec's experience that the cover vegetation becomes fully stabilized and experiences less erosion.

In conclusion, the use of the two-foot-thick final protective layer will exceed the minimum requirements of Section 845.750 c) 2) E), using a robust program to support the establishment of protective vegetation, prevent and address any erosion that may occur during vegetation establishment, and monitor and maintain the vegetation during post-closure conditions.

ADDITIONAL CONSIDERATIONS

Infiltration Analysis

The use of the proposed two-foot-thick final protective layer, when coupled with a geomembrane low permeability layer, will also meet the criteria contained within Section 845.750 (a) (1). Section 845.750 (a) (1) provides the following requirement:

Section 845.750(a)(1) Control, minimize or eliminate, to the maximum extent feasible, post-closure infiltration of liquids into the waste and releases of CCR, leachate, or contaminated run-off to the ground or surface waters or to the atmosphere;

Section 845.750(a)(1) is an important overall measure of the effectiveness of the final cover system because it requires control of post-closure infiltration of liquids through the final cover and into the waste and releases of CCR.

An infiltration analysis was performed to by Ramboll, within the CPP GSP Construction Permit Application [8], to estimate post-closure liquid infiltration rates through both the default and the proposed alternate final cover systems at the CPP GSP. The infiltration analysis used the Hydrologic Evaluation of Landfill Performance (HELP) software promulgated by the USEPA [9]. The HELP model estimates the infiltration rates from the top of the cover, through the final protective layer and through the low permeability layer (either a geomembrane or the three-foot thick compacted earth layer). The results are included in **Appendix A**. The resulting estimated infiltration rates are provided in **Table 1**.

Table 1 – CPP GSP Final Cover Systems for Infiltration Analysis

Description	Low Permeability Layer¹	Final Protective Layer	Infiltration Rate²
Proposed Alternative Final Cover System	40-mil Linear Low-Density Polyethylene (LLDPE) Geomembrane	2 ft of cover material, including, from bottom to top, a geocomposite drainage layer, 1.5 ft of silty clay and 0.5 ft of silty clay loam	0.0026 in/yr
Default Cover with Geomembrane Barrier	40-mil LLDPE Geomembrane	3 ft of cover material, including, from bottom to top, a geocomposite drainage layer, 2.5 ft of silty clay and 0.5 ft of silty clay loam	0.002 in/yr
Default Cover with Compacted Earth Layer	3-ft thick compacted earth layer (1×10^{-7} cm/sec)	3 ft of cover material, including, from bottom to top, 2.5 ft of silty clay and 0.5 ft of silty clay loam	1.98 in/yr

The CPP GSP analysis indicated that the performance of the proposed alternative final cover system with a geomembrane and a two-foot-thick final protective cover exceeds the performance offered by the default final cover system utilizing a geomembrane with the default three-foot-thick protective layer and cushion layer, with the infiltration rate reduced by a factor of 79.

Furthermore, the proposed alternate final cover system performance is equivalent to the performance offered by the default final cover system utilizing a geomembrane with the default three-foot-thick protective layer and drainage layer. This is because the infiltration rates are nearly identical.

¹ All HELP run versions used a pinhole density of 1 hole per acre, installation defects of 1 hole/acre, and construction quality as “good”.

² Infiltration is out the bottom of the low permeability layer.

Environmental and Societal Benefits

The use of the proposed two-foot-thick final protective layer will provide the following additional environmental and societal benefits, relative to the default three-foot-thick final protective layer:

- The final cover system earthwork quantities will be reduced by 33%. This will result in a corresponding 33% reduction in the amount of onsite soil fill that needs to be excavated, hauled to the construction location, and placed. This provides multiple benefits, such as:
 - Reduced disruption to onsite areas caused by the excavation of fill materials and corresponding disturbance to the natural environment.
 - Reduced haul truck traffic on site access roadways, thereby reducing traffic impacts, roadway damage, air pollution, and carbon emissions.
 - Reduced earthwork effort during installation of the final cover system, thereby reducing air pollution and carbon emissions.
- Construction of the alternate final cover system can be completed faster than the default final cover, providing multiple benefits, such as:
 - Initiation of the reduction of infiltration at a sooner date than with the default final cover system.
 - Ceasing construction-related impacts to offsite residents (e.g., air pollution, carbon emissions) at a sooner date than otherwise possible.

SUMMARY

The proposed alternate final protective layer will:

- Provide equivalent or superior performance to the requirements of Section 845.750 (c)(2).
- Have a drainage layer, which is not required by Section 845.750, over the geomembrane that adds both lateral drainage to reduce the amount of water available for infiltration through the geomembrane and physical protection for the geomembrane.
- Have a lower infiltration rate than the infiltration through the default soil final cover system.
- Meet or exceed the same criteria for long term performance and all other requirements of Section 845.750(c)(2).

Alternate Protective Layer Proposal

Coffeen Power Plant, Gypsum Management Facility Gypsum Stack Pond

July 25, 2022

Page 11

- Provide other benefits by reducing the amount of final cover earthwork by 33% for the CPP GSP.

REFERENCES

- [1] Illinois Environmental Protection Agency, "35 Ill. Adm. Code Part 845, Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments," Springfield, IL, 2021.
- [2] Golder Associates USA Inc., "Final Closure Plan for the Gypsum Management Facility Gypsum Stack Pond, Coffeen Power Plant," Creve Coeur, MO, 2022.
- [3] A. L. Rollin, J. Lafleur, M. Marcotte, O. Dascal and Z. Akber, "Selection Criteria for the Use of Geomembranes in Dams and Dykes in Northern Climate," in *Proceedings of the International Conference on Geomembranes*, Denver, Colorado, 1945.
- [4] D. E. Thorton and P. Blackall, "Report EPA-3-76-13: Field Evaluation of Plastic Film Liners for Petroleum Storage Areas in the Mackenzie Delta," Canadian Environmental Protection Service, 1976.
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- [6] Y. G. Hsuan, R. M. Koerner and A. I. Comer, "GSI White Paper #28: Cold Temperature and Freeze-Thaw Cycling Behavior of Geomembranes and their Seams," Geosynthetic Institute, Folsom, Pennsylvania, 2013.
- [7] United States Environmental Protection Agency, "(Draft) Technical Guidance For RCRA/CERCLA Final Covers," Office of Solid Waste and Emergency Response, Washington D.C., 2004.
- [8] Golder Associates USA, Inc., "Construction Permit Applicaiton, Coffeen Power Plant, Gypsum Management Facility Gypsum Stack Pond," Creve Coeur, Missouri, 2022.
- [9] T. Tolaymat and M. Krause, "Hydrologic Evaluation of Landfill Performance: HELP 4.0 User Manual," United States Environmental Protection Agency, Washington, DC, 2020.

APPENDIX A: HELP MODEL OUTPUT

A-1: CPP GSP- 2-FT FINAL PROTECTIVE COVER SOIL

A-2: CPP GSP-3-FT FINAL PROTECTIVE COVER SOIL

A-3: CPP GSP-3-FT COMPACTED EARTH LAYER, 3-FT FINAL PROTECTIVE COVER SOIL

APPENDIX A-1

CPP GSP- 2-FT FINAL PROTECTIVE COVER SOIL

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.04 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 5

Type 1 - Vertical Percolation Layer (Waste)

Gypsum waste material (Sandy Loam)

Material Texture Number 83

Thickness	=	144 inches
Porosity	=	0.437 vol/vol
Field Capacity	=	0.105 vol/vol
Wilting Point	=	0.047 vol/vol
Initial Soil Water Content	=	0.105 vol/vol
Effective Sat. Hyd. Conductivity	=	6.70E-04 cm/sec

Layer 6

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.06 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 7

Type 3 - Barrier Soil Liner

C (Moderate)

Material Texture Number 29

Thickness	=	36 inches
Porosity	=	0.451 vol/vol
Field Capacity	=	0.419 vol/vol
Wilting Point	=	0.332 vol/vol
Initial Soil Water Content	=	0.451 vol/vol
Effective Sat. Hyd. Conductivity	=	6.80E-07 cm/sec

Note: Initial moisture content of the layers and snow water were computed as nearly steady-state values by HELP.

General Design and Evaporative Zone Data

SCS Runoff Curve Number	=	87.5
Fraction of Area Allowing Runoff	=	100 %
Area projected on a horizontal plane	=	12.4 acres
Evaporative Zone Depth	=	18 inches
Initial Water in Evaporative Zone	=	5.708 inches
Upper Limit of Evaporative Storage	=	8.574 inches
Lower Limit of Evaporative Storage	=	4.272 inches
Initial Snow Water	=	0.088112 inches
Initial Water in Layer Materials	=	39.293 inches
Total Initial Water	=	39.381 inches
Total Subsurface Inflow	=	0 inches/year

 Note: SCS Runoff Curve Number was calculated by HELP.

Evapotranspiration and Weather Data

Station Latitude	=	39.07 Degrees
Maximum Leaf Area Index	=	4.5
Start of Growing Season (Julian Date)	=	97 days
End of Growing Season (Julian Date)	=	302 days
Average Wind Speed	=	8 mph
Average 1st Quarter Relative Humidity	=	72 %
Average 2nd Quarter Relative Humidity	=	64 %
Average 3rd Quarter Relative Humidity	=	71 %
Average 4th Quarter Relative Humidity	=	72 %

 Note: Evapotranspiration data was obtained for Coffeen, Illinois

Normal Mean Monthly Precipitation (inches)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
2.353618	2.511085	2.81508	3.241374	3.956664	4.312863
4.375035	2.656228	3.284204	3.675466	3.677412	3.106835

 Note: Precipitation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Normal Mean Monthly Temperature (Degrees Fahrenheit)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
37.3	37.1	50	61.9	69.7	80.6
84.2	81	72.2	62.4	48.1	38

363	*	0.00	0.000	0.002	0.2669	0.0000	0.0000	0.00E+00	0.2185	0.0000	1.02E-06
364		0.09	0.000	0.047	0.2691	0.0000	0.0000	0.00E+00	0.2185	0.0000	1.02E-06
365		0.00	0.000	0.037	0.2670	0.0000	0.0000	0.00E+00	0.2185	0.0000	1.02E-06

* = Frozen (air or soil)

Annual Totals for Year 30			
	inches	cubic feet	percent
Precipitation	33.77	1,519,877.6	100.00
Runoff	2.595	116,818.2	7.69
Evapotranspiration	30.284	1,363,135.4	89.69
Drainage Collected from Layer 3	3.4145	153,694.9	10.11
Percolation/Leakage through Layer 4	0.001014	45.7	0.00
Average Head on Top of Layer 4	0.0027	---	---
Percolation/Leakage through Layer 7	0.000368	16.6	0.00
Average Head on Top of Layer 6	0.2159	---	---
Change in Water Storage	-2.5279	-113,787.5	-7.49
Soil Water at Start of Year	42.2513	1,901,813.9	125.13
Soil Water at End of Year	39.7233	1,788,026.4	117.64
Snow Water at Start of Year	0.0000	0.0000	0.00
Snow Water at End of Year	0.0000	0.0000	0.00
Annual Water Budget Balance	0.0000	0.0000	0.00

Average Annual Totals Summary

Title: Cof GMF - CIP Cons
Simulated on: 6/27/2022 18:30

	Average Annual Totals for Years 1 - 30*			
	(inches)	[std dev]	(cubic feet)	(percent)
Precipitation	39.97	[4.83]	1,798,943.4	100.00
Runoff	3.963	[2.529]	178,369.0	9.92
Evapotranspiration	29.888	[3.283]	1,345,302.1	74.78
Subprofile1				
Lateral drainage collected from Layer 3	6.1459	[2.4217]	276,641.4	15.38
Percolation/leakage through Layer 4	0.002554	[0.001385]	115.0	0.01
Average Head on Top of Layer 4	0.0155	[0.0128]	---	---
Subprofile2				
Percolation/leakage through Layer 7	0.000172	[0.000108]	7.7305	0.00
Average Head on Top of Layer 6	0.0947	[0.0648]	---	---
Water storage				
Change in water storage	-0.0306	[1.2297]	-1,376.8	-0.08

* Note: Average inches are converted to volume based on the user-specified area.

Peak Values Summary

Title: Cof GMF - CIP Cons
Simulated on: 6/27/2022 18:31

	Peak Values for Years 1 - 30*	
	(inches)	(cubic feet)
Precipitation	3.44	154,722.1
Runoff	2.903	130,677.3
Subprofile1		
Drainage collected from Layer 3	0.6967	31,360.7
Percolation/leakage through Layer 4	0.002325	104.6
Average head on Layer 4	9.9873	---
Maximum head on Layer 4	17.4269	---
Location of maximum head in Layer 3	82.00 (feet from drain)	
Subprofile2		
Percolation/leakage through Layer 7	0.000001	0.0460
Average head on Layer 6	0.2190	
Other Parameters		
Snow water	6.8181	306,896.9
Maximum vegetation soil water	0.4664 (vol/vol)	
Minimum vegetation soil water	0.2373 (vol/vol)	

Final Water Storage in Landfill Profile at End of Simulation Period

Title: Cof GMF - CIP Cons
Simulated on: 6/27/2022 18:31
Simulation period: 30 years

Layer	Final Water Storage	
	(inches)	(vol/vol)
1	1.5117	0.2519
2	5.5211	0.3067
3	0.0020	0.0100
4	0.0000	0.0000
5	15.1925	0.1055
6	0.0000	0.0000
7	16.2360	0.4510
Snow water	0.0000	---

APPENDIX A-2

CPP GSP- 3-FT FINAL PROTECTIVE COVER SOIL

HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
HELP MODEL VERSION 4.0 BETA (2018)
DEVELOPED BY USEPA NATIONAL RISK MANAGEMENT RESEARCH LABORATORY

Title: Cof GMF - CIP Cons **Simulated On:** 6/23/2022 14:26

Layer 1

Type 1 - Vertical Percolation Layer (Cover Soil)

SiCL - Silty Clay Loam

Material Texture Number 12

Thickness	=	6 inches
Porosity	=	0.471 vol/vol
Field Capacity	=	0.342 vol/vol
Wilting Point	=	0.21 vol/vol
Initial Soil Water Content	=	0.2608 vol/vol
Effective Sat. Hyd. Conductivity	=	4.20E-05 cm/sec

Layer 2

Type 1 - Vertical Percolation Layer

SiC - Silty Clay

Material Texture Number 14

Thickness	=	30 inches
Porosity	=	0.479 vol/vol
Field Capacity	=	0.371 vol/vol
Wilting Point	=	0.251 vol/vol
Initial Soil Water Content	=	0.3648 vol/vol
Effective Sat. Hyd. Conductivity	=	2.50E-05 cm/sec

Layer 3

Type 2 - Lateral Drainage Layer

Drainage Net (0.5 cm)

Material Texture Number 20

Thickness	=	0.2 inches
Porosity	=	0.85 vol/vol
Field Capacity	=	0.01 vol/vol
Wilting Point	=	0.005 vol/vol
Initial Soil Water Content	=	0.0229 vol/vol
Effective Sat. Hyd. Conductivity	=	1.00E+01 cm/sec
Slope	=	4 %
Drainage Length	=	650 ft

Layer 4

Type 4 - Flexible Membrane Liner
LDPE Membrane

Material Texture Number 36

Thickness	=	0.04 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 5

Type 1 - Vertical Percolation Layer (Waste)

Gypsum waste material (Sandy Loam)

Material Texture Number 83

Thickness	=	144 inches
Porosity	=	0.437 vol/vol
Field Capacity	=	0.105 vol/vol
Wilting Point	=	0.047 vol/vol
Initial Soil Water Content	=	0.105 vol/vol
Effective Sat. Hyd. Conductivity	=	6.70E-04 cm/sec

Layer 6

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.06 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 7

Type 3 - Barrier Soil Liner

C (Moderate)

Material Texture Number 29

Thickness	=	36 inches
Porosity	=	0.451 vol/vol
Field Capacity	=	0.419 vol/vol
Wilting Point	=	0.332 vol/vol
Initial Soil Water Content	=	0.451 vol/vol
Effective Sat. Hyd. Conductivity	=	6.80E-07 cm/sec

Note: Initial moisture content of the layers and snow water were computed as nearly steady-state values by HELP.

General Design and Evaporative Zone Data

SCS Runoff Curve Number	=	87.5
Fraction of Area Allowing Runoff	=	100 %
Area projected on a horizontal plane	=	12.4 acres
Evaporative Zone Depth	=	18 inches
Initial Water in Evaporative Zone	=	5.708 inches
Upper Limit of Evaporative Storage	=	8.574 inches
Lower Limit of Evaporative Storage	=	4.272 inches
Initial Snow Water	=	0.088112 inches
Initial Water in Layer Materials	=	43.871 inches
Total Initial Water	=	43.959 inches
Total Subsurface Inflow	=	0 inches/year

 Note: SCS Runoff Curve Number was calculated by HELP.

Evapotranspiration and Weather Data

Station Latitude	=	39.07 Degrees
Maximum Leaf Area Index	=	4.5
Start of Growing Season (Julian Date)	=	97 days
End of Growing Season (Julian Date)	=	302 days
Average Wind Speed	=	8 mph
Average 1st Quarter Relative Humidity	=	72 %
Average 2nd Quarter Relative Humidity	=	64 %
Average 3rd Quarter Relative Humidity	=	71 %
Average 4th Quarter Relative Humidity	=	72 %

 Note: Evapotranspiration data was obtained for Coffeen, Illinois

Normal Mean Monthly Precipitation (inches)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
2.353618	2.511085	2.81508	3.241374	3.956664	4.312863
4.375035	2.656228	3.284204	3.675466	3.677412	3.106835

 Note: Precipitation was simulated based on HELP V4 weather simulation for:
 Lat/Long: 39.07/-89.4

Normal Mean Monthly Temperature (Degrees Fahrenheit)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
37.3	37.1	50	61.9	69.7	80.6
84.2	81	72.2	62.4	48.1	38

Average Annual Totals Summary

Title: Cof GMF - CIP Cons
Simulated on: 6/23/2022 14:28

	Average Annual Totals for Years 1 - 30*			
	(inches)	[std dev]	(cubic feet)	(percent)
Precipitation	39.97	[4.83]	1,798,943.4	100.00
Runoff	3.962	[2.529]	178,360.0	9.91
Evapotranspiration	29.891	[3.287]	1,345,434.5	74.79
Subprofile1				
Lateral drainage collected from Layer 3	6.1480	[2.4572]	276,733.5	15.38
Percolation/leakage through Layer 4	0.001912	[0.000902]	86.0	0.00
Average Head on Top of Layer 4	0.0068	[0.0082]	---	---
Subprofile2				
Percolation/leakage through Layer 7	0.000141	[0.000083]	6.3525	0.00
Average Head on Top of Layer 6	0.0758	[0.0487]	---	---
Water storage				
Change in water storage	-0.0353	[1.3562]	-1,590.9	-0.09

* Note: Average inches are converted to volume based on the user-specified area.

Peak Values Summary

Title: Cof GMF - CIP Cons
Simulated on: 6/23/2022 14:28

	Peak Values for Years 1 - 30*	
	(inches)	(cubic feet)
Precipitation	3.44	154,722.1
Runoff	2.903	130,677.6
Subprofile1		
Drainage collected from Layer 3	0.6202	27,916.9
Percolation/leakage through Layer 4	0.001856	83.5
Average head on Layer 4	8.4412	---
Maximum head on Layer 4	13.4190	---
Location of maximum head in Layer 3	68.69 (feet from drain)	
Subprofile2		
Percolation/leakage through Layer 7	0.000001	0.0353
Average head on Layer 6	0.1636	
Other Parameters		
Snow water	6.8181	306,896.9
Maximum vegetation soil water	0.4665 (vol/vol)	
Minimum vegetation soil water	0.2373 (vol/vol)	

Final Water Storage in Landfill Profile at End of Simulation Period

Title: Cof GMF - CIP Cons
Simulated on: 6/23/2022 14:28
Simulation period: 30 years

Layer	Final Water Storage	
	(inches)	(vol/vol)
1	1.5117	0.2519
2	9.9750	0.3325
3	0.0020	0.0100
4	0.0000	0.0000
5	15.1742	0.1054
6	0.0000	0.0000
7	16.2360	0.4510
Snow water	0.0000	---

APPENDIX A-3

**CPP GSP-3-FT COMPACTED EARTH LAYER, 3-FT
FINAL PROTECTIVE COVER SOIL**

Material Texture Number 83

Thickness	=	144 inches
Porosity	=	0.437 vol/vol
Field Capacity	=	0.105 vol/vol
Wilting Point	=	0.047 vol/vol
Initial Soil Water Content	=	0.1159 vol/vol
Effective Sat. Hyd. Conductivity	=	6.70E-04 cm/sec

Layer 5

Type 4 - Flexible Membrane Liner

LDPE Membrane

Material Texture Number 36

Thickness	=	0.06 inches
Effective Sat. Hyd. Conductivity	=	4.00E-13 cm/sec
FML Pinhole Density	=	1 Holes/Acre
FML Installation Defects	=	1 Holes/Acre
FML Placement Quality	=	3 Good

Layer 6

Type 3 - Barrier Soil Liner

C (Moderate)

Material Texture Number 29

Thickness	=	36 inches
Porosity	=	0.451 vol/vol
Field Capacity	=	0.419 vol/vol
Wilting Point	=	0.332 vol/vol
Initial Soil Water Content	=	0.451 vol/vol
Effective Sat. Hyd. Conductivity	=	6.80E-07 cm/sec

Note: Initial moisture content of the layers and snow water were computed as nearly steady-state values by HELP.

General Design and Evaporative Zone Data

SCS Runoff Curve Number	=	87.5
Fraction of Area Allowing Runoff	=	100 %
Area projected on a horizontal plane	=	12.4 acres
Evaporative Zone Depth	=	18 inches
Initial Water in Evaporative Zone	=	5.667 inches
Upper Limit of Evaporative Storage	=	8.574 inches
Lower Limit of Evaporative Storage	=	4.272 inches
Initial Snow Water	=	0.088112 inches
Initial Water in Layer Materials	=	62.588 inches
Total Initial Water	=	62.676 inches

Total Subsurface Inflow = 0 inches/year

Note: SCS Runoff Curve Number was calculated by HELP.

Evapotranspiration and Weather Data

Station Latitude = 39.07 Degrees
Maximum Leaf Area Index = 4.5
Start of Growing Season (Julian Date) = 97 days
End of Growing Season (Julian Date) = 302 days
Average Wind Speed = 8 mph
Average 1st Quarter Relative Humidity = 72 %
Average 2nd Quarter Relative Humidity = 64 %
Average 3rd Quarter Relative Humidity = 71 %
Average 4th Quarter Relative Humidity = 72 %

Note: Evapotranspiration data was obtained for Coffeen, Illinois

Normal Mean Monthly Precipitation (inches)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
2.353618	2.511085	2.81508	3.241374	3.956664	4.312863
4.375035	2.656228	3.284204	3.675466	3.677412	3.106835

Note: Precipitation was simulated based on HELP V4 weather simulation for:
Lat/Long: 39.07/-89.4

Normal Mean Monthly Temperature (Degrees Fahrenheit)

<u>Jan/Jul</u>	<u>Feb/Aug</u>	<u>Mar/Sep</u>	<u>Apr/Oct</u>	<u>May/Nov</u>	<u>Jun/Dec</u>
37.3	37.1	50	61.9	69.7	80.6
84.2	81	72.2	62.4	48.1	38

Note: Temperature was simulated based on HELP V4 weather simulation for:
Lat/Long: 39.07/-89.4
Solar radiation was simulated based on HELP V4 weather simulation for:
Lat/Long: 39.07/-89.4

Average Annual Totals Summary

Title: Cof GMF - CIP Cons
Simulated on: 6/23/2022 14:38

	Average Annual Totals for Years 1 - 30*			
	(inches)	[std dev]	(cubic feet)	(percent)
Precipitation	39.97	[4.83]	1,798,943.4	100.00
Runoff	7.080	[3.444]	318,685.9	17.72
Evapotranspiration	30.973	[3.376]	1,394,153.9	77.50
Subprofile1				
Percolation/leakage through Layer 3	1.976199	[0.075661]	88,952.7	4.94
Average Head on Top of Layer 3	21.2644	[2.1745]	---	---
Subprofile2				
Percolation/leakage through Layer 6	0.094011	[0.06158]	4,231.6	0.24
Average Head on Top of Layer 5	81.6880	[52.8561]	---	---
Water storage				
Change in water storage	1.8189	[1.5745]	81,872.0	4.55

* Note: Average inches are converted to volume based on the user-specified area.

Peak Values Summary

Title: Cof GMF - CIP Cons
Simulated on: 6/23/2022 14:38

	Peak Values for Years 1 - 30*	
	(inches)	(cubic feet)
Precipitation	3.44	154,722.1
Runoff	2.971	133,752.0
Subprofile1		
Percolation/leakage through Layer 3	0.006803	306.2
Average head on Layer 3	35.9998	
Subprofile2		
Percolation/leakage through Layer 6	0.000461	20.7
Average head on Layer 5	144.0000	
Other Parameters		
Snow water	6.8181	306,896.9
Maximum vegetation soil water	0.4763 (vol/vol)	
Minimum vegetation soil water	0.2373 (vol/vol)	

Final Water Storage in Landfill Profile at End of Simulation Period

Title: Cof GMF - CIP Cons
Simulated on: 6/23/2022 14:38
Simulation period: 30 years

Layer	Final Water Storage	
	(inches)	(vol/vol)
1	1.4270	0.2378
2	11.0513	0.3684
3	15.3720	0.4270
4	73.1567	0.5080
5	0.0000	0.0000
6	16.2360	0.4510
Snow water	0.0000	---



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APPENDIX H

Groundwater Monitoring Plan

Intended for
Illinois Power Generating Company

Date
October 25, 2021

Project No.
1940100806-002

GROUNDWATER MONITORING PLAN

GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS

GROUNDWATER MONITORING PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND


Project Name **Coffeen Power Plant GMF Gypsum Stack Pond**
Project No. **1940100806-002**
Recipient **Illinois Power Generating Company**
Document Type **Groundwater Monitoring Plan**
Revision **FINAL**
Date **October 25, 2021**

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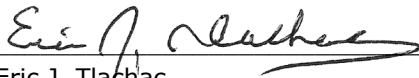


Chase J. Christenson, PG
Hydrogeologist

LICENSED PROFESSIONAL CERTIFICATIONS

35 I.A.C. § 845.630 Groundwater Monitoring Systems (PE)

I, Eric J. Tlachac, a qualified professional engineer in good standing in the State of Illinois, certify that the groundwater monitoring system described in this document (Groundwater Monitoring Plan, Coffeen Power Plant GMF Gypsum Stack Pond), has been designed and constructed to meet the requirements of 35 I.A.C. § 845.630. The monitoring system was developed based on information included in the Hydrogeologic Site Characterization Report (Ramboll 2021; included in the Operating Permit to which this Groundwater Monitoring Plan is attached).



Eric J. Tlachac
Qualified Professional Engineer
062-063091
Illinois
Date: October 25, 2021



35 I.A.C. § 845.630 Groundwater Monitoring Systems (PG)

I, Brian G. Hennings, a qualified professional geologist in good standing in the State of Illinois, certify that the groundwater monitoring system described in this document (Groundwater Monitoring Plan, Coffeen Power Plant GMF Gypsum Stack Pond), has been designed and constructed to meet the requirements of 35 I.A.C. § 845.630. The monitoring system was developed based on information included in the Hydrogeologic Site Characterization Report (Ramboll 2021; included in the Operating Permit to which this Groundwater Monitoring Plan is attached).



Brian G. Hennings
Professional Geologist
196.001482
Illinois
Date: October 25, 2021



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Table 2-1	Monitoring Well Locations and Construction Details
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FIGURES (ATTACHED)

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Figure 1-2	Site Map
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APPENDICES

Appendix A	Statistical Analysis Plan
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ACRONYMS AND ABBREVIATIONS

35 I.A.C.	Title 35 of the Illinois Administrative Code
40 C.F.R.	Title 40 of the Code of Federal Regulations
ASD	Alternate Source Demonstration
bgs	below ground surface
CCR	coal combustion residuals
cm/s	centimeters per second
CPP	Coffeen Power Plant
DA	deep aquifer
DCU	deep confining unit
GMF	Gypsum Management Facility
GMP	Groundwater Monitoring Plan
GSP	Gypsum Stack Pond
GWPS	Groundwater Protection Standard
HCR	Hydrogeologic Site Characterization Report
HDPE	high-density polyethylene
ID	identification
IEPA	Illinois Environmental Protection Agency
IPGC	Illinois Power Generating Company
LCU	lower confining unit
mg/L	milligrams per liter
NA	not applicable
NID	National Inventory of Dams
No.	number
NRT/OBG	Natural Resources Technology, Inc., an OBG Company
Part 845	Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments: Title 35 of the Illinois Administrative Code § 845
PMP	Potential Migration Pathway
QA/QC	quality assurance/quality control
Ramboll	Ramboll Americas Engineering Solutions, Inc.
RL	reporting limit
RP	Recycle Pond
SI	surface impoundment
TDS	total dissolved solids
UA	uppermost aquifer
UCU	upper confining unit
USEPA	United States Environmental Protection Agency
WLO	water level only
WPCP	Water Pollution Control Permit

1. INTRODUCTION

1.1 Overview

In accordance with requirements of the Standards for the Disposal of Coal Combustion Residuals (CCR) in Surface Impoundments (SIs): Title 35 of the Illinois Administrative Code (35 I.A.C.) § 845 (Part 845) (Illinois Environmental Protection Agency [IEPA], April 15, 2021), Ramboll Americas Engineering Solutions, Inc. (Ramboll) has prepared this Groundwater Monitoring Plan (GMP) on behalf of Coffeen Power Plant (CPP), operated by Illinois Power Generating Company (IPGC). This report will apply specifically to the CCR Units referred to as the Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP), Vistra identification (ID) number (No.) 103, IEPA ID No. W1350150004-03, and National Inventory of Dams (NID) No. IL50579. This GMP includes Part 845 content requirements specific to 35 I.A.C. § 845.630 (Groundwater Monitoring System), 35 I.A.C. § 845.640 (Groundwater Sampling and Analysis), and 35 I.A.C. § 845.650 (Groundwater Monitoring Program) for the GMF GSP at the CPP.

A checklist which identifies the specific requirements of 35 I.A.C. § 845.630, 35 I.A.C. § 845.640, and 35 I.A.C. § 845.650 is included in **Table 1-1**. The table provides references to sections, tables, and figures included in this document to locate the information that meets specific requirements of 35 I.A.C. § 845.630, 35 I.A.C. § 845.640, and 35 I.A.C. § 845.650.

1.2 Site Location and Background

The CPP is approximately two miles south of the city of Coffeen, Illinois and approximately eight miles southeast of the city of Hillsboro, Illinois (**Figure 1-1**). The GMF GSP is located in Montgomery County, in central Illinois, within Section 11, Township 7 North, and Range 7 East. The CPP is located between the two lobes of Coffeen Lake to the west, east, and south, and is bordered by agricultural land to the north. The CPP operated as a coal-fired power plant from 1964 to November 2019 and has five CCR management units. The approximately 1,100-acre Coffeen Lake was built by damming the McDavid Branch of the East Fork of Shoal Creek in 1963 for use as an artificial cooling lake for the CPP.

The two GMFs, consisting of the 77-acre GMF GSP and the 17-acre GMF Recycle Pond (RP) (**Figure 1-2**), receive blowdown from the air emission scrubbers and have been in operation since 2010. Construction of the GMFs were in accordance with the IEPA Water Pollution Control Permit (WPCP) No. 2008-EA-4661 and feature a composite high-density polyethylene (HDPE) liner with three feet of compacted soil with a hydraulic conductivity of 1×10^{-7} centimeters per second (cm/s). Both GMF ponds have a groundwater underdrain system. The GMF GSP system was actively pumped during construction but is currently not used. The GMF RP underdrain is a passive, gravity drained system. IPGC ceased receipt of waste to the GMF GSP prior to April 11, 2021.

1.3 Conceptual Model

Significant site investigation has been completed at the CPP to characterize the geology, hydrogeology, and groundwater quality. Based on extensive investigation and monitoring, the CPP has been well characterized and detailed in the Hydrogeologic Site Characterization Report (HCR; Ramboll, 2021), included in the Operating Permit to which this Plan is attached. A site conceptual model has been developed and is discussed below.

In addition to the CCR present at the GMF GSP, there are five principal layers of unlithified material present above the bedrock, which are categorized into hydrostratigraphic units below based on stratigraphic relationships and common hydrogeologic characteristics.

- **Upper Confining Unit (UCU):** Composed of the Roxana and Peoria Silts (Loess Unit) and the upper clayey portion of the Hagarstown member which are classified as silts to clayey silts and gravelly clay below the surficial soil. Construction of the GMF GSP required the excavation and removal of this layer within the unit footprint and the UCU has been eroded east of the GMF GSP, near the Unnamed Tributary.
- **Uppermost Aquifer:** The uppermost aquifer is the Hagarstown Member which is classified as primarily sandy to gravelly silts and clays with thin beds of sands. Similar to the Loess Unit, the Hagarstown Member was excavated to facilitate construction of the GMF GSP and the Hagarstown is also absent in some locations near the Unnamed Tributary.
- **Lower Confining Unit (LCU):** Comprised of the Vandalia Member, Mulberry Grove Member, and Smithboro Member. These units include a sandy to silty till with thin, discontinuous sand lenses, a discontinuous and limited extent sandy silt which has infilled prior erosional features, and silty to clayey diamicton, respectively.
- **Deep Aquifer (DA):** Sand and sandy silt/clay units of the Yarmouth Soil, which include accretionary deposits of fine sediment and organic materials, typically less than five feet thick and discontinuous across the site. Where present, the DA has been identified as a potential migration pathway (PMP) due to presence of downward gradients and the relatively greater hydraulic conductivities measured in the DA.
- **Deep Confining Unit (DCU):** Comprised of the Banner Formation, generally consists of clays, silts, and sands. The Lierle Clay Member is the upper layer of the Banner Formation which was encountered at the Site.

Flow of groundwater from central portions of the CPP to Coffeen Lake or the Unnamed Tributary through the uppermost aquifer are the primary pathways for contaminant migration. Groundwater elevations are primarily controlled by surface topography, geologic unit topography, and water levels within Coffeen Lake and the Unnamed Tributary. A groundwater divide trending north-south is observed running through the approximate center of the CPP (**Figure 1-3**). Water levels within the SIs are generally consistent and have not been observed to fluctuate with groundwater elevations indicating limited hydraulic connection with the SIs.

Part 845 parameters were monitored in the uppermost aquifer monitoring wells at the GMF GSP as part of the IEPA WPCP No. 2020-EO-65043 monitoring program and Title 40 of the Code of Federal Regulations (40 C.F.R.) § 257 monitoring program beginning in 2015. These data were supplemented by sampling of additional locations in 2021. The results indicate that the following parameters were detected at concentrations/measurements greater than (or less than for pH) the applicable 35 I.A.C. § 845.600 groundwater protection standards (GWPSs) and are considered potential exceedances:

- Arsenic in compliance uppermost aquifer well G215.
- Beryllium in compliance uppermost aquifer well G209.
- Cobalt in compliance uppermost aquifer wells G209, G213, and G217.
- Lead in compliance uppermost aquifer wells G209 and G213.

- pH (lower limit) in compliance uppermost aquifer well G206.
- Sulfate in compliance uppermost aquifer well G215 and in compliance DA well G206D.
- Thallium in compliance uppermost aquifer well G209.
- Total dissolved solids (TDS) in compliance DA well G206D.

Concentration results for the above parameters were compared directly to 35 I.A.C. § 845.600 GWPS, without an evaluation of background concentrations. Evaluation of background groundwater quality has been completed as part of this GMP, and compliance with Part 845 will be determined following the first round of groundwater sampling. The first round of groundwater sampling for compliance will be completed the quarter following issuance of the Operating Permit and in accordance with this GMP.

2. GROUNDWATER MONITORING SYSTEMS

2.1 Existing Monitoring Well Network and Analysis

This GMP is being provided to propose a groundwater monitoring network and monitoring program specific to the GMF GSP that will comply with Part 845. Monitoring networks and programs that apply to other units are not discussed in this GMP. Those programs will continue to be performed as specified in IEPA approvals. Any future modifications will be proposed and submitted to IEPA for approval in a separate document. The remaining discussion in this document will include only these networks and monitoring programs that are applicable and specific to the GMF GSP, specifically the IEPA WPCP monitoring network, the 40 C.F.R. § 257 network, and the proposed Part 845 monitoring network.

2.1.1 IEPA Groundwater Monitoring Program

Routine quarterly groundwater monitoring is completed for a monitoring well network that includes wells for both the GMF GSP and GMF RP. The monitoring well network consists of thirty-one monitoring wells screened in the uppermost aquifer (G102, G103, R104, G105, G106, G200, G205, G206, G207, G208, G209, G210, G211, G212, G213, G214, G215, G216, G217, G218, G270, G271, G272, G273, G274, G275, G276, G277, G279, G280, and R201) in accordance with IEPA WPCP No. 2020-EO-65043, issued on March 11, 2020. The boring logs and well construction forms for the GMF well network are included in Appendix C of the HCR (included in the Operating Permit to which this Plan is attached). Quarterly and annual samples are analyzed for the following field and laboratory parameters listed in **Table A** below.

Table A. IEPA Groundwater Monitoring Program Parameters

Field Parameters¹			
pH	Elevation of Measuring Point	Specific Conductance	
Depth to Water (below measuring point, below ground surface)	Elevation of Groundwater Surface	Temperature	
Metals (Dissolved)			
Antimony	Cadmium	Manganese	Thallium
Arsenic	Chromium	Mercury	Vanadium
Aluminum	Cobalt	Molybdenum	Zinc
Barium	Copper	Nickel	
Beryllium	Iron	Selenium	
Boron	Lead	Silver	
Inorganics (Dissolved)			
Chloride	Fluoride	TDS	
Cyanide	Sulfate		
Other (Total)			
Phenols			

Note: Parameters are monitored as dissolved quarterly, and as dissolved and total annually.

¹Dissolved oxygen, oxidation/reduction potential, and turbidity were recorded during sample collection.

2.1.2 40 C.F.R. § 257 Monitoring Program

The 40 C.F.R. § 257 well network for the GMF GSP consists of seven monitoring wells installed nearby or adjacent to the GMF GSP within the uppermost aquifer. The GMF GSP 40 C.F.R. § 257 well network consists of two background monitoring wells (G200 and R201) and five compliance monitoring wells (G206, G209, G212, G215, and G218). The boring logs, well construction forms, and other related monitoring well forms are available in the Operating Records as required by 40 C.F.R. § 257.91 for each monitored CCR Unit or CCR Multi-Unit, and are included in Appendix C of the HCR (included in the Operating Permit to which this Plan is attached).

Groundwater is being monitored at the GMF GSP in accordance with the Detection Monitoring Program requirements specified in 40 C.F.R. § 257.94. Details on the procedures and techniques used to fulfill the groundwater sampling and analysis program requirements are found in the Sampling and Analysis Plan for GMF GSP (Natural Resource Technology, an OBG Company [NRT/OBG], 2017).

Groundwater samples are collected semiannually and analyzed for the laboratory and field parameters from Appendix III of 40 C.F.R. § 257, summarized in **Table B** below.

Table B. 40 C.F.R. § 257 Groundwater Monitoring Program Parameters

Field Parameters ¹		
Groundwater Elevation	pH	
Appendix III Parameters (Total, except TDS)		
Boron	Chloride	Sulfate
Calcium	Fluoride	TDS

¹Dissolved oxygen, temperature, specific conductance, oxidation/reduction potential, and turbidity are recorded during sample collection.

Results and analysis of groundwater sampling are reported annually by January 31 of the following year and made available on the CCR public website as required by 40 C.F.R. § 257.

2.1.3 Part 845 Well Installation and Monitoring

In 2021, one additional monitoring well (G206D), one source sample collection point (NE Riser), and one soil boring (G289) were installed along the perimeter of the GMF GSP to assess the vertical and horizontal lithology, stratigraphy, chemical properties, and physical properties of geologic layers to a minimum of 100 feet below ground surface (bgs) as specified in 35 I.A.C. § 845.620(b).

Prospective Part 845 monitoring wells were sampled for eight rounds from March to August 2021 and the results were assessed for selection of the GMF GSP Part 845 monitoring well network. Groundwater samples were collected and analyzed for 35 I.A.C. § 845.600 parameters as summarized in **Table C** below.

Table C. Part 845 Groundwater Monitoring Program Parameters

Field Parameters¹			
Groundwater Elevation	pH	Turbidity	
Metals (Total)			
Antimony	Boron	Cobalt	Molybdenum
Arsenic	Cadmium	Lead	Selenium
Barium	Calcium	Lithium	Thallium
Beryllium	Chromium	Mercury	
Inorganics (Total, except TDS)			
Fluoride	Sulfate	Chloride	TDS
Other (Total)			
Radium 226 and 228 combined			

¹ Dissolved oxygen, temperature, specific conductance, and oxidation/reduction potential were recorded during sample collection.

Data and results from the Part 845 background monitoring were included in the water quality discussion included in the HCR (included in the Operating Permit to which this Plan is attached). The data collected from background locations during the Part 845 monitoring were used to evaluate and calculate background concentrations for the GMF GSP. The evaluation and discussion are included in **Section 3.2** of this report.

Data collected from the 40 C.F.R. § 257 monitoring network from 2015 to 2021, and from the Part 845 background monitoring were used for selection of the Part 845 monitoring well network proposed in **Section 2.2**.

2.2 Proposed Part 845 Monitoring Well Network

The groundwater monitoring network proposed in this plan will include nine monitoring wells screened in the uppermost aquifer (G200, G206, G209, G212, G213, G215, G217, G218, and R201), one monitoring well screened in the DA (G206D), one temporary water level only well (NE Riser), and one temporary water level only surface water staff gage (SG-04). The proposed network is summarized in **Table D** below and displayed on **Figure 2-1**. Ten wells (two background and eight compliance) will be used to monitor groundwater concentrations within the hydrostratigraphic units.

The groundwater samples collected from the ten wells will be used to monitor and evaluate groundwater quality and demonstrate compliance with the groundwater quality standards listed in 35 I.A.C. § 845.600(a). The proposed monitoring wells will yield groundwater samples that represent the quality of downgradient groundwater at the CCR boundary (as required in 35 I.A.C. § 845.630(a)(2)). Monitoring well depths and construction details are listed in **Table 2-1** and summarized in **Table D** below.

Table D. Proposed Part 845 Monitoring Well Network

Well ID	Monitored Unit	Well Screen Interval (feet bgs)	Well Type ¹
G200	UA	12.2 – 17.0	Background
G206	UA	17.5 – 21.9	Compliance
G206D	DA	49.2 – 59.0	Compliance
G209	UA	17.7 – 22.3	Compliance
G212	UA	16.7 – 21.3	Compliance
G213	UA	16.8 – 21.3	Compliance
G215	UA	19.4 – 23.8	Compliance
G217	UA	20.5 – 24.9	Compliance
G218	UA	20.3 – 24.8	Compliance
R201	UA	14.6 – 19.3	Background
NE Riser ^{2, 3}	CCR	NA	WLO
SG-04 ^{3, 4}	Surface Water	NA	WLO

¹ Well Type refers to the role of the well in the monitoring network.

² Well is to be for water level data collection only.

³ Location is temporary pending implementation of impoundment closure per an approved Construction Permit Application.

⁴ Surface water level measuring point.

NA = Not Applicable

UA = uppermost aquifer

WLO = water level only

2.3 Well Abandonment

No wells are currently proposed for abandonment.

3. APPLICABLE GROUNDWATER QUALITY STANDARDS

3.1 Groundwater Classification

Groundwater within the uppermost aquifer at the GMF GSP meets the definition of Class I - Potable Resource Groundwater (35 I.A.C. § 620.210), based on the following criteria:

- Groundwater in the uppermost aquifer is located 10 feet or more below the land surface.
- Field hydraulic conductivity testing performed in the uppermost aquifer resulted in an overall (geometric mean) horizontal hydraulic conductivity of 1.4×10^{-3} cm/s, exceeding the 1×10^{-4} cm/s criterion.

3.2 Statistical Evaluation of Background Groundwater Data

A Statistical Analysis Plan (**Appendix A**) has been developed to describe procedures that will be used to establish background conditions and implement compliance monitoring as necessary and required by 35 I.A.C. § 845.640 and 35 I.A.C. § 845.650. The Statistical Analysis Plan was prepared in accordance with the requirements of 35 I.A.C. § 845.640(f), with reference to the acceptable statistical procedures provided in United States Environmental Protection Agency's (USEPA) *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Unified Guidance (Unified Guidance, March 2009)*, and is intended to provide a logical process and framework for conducting the statistical analysis of the data obtained during groundwater monitoring.

In accordance with 35 I.A.C. § 845.640(f)(1), the statistical method chosen for analysis of background groundwater quality was either the tolerance interval or the prediction interval procedure for each constituent listed in 35 I.A.C. § 845.600(a)(1) at this CCR unit per 35 I.A.C. § 845.640(f)(1)(C). A comparison of the statistical background concentrations and groundwater quality standards listed in 35 I.A.C. § 845.600(a)(1) and the resulting GWPSs are summarized in **Table 3-1**.

3.3 Applicable Groundwater Protection Standards

The applicable GWPS will be established in accordance with 35 I.A.C. § 845.600(a) (greater of the background concentration or numerical limit specified in 35 I.A.C. § 845.600(a)(1)). The results of the statistical analysis of background groundwater data (**Table 3-1**) indicate that most background concentrations in the uppermost aquifer and DA are less than the groundwater quality standards listed in 35 I.A.C. § 845.600(a)(1). Therefore, for these parameters, the groundwater quality standards listed in 35 I.A.C. § 845.600(a)(1) will be applied to the results from the proposed groundwater monitoring network. The only exception being arsenic, where the background concentration is greater than the 35 I.A.C. § 845.600(a)(1) standard (0.011 milligrams per liter [mg/L] versus 0.010 mg/L). In this instance, the GWPS will be the background concentration.

Under most circumstances, the GWPS will be compared to the lower confidence limit for the observed concentrations for each constituent in each compliance well. Exceptions are when there are high percentages (greater than 50 percent) of non-detects in compliance well data, for which a future mean (for 50 to 70 percent non-detects) or median (for greater than 70 percent non-detects) will be compared to the GWPS. Consistent with the *Unified Guidance*, the same general statistical method of confidence interval testing against a fixed GWPS is recommended in compliance and corrective action programs. Confidence intervals provide a flexible and

statistically accurate method to test how a parameter estimated from a single sample compares to a fixed numerical limit. Confidence intervals explicitly account for variation and uncertainty in the sample data used to construct them.

Evaluation of the applicable standards will occur in conjunction with the analysis of groundwater quality results. Background calculations and the resulting concentrations may be updated as appropriate, in accordance with the Statistical Analysis Plan included in **Appendix A**.

4. GROUNDWATER MONITORING PLAN

The groundwater monitoring plan will monitor and evaluate groundwater quality to demonstrate compliance with the groundwater quality standards included in 40 C.F.R. § 257.94(e), 40 C.F.R. § 257.95(h), and 35 I.A.C. § 845.600(a). The groundwater monitoring program will include sampling and analysis procedures that are consistent and that provide an accurate representation of groundwater quality at the background and compliance wells as required by 35 I.A.C. § 845.630. As discussed in **Section 2**, three monitoring programs specific to GMF GSP exist: the IEPA WPCP monitoring program, the 40 C.F.R. § 257 monitoring program, and the proposed Part 845 monitoring program. These networks will continue to be monitored until USEPA approves Part 845. Upon approval of the Operating Permits (and by extension the GMPs) for the GMF GSP and RP, the IEPA WPCP monitoring will be replaced by the Part 845 monitoring program. It is expected that upon USEPA approval of Part 845, the 40 C.F.R. § 257 monitoring program and reporting will be eliminated, and the proposed Part 845 monitoring and reporting included in this Plan will continue until requirements of Part 845 have been achieved.

4.1 Monitoring Networks and Parameters

4.1.1 IEPA Monitoring Program

The existing IEPA monitoring program was discussed in detail in **Section 2.1.1**. Thirty-one monitoring wells are sampled quarterly for dissolved analyses, and annually for totals. No changes are proposed to this monitoring network. Well locations and parameters will continue to be monitored and reported as required by the WPCP until IEPA approves this GMP and the GMP prepared for the GMF RP.

4.1.2 40 C.F.R. § 257 Groundwater Monitoring

The existing 40 C.F.R. § 257 monitoring program was discussed in detail in **Section 2.1.2**. Seven wells (two background and five compliance) are sampled for Appendix III parameters on a semi-annual frequency. No changes are proposed to this monitoring network. Well locations and parameters will continue to be monitored and reported as required by 40 C.F.R. § 257 until USEPA approves Part 845.

4.1.3 Part 845 Groundwater Monitoring

The proposed Part 845 Monitoring Network will consist of two background monitoring wells (G200 and R201), eight compliance monitoring wells (G206, G206D, G209, G212, G213, G215, G217, and G218), one temporary water level only well (NE Riser) and one temporary water level only surface staff gage (SG-04) to monitor potential impacts from the GMF GSP (**Figure 2-1**). These monitoring wells are screened within the uppermost aquifer and DA along the perimeter of the GMF GSP. Groundwater samples will be collected and analyzed for the laboratory and field parameters in **Table E** below.

Table E. Part 845 Groundwater Monitoring Program Parameters

Field Parameters¹			
Groundwater Elevation	pH	Turbidity	
Metals (Total)			
Antimony	Boron	Cobalt	Molybdenum
Arsenic	Cadmium	Lead	Selenium
Barium	Calcium	Lithium	Thallium
Beryllium	Chromium	Mercury	
Inorganics (Total, except TDS)			
Fluoride	Sulfate	Chloride	TDS
Other (Total)			
Radium 226 and 228 combined			

¹ Dissolved oxygen, temperature, specific conductance, and oxidation/reduction potential will be recorded during sample collection.

All parameters listed above were sampled a minimum of eight times by October 18, 2021 to establish background groundwater quality in accordance with 35 I.A.C. § 845.650 (b)(1)(A). Discussion of background groundwater quality is included in **Section 3.2**.

4.2 Sampling Schedule

Groundwater sampling for the Part 845 monitoring well network will initially be performed quarterly according to the following schedule:

Table F. Part 845 Sampling Schedule

Frequency	Duration
Monthly (groundwater elevations only)	Begins: the quarter following approval of this plan and issuance of the Operating Permit.
	Ends: Following the 30-year post closure care period and following IEPA approval of documentation that groundwater concentrations are below standards in 35 I.A.C. § 845.600 and concentrations exceeding background are not increasing and meet requirements in 35 I.A.C. § 845.780 (c)(2)(B)(i) and (ii).
Quarterly (groundwater quality)	Begins: the quarter following approval of this plan and issuance of the Operating Permit.
	Ends: Following the 30-year post closure care period and following IEPA approval of documentation that groundwater concentrations are below standards in 35 I.A.C. § 845.600 and concentrations exceeding background are not increasing and meet requirements in 35 I.A.C. § 845.780 (c)(2)(B)(i) and (ii), or upon IEPA approval of an alternate schedule as allowed by 35 I.A.C. § 845.650(b)(4).
Semi-annual (groundwater quality)	Begins: Following 5 years of quarterly groundwater monitoring and IEPA approval of a demonstration that groundwater concentrations are below standards in 35 I.A.C. § 845.600 and not exhibiting statistically-significant increasing trends, monitoring effectiveness is not compromised by a semi-annual schedule, and sufficient data has been collected to characterize groundwater.
	Ends: Following detection of a statistically-significant increasing trend in groundwater concentrations or an exceedance of the standards in 35 I.A.C. § 845.600 (quarterly monitoring shall be resumed in these circumstances), or following the 30-year post closure care period and following IEPA approval of documentation that groundwater concentrations

are below standards in 35 I.A.C. § 845.600 and concentrations exceeding background are not increasing and meet requirements in 35 I.A.C. § 845.780 (c)(2)(B)(i) and (ii).

4.3 Groundwater Sample Collection

Groundwater sampling procedures have been developed and the collection of groundwater samples is being implemented to meet the requirements of 35 I.A.C. § 845.640. In addition to groundwater well samples, quality assurance samples will be collected as described in **Section 4.5 (Table 4-1)**.

4.4 Laboratory Analysis

Laboratory analysis will be performed consistent with the requirements of 35 I.A.C. § 845.640(j) by a state-certified laboratory using methods approved by IEPA and USEPA. Laboratory methods may be modified based on laboratory equipment availability or procedures, but the Reporting Limit (RL) for all parameters analyzed, regardless of method, will be lower than the applicable groundwater quality standard. RLs for the applicable parameters are summarized in **Table 4-2**. Concentrations lower than the RL will be reported as less than the RL.

4.5 Quality Assurance Program

Consistent with the requirements of 35 I.A.C. § 845.640(a)(5), the sampling and analysis program includes procedures and techniques for quality assurance/quality control (QA/QC). Additional quality assurance samples to be collected will include the following:

- Field duplicates will be collected at a frequency of one per group of ten or fewer investigative water samples.
- One equipment blank sample will be collected and analyzed for each day of sampling. If dedicated sampling equipment is used, then equipment blank samples will not be collected.
- The duplicate and equipment blank quality assurance samples will be supplemented by the laboratory QA/QC program, which typically includes:
 - Regular generation of instrument calibration curves to assure instrument reliability
 - Laboratory control samples and/or quality control check standards that have been spiked, and analyses to monitor the performance of the analytical method
 - Matrix spike/matrix spike duplicate analyses to determine percent recoveries and relative percent differences for each of the parameters detected
 - Analysis of replicate samples to check the precision of the instrumentation and/or methodology employed for all analytical methods
 - Analysis of method blanks to assure that the system is free of contamination

Water quality meters used to measure pH and turbidity will be calibrated according to manufacturer's specifications. At a minimum, it is recommended that calibration of pH occur daily prior to sampling and checked for accuracy at the end of each day. Unusual or suspect pH measurements during sampling events will be flagged, evaluated, and additional calibration may be performed throughout the sampling events. Turbidity meters will be checked daily, prior to and following sampling. Unusual measurements or erratic meter performance will be flagged and evaluated for overall effects on the data prior to reporting.

4.6 Groundwater Monitoring System Maintenance Plan

Consistent with the requirements of 35 I.A.C. § 845.630(e)(2), maintenance will be performed as needed to assure that the monitoring wells provide representative groundwater samples. Monitoring wells will be inspected during each groundwater sampling event; inspections will consist of the following:

- Visual inspection, clearing of vegetation, replacement of markers, and painting of protective casings as needed to assure that monitoring wells are clearly marked and accessible
- Visual inspection and repair or replacement of well aprons as needed to assure that they are intact, drain water away from the well, and have not heaved
- Visual inspection and repair or replacement of protective casings as needed to assure that they are undamaged, and that locks are present and functional
- Checks to assure that well caps are intact and vented, unless in flood-prone areas in which case caps will not be vented
- Annual measurement of monitoring well depths to determine the degree of siltation within the wells. Wells will be redeveloped as needed to remove siltation from the screened interval if it impedes flow of water into the well
- Checks to assure that wells are clear of internal obstructions, and flow freely

If maintenance of a monitoring well cannot address an identified deficiency, a replacement well will be installed.

4.7 Statistical Analysis

Statistical analysis will be consistent with procedures listed in 35 I.A.C. § 845.640(f). A Statistical Analysis Plan, provided in **Appendix A**, has been developed to summarize the statistical procedures that will be used to evaluate the groundwater results.

4.8 Data Reporting

Data reporting for the 40 C.F.R. § 257 monitoring program will be consistent with recordkeeping, notification, and internet posting requirements described in 40 C.F.R. § 257.105 through 257.107.

Groundwater monitoring and analysis completed in accordance with the Part 845 monitoring under an approved monitoring program will be reported to IEPA within 60 days after completion of sampling and the data placed in the facility's operating record as required by 35 I.A.C. § 845.610(b)(3)(D). Within 14 days of posting to the operating record, information will be posted to the publicly accessible internet site "Illinois CCR Rule Compliance Data and Information" as required by 35 I.A.C. § 845.810(d). Information will also be submitted to IEPA annually by January 31 as required by 35 I.A.C. § 845.550, for data collected the preceding year. The annual report will include the status of the groundwater monitoring and any required corrective action plan for the GMP GSP in addition to other requirements detailed in 35 I.A.C. § 845.610(e).

4.9 Compliance with Applicable On-site Groundwater Protection Standards

In accordance with 35 I.A.C. § 845.600(a)(1), the groundwater protection standard at the waste boundary will be the higher of either the 35 I.A.C. § 845.600 standard or the concentration determined by background groundwater monitoring.

As provided in 35 I.A.C. § 845.780(c)(2), at the end of the 30-year post-closure care period, groundwater monitoring will continue to be conducted in post-closure care until the groundwater results show the concentrations are:

- Below the GWPS in 35 I.A.C. § 845.600; and
- Not increasing for those constituents over background, using the statistical procedures and performance standards in 35 I.A.C. § 845.640(f) and (g), provided that:
 - Concentrations have been reduced to the maximum extent feasible; and
 - Concentrations are protective of human health and the environment.

Following detection of an exceedance of the GWPS, an Alternate Source Demonstration (ASD) will be evaluated as described in **Section 4.10**.

4.10 Alternate Source Demonstrations

As allowed in 35 I.A.C. § 845.650(e), following detection of an exceedance of the GWPS, an ASD will be evaluated and, if completed, submitted to IEPA within 60 days. The ASD will provide lines of evidence that a source other than the GMF GSP caused the contamination and the GMF GSP did not contribute to the contamination, or that the exceedance of the GWPS resulted from error in sampling, analysis, statistical evaluation, natural variation in groundwater quality, or a change in the potentiometric surface and groundwater flow direction.

The ASD will include information and analysis that supports the conclusions and a certification of accuracy by a qualified professional engineer. Once the ASD is approved by IEPA, the Part 845 groundwater monitoring will continue as defined in **Section 4.1.3**.

If an ASD is not completed and submitted, or IEPA does not approve the ASD, a notification of the exceedance will be provided to IEPA and placed in the operating record. Additional actions will also be completed as required by 35 I.A.C § 845.650(d)(1) through (3), including initiation of an assessment of corrective measures under 35 I.A.C § 845.660. As allowed in 35 I.A.C § 845.650(e)(7) a petition for review of IEPA's non-concurrence under 35 I.A.C. § 105 may also be filed.

4.11 Assessment of Corrective Measures and Corrective Action

As described in 35 I.A.C. § 845.660, if the ASD summarized in **Section 4.10** has not been approved by IEPA, an assessment of corrective measures will be initiated within 90 days of the detection of a result exceeding 35 I.A.C. § 845.600 standards (*i.e.*, receipt of laboratory data). The assessment of corrective measures will include at least the following (35 I.A.C. § 845.660 (c)):

- The performance, reliability, ease of implementation, and potential impacts of appropriate potential remedies, including safety impacts, cross-media impacts, and control of exposure to any residual contamination;
- The time required to begin and complete the corrective action plan; and
- The institutional requirements, such as State or local permit requirements or other environmental or public health requirements that may substantially affect implementation of the corrective action plan.

Within one year of completing the assessment of corrective measures, a corrective action plan will be developed to identify the selected remedy in accordance with 35 I.A.C. § 845.670. If closure of the CCR Unit is required, a closure alternatives analysis will be completed as specified in 35 I.A.C. § 845.710. The analysis and selected alternative will be submitted to IEPA in a Closure Plan as specified by 35 I.A.C. § 845.720. Groundwater monitoring proposed in this Addendum will continue as specified until the post closure care period has expired and IEPA has approved termination of post-closure care.

5. REFERENCES

Illinois Environmental Protection Agency, 2021. *Standards for the Disposal of Coal Combustion Residuals in Surface Impoundments: Title 35 of the Illinois Administrative Code § 845*, April 15, 2021.

Natural Resource Technology, an OBG Company (NRT/OBG), 2017. *Sampling and Analysis Plan, Coffeen GMF Gypsum Stack Pond, Coffeen Power Station, Coffeen, Illinois, Project No. 2285, Revision 0, October 17, 2017.*

Ramboll Americas Engineering Solutions, Inc. (Ramboll), 2021. *Hydrogeologic Site Characterization Report. Coffeen GMF Gypsum Stack Pond, Coffeen Power Plant. Coffeen, Illinois.*

United States Environmental Protection Agency (USEPA), March 2009. *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities – Unified Guidance*. Office of Resource Conservation and Recovery, Program Implementation and Information Division, United States Environmental Protection Agency, Washington D.C. EPA/530/R-09/007.

TABLES

TABLE 1-1. PART 845 REQUIREMENTS CHECKLIST

GROUNDWATER MONITORING PLAN

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Part 845 Reference	Part 845 Components	Location of Information in GMP
845.630	Groundwater Monitoring Systems	
845.630(a)(2)	Potential contaminant pathways must be monitored.	Sections 2.2 & 4.1.3
845.630(a) 845.630(b) 845.630(c)	At least two upgradient wells and four downgradient wells (min. 1 and 3, but requires additional documentation)	Sections 2.2 & 4.1.3 Table 2-1 Figure 2-1
845.630(a) 845.630(b) 845.630(c)	Downgradient Well Density	Figure 2-1
845.630(a)(2)	Downgradient wells at waste boundary	Figure 2-1
845.640	Groundwater Sampling and Analysis Requirements	
845.640(a)	Consistent sampling and analysis procedures	Section 4 Tables 4-1 & 4-2
845.640(b)	Methods are appropriate	Section 4 Tables 4-1 & 4-2
845.640(c)	Groundwater elevations must be measured in each well prior to purging, each time groundwater is sampled.	Section 4.3
845.640 (d)(e)(f)(g)(h)	Establishment of background and application of statistical methods	Sections 3.2 & 4.7 Appendix A
845.640(i)	Analyze total recoverable metals	Section 4.1.3
845.640(j)	Analyze groundwater samples using a certified laboratory	Section 4.4

TABLE 1-1. PART 845 REQUIREMENTS CHECKLIST

GROUNDWATER MONITORING PLAN

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Part 845 Reference	Part 845 Components	Location of Information in GMP
845.650	Groundwater Monitoring Program	
845.650(a)	Must include monitoring for all constituents with a groundwater protection standard in Section 845.600(a), calcium, and turbidity	Section 4.1.3
845.650(b)(c)	Groundwater Monitoring Frequency	Sections 4.1.3 & 4.2
845.650(d)(e)	Exceedances of the groundwater protection standard	Sections 4.9, 4.10 & 4.11
845.650(b)(2) and (3)	Staff gauge/ piezometer to monitor head in impoundment	Sections 2.2 & 4.1.3 Figure 2-1 (NE Riser)
NA	Staff gauge/ piezometer to monitor head of neighboring surface water body	Sections 2.2 & 4.1.3 Figure 2-1 (SG-04)

[O: CJC 09/02/21; C: SSW 09/16/21]

Notes:

GMP = Groundwater Monitoring Plan

NA = Not Applicable

TABLE 2-1. MONITORING WELL LOCATIONS AND CONSTRUCTION DETAILS

GROUNDWATER MONITORING PLAN
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND
 COFFEEN, ILLINOIS

Well Number	Type	HSU	Date Constructed	Top of PVC Elevation (ft)	Measuring Point Elevation (ft)	Measuring Point Description	Ground Elevation (ft)	Screen Top Depth (ft BGS)	Screen Bottom Depth (ft BGS)	Screen Top Elevation (ft)	Screen Bottom Elevation (ft)	Well Depth (ft BGS)	Bottom of Boring Elevation (ft)	Screen Length (ft)	Screen Diameter (inches)	Latitude (Decimal Degrees)	Longitude (Decimal Degrees)
G200	B	UA	02/25/2008	--	625.94	Top of Disk	623.27	12.19	16.98	611.08	606.29	17.36	605.30	4.8	2	39.075139	-89.395009
G206	C	UA	10/14/2010	--	632.82	Top of Disk	630.53	17.51	21.92	613.02	608.61	22.42	606.50	4.4	2	39.067399	-89.398548
G206D	C	DA	01/25/2021	634.14	634.14	Top of PVC	631.41	49.20	59.00	582.21	572.41	59.39	571.41	9.8	2	39.067428	-89.398493
G209	C	UA	10/07/2010	--	632.91	Top of Disk	630.57	17.74	22.28	612.83	608.29	22.81	606.60	4.5	2	39.067923	-89.39685
G212	C	UA	10/11/2010	--	632.89	Top of Disk	630.59	16.74	21.29	613.85	609.30	21.81	606.60	4.6	2	39.06843	-89.395318
G213	C	UA	10/12/2010	--	632.81	Top of Disk	630.34	16.75	21.29	613.59	609.05	21.82	606.30	4.5	2	39.068585	-89.394822
G215	C	UA	10/13/2010	--	633.06	Top of Disk	630.48	19.41	23.80	611.07	606.68	24.31	606.20	4.4	2	39.069309	-89.39394
G217	C	UA	10/12/2010	--	633.10	Top of Disk	630.67	20.49	24.88	610.18	605.79	25.38	604.70	4.4	2	39.07034	-89.393959
G218	C	UA	10/12/2010	--	633.11	Top of Disk	630.64	20.33	24.77	610.31	605.87	25.27	604.60	4.4	2	39.070876	-89.393956
R201	B	UA	10/08/2010	--	626.34	Top of Disk	624.02	14.59	19.32	609.43	604.70	19.85	604.20	4.7	2	39.075142	-89.397855
NE Riser	WLO	S	--	--	626.13	--	--	--	--	--	--	--	--	--	--	39.071111	-89.393889
SG-04	WLO	SW	--	--	599.52	Top of Prot Casing	599.52	--	--	--	--	--	--	--	--	39.064146	-89.390504

Notes:

All elevation data are presented relative to the North American Vertical Datum 1988 (NAVD88), GEOID 12A
 Type refers to the role of the well in the monitoring network: background (B), compliance (C), or water level measurements only (WLO)
 WLO wells are temporary pending implementation of impoundment closure per an approved Construction Permit application
 -- = data not available
 BGS = below ground surface
 DA = deep aquifer
 ft = foot or feet
 HSU = Hydrostratigraphic Unit
 PVC = polyvinyl chloride
 S = source water
 SW = surface water
 UA = uppermost aquifer

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TABLE 3-1. BACKGROUND GROUNDWATER QUALITY AND STANDARDS
GROUNDWATER MONITORING PLAN
COFFEEN POWER PLANT
GMF GYPSUM STACK POND
COFFEEN, ILLINOIS

Parameter	Background Concentration	845 Limit	Groundwater Protection Standard	Unit
Antimony, total	0.003	0.006	0.006	mg/L
Arsenic, total	0.011	0.010	0.011	mg/L
Barium, total	0.13	2.0	2.0	mg/L
Beryllium, total	0.001	0.004	0.004	mg/L
Boron, total	0.11	2	2	mg/L
Cadmium, total	0.001	0.005	0.005	mg/L
Chloride, total	94.9	200	200	mg/L
Chromium, total	0.0096	0.1	0.1	mg/L
Cobalt, total	0.0037	0.006	0.006	mg/L
Fluoride, total	0.552	4.0	4.0	mg/L
Lead, total	0.0059	0.0075	0.0075	mg/L
Lithium, total	0.02	0.04	0.04	mg/L
Mercury, total	0.0011	0.002	0.002	mg/L
Molybdenum, total	0.044	0.1	0.1	mg/L
pH (field)	7.4 / 6.8	9.0 / 6.5	9.0 / 6.5	SU
Radium 226 and 228 combined	1.48	5	5	pCi/L
Selenium, total	0.0035	0.05	0.05	mg/L
Sulfate, total	387	400	400	mg/L
Thallium, total	0.001	0.002	0.002	mg/L
Total Dissolved Solids	975	1200	1200	mg/L

Notes:

For pH, the values presented are the upper / lower limits
Groundwater protection standards for calcium and turbidity do not apply per 35 I.A.C. § 845.600(b)
mg/L = milligrams per liter
SU = standard units
pCi/L = picocuries per liter

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TABLE 4-1. SAMPLING AND ANALYSIS SUMMARY

GROUNDWATER MONITORING PLAN
 COFFEEN POWER PLANT
 GMF GYPSUM STACK POND
 COFFEEN, ILLINOIS

Parameter	Analytical Method ¹	Number of Samples	Field Duplicates ²	Field Blanks ³	Equipment Blanks ³	MS/MSD ⁴	Total	Container Type	Minimum Volume ⁵	Preservation (Cool to 4 °C for all samples)	Sample Hold Time from Collection Date
Metals											
Metals ⁶	6020, Li - EPA 200.7	10	1	0	0	1	12	plastic	600 mL	HNO ₃ to pH<2	6 months
Mercury	7470A or 6020	10	1	0	0	1	12	plastic	400 mL	HNO ₃ to pH<2	28 days
Inorganic Parameters											
Fluoride	9214 or EPA 300	10	1	0	0	1	12	plastic	300 mL	Cool to 4 °C	28 days
Chloride	9251 or EPA 300	10	1	0	0	1	12	plastic	100 mL	Cool to 4 °C	28 days
Sulfate	9036 or EPA 300	10	1	0	0	1	12	plastic	50 mL	Cool to 4 °C	28 days
Total Dissolved Solids	SM 2540 C	10	1	0	0	1	12	plastic	200 mL	Cool to 4 °C	7 days
Radium											
Radium 226	9315 or EPA 903	10	0	0	0	0	10	plastic	1000 mL	HNO ₃ to pH<2	6 months
Radium 228	9320 or EPA 904	10	0	0	0	0	10	plastic	1000 mL	HNO ₃ to pH<2	6 months
Field Parameters											
pH	SM 4500-H+ B	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Dissolved Oxygen ⁸	SM 4500-O/405.1	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Temperature ⁸	SM 2550	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Oxidation/Reduction Potential ⁸	SM 2580 B	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Specific Conductance ⁸	SM 2510 B	10	NA	NA	NA	NA	10	flow-through cell	NA	none	immediately
Turbidity ⁷	SM 2130 B	10	NA	NA	NA	NA	10	flow-through cell or hand-held turbidity meter	NA	none	immediately

[O: CJC 09/02/21; C: SSW 09/16/21]

Notes:

- ¹ Analytical method numbers are from SW-846 unless otherwise indicated. Analytical methods may be updated with more recent versions as appropriate.
 - ² Field duplicates will be collected at a frequency of one per group of 10 or fewer investigative water samples. Field duplicates will not be collected for radium analysis.
 - ³ Field blanks will be collected at the discretion of the project manager; Equipment blanks will be collected at a rate of 1 per sampling event if non-dedicated equipment is used.
 - ⁴ Matrix Spike/Matrix Spike Duplicate (MS/MSD) samples will be collected at a frequency of one per group of 20 or fewer investigative water samples per CCR unit/multi-unit. Additional volume to be determined by laboratory.
 - ⁵ Sample volume is estimated and will be determined by the laboratory.
 - ⁶ Metals = antimony, arsenic, barium, beryllium, boron, cadmium, calcium, chromium, cobalt, lead, lithium, molybdenum, selenium, thallium. Metals may be analyzed via ICP/ ICP-MS USEPA methods 6010 or 6020 depending on laboratory instrument availability
 - ⁷ If turbidity exceeds 10 NTUs, a duplicate sample filtered through a .45 micron filter may be collected for metals analysis in addition to the unfiltered sample. Both samples would be submitted for analysis.
 - ⁸ Parameter collected for quality assurance and quality control for field sampling purposes only; not required to be collected or reported under Part 845; collection of parameter may be discontinued without notification.
- < = less than
 °C = degrees Celsius
 HNO₃ = nitric acid
 mL = milliliter
 NA = not applicable
 NTU = nephelometric turbidity unit

TABLE 4-2. DETECTION AND REPORTING LIMITS FOR PART 845 PARAMETERS

GROUNDWATER MONITORING PLAN

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Constituent	CAS	Unit	Analytical Methods ¹	USEPA MCL ²	35 I.A.C. § 845.600	RL ^{4,5}	MDL ⁵
Metals							
Antimony	7440-36-0	mg/L	6020	0.006	0.006	0.003	0.00036
Arsenic	7440-38-2	mg/L	6020	0.01	0.01	0.001	0.00013
Barium	7440-39-3	mg/L	6020	2	2	0.001	0.00028
Beryllium	7440-41-7	mg/L	6020	0.004	0.004	0.001	0.000017
Boron	7440-42-8	mg/L	6020	NS	2	0.01	0.0023
Cadmium	7440-43-9	mg/L	6020	0.005	0.005	0.001	0.000042
Calcium	7440-70-2	mg/L	6020	NS	NS	0.15	0.15
Chromium	7440-47-3	mg/L	6020	0.1	0.1	0.004	0.00027
Cobalt	7440-48-4	mg/L	6020	0.006	0.006	0.002	0.000017
Lead	7439-92-1	mg/L	6020	0.015	0.0075	0.001	0.000025
Lithium	7439-93-2	mg/L	6020 or EPA 200.7	0.04	0.04	0.02	0.0001
Mercury	7439-97-6	mg/L	6020 or 7470A	0.002	0.002	0.0002	0.000078
Molybdenum	7439-98-7	mg/L	6020	0.1	0.1	0.001	0.000063
Selenium	7782-49-2	mg/L	6020	0.05	0.05	0.001	0.00032
Thallium	7440-28-0	mg/L	6020	0.002	0.002	0.001	0.000062
Inorganics							
Fluoride	7681	mg/L	9214 or EPA 300	4	4	0.25	0.065
Chloride	16887-00-6	mg/L	9251 or EPA 300	250 ³	200	1	0.15
Sulfate	18785-72-3	mg/L	9036 or EPA 300	250 ³	400	1	0.24
Total Dissolved Solids	10052	mg/L	SM 2540C	500 ³	1200	17	--
Other							
Radium 226 and 228 combined	7440-14-4	pCi/L	9315/9320 or EPA 903/904	5	5	-- ⁶	-- ⁷

TABLE 4-2. DETECTION AND REPORTING LIMITS FOR PART 845 PARAMETERS

GROUNDWATER MONITORING PLAN

COFFEEN POWER PLANT

GMF GYPSUM STACK POND

COFFEEN, ILLINOIS

Constituent	CAS	Unit	Analytical Methods ¹	USEPA MCL ²	35 I.A.C. § 845.600	RL ^{4, 5}	MDL ⁵
Field							
pH	NA	SU	SM 4500-H+ B	NS	6.5-9.0	NA	NA
Oxidation/Reduction Potential	NA	mV	SM 2580 B	NS	NS	NA	NA
Dissolved Oxygen	NA	mg/L	SM 4500-O/405.1	NS	NS	NA	NA
Temperature	NA	°C	SM 2550	NS	NS	NA	NA
Specific Conductivity	NA	µS/cm	SM 2510 B	NS	NS	NA	NA
Turbidity	NA	NTU	SM 2130 B	NS	NS	NA	NA

[O: CJC 09/02/21; C: SSW 09/16/21]

Notes:

¹ Analytical method numbers are from SW-846 unless otherwise indicated. Metals will be analyzed via Method 6020 or 6010 depending on laboratory equipment availability. Selected method will ensure reporting limits (RL) are below Title 35 of the Illinois Administrative Code (35 I.A.C.) § 845.600 groundwater protection standards.

² USEPA MCL = United States Environmental Protection Agency Maximum Contaminant Level.

³ USEPA SMCL = United States Environmental Protection Agency Secondary Maximum Contaminant Level.

⁴ RLs will be less than the 35 I.A.C. § 845.600 groundwater protection standards.

⁵ RLs and method detection limits (MDL) will vary depending on the laboratory performing the work.

⁶ All radium results will be reported (values may be positive or negative) and will include uncertainty and the calculated MDC.

⁷ Laboratories calculate a minimum detectable concentration (MDC) based on the sample.

°C = degrees Celsius

µS/cm = microSiemens per centimeter

CAS = Chemical Abstract Number

MDL = Method detection limit as established by the laboratory

mg/L = milligrams per liter

mV = millivolts

NS = No standard

NTU = nephelometric turbidity unit

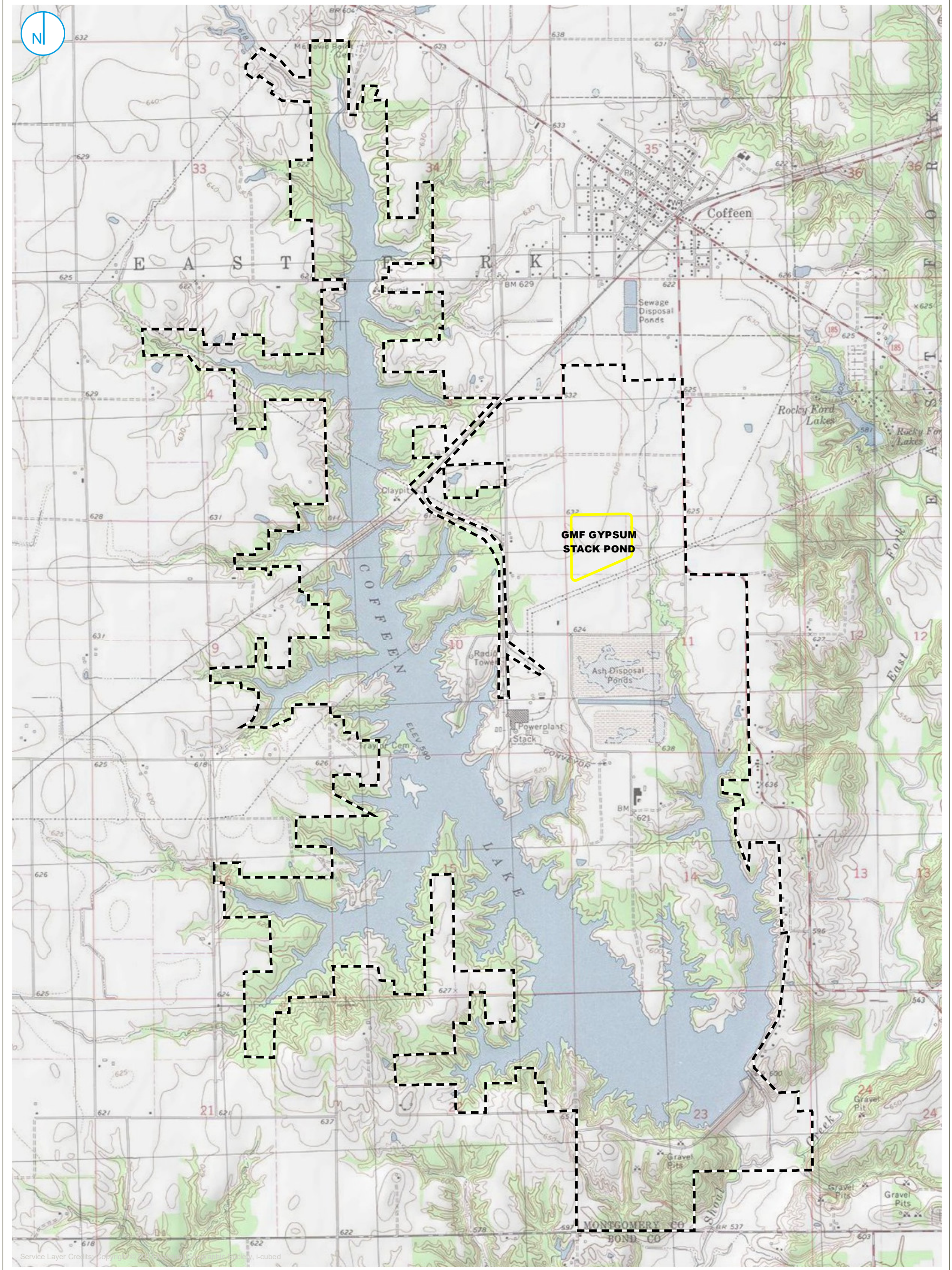
pCi/L = picoCuries per liter

RL = Reporting limit as established by the laboratory

SM = Standard Methods for the Examination of Water and Wastewater

SU = standard units

FIGURES



PART 845 REGULATED UNIT (SUBJECT UNIT)
 PROPERTY BOUNDARY

SITE LOCATION MAP

FIGURE 1-1

0 1,000 2,000
Feet

GROUNDWATER MONITORING PLAN
GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.





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- COAL MINE SHAFT
- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- ▨ LIMITS OF FINAL COVER
- ▭ PROPERTY BOUNDARY

0 275 550 Feet

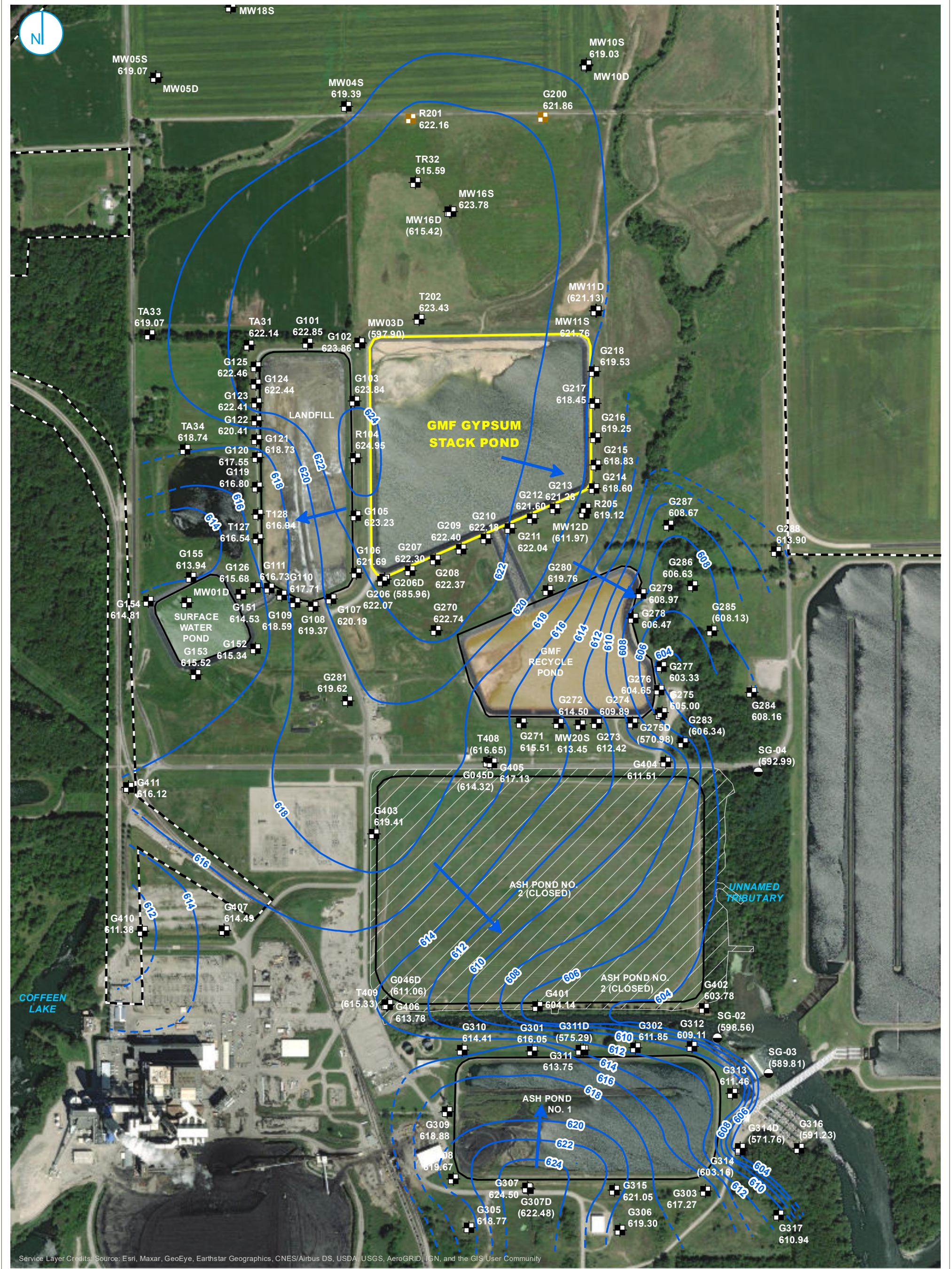
SITE MAP

FIGURE 1-2

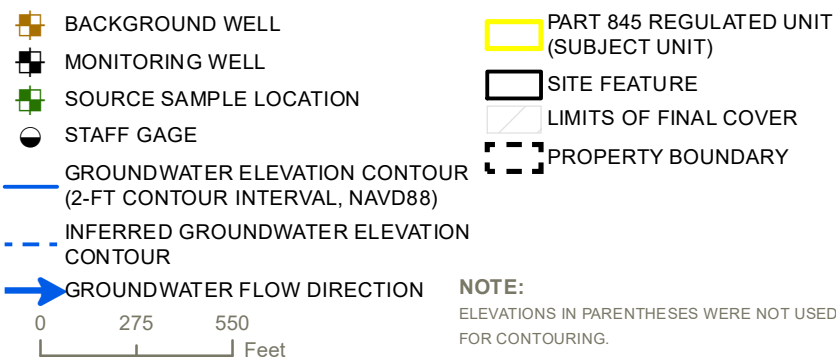
GROUNDWATER MONITORING PLAN
GMF GYPSUM STACK POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

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UPPERMOST AQUIFER GROUNDWATER ELEVATION CONTOURS
APRIL 20, 2021

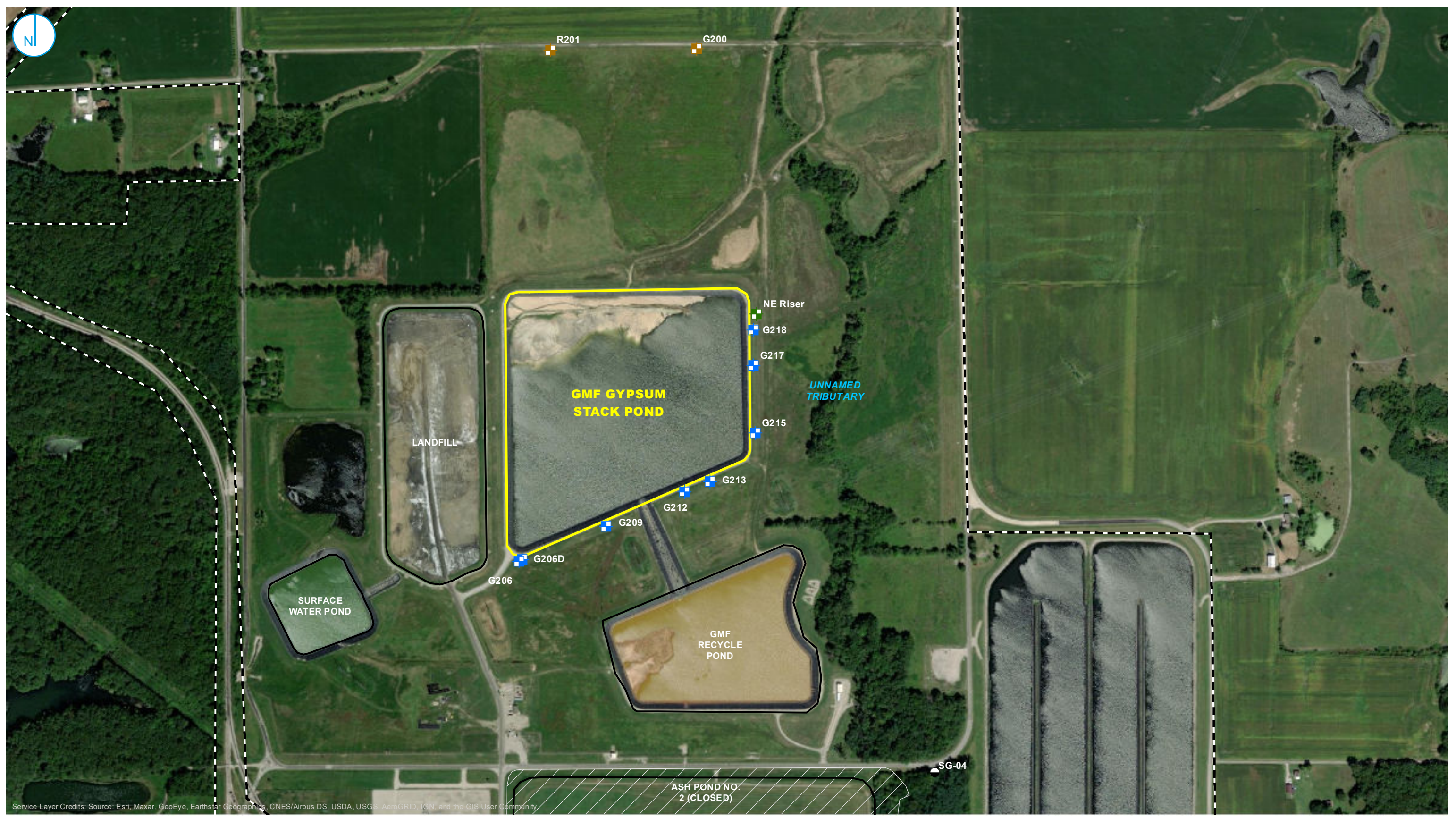
GROUNDWATER MONITORING PLAN
GMF GYPSUM STACK POND
COFFEEN POWER PLANT
COFFEEN, ILLINOIS

FIGURE 1-3

RAMBOLL AMERICAS
ENGINEERING SOLUTIONS, INC.



PROJECT: 169000XXXX | DATED: 10/12/2021 | DESIGNER: HOTCALD
 Y:\Mapping\Projects\22\2285\MXD\845_Operating_Permit\Coffeen\GMF_GSP\GMF\Figure 2-1_Proposed Monitoring Well Network.mxd



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- BACKGROUND WELL
- COMPLIANCE WELL
- SOURCE SAMPLE LOCATION
- STAFF GAGE
- PART 845 REGULATED UNIT (SUBJECT UNIT)
- SITE FEATURE
- LIMITS OF FINAL COVER
- PROPERTY BOUNDARY



PROPOSED PART 845 GROUNDWATER MONITORING WELL NETWORK

GROUNDWATER MONITORING PLAN
 GMF GYPSUM STACK POND
 COFFEEN POWER PLANT
 COFFEEN, ILLINOIS

FIGURE 2-1

RAMBOLL AMERICAS
 ENGINEERING SOLUTIONS, INC.



**APPENDIX A
STATISTICAL ANALYSIS PLAN**

Prepared for
Illinois Power Generating Company

Date
October 25, 2021

Project No.
1940100806-002

STATISTICAL ANALYSIS PLAN

GMF GYPSUM STACK POND COFFEEN POWER PLANT COFFEEN, ILLINOIS

STATISTICAL ANALYSIS PLAN COFFEEN POWER PLANT GMF GYPSUM STACK POND

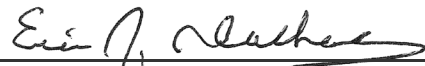
Project Name **Coffeen Power Plant GMF Gypsum Stack Pond**
Project No. **1940100806-002**
Recipient **Illinois Power Generating Company**
Document Type **Statistical Analysis Plan**
Version **FINAL**
Date **October 25, 2021**

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Senior Managing Hydrogeologist



Eric J. Tlachac, PE
Senior Managing Engineer



Rachel A. Banoff, EIT
Project Statistician


LICENSED PROFESSIONAL CERTIFICATIONS

This certification is based on the description of the statistical methods selected to evaluate groundwater as presented in the following Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond. The procedures described in the plan will be used to establish background conditions and implement compliance monitoring as necessary and required by 35 I.A.C. § 845.640 and 35 I.A.C. § 845.650. The Statistical Analysis Plan was prepared in accordance with the requirements of 35 I.A.C. § 845.640(f), with reference to the acceptable statistical procedures provided in the United States Environmental Protection Agency (USEPA)'s *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Unified Guidance (Unified Guidance, March 2009)*, and is intended to provide a logical process and framework for conducting the statistical analysis of the data obtained during groundwater monitoring. In accordance with 35 I.A.C. § 845.640(f)(1), the statistical method chosen for analysis of background groundwater quality will be either the tolerance interval or the prediction interval procedure for each constituent listed in 35 I.A.C. § 845.600(a)(1) at this CCR unit per 35 I.A.C. § 845.640(f)(1)(C). Groundwater Protection Standards (GWPS) will be established in accordance with 35 I.A.C. § 845.600(a) (greater of the background concentration or numerical limit specified in 35 I.A.C. § 845.600(a)(1)). The GWPS will be compared to the lower confidence limit for the observed concentrations for each constituent in each compliance well. Consistent with the *Unified Guidance*, the same general statistical method of confidence interval testing against a fixed GWPS is recommended in compliance and corrective action programs. Confidence intervals provide a flexible and statistically accurate method to test how a parameter estimated from a single sample compares to a fixed numerical limit. Confidence intervals explicitly account for variation and uncertainty in the sample data used to construct them.

Description of the statistical methods chosen for analysis of groundwater monitoring data and application of these methods for determining exceedances of the GWPS identified in 35 I.A.C. § 845.600(a) is provided in this Statistical Analysis Plan.

35 I.A.C. § 845.640 Statistical Analysis (PE)

I, Eric J. Tlachac, a qualified professional engineer in good standing in the State of Illinois, certify that the statistical methods summarized above and described in this document (Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond) are appropriate for evaluating the groundwater monitoring data collected as described in the attached document and are in substantial compliance with 35 I.A.C. § 845.640.



Eric J. Tlachac
Qualified Professional Engineer
062-063091
Illinois
Date: October 25, 2021



35 I.A.C. § 845.640 Statistical Analysis (PG)

I, Brian G. Hennings, a qualified professional geologist in good standing in the State of Illinois, certify that the statistical methods described in this document (Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond) are appropriate for evaluating the groundwater monitoring data collected as described in the attached document and are in substantial compliance with 35 I.A.C. § 845.640.

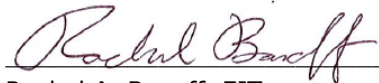


Brian G. Hennings
Professional Geologist
196.001482
Illinois
Date: October 25, 2021



35 I.A.C. § 845.640 Statistical Analysis

I, Rachel A. Banoff, a qualified professional, certify that the statistical methods described in this document (Statistical Analysis Plan; Coffeen Power Plant GMF Gypsum Stack Pond), are appropriate for evaluating the groundwater monitoring data collected as described in the attached document and are in substantial compliance with 35 I.A.C. § 845.640.



Rachel A. Banoff, EIT
Project Statistician
Date: October 25, 2021

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Table A	Statistical Calculations Used in Compliance Monitoring Procedures
---------	---

ACRONYMS AND ABBREVIATIONS

§	Section
35 I.A.C.	Title 35 of the Illinois Administrative Code
ANOVA	analysis of variance
CCR	coal combustion residuals
COC	constituents of concern
GWPS	groundwater protection standard
IEPA	Illinois Environmental Protection Agency
LCL	lower confidence limit
LTL	lower tolerance limit
MSE	mean squared error
P	probability
Part 845	Residuals in Surface Impoundments: Title 35 of the Illinois Administrative Code § 845
RCRA	Resource Conservation and Recovery Act
RL	reporting limit
ROS	regression on order statistics
SI	surface impoundment
SSI	statistically significant increase
SWFPR	site-wide false positive rate
<i>Unified Guidance</i>	<i>Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Unified Guidance (USEPA, 2009)</i>
UPL	upper prediction limit
USEPA	United States Environmental Protection Agency
UTL	upper tolerance limit

1. INTRODUCTION

In April 2021, the Illinois Environmental Protection Agency (IEPA) issued a final rule for the regulation and management of Coal Combustion Residuals (CCR) in surface impoundments (SIs) under the Standards for the Disposal of CCR in Surface Impoundments: Title 35 of the Illinois Administrative Code (35 I.A.C.) § 845 (Part 845). Facilities regulated under Part 845 are required to develop and sample a groundwater monitoring well network to evaluate whether impounded CCR materials are impacting downgradient groundwater quality. The groundwater quality evaluation must include selection and certification by a qualified professional engineer of the statistical procedures to be used. The procedures described in the evaluation will be used to establish background conditions and implement compliance and corrective action monitoring as necessary and required by 35 I.A.C. § 845.640 and 35 I.A.C. § 845.650. This Statistical Analysis Plan was prepared in accordance with the requirements of 35 I.A.C. § 845.640(f), with reference to the acceptable statistical procedures provided in United States Environmental Protection Agency's (USEPA's) *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Unified Guidance (Unified Guidance)* (March 2009).

This Statistical Analysis Plan does not include procedures for groundwater sample collection and analysis, as these activities are conducted in accordance with the Sampling and Analysis Plan prepared for each CCR unit in accordance with 35 I.A.C. § 845.640. This Statistical Analysis Plan will be used as the primary reference for evaluating groundwater quality during operation and post-closure care.

1.1 Statistical Analysis Objectives

This Statistical Analysis Plan is intended to provide a logical process and framework for conducting the statistical analyses of data obtained during groundwater monitoring conducted in accordance with the Sampling and Analysis Plan for each CCR unit. The Statistical Analysis Plan will enable a qualified professional engineer to certify that the selected statistical methods are appropriate for evaluating the groundwater monitoring data for the applicable CCR unit(s).

1.2 Statistical Analysis Plan Approach

The main sections of this Statistical Analysis Plan should be viewed as a "generic" outline of statistical methods utilized for each CCR unit and constituent required to be monitored. The statistical analysis of the groundwater monitoring data, however, will be conducted on an individual-constituent or well basis, and may involve the use of appropriate statistical procedures depending on multiple factors such as detection frequency and normality distributions.

The CCR Rule outlines two phases of groundwater monitoring:

- Background Monitoring in accordance with 35 I.A.C. § 845.650(b)(1)
- Compliance Monitoring in accordance with 35 I.A.C. § 845.650

Each phase of the groundwater monitoring program requires specific statistical procedures to accomplish the intended purpose. During the background monitoring phase, background groundwater quality will be established utilizing upgradient and background wells and downgradient groundwater quality data will be collected to facilitate statistics in subsequent phases. Compliance Monitoring is then initiated through the evaluation of the downgradient

groundwater monitoring data for exceedances of the groundwater protection standard (GWPS) established by Part 845 (concentration specified in 35 I.A.C. § 845.600 or an IEPA-approved background concentration). The developed statistical analysis plan will be implemented for each monitoring phase and in accordance with the statistical procedures.

2. BACKGROUND MONITORING AND DATA PREPARATION

The background and compliance monitoring wells were sampled and analyzed for constituents, as listed in Part 845 (antimony, arsenic, barium, beryllium, boron, cadmium, calcium, chloride, chromium, cobalt, fluoride, lead, lithium, mercury, molybdenum, pH, radium 226 and 228 combined, selenium, sulfate, thallium, total dissolved solids, and turbidity), during the baseline phase of the groundwater monitoring program.

The background monitoring well(s) were placed upgradient of the CCR unit, or at an alternative background location, where they are not affected by potential leakage from the CCR unit. Compliance monitoring wells were placed at the waste boundary of the CCR unit, along the same groundwater flow path. As 35 I.A.C. § 845.630(a) specifies, the location of these wells ensures that background accurately represents the quality of unaffected groundwater, while compliance wells accurately represent groundwater quality at the waste boundary and monitor all potential contaminant pathways.

As required by 35 I.A.C. § 845.650(a)(1), eight sampling events were completed within 180 days of April 21, 2021. As outlined, groundwater sampling procedures included sampling of the background and compliance wells using low-flow sampling methods, collection of one field quality control sample per event, and groundwater samples were not field filtered before laboratory analysis of total recoverable metals.

Following completion of the eight sampling events, background groundwater quality was established for Part 845 constituents. Groundwater monitoring will be conducted quarterly for at least the first five years. In accordance with 35 I.A.C. § 845.650(b)(4), after the first five years, a request to reduce the monitoring frequency to semiannual may be submitted to IEPA if all of the following can be demonstrated:

- Groundwater monitoring effectiveness will not be compromised by the reduced frequency
- Sufficient data has been collected to characterize groundwater
- Monitoring to date does not show any statistically significant increasing trends
- The concentrations of monitored constituents at the compliance monitoring wells are below the applicable GWPSs established in 35 I.A.C. § 845.600

The following subsections outline the statistical tests and procedures (methods) that will be utilized to evaluate data collected for each constituent in both background and compliance wells for Background and Compliance Monitoring. When necessary and contingent upon equivalent statistical power, an alternative test not included in this Statistical Analysis Plan may be chosen due to site-specific data requirements.

2.1 Sample Independence

Independence of sample results is a major assumption for most statistical analyses. To ensure physical independence of groundwater sampling results, the minimum time between sampling events must be longer than the time required for groundwater to move through the monitoring well. The sampling schedules for both the baseline and compliance monitoring periods are specified in 35 I.A.C. § 845.650(b) and may conflict with the statistical assumption of independence of sample results.

2.2 Non-Detect Data Processing

The reporting limit (RL) will be used as the lower level for the reporting of non-detected groundwater quality data. For all summary statistics (box plots, timeseries, etc.), the RL will be substituted for concentrations reported below the RL, including non-detects. With professional judgement, analytical results between the RL and the method detection limit, *i.e.*, estimated values, typically identified with a "J" flag, may be utilized if provided by the laboratory.

For all statistical test procedures:

- If the frequency of non-detect data are less than or equal to 15 percent, half of the RL will be substituted for these data
- If the non-detect frequency is between 15 percent and 50 percent, either the Kaplan-Meier or robust regression on order statistics (ROS) will be used to estimate the mean and standard deviation adjusted for the presence of left-censored values
- If the non-detect frequency is greater than 50 percent, a non-parametric test will be used
- If only one background result is detected that value will be used as the non-parametric upper prediction limit (UPL)

2.3 Testing for Normality

Many statistical analyses assume that sample data are normally distributed (parametric). However, environmental data are frequently not normally distributed (nonparametric). 35 I.A.C. § 845.640(g) requires the knowledge of the background data distribution for comparison to compliance results. The *Unified Guidance* document recommends the Shapiro-Wilk normality test for sample sizes of 50 or less, and the Shapiro-Francia normality test for sample sizes greater than 50.

When possible, transformation of datasets to achieve normal distributions is preferred.

2.4 Testing for Outliers

Part 845 constituents will be screened for the existence of outliers using a method described by the *Unified Guidance*. Outliers are extreme data points that may represent an anomaly or erroneous data point. To test for outliers, one or more of the following outlier tests will be utilized:

- Dixon's test, for well-constituent pairs with less than 25 samples, assumes normally distributed data.
- Rosner's test, for well-constituent pairs with more than 20 samples, assumes normally distributed data.
- Grubb's test for well-constituent pairs with seven or more samples, assumes normally distributed data.
- Time series, box-whisker plots, and probability plots provide visual tools to identify potential outliers, and evaluation of seasonal, spatial, or temporal variability for both normally and non-normally distributed data.

Data quality control, groundwater geochemistry, and sampling procedures will be evaluated as potential sources of error leading to an outlier result. The outlier tests cannot be used alone to determine whether a value is a true outlier that should be excluded from future statistical

analysis. Corroborating evidence needed to exclude values includes a discrete data reporting or analytical error, or potential laboratory bias. Absent corroborating evidence, the flagged values are considered true, but extreme, values in the data set. Professional judgement will be used to exclude extreme outliers from further statistical analyses. Outliers will be retained in the database.

With professional judgement, a confirmatory sample may be collected to allow for the distinction between an outlier and a true representation of groundwater quality at the monitoring point. If re-sampling is conducted, this sample will be collected within 90 days following outlier identification. If the confirmatory sample indicates the original result as an outlier, it will be reported as such.

2.5 Trend Analysis

Statistical analyses supporting the lack of trend are a fundamental step to confirm the assumption that groundwater quality values are stationary or constant over time at a CCR unit. These analyses allow for evaluation of variation in the background and compliance data for each constituent over time. A statistically significant increasing trend in background data could indicate an existing release from the CCR unit or alternate source, requiring further investigation. In addition, statistically significant trending background data can result in increased standard deviation and, therefore, greater prediction or control limits. Consequently, the increased prediction or control limit will have less power or ability to identify a release from the CCR unit.

A linear regression, coupled with a t-test for slope significance at a 95 percent confidence level (0.05 significance level), may be used on datasets for each constituent with few non-detects and a normally distributed variance of the mean to evaluate time trends. The Theil-Sen trend line, coupled with the Mann-Kendall test for slope significance at a 95 percent confidence level (0.05 significance level), will be used for datasets with frequent non-detects or non-normal variance. Similarly, trend analyses could also be used on compliance data to evaluate a possible release from the CCR unit.

2.6 Spatial Variation

Spatial trends and/or variation between background wells could indicate an existing release from a CCR unit. If the spatial variability is not due to an existing release, intrawell comparisons in compliance wells may be used to account for spatial variability and monitor for a future release. However, the CCR unit being monitored was placed into service prior to the start of groundwater monitoring and it is unknown whether a previous release has occurred. Accordingly, intrawell comparisons in compliance wells cannot be used to determine the occurrence of a future release. Interwell comparisons between compliance wells and background wells will be used.

2.7 Temporal Variation

Time series plots can be used to identify temporal dependence. Potentially significant temporal components of variability can be identified by graphing single constituent data from multiple wells together on a time series plot. With temporal dependence, the time series plot as a pattern of parallel traces, in which the individual wells will tend to rise and fall together across the sequence of sampling dates. Time series plots can be helpful by plotting multiple constituents over time for the same well, or averaging values for each constituent across wells on each sampling event and then plotting the averages over time. In either case, the plots can signify whether the general concentration pattern over time is simultaneously observed for different

constituents. If so, it may indicate that a group of constituents is highly correlated in groundwater or that the same artifacts of sampling and/or lab analysis impacted the results of several monitoring parameters.

Hydrologic factors such as drought, recharge patterns or regular (e.g., seasonal) water table fluctuations may be responsible for the temporal variation. In these cases, it may be useful to test for the presence of a significant temporal effect by first constructing a parallel time series plot and then running a formal one-way analysis of variance (ANOVA) ($\alpha = 0.05$) for temporal effects. A one-way ANOVA for temporal effects considers multiple well data sets for individual sampling events or seasons as the relevant statistical factor. If event-specific analytical differences or seasonality appear to be an important temporal factor, the one-way ANOVA for temporal effects can be used to formally identify seasonality, parallel trends, or changes in lab performance that affect other temporal effects. The one-way ANOVA for temporal effects assumes that the data groups are normally distributed with constant variance. It is also assumed that for each of a series of background wells, measurements are collected at each well on sampling events or dates common to all the wells. Results of the ANOVA can also be used to create temporally stationary residuals, where the temporal effect has been 'subtracted from' the original measurements. These stationary residuals may be used to replace the original data in subsequent statistical testing.

If the data cannot be normalized, a similar test for a temporal or seasonal effect can be performed using the Kruskal-Wallis test ($\alpha = 0.05$). Each sampling event should be treated as a separate 'well,' while each well is treated as a separate 'sampling event.' In this case, no residuals can be computed since the Kruskal-Wallis test employs ranks of the data rather than the measurements themselves.

Where both spatial and temporal variation occur, two-way ANOVA can be considered where both well location and sampling event/season are treated as statistical factors. This procedure is described in Davis (1994).

2.8 Updating Background

Updating the background dataset periodically by adding recent results to an existing background dataset can improve the statistical power and accuracy of the statistical analysis, especially for non-parametric prediction intervals. The *Unified Guidance* recommends updating statistical limits (background) when at least four to eight new measurements (every 1 to 2 years under a quarterly monitoring program), are available for comparison to historical data. Professional judgement will be used to evaluate whether any background data appear to be affected by a release and need to be excluded from a background update. A t-test for equal means (if normal data distribution) or appropriate non-parametric test (if non-normal data distribution) such as a Mann-Whitney (or Wilcoxon) rank-sum or box-whisker plots, will be conducted to evaluate whether the two groups of background sample populations are statistically different prior to updating any background datasets. A 0.05 significance level will be utilized when evaluating the two populations, with the null hypothesis that they are equivalent. In addition, time series graphs or other trend evaluation statistics will be conducted on the new background dataset to verify the absence of a release or changing groundwater quality. If the tests indicate that there are no statistical differences between the two background populations, the new data will be combined with the existing dataset. If the two populations are found to be different, the data will be reviewed to evaluate the cause of the difference. If the differences appear to be caused by a

release (if the new data are significantly higher, or lower for pH), then the previous background dataset may continue to be used. Furthermore, verified outliers will not be added to an existing background dataset. In accordance with the *Unified Guidance*, continual background updates will not be conducted due to the lack of sufficient samples for a statistical comparison.

3. COMPLIANCE MONITORING

Compliance monitoring is designed to monitor groundwater for evidence of a release by comparing Part 845 constituents in compliance wells to both background concentrations and the GWPS. Compliance Monitoring will begin the quarter following approval of this Groundwater Monitoring Plan and issuance of the Operating Permit. The selected Compliance Monitoring statistical method used to compare compliance groundwater quality data for each constituent to the GWPS will provide for adequate statistical power, error levels and individual test false positive rates, and be appropriate for the distribution and detection frequency of the background dataset. Statistical power is the ability of a statistical test to detect a true exceedance.

In accordance with 35 I.A.C. § 845.610(b)(3)(D), compliance monitoring statistical analyses will be completed and submitted to IEPA within 60 days after completion of sampling.

3.1 GWPS Establishment and Exceedance Determination

In accordance with 35 I.A.C. § 845.600(a), the GWPS will be the constituent concentrations specified in 35 I.A.C. § 845.600(a)(1) except for when the background concentration is greater, or no concentration is specified (*i.e.*, for calcium and turbidity), in which case the GWPS will be the background concentration. The GWPS based on background concentration will be calculated using a parametric upper tolerance limit (UTL), a parametric UPL for a future mean, or a non-parametric UPL for a future median.

Statistical calculations that will be utilized in Compliance Monitoring procedures are summarized in **Table A** below and listed in **Sections 3.1.1** through **3.1.7**. Depending on the distribution of the data and the percentage of non-detects, it may be more appropriate to use a parametric model over a non-parametric model. As necessary, other techniques as mentioned in the *Unified Guidance* and/or new methods will be implemented.

Table A. Statistical Calculations Used in Compliance Monitoring Procedures

Compliance Monitoring						
Significant Trend?	Background Data			Compliance Data		
	Percent Non-Detects	Distribution	GWPS Determination	Percent Non-Detects	Distribution	Method to Determine Exceedance
No	0 ≤ 50	Normal	35 I.A.C § 845.600(a)(1) constituent concentration or The Upper Tolerance Limit	≤75	Normal	Parametric Lower Confidence Limit around a Normal Mean
				≤75	Log-Normal	Parametric Lower Confidence Limit around a Lognormal Geometric Mean
				NA	Non-Normal	Non-Parametric Lower Confidence Limit around a Median
				>75	Unknown/ Cannot be determined	
	50 ≤ 70	Normal	The Upper Prediction Limit for a Future Mean	NA	NA	Future mean
	>70	Non-Normal	Upper Prediction Limit for a Future Median	NA	NA	Future median
100	Non-Normal	Double Quantification Rule	NA	NA	Individual Retesting Values	
Yes	0 ≤ 50	Normal	UCL of Confidence Band around Linear Regression	≤75	Residuals after subtracting trend are normal, equal variance	Lower Limit from Confidence Band around Linear Regression
	50 ≤ 100	Non-Normal	UCL of Confidence Band around Thiel-Sen trend line	≤75	Residuals not normal	Lower Limit from Confidence Band around Thiel-Sen

3.1.1 The Upper Tolerance Limit

The UTL will be used to calculate the GWPS when pooled background data are normally distributed, with a non-detect frequency of 50 percent or less. When non-detect frequency is 15 percent or less, half the RL will be substituted for non-detects. The *Unified Guidance* recommends 95 percent confidence level and 95 percent coverage (95/95 tolerance interval).

- When non-detect frequency is 15 percent or less, half the RL will be substituted for non-detects (simple substitution), and the normal mean and standard deviation will be calculated.

- The Kaplan-Meier or the ROS method will be used when the detection frequency is between 15 percent and 50 percent. The Kaplan-Meier method assesses the linearity of a censored probability plot to determine whether the background sample can be approximately normalized. If so, then the Kaplan-Meier method will be used to compute estimates of the mean and standard deviation adjusted for the presence of left-censored values. The Kaplan-Meier or ROS estimate of the mean and standard deviation will be substituted for the sample mean and standard deviation.
- If background normality cannot be achieved, non-parametric UTLs will not be calculated until a minimum of 60 background samples have been collected (to achieve 95 percent coverage).

The parametric UTL on a future mean will be calculated from the background dataset as follows:

$$UTL = \bar{x} + \kappa(n, \gamma, \alpha - 1) \cdot s$$

\bar{x} = background sample mean

s = background sample standard deviation

$\kappa(n, \gamma, \alpha - 1)$ = one-sided normal tolerance factor based on the chosen coverage (γ) and confidence level ($\alpha - 1$) and the size of the background dataset (n). Values are tabulated in Table 17-3 in Appendix D of the *Unified Guidance*. If exact values are not provided, then κ values can be estimated by linear interpolation.

If the UTL is constructed on the logarithms of original observations to achieve normality, where \bar{y} and s_y are the log-mean and log-standard deviation, the limit will be exponentiated for back-transformation to the concentration scale as follows:

$$UTL = \exp[\bar{y} + \kappa(n, \gamma, \alpha - 1) \cdot s_y]$$

\bar{y} = background sample log-mean

s_y = background sample log-standard deviation

When the GWPS is based on the 35 I.A.C. § 845.600(a)(1) constituent concentrations or a UTL derived from the background dataset, an exceedance in compliance wells relative to the GWPS will be evaluated using confidence intervals. A confidence interval defines the upper and lower bound of the true mean of a constituent concentration in groundwater within a specified confidence range.

- Non-detects in compliance data will be handled similarly to upgradient analyses, with half the RL substituted for non-detects when the frequency is 15 percent or less.
- The Kaplan-Meier, or the ROS method, will be used when the detection frequency is between 15 percent and 50 percent to compute estimates of the mean and standard deviation adjusted for the presence of left-censored values. These estimates will then be substituted for the sample mean and standard deviation.

Once the GWPS is established for background data using the UTL, either parametric or non-parametric confidence intervals will be computed for each constituent in compliance wells to identify GWPS exceedances.

3.1.2 Parametric Confidence Intervals around a Mean

If compliance data are approximately normal, one-sided parametric confidence intervals around a sample mean will be constructed for each constituent and well pair. The lower confidence limit (LCL) will be calculated as:

$$LCL_{1-\alpha} = \bar{x} - t_{1-\alpha, n-1} \cdot \frac{s}{\sqrt{n}}$$

\bar{x} = compliance sample mean

s = compliance sample standard deviation

n = compliance sample size

$t_{1-\alpha, n-1}$ = obtained from a Student's t-table with (n-1) degrees of freedom (Table 16-1 in Appendix D of the *Unified Guidance*)

The chosen t value will aim to achieve both a low false-positive rate, and high statistical power. Minimum α values are tabulated in Table 22-2 of Appendix D of the *Unified Guidance*. The selected minimum α value, from which the t value will be derived, will have at least 80 percent power ($1-\beta = 0.8$) when the underlying mean concentration is twice the GWPS.

If compliance data are distributed lognormally, the LCL will be computed around the lognormal geometric mean as:

$$LCL_{1-\alpha} = \exp\left(\bar{y} - t_{1-\alpha, n-1} \cdot \frac{s_y}{\sqrt{n}}\right)$$

\bar{y} = compliance sample log-mean

s_y = compliance sample log-standard deviation

3.1.3 Non-Parametric Confidence Intervals around a Median

Non-parametric confidence intervals around the median will be computed if the compliance data contain greater than 50 percent non-detects or are not normally distributed. The mathematical algorithm used to construct non-parametric confidence intervals is based on the probability (P) that any randomly selected measurement in a sample of n concentration measurements will be less than an unknown $P \times 100^{\text{th}}$ percentile of interest (where P is between 0 and 1). Then the probability that the measurement will exceed the $P \times 100^{\text{th}}$ percentile is $(1-P)$. The number of sample values falling below the $P \times 100^{\text{th}}$ percentile out of a set of n should follow a binomial distribution with parameters n and success probability P , where 'success' is defined as the event that a sample measurement is below the $P \times 100^{\text{th}}$ percentile. The probability that the interval formed by a given pair of order statistics will contain the percentile of interest will then be determined by a cumulative binomial distribution $Bin(x; n, p)$, representing the probability of x or fewer successes occurring in n trials with success probability p . P will be set to 0.50 for an interval around the median.

The sample size n will be ordered from least to greatest. Given $P = 0.50$, candidate interval endpoints will be chosen by ordered data values with ranks close to the product of $(n+1) \times 0.50$. If the result of $(n+1) \times 0.50$ is a fraction (for even-numbered sample sizes), the rank values immediately above and below will be selected as possible candidate endpoints. If the result of $(n+1) \times 0.50$ is an integer (for odd-numbered sample sizes), one will be added to and subtracted

from the result to get the upper and lower candidate endpoints. The ranks of the endpoints will be denoted L^* and U^* . For a one-sided LCL, the confidence level associated with endpoint L^* will be computed as:

$$1 - \alpha = \text{Bin}(L^* - 1; n, 0.50) = \sum_{x=L^*}^n \binom{n}{x} \left(\frac{1}{2}\right)^n$$

If the candidate endpoint(s) do not achieve the desired confidence level, new candidate endpoints (L^*-1) and (U^*+1) and achieved confidence levels will be calculated. If one candidate endpoint equals the data minimum or maximum, only the rank of the other endpoint will be changed. Achievable confidence levels are tabulated using these equations in Table 21-11 in Appendix D of the *Unified Guidance*.

Both parametric and non-parametric confidence limits will then be compared to the GWPS. The CCR unit is considered to be in compliance if the LCL is equal to or lower than the GWPS for all detected constituents at all compliance monitoring wells. A GWPS exceedance is determined if the LCL exceeds the GWPS.

3.1.4 The Upper Prediction Limit for a Future Mean

The parametric UPL for a future mean will be used to calculate the GWPS if the pooled background data contain 50 to 70 percent non-detects and normality can be achieved. The Kaplan-Meier or ROS methods will be used to estimate the mean and standard deviation. The non-parametric UPL for a future median will be calculated as the GWPS if background samples cannot be normalized or contain greater than 70 percent non-detects. The parametric UPL for a future mean will be calculated from the background dataset at follows:

$$UPL_{1-\alpha} = \bar{x} + \kappa s$$

\bar{x} = background sample mean

s = background standard deviation

κ = multiplier based on the order (p) of the future mean to be predicted, the number of compliance wells to be tested (w), the background sample size (n) the number (c) of constituents of concern (COCs), the "1-of- m " retesting scheme, and the evaluation schedule (annual, semi-annual, quarterly). Values are tabulated in 19-5 to 19-9 in Appendix D of the *Unified Guidance*.

The mean of order p will be computed for each well and compared against the UPL. For any compliance point mean that exceeds the limit, p additional resamples may be collected at that well for a 1-of-2 retesting scheme. Resample means will then be compared to the UPL. A GWPS exceedance has been deemed to occur at a compliance well when the initial mean and all resample means exceed the UPL.

3.1.5 The Non-Parametric Upper Prediction Limit for a Future Median

The non-parametric UPL for a future median will be used to calculate the GWPS if the pooled background data contain greater than 70 percent non-detects and normality cannot be achieved. Non-parametric methods assume that the data does not have an underlying distribution. To calculate the non-parametric UPL on a future value, the target per-constituent false positive rate (a_{const}) will be determined as follows:

$$\alpha_{const} = 1 - (1 - \alpha)^{1/c}$$

α = the site-wide false positive rate (SWFPR) of 0.10 recommended by the *Unified Guidance*

c = the number of monitoring constituents

The number of yearly statistical evaluation (nE) will be multiplied by the number of compliance wells (w) to determine the look-up table entry, w^* . The background sample size (n) and w^* will be used to select an achievable per-constituent false positive rate value in Table 19-24 of Appendix D in the *Unified Guidance*. The chosen achievable per-constituent false positive rate value will determine the type of non-parametric prediction limit (maximum or 2nd highest value in background) and a retesting scheme for a future median. The background data will be sorted in ascending order, and the upper prediction limit will be set to the appropriate order statistic previously determined by the achievable per-constituent false positive rate value in Table 19-24. If all constituent measurements in a background sample are non-detect, the Double Quantification rule will be used. The use of the Double Quantification rule in Compliance Monitoring will only be applicable if the RL is above the 35 I.A.C. § 845.600(a)(1) constituent concentration or a constituent concentration is not specified in § 845.600(a)(1). This scenario is highly unlikely. The constituent will also be removed from calculations identifying the target false positive rate.

Two initial measurements per compliance well will be collected. If both do not exceed the upper prediction limit, a third initial measurement will not be collected since the median of order 3 will also not exceed the limit. If both exceed the prediction limit, a third initial measurement will not be collected since the median will also exceed the limit. If one initial measurement is above and one below the limit, a third initial observation may be collected to determine the position of the median relative to the UPL. Up to three resamples will be collected in order to assess the resample median. In all cases, if two or more of the compliance point observations are non-detect, the median will be set equal to the RL. The median value for each compliance well will be compared to the UPL. For the 1-of-2 retesting scheme, if any compliance point median exceeds the limit, up to three additional resamples will may be collected from that well. The resample median will be computed and compared to the UPL. A GWPS exceedance has been deemed to occur at a compliance well when either the initial median, or both the initial median and resample median exceed the UPL.

If the concentrations of detected constituents are below the established GWPS, Compliance Monitoring will continue.

3.1.6 Parametric Linear Regression and Confidence Band

If the t-test detects a significant trend in the parametric linear regression line using either background or compliance data for a particular constituent, confidence bands accounting for trends will be constructed to account for the trend-induced variation. If this is not accounted for, a wider confidence interval will inevitably be calculated for a given confidence level and sample size (n). A wider confidence interval will result in less statistical power, or ability to demonstrate an exceedance or return to compliance. When a linear trend line has been estimated, a series of confidence intervals is estimated at each point along the trend. This creates a simultaneous confidence band that follows the trend line. As the underlying population mean increases or decreases, the confidence band does also to reflect this change at that point in time.

Linear regression will be used when background or compliance data are approximately normally distributed, with a constant sample variance around the mean, and the frequency of non-detects is low. The linear regression of concentration against sampling date (time) will be computed as follows:

$$\hat{b} = \sum_{i=1}^n (t_i - \bar{t}) \cdot x_i / (n - 1) \cdot s_t^2$$

x_i = i^{th} concentration value and

t_i = i^{th} sampling date

\bar{t} = sampling mean date

s_t^2 = variance of the sampling dates

This estimate leads to the following regression equation:

$$\hat{x} = \bar{x} + \hat{b} \cdot (t - \bar{t})$$

\bar{x} = mean concentration level

\hat{x} = estimated mean concentration at time t

The regression residuals will also be computed at each sampling event to ensure uniformity and lack of significant skewness. Regression residuals will be computed at each sampling event as follows:

$$r_i = x_i - \hat{x}_i$$

The estimated variance around the regression line, or mean squared error (MSE) will be computed as follows:

$$s_e^2 = \frac{1}{n - 2} \sum_{i=1}^n r_i^2$$

The confidence intervals around a linear regression trend line given confidence level (1- α) and a point in time (t_0), will be computed as follows:

$$LCL_{1-\alpha} = \hat{x}_0 - \sqrt{2s_e^2 \cdot F_{1-2\alpha,2,n-1} \cdot \left[\frac{1}{n} + \frac{(t_0 - \bar{t})^2}{(n-1) \cdot s_t^2} \right]}$$

$$UCL_{1-\alpha} = \hat{x}_0 + \sqrt{2s_e^2 \cdot F_{1-2\alpha,2,n-2} \cdot \left[\frac{1}{n} + \frac{(t_0 - \bar{t})^2}{(n-1) \cdot s_t^2} \right]}$$

\hat{x}_0 = estimated mean concentration from the regression equation at time t_0

$F_{1-2\alpha,2,n-2}$ = upper (1-2 α)th percentage point from an F-distribution with 2 and (n-2) degrees of freedom

For background data, the UCL around the linear regression line will be used as the GWPS for the trending constituent. For compliance data, confidence bands around the linear regression line will be compared to the GWPS. The CCR unit is considered to be in compliance if the LCL is equal to or lower than the GWPS for all detected constituents at all compliance wells. A GWPS exceedance is determined when the LCL based on the trend line first exceeds the GWPS.

3.1.7 Non-Parametric Thiel-Sen Trend Line and Confidence Band

If the Mann-Kendall test detects a significant trend in the non-parametric Thiel-Sen line using either background or compliance data for a particular constituent, confidence bands accounting for trends will be constructed to account for the trend-induced variation. The Thiel-Sen trend line will be used as a non-parametric alternative to linear regression when trend residuals cannot be normalized or if there are a higher percentage of non-detects in either background or compliance data. The Thiel-Sen trend line estimates the median concentration over time by combining the median pairwise slope with the median concentration value and the median sample date. To compute the Thiel-Sen line, the data will first be ordered by sampling event x_1, x_2, \dots, x_n . All possible distinct pairs of measurements (x_i, x_j) for $j > i$ will be considered and the simple pairwise slope estimate will be computed for each pair as follows:

$$m_{ij} = (x_j - x_i)/(j - i)$$

With a sample size of n , there will be a total of $N = n(n-1)/2$ pairwise estimates (m_{ij}) . If a given observation is a non-detect, half the RL will be substituted. The N pairwise slope estimates (m_{ij}) will be ordered from least to greatest (renamed $m(1), m(2), \dots, m(N)$). The Thiel-Sen estimate of slope (Q) will be calculated as the median value of the list depending on whether N is even or odd as follows:

$$Q = \begin{cases} m_{([N+1]/2)} & \text{if } N \text{ is odd} \\ (m_{(N/2)} + m_{([N+2]/2)})/2 & \text{if } N \text{ is even} \end{cases}$$

The sample concentration magnitude will be ordered from least to greatest, $x(1), x(2), \dots, x(n)$ and the median concentration will be calculated as follows:

$$\tilde{x} = \begin{cases} x_{([n+1]/2)} & \text{if } n \text{ is odd} \\ (x_{(n/2)} + x_{([n+2]/2)})/2 & \text{if } n \text{ is even} \end{cases}$$

The median sampling date (\tilde{t}) with ordered times ($t(1), t(2), \dots, t(n)$) will also be determined in this way. The Thiel-Sen trend line will then be computed for an estimate at any time (t) of the expected median concentration (x) as follows:

$$x = \tilde{x} + Q \cdot (t - \tilde{t}) = (\tilde{x} - Q \cdot \tilde{t}) + Q \cdot t$$

To construct a confidence band around the Thiel-Sen line, sample pairs (t_i, x_i) will be formed with a sample date (t_i) and the concentration measurement from that date (x_i). Bootstrap samples (B) will be formed by repeatedly sampling n pairs at random with replacement from the original sample pairs. This will be repeated 500 times. For each bootstrap sample, a Thiel-Sen trend line will be constructed using the equation above. A series of equally spaced time points (t_j) will be identified along the range of sampling dates represented in the original sample, $j = 1$ to m . The Thiel-Sen trend line associated with each bootstrap replicate will be used to compute an estimated concentration (\hat{x}_j^B) . An LCL will be constructed for the lower α^{th} percentile $\hat{x}_j^{[\alpha]}$ from the distribution of estimated concentrations at each time point (t_j). For a UCL, compute the upper $(1-\alpha)^{\text{th}}$ percentile, $\hat{x}_j^{[1-\alpha]}$ at each time point (t_j).

For background data, the UCL around the Thiel-Sen trend line will be used as the GWPS for the trending constituent. For compliance data, confidence bands around the Thiel-Sen trend line will be compared to the GWPS. The CCR unit is considered to be in compliance if the LCL is equal to or lower than the GWPS for all detected constituents at all compliance wells. A GWPS exceedance is confirmed when the LCL based on the trend line first exceeds the GWPS.

3.2 Determination of Statistically Significant Increases over Background

In accordance with 35 I.A.C. §§ 845.610(b)(3)(B) and 845.640(h), individual monitoring event concentrations for each constituent detected in the compliance monitoring wells during compliance monitoring sampling events will be compared to the background concentration as determined by the methods described above. An exceedance of the background concentration for any constituent measured at any compliance monitoring well, or constituent detection if not detected in the background samples, constitutes a Statistically Significant Increase (SSI). An exception to this method is pH, where two-sided (upper and lower) tolerance limits are established from the distribution of the background groundwater quality data. An exceedance of either the UTL or lower tolerance limit (LTL) would constitute an SSI for pH.

4. REFERENCES

Davis, C.B., 1994. *Environmental Regulatory Statistics*. In GP Patil & CR Rao (Eds.) *Handbook of Statistics, Volume 12: Environmental Statistics*, Chapter 26. New York: Elsevier Science B.V.

United States Environmental Protection Agency (USEPA), 2009. *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities: Unified Guidance*. EPA 530-R-09-007. March 2009.

APPENDIX I

Post-closure Care Plan

POST-CLOSURE PLAN FOR EXISTING CCR SURFACE IMPOUNDMENT
40 C.F.R. § 257.104 and 35 I.A.C. 845.780
REV 0 – 10/30/2021

SITE INFORMATION

Site Name / Address	Coffeen Power Plant / 134 Cips Lane, Coffeen, IL 61207		
Owner Name / Address	Illinois Power Generating Company / 6555 Sierra Drive Irving, Texas 75039		
CCR Unit	GMF Pond	Closure Method and Final Cover Type	Close In-Place Clayey Soil Cover with Vegetation

POST-CLOSURE PLAN DESCRIPTION

40 C.F.R. § 257.104(c)(1) and 35 I.A.C. 845.780(c)(1) – Length of post-closure care period.	<p>Post-closure care will be conducted for a period of 30 years as required by 40 C.F.R. § 257.104(c)(1) and 35 I.A.C. 845.780(c)(1), except as provided by 40 C.F.R. § 257.104(c)(2) and 35 I.A.C. 845.780(c)(2).</p>
40 C.F.R. § 257.104(c)(2) and 35 I.A.C. 845.780(c)(2) – Circumstances extending the post closure care period.	<p>If at the end of the post-closure care period the CCR unit is operating under assessment monitoring in accordance with §257.95, the post-closure care as described in this plan will continue until returning to detection monitoring in accordance with §257.95.</p> <p>Under 35 I.A.C. 845.780(c)(2), the post-closure care period will be extended until groundwater monitoring data demonstrate that concentrations are below the groundwater protection standards in Section 845.600 and are not increasing for those constituents over background, using the statistical procedures and performance standards in Section 845.640(f) and (g), provided that concentrations have been reduced to the maximum extent feasible and concentrations are protective of human health and the environment.</p>
40 C.F.R. § 257.104(d)(1)(i) and 35 I.A.C. 845.780(d)(1)(A) – A description of the monitoring and maintenance activities required in 40 C.F.R. § 257.104(b) and 35 I.A.C. 845.780(b), and the frequency at which these activities will be performed, to maintain the integrity and effectiveness of the final cover system, maintain the groundwater monitoring system and monitor the groundwater.	<p>Pursuant to § 257.104(b)(1) and 35 I.A.C. 845.780(b)(1), throughout the post-closure care period, periodic visual observations of the final cover system and stormwater management system will be performed at least annually for evidence of settlement, subsidence, erosion, or other damage that may adversely affect the integrity and effectiveness of the final cover system. When practical, visual observations of the final cover will be made concurrent with groundwater monitoring activities.</p> <p>Noted evidence of damage, such as rills, surface cracks and settlement, will be repaired to maintain the integrity and effectiveness of the final cover system. Vegetation will be established and maintained on the final cover system, including storm drainage areas, where appropriate, to provide long-term erosion control. Established vegetation and the slope design of the final cover system will prevent potential erosion and damage that may be caused by run-on and run-off.</p> <p>Repair activities may include, but are not limited to, replacing and compacting soil cover, repairing drainage channels that have been eroded, filling in depressions with soil, regrading, and reseeding areas of</p>

	<p>failed vegetation, as necessary.</p> <p>Pursuant to § 257.104(b)(3) and 35 I.A.C. 845.780(b)(3), the groundwater monitoring system will be maintained, and groundwater will be monitored as required by 40 C.F.R. § 257.90 through 40 C.F.R. § 257.98 and 35 I.A.C. 845.600 through 35 I.A.C. 845.680. Monitoring wells will be inspected during each groundwater sampling event. Monitoring wells and associated instrumentation will be maintained so that they perform to the design specifications throughout the life of the monitoring program. Groundwater monitoring frequency will be at least quarterly, except as provided in 40 C.F.R. § 257.94(d), 257.94(c), and 35 I.A.C. 845.650(b)(4).</p>
<p>40 C.F.R. § 257.104(d)(1)(ii) and 35 I.A.C. 845.780(d)(1)(B) – The name, address, telephone number and email address of the person or office to contact about the facility during the post-closure care period.</p>	<p>Illinois Power Generating Company 6555 Sierra Drive Irving, Texas 75039 800.633.4704 ccr@dynegy.com</p>
<p>40 C.F.R. § 257.104(d)(1)(iii) and 35 I.A.C. 845.780(d)(1)(C) – A description of the planned uses of the property during the post-closure period.</p>	<p>The CCR unit is located at a retired electric generation facility. Planned uses of the property during the post-closure period are currently unknown, except for post-closure care of the CCR unit.</p> <p>Post-closure use of the property will not disturb the integrity of the final cover system or other components of the containment system, or the function of the monitoring systems unless necessary to comply with the requirements of 40 C.F.R. Part § 257, Subpart D and 35 I.A.C. Part 845. Any other disturbance will be conducted following a demonstration that it will not increase the potential threat to human health or the environment, as required by 40 C.F.R. § 257.104(d)(1)(iii) and 35 I.A.C. 845.780 (d)(1)(C). The demonstration will be certified by a qualified professional engineer and submitted to the Illinois Environmental Protection Agency (IEPA). Per 40 C.F.R. § 257.104(d)(1)(iii) notification shall be provided to the State Director that the demonstration has been placed in the operating record and on the owners or operator's publicly accessible internet site.</p> <p>Following closure of the CCR unit, a notation on the deed to the property, or some other instrument that is normally examined during title search, will be recorded in accordance with 40 C.F.R. § 257.102(i) and 35 I.A.C. 845.760(h). The notation will notify potential purchasers of the property that the land has been used as a CCR unit and its use is restricted under the post-closure care requirements in 40 C.F.R. § 257.104(d)(1)(iii) and 35 I.A.C. 845.780(d)(1)(C) or groundwater monitoring requirements per 35 I.A.C. 845.740(b). Within 30 days of recording the deed notation, a notification stating that the notation has been recorded will be submitted to the IEPA and placed in the facility's operating record per 35 I.A.C. 845.760(h)(3). The notification will be placed on the owner or operator's publicly accessible CCR Web site in accordance with 40 C.F.R. § 257.107(i)(9) and 35 I.A.C. 845.810(e) and placed in the facility's operating record as required by 35 I.A.C. 845.800(d)(26) and §257.105(i)(9).</p>

<p>40 C.F.R. § 257.104(d)(3) and 35 I.A.C. 845.780(d)(3) - Amendments to the initial or subsequent written post-closure plan.</p>	<p>Pursuant to 40 C.F.R. § 257.104(d), the initial post closure care plan for the Coffeen GMF Pond was prepared on October 17, 2016. That plan is being amended pursuant to 40 C.F.R. § 257.104(d)(3)(i). This plan also serves as the initial post-closure care plan, prepared in accordance with 35 I.A.C. 845.780(d).</p> <p>Pursuant to § 257.104(d)(3) and 35 I.A.C. 845.780(d)(3), an operating permit modification application to amend the initial or any subsequent written post-closure care plan developed under 35 I.A.C. 845.780 (d)(1) and § 257.104(d)(1) will be submitted to IEPA. The written post-closure care plan will be amended whenever there is a change in the operation of the CCR surface impoundment that would substantially affect the written post-closure care plan in effect; or unanticipated events necessitate a revision of the written post-closure care plan, after post-closure activities have started.</p> <p>The written post-closure care plan will be amended at least 60 days before a planned change in the operation of the facility or CCR surface impoundment, or within 60 days after an unanticipated event requires the need to revise the existing plan. If the plan is revised after post-closure activities have started, a request to modify the operating permit, including an amended written post-closure care plan, will be submitted to the IEPA within 30 days following the triggering event.</p>
<p>40 C.F.R. § 257.104(d)(4) and 35 I.A.C. 845.780(d)(4) – Qualified professional engineering certification.</p>	<p>Certification by a qualified professional engineer will be appended to this plan and any amendment of this plan.</p>
<p>35 I.A.C. 845.780(e) – Termination of post-closure care</p>	<p>Upon completion of the post-closure period, a request to terminate post-closure care will be submitted to the IEPA. The request will include a certification by a qualified professional engineer verifying that post-closure care has been completed in accordance with the post-closure care plan specified in 35 I.A.C. 845.780(d) and the requirements of 35 I.A.C. 845.780.</p>
<p>40 C.F.R. § 257.104(e) and 35 I.A.C. 845.780(f) – Notification of completion of the post-closure care period.</p>	<p>A notification of completion of post-closure care will be prepared and placed in the facility’s operating record within 30 days after IEPA approval of the request to terminate post-closure care. The notification will be placed in the facility's operating record in accordance with 35 I.A.C. 845.800(d)(31) and § 257.105(i)(13).</p> <p>The notification will be placed on the owner or operator's publicly accessible CCR Internet site in accordance with the requirements of § 257.107(i)(13) and 35 I.A.C. 845.810(e). The IEPA will be notified when the notification has been placed in the operating record and on the owner or operator's publicly accessible Internet site in accordance with the requirements of § 257.106(i)(13).</p>

**Certification Statement 40 C.F.R. § 257.104 (d)(4) and 35 I.A.C. 845.780(d)(4) – Amended/Initial
Written Post Closure Plan for a CCR Surface Impoundment**

CCR Unit: Illinois Power Generating Company; Coffeen Power Plant; GMF Pond

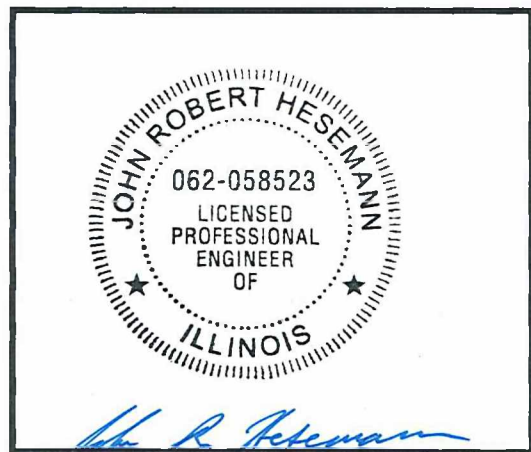
I, John R. Hesemann, being a Registered Professional Engineer in good standing in the State of Illinois, do hereby certify, to the best of my knowledge, information, and belief, that the information contained in this certification has been prepared in accordance with the accepted practice of engineering. I certify, for the above referenced CCR Unit, that the information contained in the amended/initial written post closure plan, dated October 30, 2021, meets the requirements of 40 C.F.R. § 257.104 and 35 I.A.C. 845.780.

John R. Hesemann

Printed Name

9/27/2021

Date



John R. Hesemann
Exp.: 11/30/2021

APPENDIX J

Legal Description

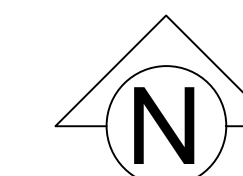
CONTROL MONUMENTATION				
POINT NO.	NORTHING	EASTING	ELEVATION	DESCRIPTION
2008	876619.65	2517127.46	623.84	FOUND IRON PIN @ NORTH 1/4 CORNER OF SEC. 11
2009	873984.91	2517176.22	614.30	FOUND 2IN PIPE @ THE CENTER OF SEC. 11
2010	873979.62	2514535.96	623.85	FOUND BRASS DISK IN CONCRETE @ THE WEST 1/4 CORNER OF SEC. 11
2016	876592.86	2514482.10	626.44	FOUND IRON PIN @ NW CORNER OF SEC. 11



Luminant

**ILLINOIS POWER GENERATING COMPANY
COFFEEN POWER PLANT**

**SW 1/4
SEC. 2**



0' 150' 300'

- LEGEND**
- SECTION LINE
 - RESTRICTED USE BOUNDARY
 - FACILITY BOUNDARY
 - FOUND SURVEY MARKER AS NOTED

SURVEY NOTE:
THIS DRAWING AND THE INFORMATION SHOWN HERE ON WAS OBTAINED FROM DATA COLLECTED FROM A FIELD SURVEY MADE BY INGENAE, LLC BETWEEN FEBRUARY 12 THROUGH JULY 20, 2021. SURVEY COORDINATES, BEARINGS & DISTANCES ARE REFERENCED TO ILLINOIS WEST 1202 STATE PLANE COORDINATE SYSTEM NAD 1983.

CCR FACILITY BOUNDARY CORNERS		
Point #	Northing	Easting
1010	873983.38	2516413.41
1011	874222.30	2516410.32
1012	874230.94	2515283.35
1013	875165.70	2514904.19
1014	875017.48	2514554.77
1015	876638.91	2514531.16
1016	876662.16	2515990.93
1017	875183.95	2516003.77
1018	875223.87	2516105.73
1019	875107.47	2516308.60
1020	874481.26	2516406.96

**Land Description of the Coffeen Power Plant
GMF Gypsum Stack Pond and GMF Recycle Pond Facility
Boundary
77.98 Acres**

Part of the Northwest Quarter of Section 11 and part of the Southwest Quarter of Section 2 all in Township 7 North, Range 3 West of the Third Principal Meridian, Montgomery County, Illinois being more particularly described as follows:

Commencing at the Brass Plug in concrete found at the West Quarter Corner of Section 11, from which bears an Iron Pin at the Northwest corner of Section 11 North 1 degree 10 minutes 50 seconds West a distance of 2613.80 feet; thence from said commencement point at the West Quarter Corner of Section 11, North 89 degrees 53 minutes 06 seconds East, along the South line of the Northwest Quarter of Section 11, a distance of 2640.27 feet to the 2 inch Iron Pipe at the Center of Section 11, from which bears an Iron Pin at the North Quarter Corner of Section 11 North 1 degree 03 minutes 37 seconds West a distance of 2635.19 feet; thence from said 2 inch Iron Pipe at the Center of Section 11 South 89 degrees 53 minutes 06 seconds West back along the South line of the Northwest Quarter of Section 11 a distance of 762.81 feet; thence North 0 degrees 44 minutes 31 seconds West a distance of 238.94 feet to the Point of Beginning of the Tract described herein; thence North 89 degrees 33 minutes 37 seconds West a distance of 1127.00 feet; thence North 22 degrees 04 minutes 41 seconds West a distance of 1008.73 feet; thence South 67 degrees 00 minutes 48 seconds West a distance of 379.57 feet; thence North 0 degrees 50 minutes 03 seconds West a distance of 1621.60 feet; thence North 89 degrees 05 minutes 16 seconds East a distance of 1459.96 feet; thence South 0 degrees 29 minutes 51 seconds East a distance of 1478.26 feet; thence North 68 degrees 37 minutes 10 seconds East a distance of 109.50 feet; thence along a curve to the right having a radius of 150.00 feet a curve length of 268.22 feet a chord bearing South 60 degrees 09 minutes 13 seconds East a chord distance of 233.89 feet; thence South 8 degrees 55 minutes 37 seconds East a distance of 633.88 feet; thence South 0 degrees 44 minutes 31 seconds East a distance of 258.99 feet to the Point of Beginning and containing 77.98 Acres.

SURVEYOR CERTIFICATE:
THIS IS TO CERTIFY THAT WE, INGENAE, LLC, HAVE AT THE REQUEST OF AND FOR THE EXCLUSIVE USE OF THE OWNERS, PERFORMED A SURVEY OF THE TRACT AS SHOWN HEREON AND THAT THIS IS A TRUE REPRESENTATION OF THAT SURVEY. THIS PLAN AND THE SURVEY FROM WHICH IT IS BASED WERE DONE IN ACCORDANCE WITH THE "MINIMUM STANDARDS OF PRACTICE" FOR LAND SURVEYING IN THE STATE OF ILLINOIS.

INGENAE, LLC
PROFESSIONAL DESIGN FIRM
LICENSE NO. 184.007588-0010

Michael J. Graminski



MICHAEL J. GRAMINSKI
I.P.L.S. NO. 035.002901
EXPIRES: 11/30/2022

DATE

IngenAE

502 Earth City Plaza, Suite 120
Earth City, MO 63045
www.ingenae.com

Submissions / Revisions: Date:

1		
2		
3		
4		
5		
6		
7		
8		
9		
10		
11		
12		
13		



Project Name & Location:

**COFFEEN
POWER PLANT**

134 CIPS Lane
Coffeen, IL 62017

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Drawing Name:

**CCR FACILITY
BOUNDARY
EXHIBIT**

Date:

9/21/2021

Project No.

Type:

SITE

Drawing No.

Drawn By:

CB

1

Approved By:

BH

Scale:

AS NOTED

**SE 1/4
SEC. 3**

2016
FOUND IRON PIN
@ N.W. CORNER
OF SEC. 11

CCR LANDFILL

**NE 1/4
SEC. 10**

SOUTHWEST
DETENTION POND

**GMF GYPSUM
STACK POND**

**TOTAL AREA
77.98 ACRES**

**GMF RECYCLE
POND**

POINT OF COMMENCEMENT
2010
FOUND BRASS DISK IN
CONCRETE @ THE WEST
1/4 CORNER OF SEC. 11

CLOSED ASH POND 2

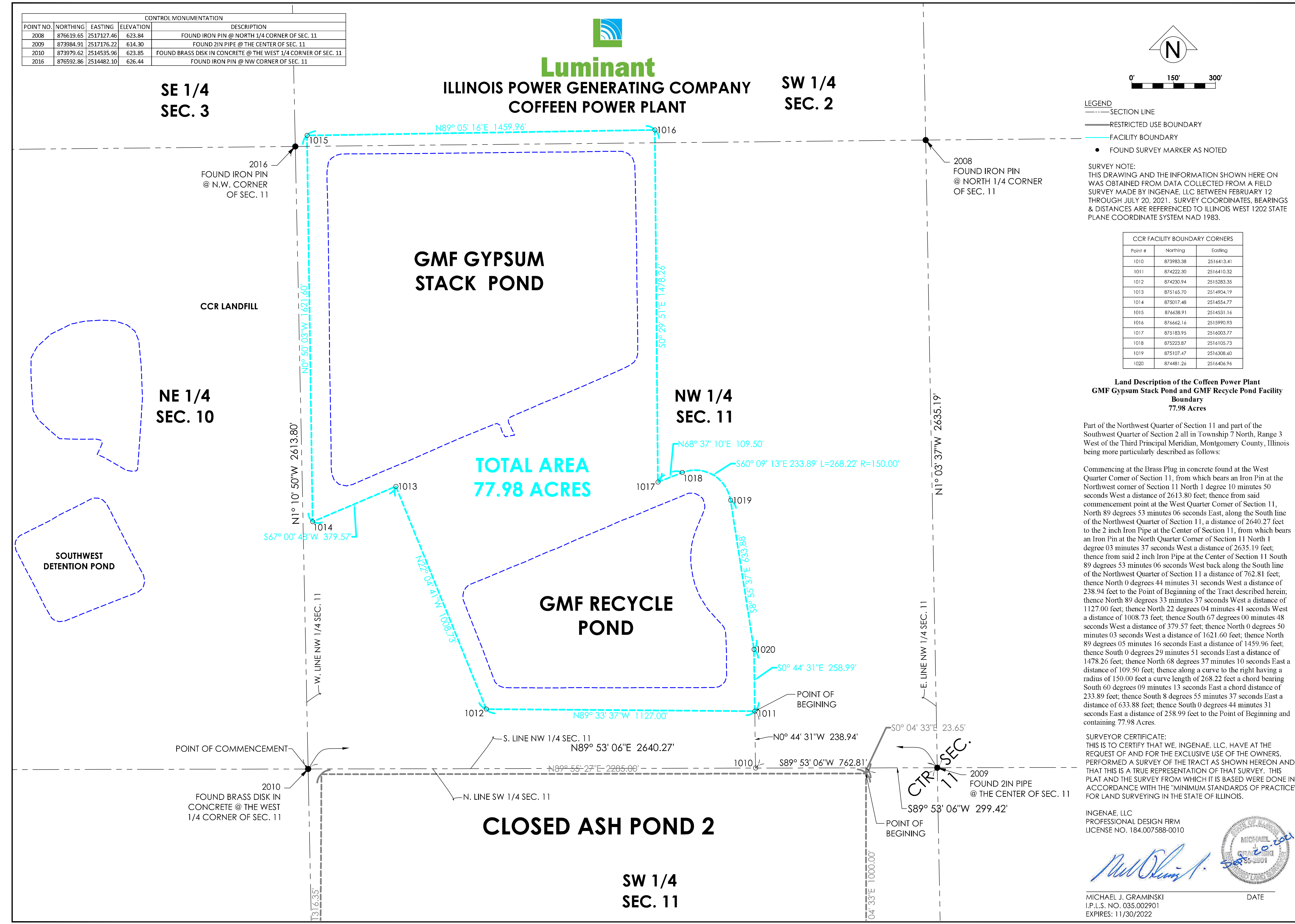
**SW 1/4
SEC. 11**

2008
FOUND IRON PIN
@ NORTH 1/4 CORNER
OF SEC. 11

2009
FOUND 2IN PIPE
@ THE CENTER OF SEC. 11

POINT OF BEGINNING

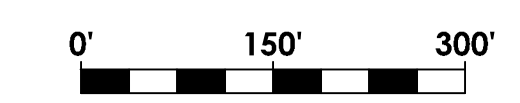
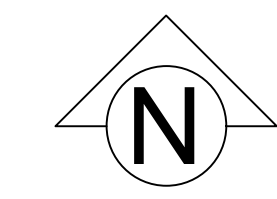
CTR. SEC. 11



CONTROL MONUMENTATION				DESCRIPTION
POINT NO.	NORTHING	EASTING	ELEVATION	
2008	876619.65	2517127.46	623.84	FOUND IRON PIN @ NORTH 1/4 CORNER OF SEC. 11
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**ILLINOIS POWER GENERATING COMPANY
COFFEEN POWER PLANT**



- LEGEND**
- SECTION LINE
 - RESTRICTED USE BOUNDARY
 - - - FACILITY BOUNDARY
 - FOUND SURVEY MARKER AS NOTED

SURVEY NOTE:
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CCR FACILITY BOUNDARY CORNERS		
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1019	875107.47	2516308.60
1020	874481.26	2514406.96



502 Earth City Plaza, Suite 120
Earth City, MO 63045
www.ingenae.com

Submissions / Revisions:	Date:
1	
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9	
10	
11	
12	
13	



Project Name & Location:
**COFFEEN
POWER PLANT**

134 CIPS Lane
Coffeen, IL 62017

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Drawing Name:
**CCR FACILITY
BOUNDARY
EXHIBIT**

Date: 9/21/2021	Project No.
Type: SITE	Drawing No. 2
Drawn By: CB	
Approved By: BH	
Scale: AS NOTED	

APPENDIX K

Public Meetings Information



Dianna Tickner
Illinois Power Generating Company
1500 Eastport Plaza Drive
Collinsville, IL 62234

July 27, 2022

Illinois Environmental Protection Agency
DWPC – Permits MC # 15
ATTN: Part 845 Coal Combustion Residual Rule Submittal
1021 North Grand Avenue East
P.O. Box 19276
Springfield, IL 62794-9276

**Re: 35 IAC 845.220(a)(9) Certification Statement
Coffeen Power Plant; Ash Pond No. 1, GMF Gypsum Stack Pond, and GMF Recycle
Pond (IEPA ID # W1350150004-01,03,04)**

Dear Mr. Darin LeCrone:

For the above-referenced CCR surface impoundments and in accordance with 35 IAC 845.220(a)(9), Illinois Power Generating Company certifies that the public notification and public meetings required under 35 IAC 845.240 were completed. Please find enclosed both the public meeting summary and listserv.

Sincerely,
Illinois Power Generating Company

A handwritten signature in blue ink that reads "Dianna Tickner".

Dianna Tickner
Director, Decommissioning

Coffeen Public Meeting Summary, June 14, 2022

On May 13, 2022, Illinois Power Generating Company (IPGC) made available to the public its plans to close Ash Pond No. 1 (AP1), Gypsum Management Facility (GMF) Gypsum Stack Pond (GSP); and GMF Recycle Pond (RP) located at the Coffeen Power Plant. On Tuesday, June 14, 2022, IPGC held in-person public meetings at 3:00 pm and 5:30 pm at the Coffeen Public School to present its decision-making process. A comparison of projected groundwater impacts for the alternatives presented, and an objective comparison of the pros and cons of each alternative were presented at these meetings. During the question-and-answer portion of the meeting, the public asked questions relating to the proposed closure which the company addressed. As required by Section 845.240(g), this document provides a general summary of the issues or comments raised by the public relating to the closure, a summary of the company’s responses to those issues or comments, and a summary of any revisions or changes made to the proposed closure action as a result of issues and comments raised by the public.

Issue/Topic		Summary of Response Provided at Meeting	Additional Written Response
1.	Closure-by-Removal with On-Site Disposal	The existing Coffeen Power Plant landfill has capacity for the Gypsum Recycle Pond and an estimated 63% of material in AP1. There is not enough space to expand the current on-site landfill due to planned redevelopment for solar generation and battery storage. Disposal of any additional material would have to be managed offsite for closure by removal.	
2.	Closure-in-Place for AP1 and GMF GSP	<p>Closure in place includes consolidating the CCR to reduce the total footprint. Ash in AP1 will be consolidated to the west side. The gypsum in the GMF GSP will be consolidated to the north side.</p> <p>The composite cap components used for final cover system include an LLDPE geomembrane liner, cover soil, vegetation, and stormwater control features.</p> <p>For AP1, the cover system will extend horizontally past the limits of waste to minimize infiltration of precipitation into the</p>	

		<p>unit. The cover system on GMF GSP will overlap the liner beneath the unit.</p> <p>Free liquids will be removed from the ash and non-contact stormwater will be discharged in accordance with the NPDES permit. If necessary, to meet permit limits, the liquids will be pre-treated.</p>	
3.	Closure Permitting	<p>The question was raised as to whom selects the closure method. Part 845 stipulates that Plant owner proposes to the agency the most appropriate closure method. The public has additional opportunities to provide input and IEPA must approve the selected method.</p>	
4.	Groundwater	<p>A question was asked as to why only sulfate was discussed during the presentation. What about other contaminants, for example cobalt?</p> <p>Sulfate is a primary indicator of potential CCR impacts to groundwater and was identified as a potential exceedance of the groundwater protection standard (GWPS) at more monitoring wells than other contaminants; therefore, sulfate was used in the groundwater model as a surrogate for all potential contaminants. Statistically significant correlations between sulfate concentrations and concentrations of boron and TDS (the other two constituents identified as potential exceedances of the GWPS) indicate sulfate is an acceptable surrogate for these parameters. Identified potential exceedances of the GWPS are expected to behave similar to sulfate following closure.</p>	<p>As indicated during the meeting, cobalt has not been identified as a potential exceedance of the GWPS for AP1, the GMF GSP, or GMF RP. Boron, sulfate, and total dissolved solids were identified as potential exceedances of the GWPS at one or more of these CCR units. Sulfate being detected in more monitoring wells than other potential exceedances is also an indication that sulfate has the largest plume and will take longer to achieve the GWPS than the other potential contaminants, making the model predictions conservative for other potential contaminants.</p>

5.	Risk Assessment	<p>An ecological and human health risk assessment was performed to evaluate potential risks associated environmental exposure to CCR related constituents. This assessment determined that there are no current or future unacceptable risks for aquatic or benthic organisms in Coffeen Lake or for recreators (e.g., anglers, boaters, etc.) on the lake.</p>	<p>The Risk Assessment follows USEPA and IEPA recommended guidance using conservative (health-protective) assumptions based on a conceptual site model (CSM). The CSM provides a basis for understanding the site conditions and exposure pathways for receptors that may be exposed to site-related constituents. Exposure pathways refer to the way that people or animals may come in contact with a constituent. They are generally referred to as either complete or incomplete. The necessary components for a complete exposure pathway consist of:</p> <ul style="list-style-type: none"> • A source and mechanism of constituent release from the source; • Retention or transport of the constituent through the environmental medium; • A point of contact between the receptor and the environmental medium; and • A route of exposure for the potential receptor at the contact point. <p>US EPA has established acceptable risk metrics. Risks above these US EPA defined metrics are termed potentially "unacceptable risks".</p> <p>This assessment evaluated potential risks to human and ecological receptors that may be exposed to groundwater or surface water near the site. These receptors include recreators on Coffeen Lake (e.g., boaters and swimmers), anglers on Coffeen Lake that consume locally caught fish, and ecological receptors in the lake and lake sediments. No unacceptable risks were identified for any receptor, which means that the calculated risks from the site are likely indistinguishable from normal background risks.</p>
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6.	Future Land Use –	<p>IPGC intends to install a solar facility capable of approximately 44 MW at the Coffeen Power Plant.</p> <p>The property is being assessed to identify potential suitable locations for installation of solar panels. The Coal-to-Solar regulation has deadlines for installation, and we anticipate ash pond closure construction may be completed along concurrent timelines.</p> <p>Work will be contracted following standard procedures.</p> <p>IPGC intends to operate an energy storage unit (battery) at the Coffeen Site. The battery containers include HVAC systems to protect the batteries and minimize risk of fires. A vendor for the batteries has not yet been selected.</p> <p>Operation of the storage units will be done in accordance with county, state, and federal environmental and safety regulations.</p> <p>The Coffeen Power Plant will continue to maintain a contract with local emergency response services.</p>	
7.	Existing Structures	There are no current plans for demolition of the existing plant structures.	Access to the site will continue to be restricted.
8.	Worker Safety	All current and future work will be performed in accordance with the Safety and Health Plan as required by 35 IAC 845.530.	

		CCR does not include volatile organic compounds capable of generating vapors, therefore, dust is the only airborne concern. In accordance with the Fugitive Dust Plan, provided to IEPA dust control methods, including spreading water on surface of ash, will be used during construction.	
9.	Beneficial Reuse	Beneficial reuse continues to be evaluated. The primary beneficial reuse of fly ash is in cement.	
10.	Meeting Notice	An attendee questioned the address provided for the meeting at the Coffeen school.	Upon further review the address provided for the meeting was correct. The address given is the address provided on the school website. https://www.hillsboroschools.net/Domain/11
11.	Off-site monitoring	An attendee questioned whether contamination was migrating onto neighboring properties triggering the need for private water well sampling.	There is no off-site contamination, thus no off-site sampling is required.
12.		An attendee questioned seasonal rainfall and if this was accounted for in the modeling.	The modeling accounts for seasonal rainfall.

In accordance with 845.240(f)(4), a list of people who requested to be added to the IEPA Listserv for Coffeen is as follows:

Coffeen construction permit public meetings	
People requesting to be added to IEPA Listserv	
Name	Email address
Dan Karban	DanKarban@gmail.com
John Blankenship	Johnblankenship@yahoo.com
Mary Ellen Declure	Jwdmed@consolidated.net
Steve Smith	520ssmith@gmail.com

APPENDIX L

Contractor Training Certification



Dianna Tickner
Illinois Power Generating Company
1500 Eastport Plaza Drive
Collinsville, IL 62234

July 28, 2022

Illinois Environmental Protection Agency
DWPC – Permits MC # 15
ATTN: Part 845 Coal Combustion Residual Rule Submittal
1021 North Grand Avenue East
P.O. Box 19276
Springfield, Illinois 62794-9276

**Re: 415 ILCS 5/22.59(b)(4) Certification Statement
Coffeen Power Plant Ash Pond 1 (IEPA ID # W1350150004-01), GMF Gypsum Stack Pond (IEPA
ID # W1350150004-03), and GMF Recycle Pond (IEPA ID# W1350150004-04)**

Dear Mr. Darin LeCrone:

For the above-referenced CCR surface impoundments and in accordance with 415 ILCS 5/22.59(b)(4), Illinois Power Generating Company certify that all contractors, subcontractors, and installers utilized to construct, install, modify, or close a CCR surface impoundment will be participants in a training program that is approved by and registered with the US Department of Labor's Employment and Training Administration and that includes instruction in the following: erosion control, environmental remediation, operation of heavy equipment and excavation.

Sincerely,
Illinois Power Generating Company

A handwritten signature in blue ink that reads "Dianna Tickner".

Dianna Tickner
Director, Decommissioning & Demolition

wsp **GOLDER**

golder.com